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DEPARTMENTAL RESEARCH

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SKEWED MULTI-BEAM BRIDGES WITH PRECAST BOX GIRDERS

STATE DEPARTMENT OF HIGHWAYS
AND PUBLIC TRANSPORTATION



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16. Abstract The research completed under this project demonstrates the reduction in required design moment that can be achieved by consideration of the skew angle and aspect ratio of a multi-beam bridge. It is primarily directed toward precast multi-beam box bridges of the type currently built in Texas. Other similiar multi-beam or solid slab structures with simple spans can also be handled. A data generator computer input assist was written which provides the data for a previously developed program so that accurate solutions can be obtained with a minimum of input. A field load test was performed on a full-scale skewed bridge. Comparison of the measured values with those predicted by the analysis methods was good.			
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SKEWED MULTI-BEAM BRIDGES
WITH PRECAST BOX GIRDERS

by

John J. Panak

Research Report Number 206-1F

Wheel Load Distribution for Precast
Multi-Beam Box Girders

Research Study 1-5-76-206

Conducted by

The Bridge Division

Texas State Department of Highways
and Public Transportation

in cooperation with the
U.S. Department of Transportation
Federal Highway Administration

September 1977

PREFACE

The report presents a study of skewed multi-beam precast box beam bridges of the types currently used in Texas. A data generator computer input assist was developed for analysis by an existing complex program. The method of analysis can be applied to most multi-beam bridges. A load test of a full-scale structure provided validation.

The work was supported by the Texas State Department of Highways and Public Transportation in cooperation with the U.S. Department of Transportation, Federal Highway Administration.

Mr. Charles C. Terry of the Bridge Division provided assistance throughout the course of the project, and his help is greatly appreciated. The advice of Mr. Robert L. Reed, of the Bridge Division, is also appreciated.

John J. Panak

September 1977

SUMMARY

This study was made to help determine more accurate design live load values for skewed multi-beam bridges constructed with precast, prestressed box beam sections of the types now in use in Texas. The results demonstrate that as much as 40 or 50 percent reduction in design live load moment is achieved by consideration of the skew angle for these structures. The span aspect ratio was also found to have an appreciable effect.

A data generator computer input assist was developed to allow simple application of the analysis method. The procedure allows any variation of skew, structure width, span, and box depth or width to be analyzed.

A field test of a full-scale skewed structure was performed which provided good correlations between predicted and measured response to a heavily loaded truck. These correlations add significant confidence to the user of the developed analysis techniques.

IMPLEMENTATION

This project has demonstrated that a significant reduction in design live load moment can be achieved in multi-beam bridges by consideration of the skew angle and span aspect ratio. It is recommended that the developed data generator computer input assist be used for future design of these structures.

DISCLAIMER

This report reflects the views of the author who is responsible for the facts presented. The contents do not necessarily reflect the views or policies of the Bridge Division, Texas State Department of Highways and Public Transportation, or the Federal Highway Administration. This report does not constitute a standard, specification, or regulation.

There was no invention or discovery made under this contract which is patentable.

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CHAPTER 1. INTRODUCTION

The objective of this study was to determine the approximate wheel load distribution for skewed precast box beam bridges of the type currently in use in Texas. At the outset, it was anticipated that a significant reduction in the required design moment could be achieved. The effect of skew was to be incorporated in the study since it produces twisting effects which are well resisted by a multi-beam structure with large shear keys such as used in Texas.

Program Data Generator

As a part of the study, a data generator computer input assist was developed to produce the necessary mass of data needed to effectively analyze a skewed structure by the solver program which had previously been developed during the period 1966 through 1972 (Ref 8). The data generator proved to require somewhat more effort to develop than first anticipated. Documentation of the data generator and instructions for its use are included herein.

Field Load Test

A field load test was performed during July 1977 of a full-scale skewed multi-beam box structure. Comparison of the measured values with those predicted by the analysis methods developed during the project were exceptionally good. The test correlation adds significant confidence to the user of the included analysis techniques for any structure configuration using precast box beams now used in Texas.

Application to Design

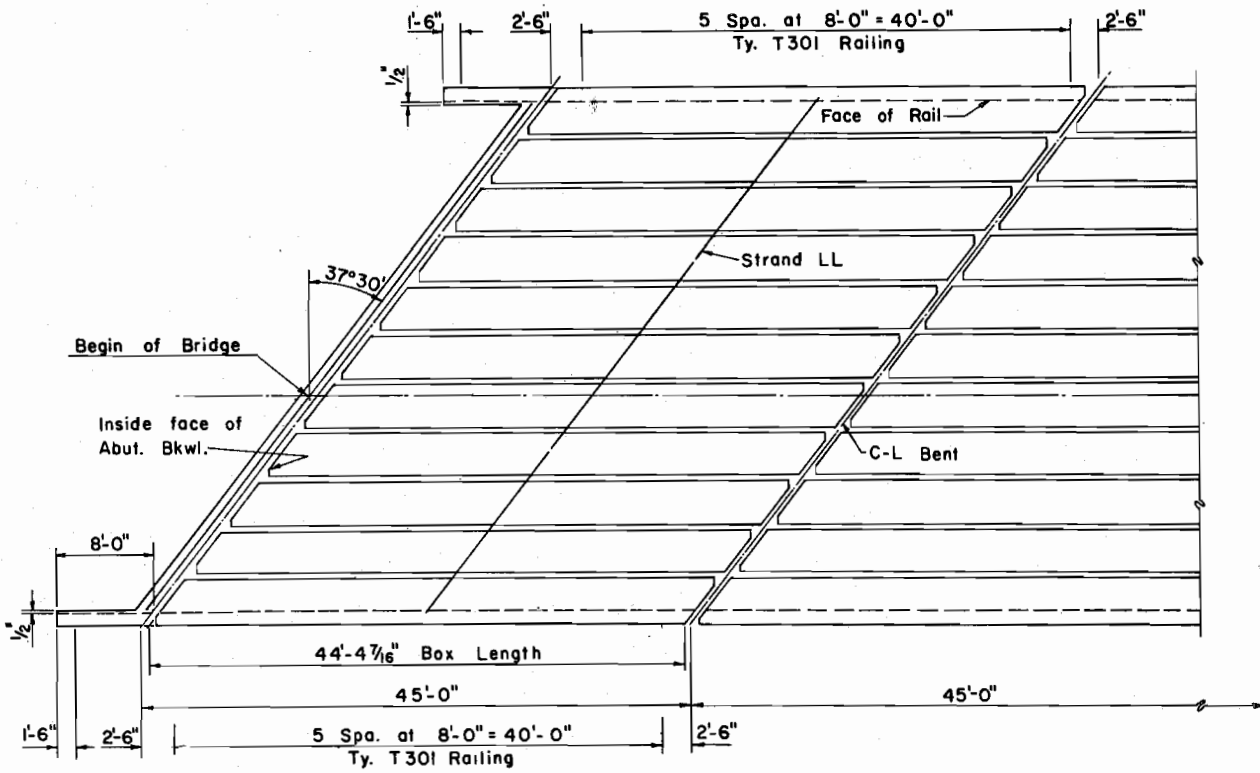
The project proposal indicated that charts or graphs might be prepared so that a designer could enter directly with his box beam structure configuration and obtain the required live load design moment. This has proven very difficult to do and still retain accuracy. The design moment per box is almost impossible to predict accurately especially when a mixture of box widths exists. Therefore, it was decided during later stages of the project to direct the emphasis toward demonstrating the beneficial effect of skew which significantly reduces the longitudinal live load design bending moment. Emphasis was also placed on making the data generator easy to use so that with as little data as possible, design moments for a particular structure can be obtained directly. As will be shown, this objective has been accomplished. A significant reduction in the required design moment can be achieved by using the proven analysis methods presented within this report.

CHAPTER 2. MULTI-BEAM BRIDGES

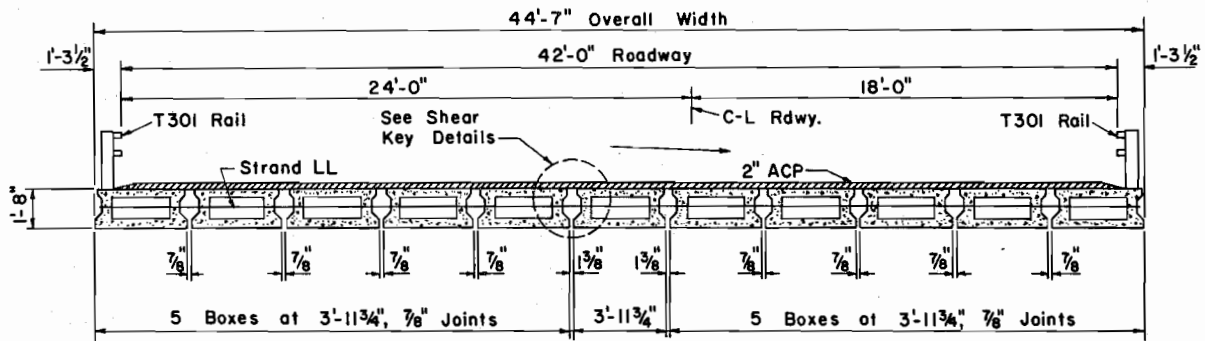
A typical skewed multi-beam precast concrete bridge as currently constructed in Texas is shown in Fig 1. The Texas State Department of Highways and Public Transportation (hereafter referred to as DHT) began building these precast multi-beam structures in 1969 and their use has increased steadily since then. The general cross-sections are as shown in Fig 2. The beams are cast with the sides formed using the same or similar configurations that have been used for precast beams in the state since about 1958. The beams are placed side by side and a cast-in-place shear key is placed in the resulting space left between beams. As can be seen in Figs 2, 3, and 4, the final cross-section has large effective webs between the voids. The direct cost of these bridges is currently about fifty percent higher than comparable precast beam and slab structures, but offsetting this added cost is the reduction in approach roadway earthwork due to the more shallow overall structure depth. The construction schedule can also be greatly accelerated, thus reducing public inconvenience. These bridges are considered more durable than cast-in-place bridges due to the use of high-strength precast concrete. It is hoped that increased usage in the future will reduce the direct cost of these bridges.

AASHTO Design Equations

The design of multi-beam precast concrete box girders was, until 1974, done with little regard to their inherently different behavior under live loading as compared to customary beam and slab structures. The AASHTO 1965 specifications (Ref 3) provided that the distribution for multi-beam bridges should not exceed that for a reinforced slab. This provision implied that pre-

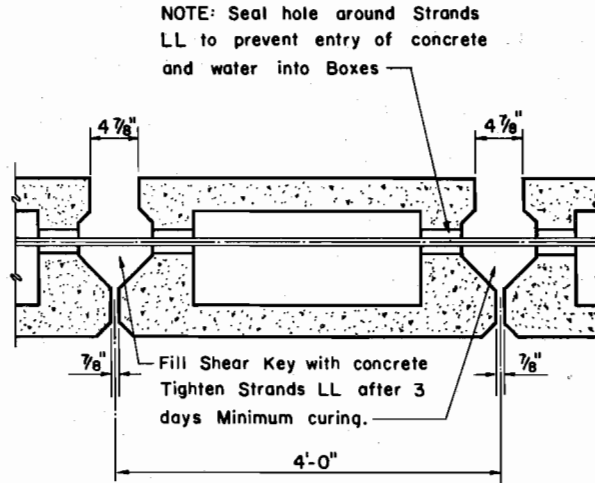


PLAN

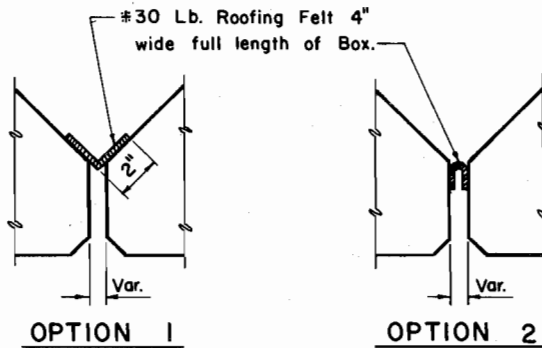


TRANSVERSE SECTION

Fig 1. Typical skewed multi-beam bridge using eleven Type 4-20 precast box sections.



SHEAR KEY DETAILS



JOINT DETAILS

*Caulking compound or other Joint Sealer may be used if approved by the Engineer.

Fig 2. Shear key and joint details

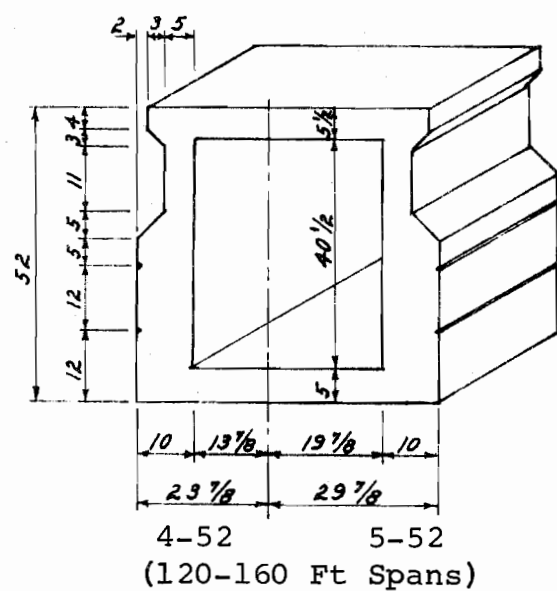
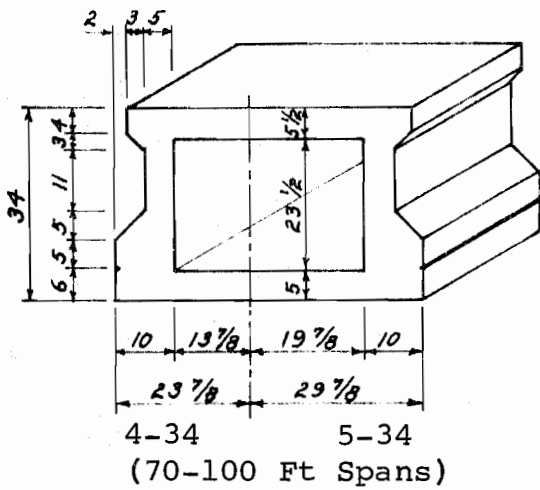
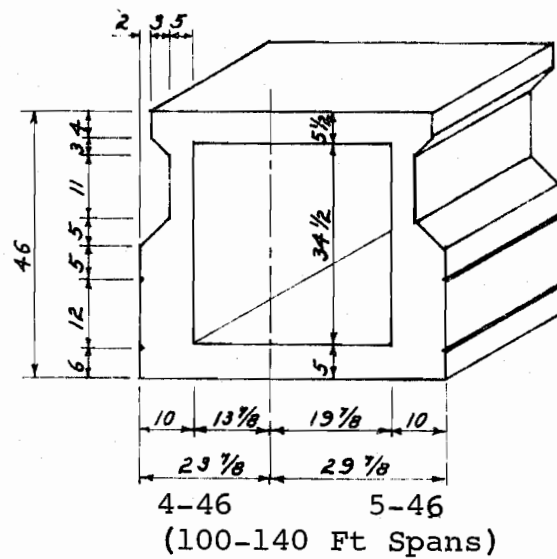
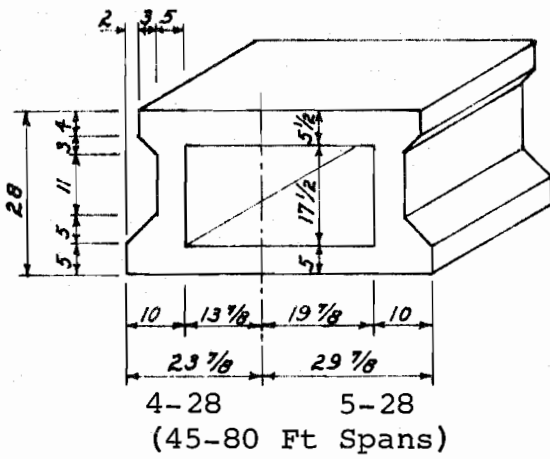
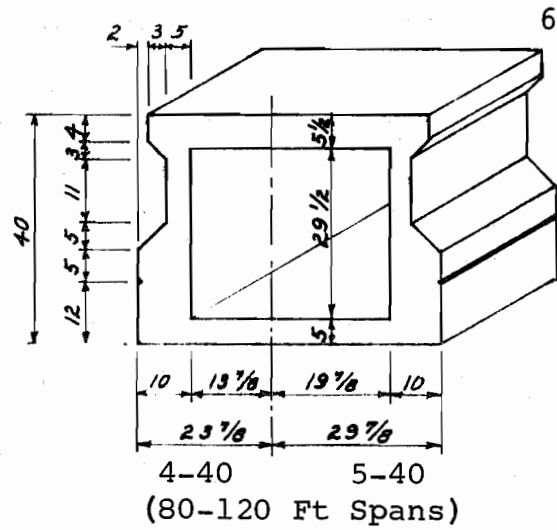
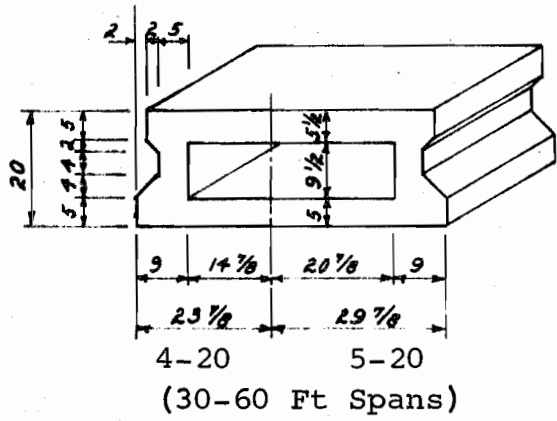
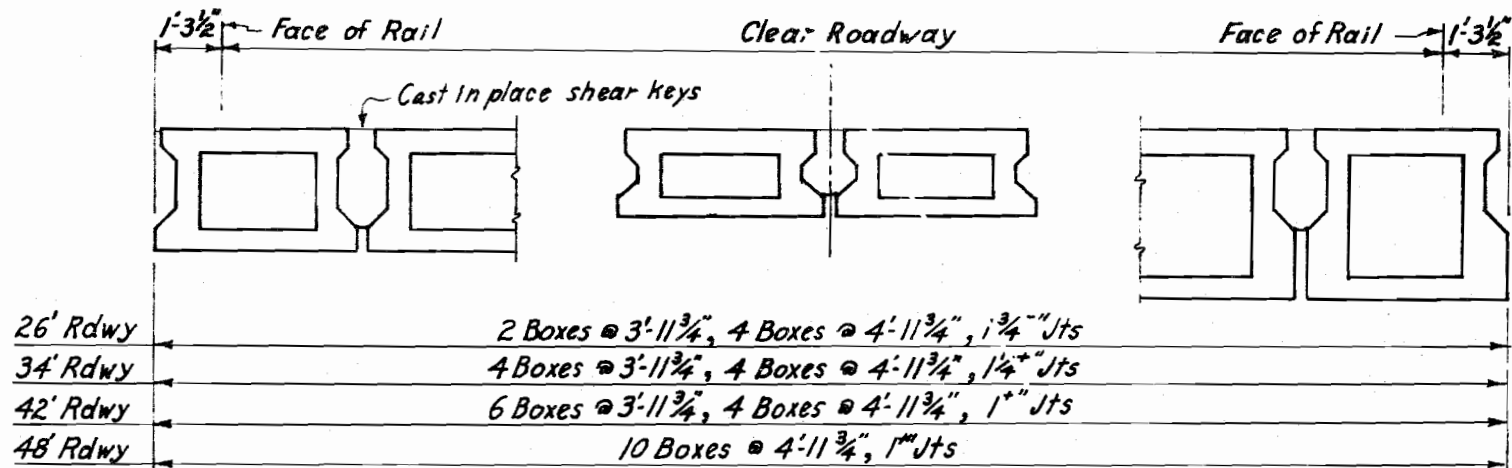
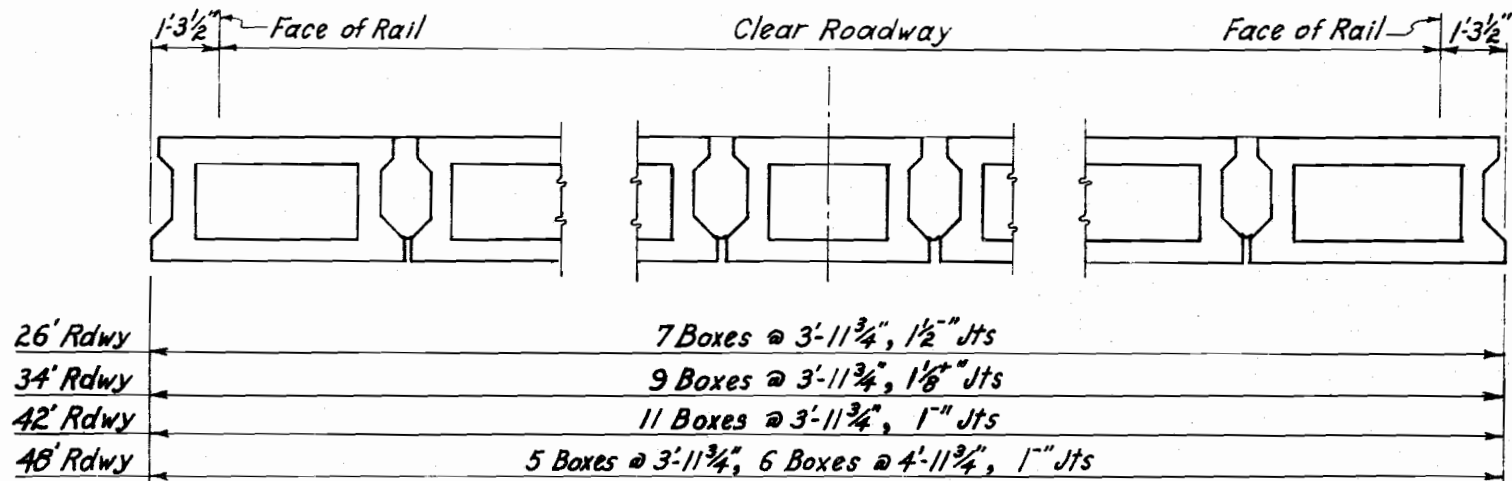


Fig 3. Typical precast multi-beam box sections.



JOINT AT CENTERLINE OF BRIDGE



NO JOINT AT CENTERLINE OF BRIDGE

Fig 4. Typical bridge cross-sections.

cast multi-beams were as good as solid slabs for distribution. The distribution formula at that time and still current for slabs is given by

$$E = 4 + 0.6S \quad (1)$$

where E is the distribution in feet and S is the span. For bridges with spans of 30 to 50 feet and longer, the resulting distribution is 5.8 to a maximum of 7.0 feet. This formula has remained since it was changed from an even more conservative formula in 1961 (Ref 4) which was

$$E = \frac{10N + W}{4N} \quad (2)$$

where N and W were numbers of lanes and bridge width respectively. For uniform 12 foot increments of lanes and widths, Eq 2 gave approximately a 5.5 foot slab distribution.

In 1970, as a result of extensive analytical investigations at Iowa State University by Sanders, Elleby, and Watanabe (Refs 10 and 12), a revised series of equations was proposed to AASHTO in 1974 (Ref 2). This interim specification is directed particularly toward multi-beam bridges, and for the first time in the history of the AASHTO bridge specifications, the torsional stiffness of the beam members and the aspect ratio of the structure were included in the effective distribution. These parameters were known to influence load distribution for many years, but simple methods to include them had not been available. The amount of bending moment for a multi-beam structure as now incorporated in Ref 2 is computed by applying the following fraction of a wheel load (line of wheels) to each beam

$$\frac{(12 N_L + 9) / N_g}{5 + N_L/10 + (3 - 2N_L/7) (1 - C/3)^2} \quad (3)$$

where N_L is the whole number of design 12 foot lanes, N_g is the number of beams, and C is a stiffness parameter given by

$$C = K W/L \quad (4)$$

where K is a constant varying from 0.7 to 2.2 with 1.0 recommended for box section multibeams, W is the overall width of the structure, and L is the span. The last term in the denominator of Eq 3 is neglected if C exceeds 3 which is only true for extremely wide structures with short spans. The K stiffness factor is supposed to help account for the variations in torsional rigidity (Ref 10), but as can be seen by Eq 3, the factor C has very little effect on the resulting distribution. This study will show that for orthogonal structures, the torsional stiffness does in fact have little effect on the maximum bending moments, but for skewed structures it is a significant parameter. This has also recently been observed by Kennedy and Gupta (Ref 6). Unfortunately, of the three general structural types investigated in Ref 10 (beam and slab, multi-beams, and cast-in-place concrete box girders), the multi-beam formulas were the least verified by comparison to experimental data. In addition, they were developed for only simple span and non-skewed structures.

Background for the Current AASHTO Equations

The current multi-beam distribution equations given above were developed by Sanders, Elleby, and Watanabe (Refs 10 and 12). Various bridge configurations over a range of spans and widths were studied by selectively varying the bending and twisting stiffness. The resulting equations are a best fit to a series of curves developed for several different multi-beam sections including channel (weak in torsion) cross-sections. Thus, the equations do not specifically suit the case of closed-box cross-sections which are almost as strong in torsion as solid slabs.

The method of analysis used by Watanabe (Ref 12) was an articulated plate analysis with limiting assumptions: (1) that the structure was orthogonal (no skew), (2) that the shear key connections were assumed completely hinged (see further comment below), (3) that the wheel loads were assumed at

mid-span, and (4) that Poisson's ratio was neglected.

The computational technique used assumed, (1) that the plate was of the same uniform bending stiffness in the longitudinal direction at all transverse locations, (2) that the transverse stiffness was taken as zero not only at the location of the shear keys but also at the areas in between, and (3) that the load was represented as a harmonic series.

Limiting assumption (2) and computational assumption (2) above disregard the effect of transverse bending and stiffness characteristics between the shear keys. This area can be proportionately quite large if the box depth is shallow in relation to the box width. The Texas DHT uses box width modules of 4 and 5 feet which are combined to achieve the desired total structure width. Thus, for the multi-beam box girders of the type shown in Figs 1 thru 5, if the distribution equations (Ref 2) or analysis procedure (Ref 12) are used, significant omissions in transverse structural strength result.

Therefore, as a partial result of the limitations in the current AASHTO distribution equations, and recognizing that many structures are built with skewed geometry as shown in Figs 1 and 5, the research described in this report is deemed to be of significant importance to the Texas DHT.

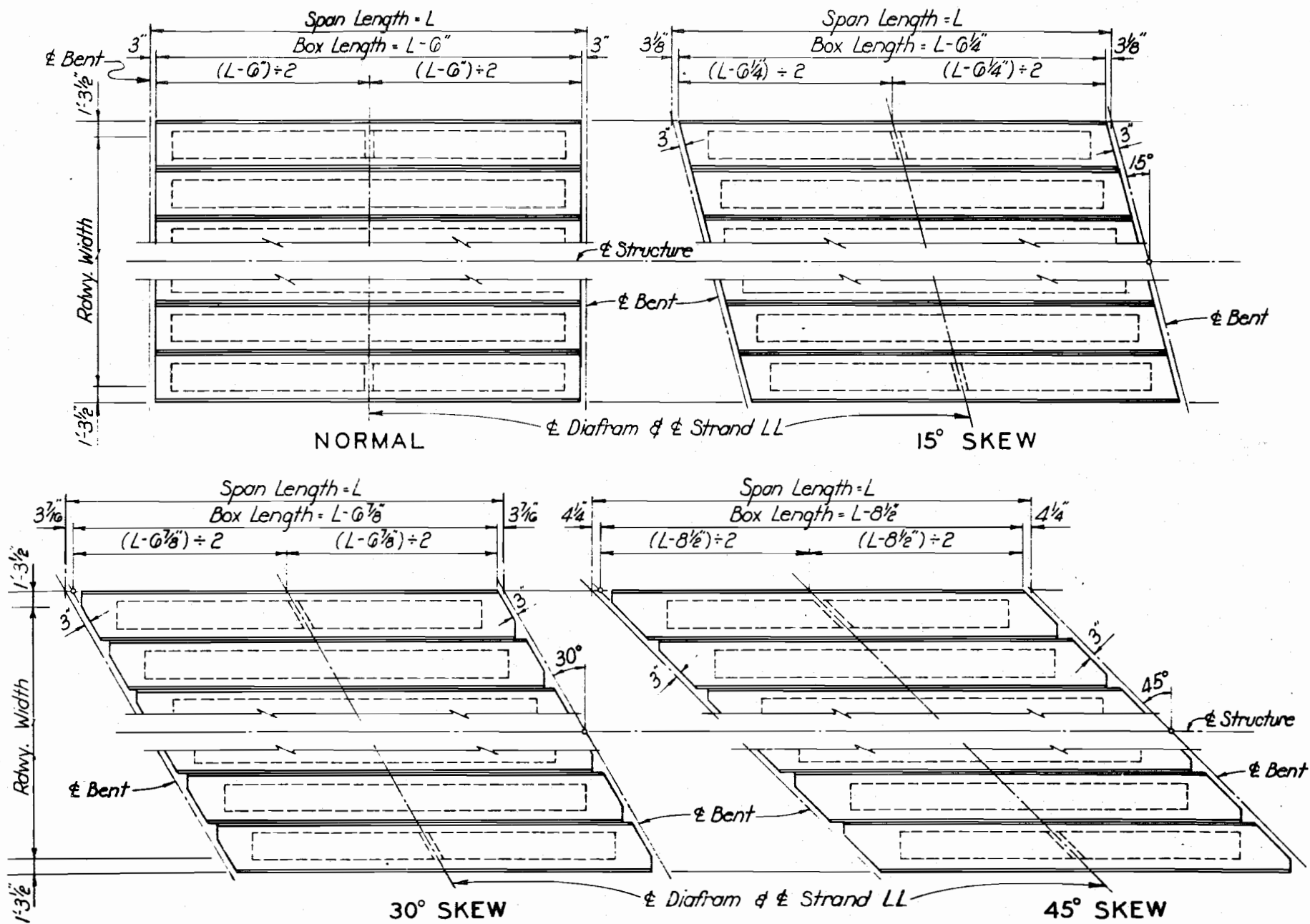


Fig 5. Typical span details. Longer spans have more diaframs.

CHAPTER 3. THE ANALYSIS METHOD

The method of analysis used throughout the study was developed on another research project during the period from 1966 through 1972. That project was "Development of Methods for Computer Simulation of Beam-Columns and Grid-Beam and Slab Systems," the major results of which are summarized in its final report (Ref 9). The specific analysis method used was one documented in Ref 8 which presents a computer program for the discrete-element analysis of isotropic or orthotropic slabs and plates on elastic supports. The program has been in successful day-to-day use by engineers of the DHT and by other agencies and organizations for several years. The program allows for the free variation of stiffness, supports, and loads. Skewed support conditions can be easily handled by appropriate definition of the support locations which procedure is described below. Another similar analysis procedure (Ref 11) was also available which was formulated specifically for the skewed anisotropic discrete-element plate solution. The selection of the first method (Ref 8) was made because of limitations of the second (Ref 11) when applied to this particular type of problem. The methods had been previously demonstrated (Ref 1) to give extremely good correlations with experimental data for the various types of beam and slab bridges considered prior to this study. The field load test performed with this study also correlated extremely well with values computed by the analysis method.

As a consequence of the generality of the analysis method (Ref 8), the SLAB 49 program requires that a significant amount of data be coded. Due to the numerous variables of this precast box girder study, it was necessary to modify the program slightly to accept the data generated by the data generator.

Skewed Supports

The SLAB 49 program allows supports to be placed at any grid intersection. Due to the geometry of most skewed spans, the support locations and grid increment sizes cannot usually be selected to properly represent the skewed support line. A substitute support system can be achieved by manipulating the program's support spring and axial thrust stiffness terms as will be described below for the support system model.

Data Generated

The data generated by SLBDG 2 for use by the SLAB 49 program solver is comprised of bending stiffnesses in both orthogonal directions, twisting stiffnesses, support springs and axial thrusts for the support systems, and loads which are allocated to the surrounding joints from any type of load pattern.

Bending stiffness is computed in the span direction on the basis of the input cross section moment of inertia of each box beam, the input modulus of elasticity, and Poisson's ratio. The transverse stiffness is based on the sum of the bending stiffness of the top and bottom slabs of the box beams with full depth stiffness used from void edge to void edge except in the center of the joint between boxes. The SLBDG 2 program allows input of any degree of transverse shear key bending stiffness, but it was found that nearly zero was appropriate. A series of problems was run with a much greater transverse void stiffness which was equal to the transformed inertia of the top and bottom slabs. This modeling approximation did not match the test data from the test loadings nearly as well as the first assumption. The first procedure of summed slab stiffness was used from the beginning of the project and is still considered to be the better approximation of the transverse stiffness. Sanders, Elleby, and Watanabe (Refs 10 and 12) assumed a zero stiffness in the transverse direction completely across the full width of the bridge. This model is

obviously only correct for articulated multi-beams which are very narrow in width. Texas box beams are four or five feet wide with nearly two feet between each void. These widths provide a significant amount of transverse stiffness in the lateral direction.

Twisting stiffness for the multi-beam boxes is computed by using the input value of estimated St. Venant torsion constant for the beam and computing the equivalent distributed twisting stiffness. As will be shown, the twisting stiffness is not a sensitive parameter for orthogonal structures, but becomes very important for skewed structures.

Sanders and Elleby (Ref 10) assumed, for a solid multi-beam cross section, that the torsional stiffness was related to a St. Venant constant of $1/6$. They stated that this value corresponds to the torsional stiffness of a small transverse section of slab and further stated that most multi-beam bridges are composed of sections with b (the width) over t (thickness) ratios of between 1.0 and 1.5. They used this as justification for the value of $1/6$ which gave them a K parameter of 0.5. This was inexplicably incorporated in the AASHTO equations as 0.7 with proportionately larger values for sections with voids up to a value of 1.0 for box sections. This investigator believes that the K of $1/6$ for solid multi-beam cross sections is incorrect and should in fact be closer to $1/3$ thus doubling the effective twisting stiffness. As stated above, the twisting stiffness has little effect on the behavior of an orthogonal structure, and since Sanders and Elleby did not investigate any skewed structures, the discrepancy therefore was not readily apparent.

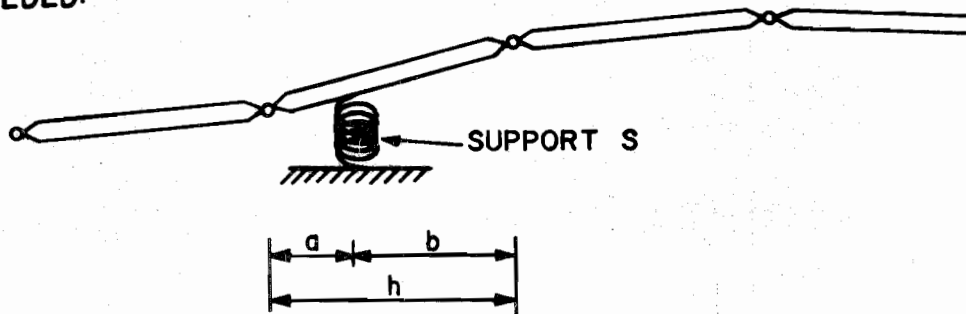
The twisting stiffness of the articulated box system may be compared to that of a slab or plate which has hinges in one of the orthogonal directions. A continuous plate with no hinges subjected to a pure twist has predictable behavior which can be solved both by SLAB 49 and in closed form. The orthogonal bending moments for a pure twist case are zero. If the plate has hinges

introduced along one or both orthogonal directions, then the moments for the pure twist case remain at zero with the same twisting moment as before. In addition, the deflections of the plate are also exactly the same as when there were no hinges. This analytic test then demonstrates that introducing bending hinges in a system, such as shear keys between multi-beams, has absolutely no effect on the twisting behavior of the system. Sanders and Elleby (Ref 10) and Watanabe (Ref 12) chose to reduce the effective twisting stiffness of the system solely because of the articulation. They did this by comparing the twisting stiffness to that of a narrow beam rather than the twisting stiffness of the equivalent slab. As stated previously, and also as demonstrated by Watanabe when he compared solutions with only longitudinal torsional stiffness to solutions with both torsional stiffnesses, the effect of torsional stiffness is small for structures which are rectangular. Therefore, the apparent error in considering torsional stiffness made in Refs 10 and 12 does not effect their results appreciably. For a skewed structure however, it is extremely important that the effective twisting stiffness be properly used in the analysis.

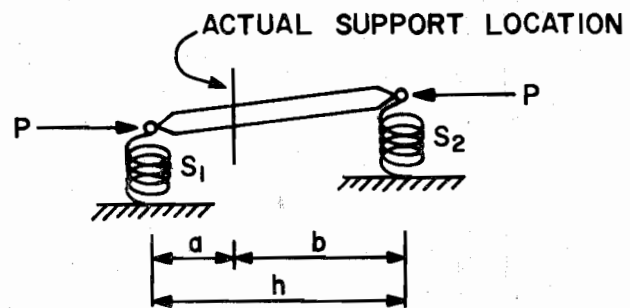
Support systems were placed along each station line in the span direction at the ends of the span. Figure 6 shows the equivalent support system which is an exact mathematical substitute for a spring which may occur between joints in either of the major grid directions. The substitute support system model is composed of two springs at adjacent joints on each side of the actual support location, the sum of whose moduli is equal to the actual spring support value as shown in Fig 6. These two springs if acting alone would provide a restraint to rotation. This restraint is exactly countered by an axial thrust acting between the two joints whose magnitude is a function of the spring value and the geometry also as shown in Fig 6.

A spring which may be located randomly within a grid area cannot, however, be represented by this simple substitute model. This case would require the

NEEDED:



EQUIV. MODEL: (SUPPORT SYSTEM)



$$P = \frac{abS}{h}$$

$$h = a + b$$

$$S = S_1 + S_2$$

$$S_1 = \frac{bS}{h}$$

$$S_2 = \frac{aS}{h}$$

Fig 6. Computer model for support systems

interaction of a counter to the additional twisting stiffness which is offered by four springs surrounding a supported grid area. The modeling relationships for this have not been formulated as yet, but it is anticipated that the countering effect may be modeled by introducing negative springs of the appropriate values.

The magnitude of the support spring S was selected to be approximately 1.0×10^6 which is large enough to represent a pinned support with nearly no base movement. If the spring value is selected much larger, solution difficulties are encountered since the axial thrust P terms are introduced in the off-diagonal terms of the SLAB 49 stiffness matrix. A spring at a grid intersection can have any arbitrarily large value since support S terms are entered in the main diagonal of the stiffness matrix.

Loads to the SLAB 49 program are assigned by SLBDG 2 to the joints surrounding the actual load placement in proportion to the relative distances from the load to the joints. Any loads falling outside the boundaries of the skewed slab are disregarded. A message is printed with the output for the sums of all loads on or off the structure which allows a check to be made on load input. Loads input within one increment of the skewed ends are assumed all on the structure even if the allocated partial load may be assigned to a joint outside the boundary. These joints are necessary to allow the support systems described above to operate correctly.

CHAPTER 4. PARAMETER STUDIES

At the beginning of the project, it was anticipated that by studying the various parameters and their individual effects on the resulting distribution, a series of relationships could be established which would allow a designer to properly account for them. Unfortunately, the complexity of the interactive effects has made it almost impossible to relate them into a usable design tool. It has been found that skew angle has the largest effect with twisting stiffness and load placement also contributing significantly. Therefore, emphasis has been placed on demonstrating the relative effects of skew angle, aspect ratio, twisting stiffness, etc., and making the developed analysis technique as simple to apply as possible.

Skew Angle

As originally anticipated prior to beginning the project, skew angle was to have a significant effect on the computed longitudinal bending moments of the individual box beams making up a multi-beam bridge. Chen, Siess, and Newmark (Ref 5), and Kennedy and Gupta (Ref 6) observed this same effect in their studies of skewed bridges. An I-beam or T-beam bridge is much more flexible in twisting stiffness so the effect of skew on reducing moment isn't as much as it is on a slab or articulated slab type structure. A multi-beam bridge composed of relatively stiff box sections with large areas of solid concrete between the voids such as shown in Fig 1 is almost as stiff in torsion as a solid slab of the same depth. Slabs have the highest torsional stiffness in comparison to bending stiffness of any bridge structure.

A particular structure was selected to demonstrate the effect of skew angle as shown in Fig 7. The span was 70 feet with 9 four foot boxes 34 inches

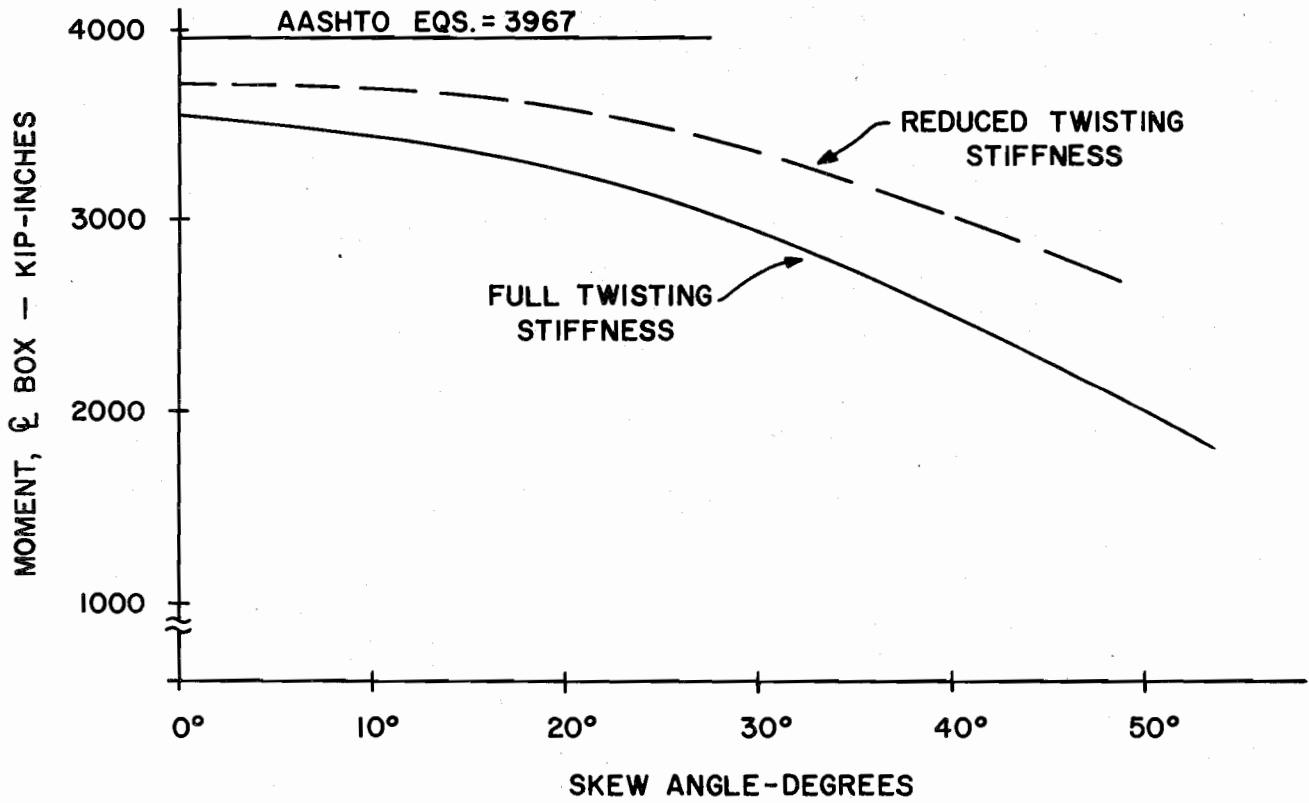
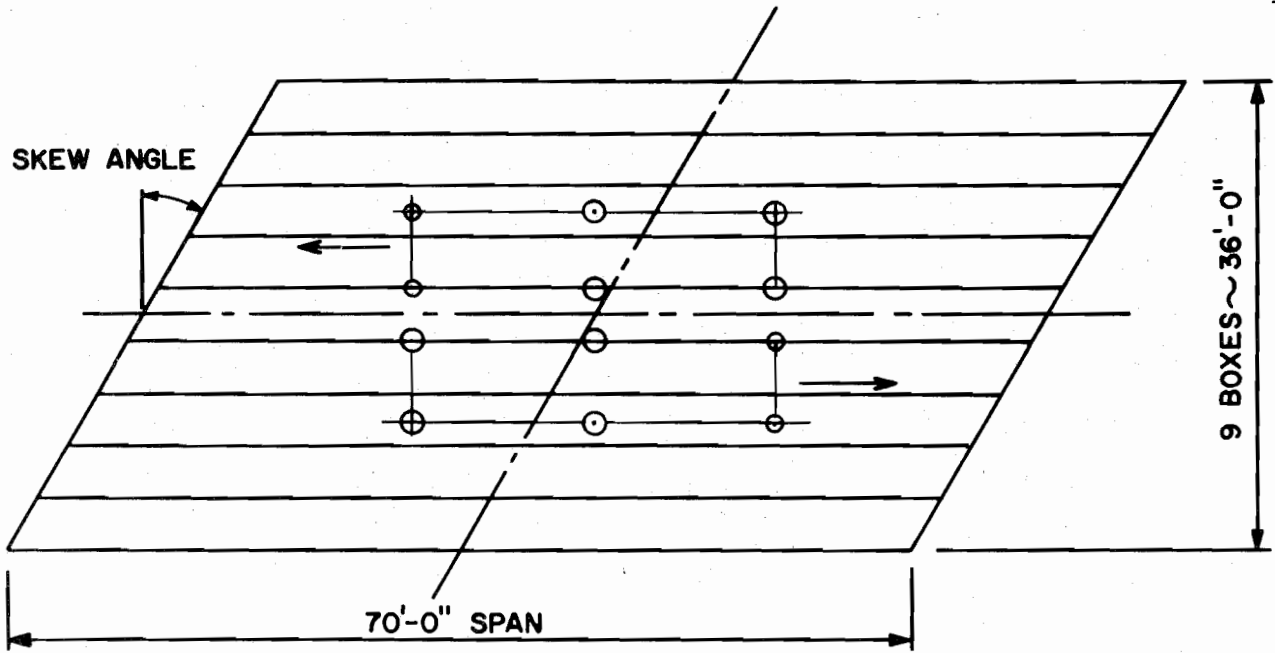


Fig 7. Showing effect of skew and twisting stiffness on bending moments for the center box. Boxes are 34 inches deep.

deep giving a 36 foot total structure width. Two HS20 trucks 4 feet apart with 25.6 percent impact were placed as shown in the plan view. The maximum bending moments for the center box are shown in the plot for various skew angles. As can be seen, there is a significant reduction in live load bending moment as the skew angle increases. At zero degree skew, the moment is about 90 percent of that as computed by using the AASHTO equations which were developed in Refs 10 and 12. This 10 percent reduction is believed to be due to the use of the transverse stiffnesses in the included method of analysis. Also, the torsional stiffness is greater than assumed in Ref 10. At a skew angle of about 50 degrees, the moment is only half of that computed by the AASHTO equations. Therefore, it is obvious that for skewed multi-beam structures, the skew must be included in the analysis. There has been some difficulty with excessive cambers due to the prestress forces in this type of bridge. By reducing the required design live load moment, there will be less prestress force and consequently less camber.

Twisting Stiffness

A supplementary study was made for the same box depth, span, and width with a variation of the twisting stiffness. This twisting stiffness was one-half that in the study described above for skew. The half value of twisting stiffness corresponds to the value used in Refs 10 and 12 which is believed to be somewhat in error as previously discussed. As shown in Fig 7, the effect of this change in twisting stiffness is small for a zero degree skew and changes the computed moment from 90 percent of AASHTO to about 94 percent of AASHTO. However, at larger skew angles, the reduced twisting stiffness causes a significantly greater effect. For instance, at a 50 degree skew, the moment is increased from 50 percent of AASHTO to 67 percent which corresponds to an actual increase in moment of about 34 percent. Therefore, it can be seen that twisting stiffness is more significant for skewed spans.

Aspect Ratio

The aspect ratio of a bridge is the measure of its span to width. For the same bridge width, a doubling of the span doubles the aspect ratio. This effect can be seen in Fig 8 which shows the computed moment as it varies with span and skew angle for a nine box arrangement with two trucks. The plot of moment is as a percent of the computed AASHTO moment (Ref 2). The zero degree skew line is nearly horizontal and therefore verifies that the AASHTO equations properly account for aspect ratio and the increase in moment with span. As the skew angle increases, the moment lines are still nearly horizontal and actually increase with longer spans for this case. Fig 9 shows the effect of aspect ratio more directly. The plot is of the amount of load carried by a central beam as it varies with span and skew angle. As can be seen, the effect of aspect ratio is very small. The zero degree skew has the largest effect. The plot of load carried by a central beam was computed by comparing one fourth of the simple beam moment for a line member subjected to two HS20 trucks with impact (four lines of wheels) to the moment computed by the discrete-element analysis procedure for the central beam of a nine box arrangement with the same two trucks. Again, as was seen in Fig 8, the skew angle is the predominant effect. The AASHTO equations give values which are about 11 to 12 percent higher than the zero degree skew solution by discrete-element analysis.

Load Placement and Skew Arrangement

A number of studies were made with different positions of two and three truck load placements for various skew angles and spans. It was found that in general, for any square or skewed multi-beam span (at least through a skew of 50 degrees) that the maximum bending moment for an HS loading occurs with the central heavy axles of two or more trucks aligned with each other perpendicular to the longitudinal centerline. The second heavy axle of each truck is then

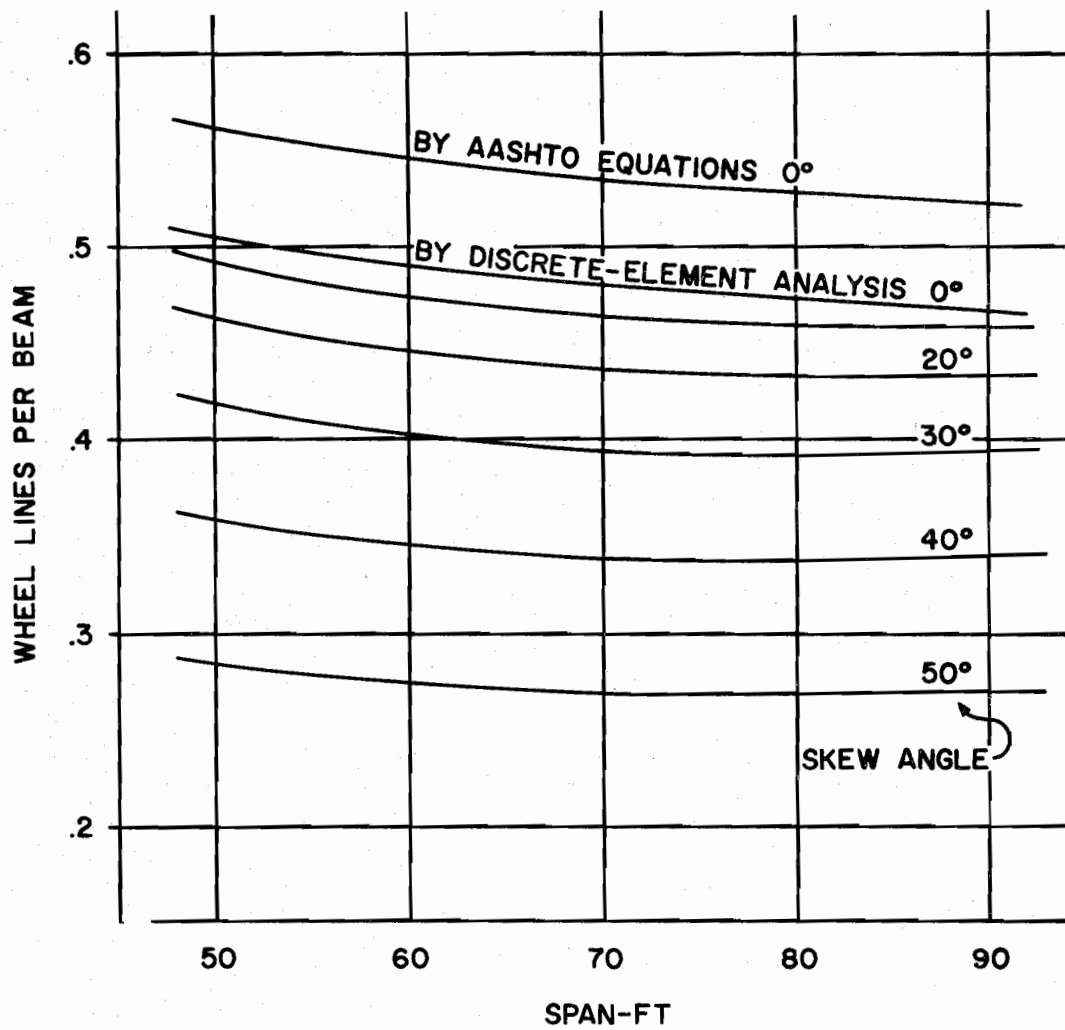


Fig 8. Effect of skew angle and span on computed maximum bending moment.

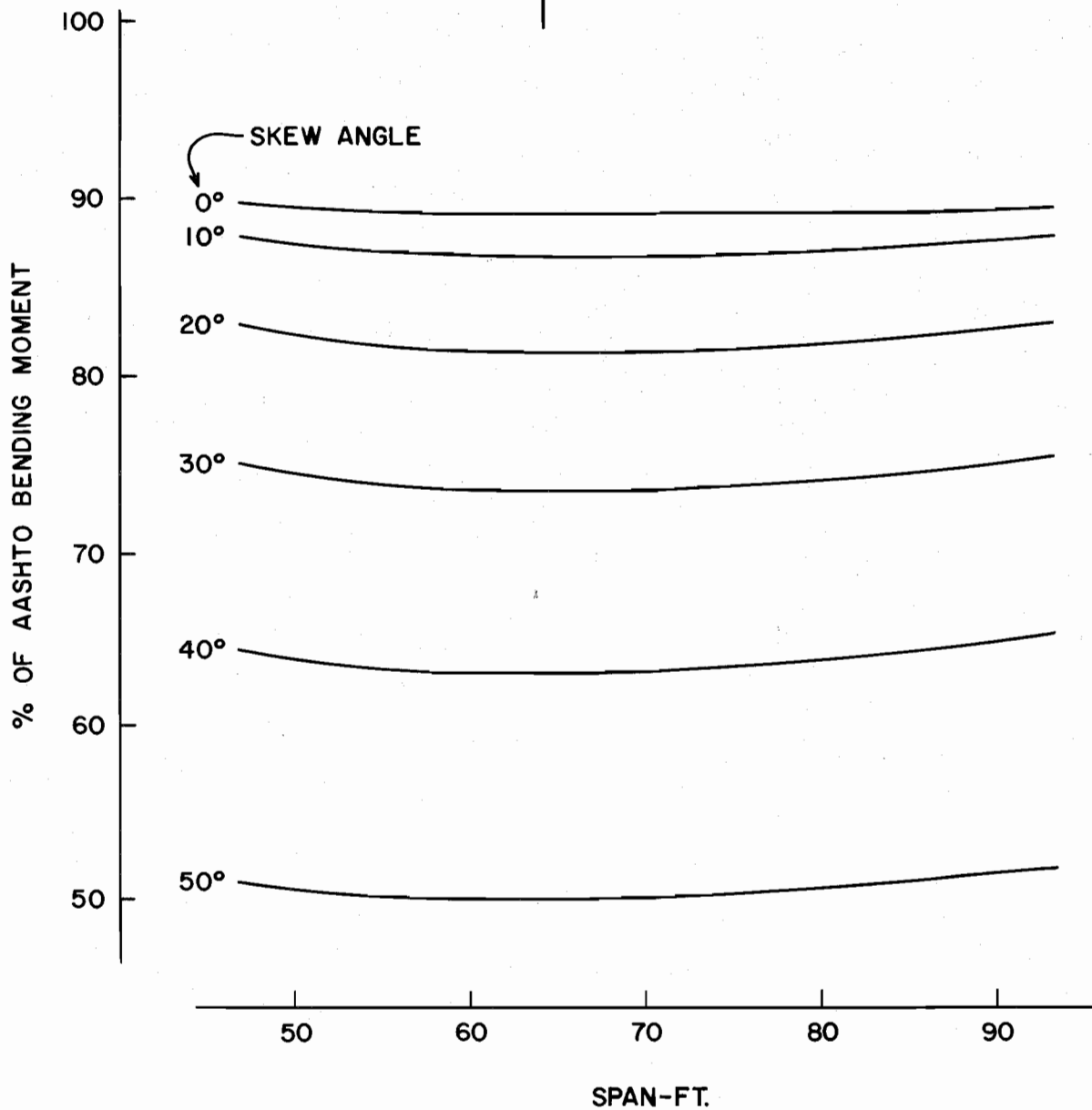
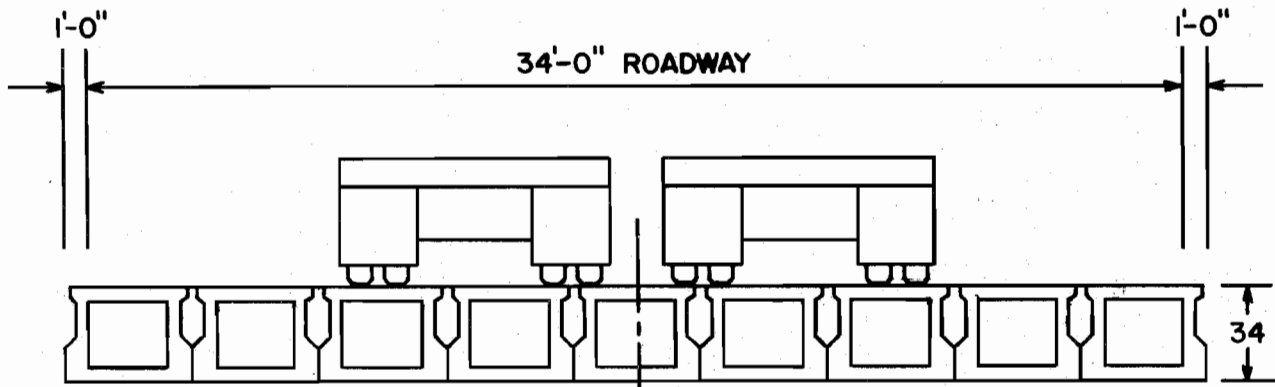


Fig 9. Effect of skew angle and span on load carried by central beams. Two trucks on nine 34 inch by 4 foot boxes.

placed on the side which puts it nearest the skew centerline as shown in Fig 7. Remaining loads (front axles) are placed on the opposite side of the central axle.

Due to the above effect of load placement, it can be seen that for spans with two lanes of traffic proceeding in opposite directions, the maximum will occur with a left forward skew. A right forward skew would have somewhat less maximum bending moment. In addition, spans with traffic proceeding in the same direction would also have slightly less moment. For these two reasons, the included studies were for two and three truck loads with the individual truck directional orientations such that they produced the maximum. Left forward skews were also always used.

CHAPTER 5. THE DATA GENERATOR PROGRAM

Program SLABDG 2 is written to essentially generate data card images which are used by program SLAB 49 (Ref 8) for analysis of multi-beam spans. As described in Chapter 3, support systems are created at the beam ends to properly account for the effect of the skewed supports.

Data Input

The procedure for input of data for analysis of a specific skewed multi-beam bridge is outlined in the following description and in The Guide for Data Input which is included as Appendix B. The guide is designed so that additional copies may be made and used for routine reference. A parallel study of the guide will help the reader understand the following discussion.

The first two cards of a problem series are for identification purposes. The Problem Series File No. (PSF No.) is for data card filing purposes. The County, Route, Control, Section, etc should usually be coded along with the date and the selected units. The complete card including spaces between the fields may be optionally used in which case the County, Route, etc heading is not printed on the output. If the date field is left blank, the current day's date is automatically printed. A consistent system of units must be used for all input data. Kips and feet or kips and inches are usually the most convenient. The second identification card must always be included. The next card is the Problem Number card with a brief description of the particular problem. The problem number may contain alphanumeric characters if desired. Successive problems in the run begin with their own Problem Number card. A final card in the run with CEASE as the Problem No. indicates the end of the data.

Table 1 is used to input the problem control data and is always comprised of two data cards that include the keep options, multiple load option, number of cards input for this problem, an option to print the generated SLAB 49 data card images, and the skew angle of the span. The skew angle is measured between a perpendicular to the span centerline and the support centerline. A left forward skew is positive. In the current version of the program, a negative skew angle is disallowed, since there has been insufficient time to verify that a negative skew angle operates properly.

Table 2 is used to specify the constants for the span. These are the number of increments in the X (span) and Y (transverse) directions. The increments in the span are along the structure centerline from support to support. A skewed span may have the same input value of number of span increments, but more total increments are generated for the SLAB 49 computer model for skewed spans. The increment lengths are also input in Table 2. The increment lengths and numbers of increments should be chosen so that there are at least 4 to 5 transverse Y increments available for each box beam and at least 12 to 14 span X increments. More increments (especially transverse) will give more accurate solutions, although increasing them to more than 6 transverse per beam or 20 in the span doesn't increase accuracy more than a fraction of a percent. Poisson's ratio is also entered which is usually 0.15 to 0.20 for concrete. The Modulus of Elasticity may be any generally accepted value for concrete such as 3,500 to 6,000 kips per square inch. This value only affects the deflections. The computed bending moments are independent of the Modulus of Elasticity.

Table 3 is used to input the data for each type of box beam in the span. Only one card is necessary if the span is made up of a single box type. Up to 10 different types may be used. The number of Y increments used for each box must be consistent with the desired width and the Y increment length defined

in Table 2. The box depth, void width, and top and bottom slab thicknesses are also entered. These four fields are decimal; a 5½ inch dimension is input as 5.5.

The shear key transverse stiffness factor may be used to define the degree of remaining transverse stiffness at the shear key between beams. It has been determined that a value of 0.0 is usually most appropriate for the types of multi-beam spans herein studied. The small amount of transverse post-tensioning (Fig 2) in Texas box beam spans is practically negligible in offering resistance to bending across a shear key.

The longitudinal box beam moment of inertia and torsional inertia are also entered. The moment of inertia is the value for the entire box cross-section about its neutral axis. The input torsional inertia is the value usually known as the St. Venant torsion constant for the same cross-section. This value is used by the program to compute equivalent slab twisting stiffnesses for both directions of the articulated slab.

Table 4 is a one card input which defines the arrangement of boxes in the span. The span must be assumed symmetrical about the longitudinal centerline. The arrangement is input by Type No. up to and including the box at centerline if it straddles the center-line. The first box type entered in column 5 is for the right-most box type with the last one the box type adjacent to or at the center.

Table 5 is for the input of the load patterns to be placed on the structure. If multiple AASHTO HS20 trucks are to be used for instance, then only one truck needs to be defined. Up to 10 different patterns may be used if desired. Each pattern is identified by a pattern number (beginning with 1). There may be from 1 to 6 loads in each pattern. If the desired pattern has more than 6 loads, then multiple patterns may be used which can be overlapped by their placement in Table 6. Each load in the pattern is referenced to a local pattern coordinate system, the origin of which may be one of the loads

or a central point within the pattern with no load. This local pattern coordinate origin is the point by which the pattern is placed by Table 6. The coordinates for the individual loads are in an F5.0 format. For instance, the number 12345 would be interpreted as 12345.0. The number 67.89 is interpreted as input. The loads are also decimal values with a negative sign implying a downward load.

Table 6 is for input of the placement of the load patterns defined by Table 5. Each pattern may be placed at as many as 6 different locations on the span for each problem. This allows for instance, definition of one truck pattern by Table 5 with up to 6 like trucks placed simultaneously. The placement is by pattern number. The coordinate placement reference is to the span geometry. The centerline of the span at the intersection with the first support is considered to be the structure origin as shown on the sketch included on the fifth page of the input guide.

Table 7 defines the locations in each box for which the total longitudinal bending moment will be tabulated. The box numbers to be summed may be any box across the transverse width of the structure. For instance, if the span had 9 boxes, the first 5 would have been defined by the arrangement input in Table 4, but sums could be made for all 9 boxes. The sum is made at the nearest X-increment in the SLAB 49 computer model to the X-distance input in Table 7 from the structure origin.

Table 8 is essentially identical to the Table 8 used by SLAB 49 with the exception that the profile input areas have from and thru distances referenced to the structure origin rather than by station numbers. The profile output however, is referenced to the nearest X or Y station of the computer model.

Table 9 is to allow the user to eliminate the majority of the detailed SLAB 49 output if desired. It is exactly analogous to the Table 9 used by SLAB 49 with the exception that the Y-bounded areas are by distances to the

structure origin rather than by station. Again, as in Table 8, the nearest station to the distance is used in presenting the output.

Parent and Offspring Problems

The SLBDG 2 program provides the user with the necessary ability to study the span with different arrangements and numbers of load patterns. As described in Chapter 4, the load placement which usually creates the maximum bending moment for a center box is one with the central heavy axles of the load patterns aligned perpendicular to the span centerline. It is sometimes advantageous however to study different placements. This is best done by a series of problems. The first is the Parent problem defined as a +1 in Table 1 input with all the required geometry and stiffness data input in subsequent tables. The second and succeeding problems in the series are Offspring problems defined as a -1 in Table 1. All the data in Tables 2 through 4 must be kept for an Offspring problem. Tables 5,7,8,or 9 may also be kept or new data input as desired. A new Table 6 is required for each Offspring problem wherein the new arrangement of loading is input.

Data Errors

All data is automatically checked for the common types of possible data errors. A count is made of the number of data errors in each table and the problem is then terminated with a message showing the number of data errors. The errors are (1) misuse of the multiple load option such as a -1 following a 0 in the preceding problem; (2) improper sequence of keep options for an Offspring problem; (3) mispunching of the keep options so that numbers greater than 1 are read; (4) more cards input than the maximum permitted for each table; (5) entering a zero or leaving blank necessary data such as Modulus of Elasticity, increment lengths, or numbers of increments; (6) incompatibility between the total number of Y-increments and the summation of Y-increments as

defined by Tables 3 and 4; and entering a zero or leaving blank any necessary number of loads, placements, sums, etc in Tables 5 thru 7.

Computed Results

Computed results begin by echo-print of the data in the same order and with essentially the same data table headings. This then allows the user to easily verify that his span was correctly coded and punched.

Generated Data. Data generated by the SLBDG 2 program is stored in the same data arrays as originally set up in the SLAB 49 program (Ref 8). If the user exercises the option in Table 1 to print the generated data, then all the generated card images that would have been required to directly solve the problem with SLAB 49 are printed. This option should be used until the user has confidence in the results obtained by SLBDG 2.

Tabulated Results. The computed results are listed in Y-station groups for the areas designated by Table 9. The results are deflection, bending moments in X (span) and Y (transverse) directions, the twisting moment, the largest principal moment, the angle to that value, and the support reaction. It should be noted that the largest principal moment is usually of little interest since it is a result of a large longitudinal moment and relatively small lateral and twisting moments. The longitudinal moment is the one with the primary importance for multi-beam box designs. The support reactions are also almost meaningless since they reflect the values in the springs of the support systems described in Chapter 3. The actual support reaction is the sum of the support system's two spring values. The reader should refer to Ref 8 for a detailed description of the SLAB 49 solution process and results if he so desires.

Following the tabulated results is a final value of the summation of the support reactions. This value, in addition to the maximum statics check error,

should always be checked to verify that the total desired load was input properly. It should be equal in magnitude to the summation of loads applied on the structure which is printed immediately after the echo-print of the SLBDG 2 data. The maximum statics check error should always be a remnant or zero value. The user can determine if the errors are significant by visualizing the effect that loads of approximately the same magnitude would have when placed at all locations on the structure.

Box Moment Summations. Following the above results (which may be abbreviated to a single Y-station group by Table 9) are printed the summations of the longitudinal bending moments across each box at the nearest station to the X-distance input in Table 7. The equivalent computed X-distance is also printed. The moment is the total bending moment in the box. The above detailed results have moments which are given in for instance, K-in./in. units, but the summations are in total K-in. units (if kips and inches are the chosen consistent units). These summations may be inspected to determine the maximum moment and that value may be confidently used to design the appropriate prestressing for the live load.

Profile Output. If profile plots are specified by Table 8, these follow the above output. Profile plots are helpful in visualizing the behavior of the span for the given loading. The arrangement of plots is the same as for SLAB 49 and the reader is again referred to Ref 8 for a detailed description.

Typical output from program SLBDG 2 which includes results from the field load test are included as Appendix D.

CHAPTER 6. THE FIELD TEST

A field test was performed on the 7th of July, 1977 of a skewed precast box beam structure on US 181 over Dry Creek, 5.5 miles South of Beeville. The structure is part of a then almost complete construction project in the area. It was not open to traffic which was one of the reasons for selecting this structure for test. Figures 10 and 11 show views of the structure and location.

The Test Structure

The structure is composed of three 45 ft spans, skewed $37^{\circ}30'$, with a total bridge width of 44 ft 7 in. Eleven standard precast boxes 20 inches in depth and nominally 4 ft wide are set side by side in each span. The structure was on a slight $0^{\circ}45'$ curve, but was built with each span straight. The bent caps are slightly tapered so that all boxes in each span are the same length. The structure geometry is shown in Fig 12. The second span was selected for test purposes to avoid any slight unsymmetrical behavior due to the abutment. The railing on this structure is an open type which did not provide any significant added restraint to the span response from the test loading. A view of the Type 301 bridge rail which was still unconnected to an approach railing is shown in Fig 13.

The Test Loading

A three axle standard dump truck shown in Fig 14 with a nominal capacity of about 9 cubic yards was overloaded with about 13 cubic yards to a total gross weight of 70,060 lbs. The front axle alone and rear axles together were weighed separately on the contractor's scales available within two miles of the test site. The sum of the individual axle weights were within about 600 pounds

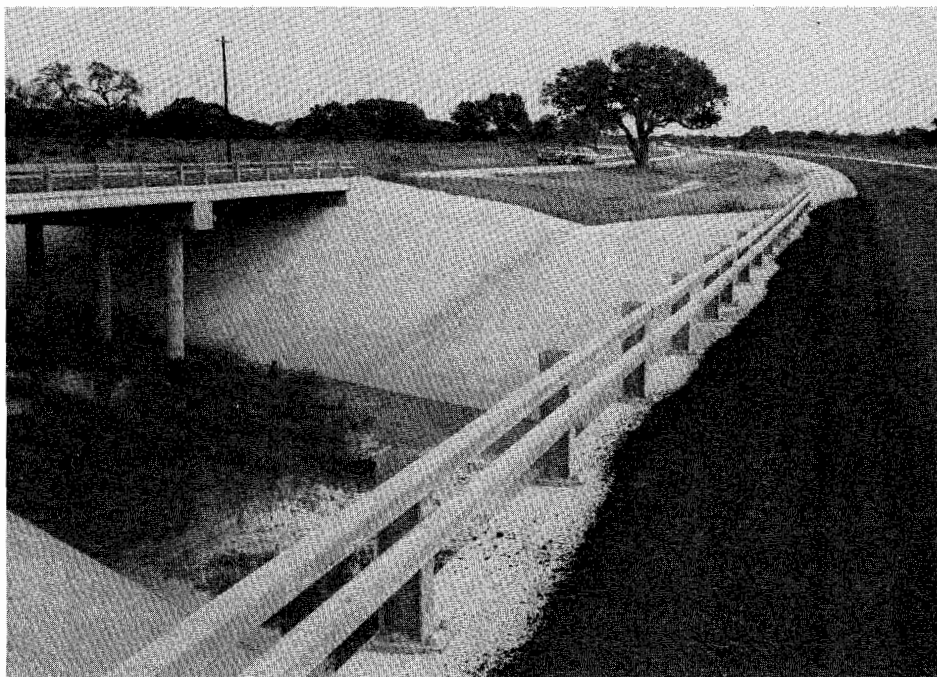


Fig 10. The test location on US 181, looking north.
The test structure is on right.

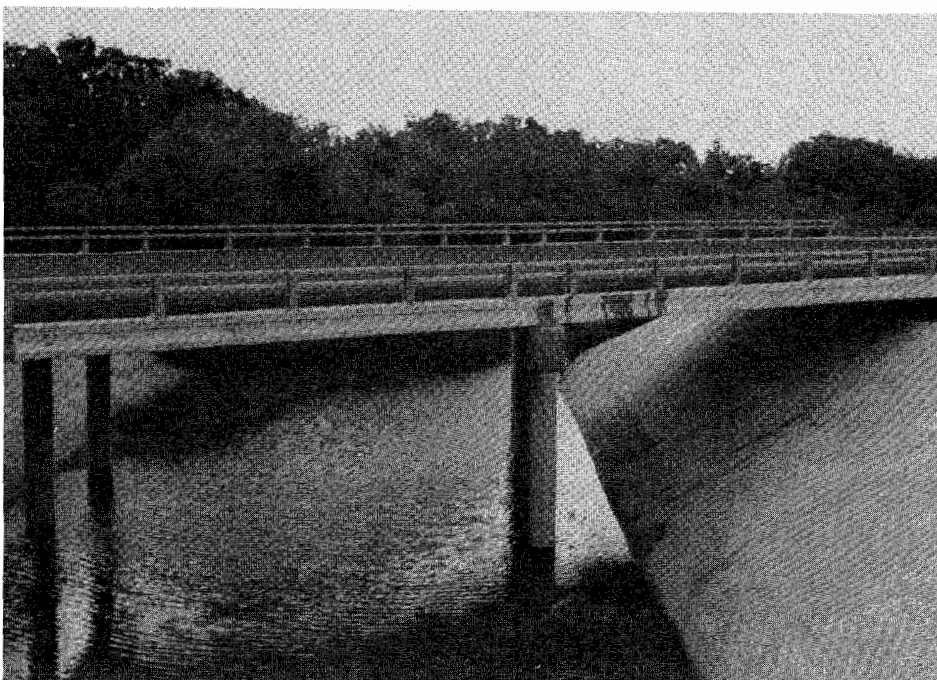
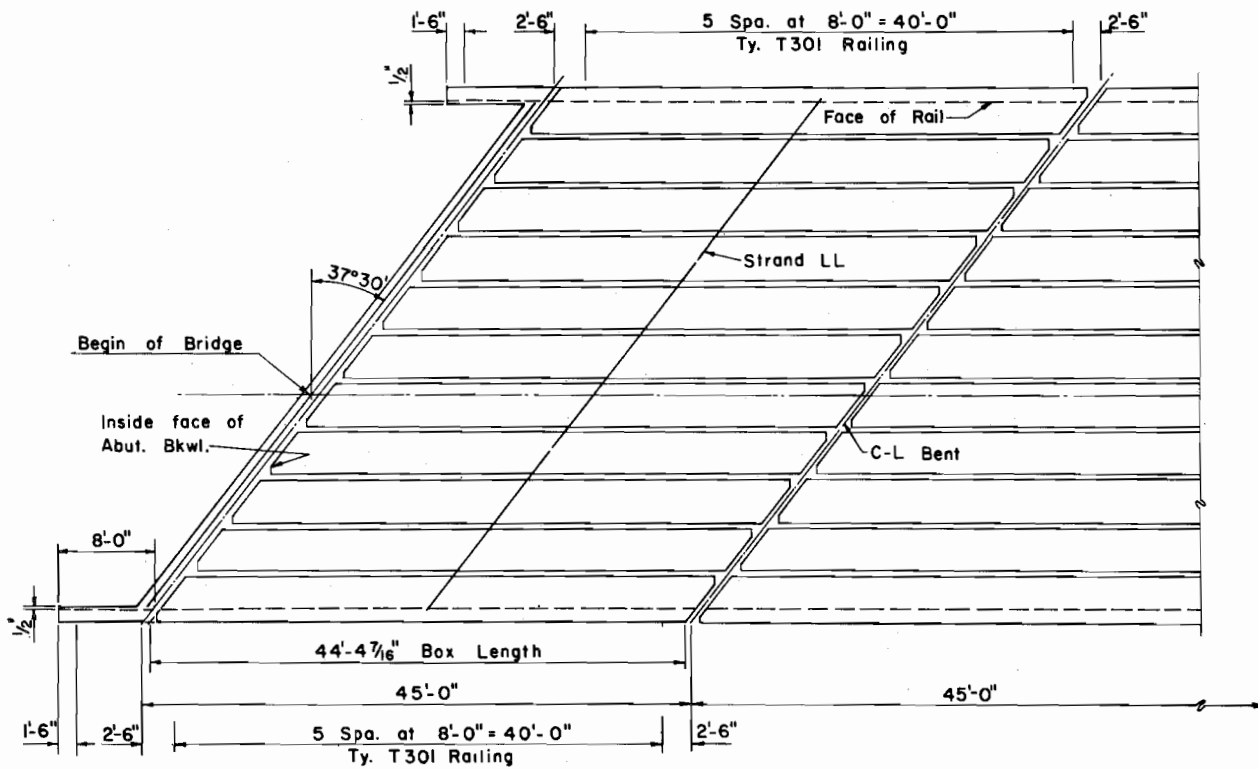
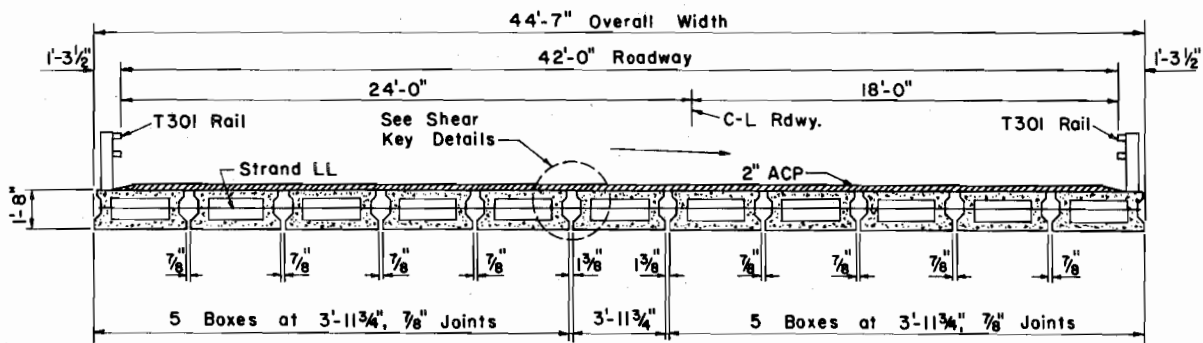


Fig 11. The test structure, looking east.



PLAN



TRANSVERSE SECTION

Fig 12. The test structure

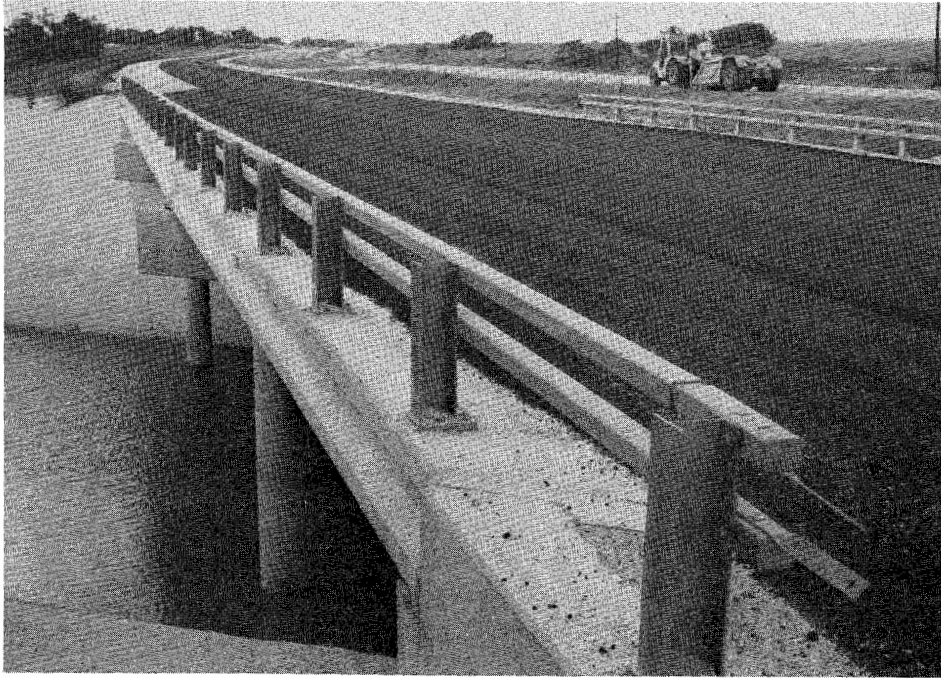


Fig. 13. The test structure looking south.



Fig 14. The test vehicle in load position 2.

of the total gross of 70,060 pounds. The slight discrepancy of less than 1 percent was most likely due to the fact that the truck was necessarily situated on a slight slope when weighing the individual axles.

The test load configuration is shown in Fig 15. The total gross of 70,060 pounds was apportioned to the wheels by means of the individual axle weights and also by assuming that the axle weight was equally shared by the two or four wheels on that axle. This load pattern was then used in the verification analyses of the test structure using the SLBDG 2 computer program.

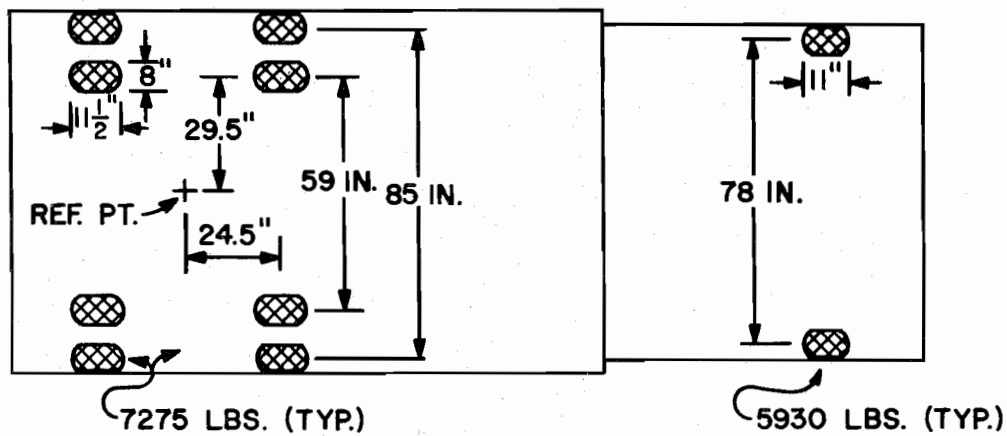
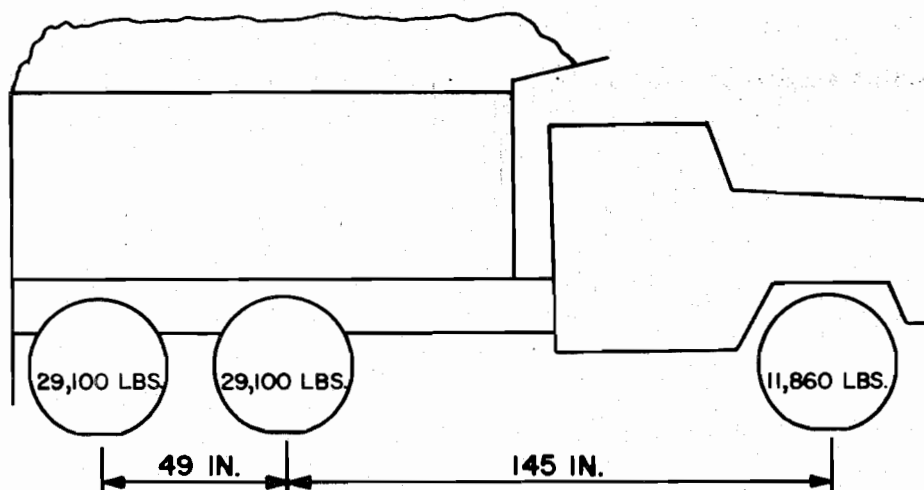
Load Placements

Three load placements were used with a complete set of readings taken for each placement. The load placement positions are shown in Fig 16. Results from placements 1 and 3 were similar but not antisymmetrical since the truck was headed north in both placements. The truck was placed as near the railing as reasonably possible with the centroid of the rear wheels positioned approximately on the diagonal centerline as shown in Fig 16.

The center to center bearing distance of 43.62 feet shown in Fig 16 was estimated to be approximately the distance to the centroid of the support under the ends of the boxes. This distance cannot be precisely defined, but is considered to be within an inch of the correct location.

Instrumentation

Deflections or stresses would have been difficult to quickly and accurately measure for a structure of this type. The maximum deflection for this load was determined to be only about 0.12 inches. Reference points, bars, and deflection gages necessary to directly measure deflections within this range would be almost impossible to establish and ensure that wind and temperature would not overpower the readings.



TOTAL GROSS LOAD=70,060 LBS.

Fig. 15. The test load

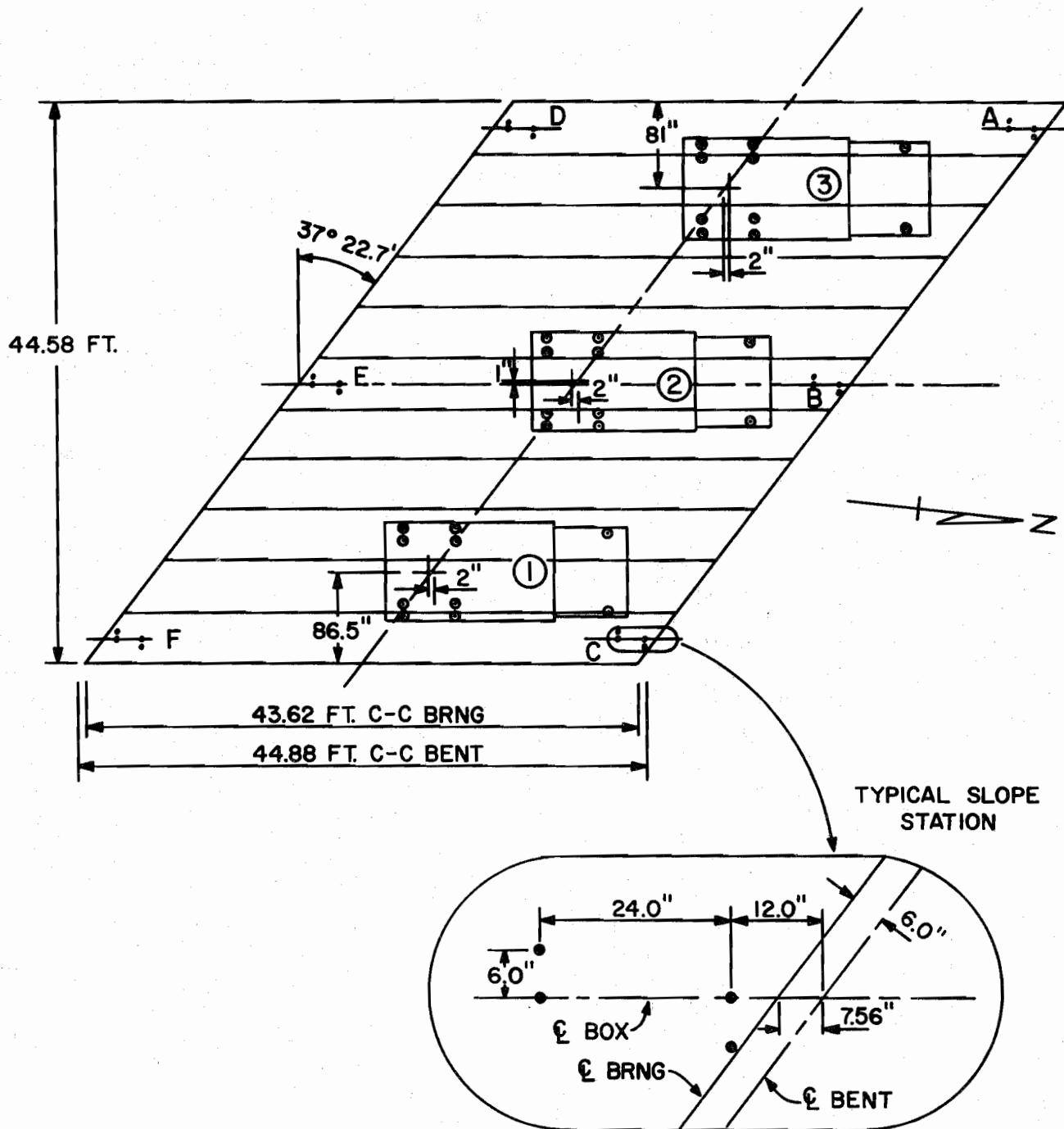
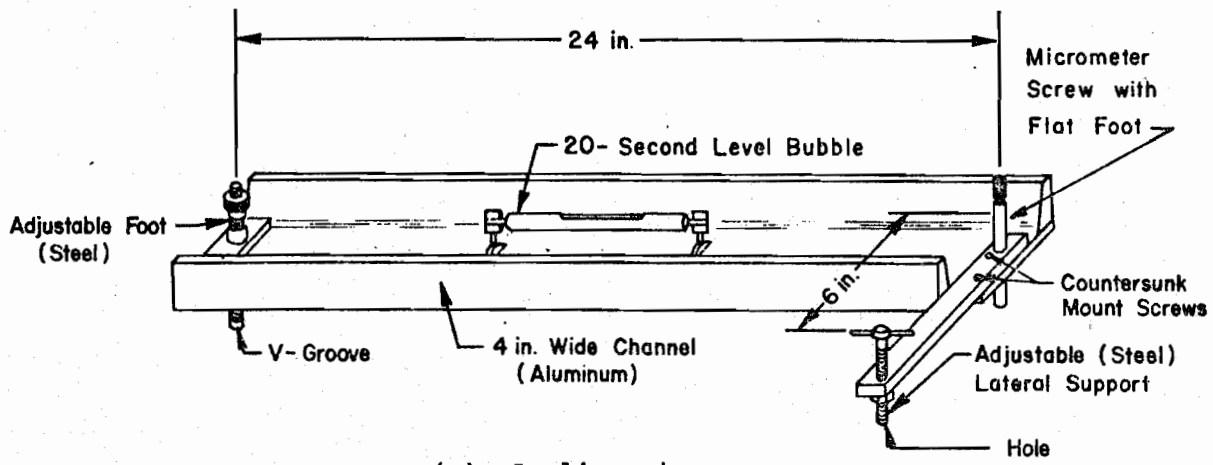


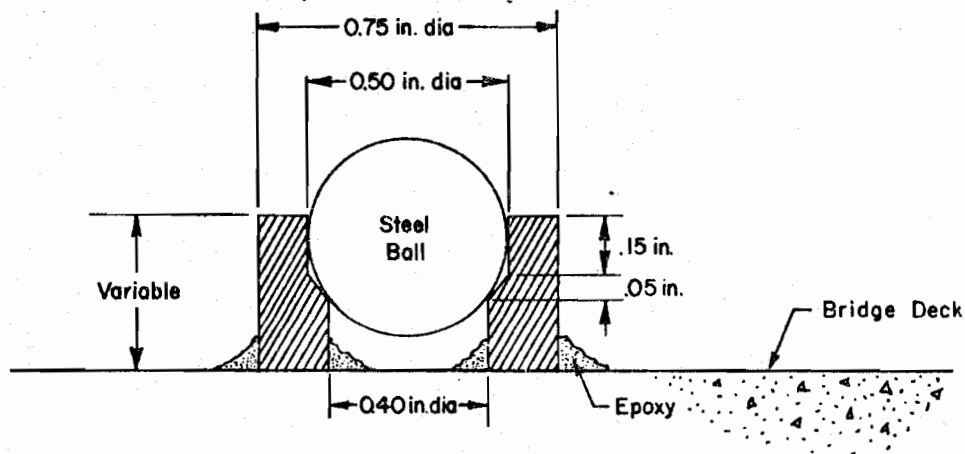
Fig 16. Showing the three load placements and six slope measurement stations.

Slope measurement has been found to be a reliable and accurate means of determining the response of bridges to test or environmental loadings. A mechanical inclinometer was developed in 1970 as part of a special study (Ref 7) to measure slope changes on a bridge tested for static live load effects in Pasadena, Texas. The inclinometer is shown in Fig 17. The inclinometer measures the change in elevation between pairs of ball bearing test points that are cemented to the bridge deck at 24-inch spacings. A typical slope station on the test structure is shown in Fig 18. The slope between the two primary points is computed by dividing the difference in elevation between the two points by the length of the inclinometer, 24 inches. Slope changes are computed by subtracting a reference slope from any other reading. As shown in Fig 17, the inclinometer has two primary steel feet in line with the level bubble, one with a V-grooved bottom and the other the flat end of a micrometer screw. An outrigger foot 6 inches to one side of the longitudinal axis of the device has a circular hole in the bottom to seat on auxiliary ball points for precise repositioning of the inclinometer. The level bubble used has a 20-second sensitivity which has been found to be approximately correct for a 0.0001 micrometer reading and a 24 inch inclinometer gage length. A finer level sensitivity creates excessive leveling times and a coarser bubble reduces accuracy.

An aluminum template, with four 7/16-inch-diameter holes was used to hold the ball points in position as they were cemented on the surface of the slab. The test points were made in various heights and were selected at each location to roughly level the balls in the direction of the inclinometer axis so as not to exceed the travel of the micrometer screw. A five-minute epoxy was used to cement the ball points to the surface of the boxes. It was necessary to remove approximately three inches of asphalt for the slope measurement points in the center of the structure. Auxillary blocks of concrete about two inches



(a) Inclinometer.



(b) Ball point.

Fig 17. Slope measuring instrument and test point (Ref 7).

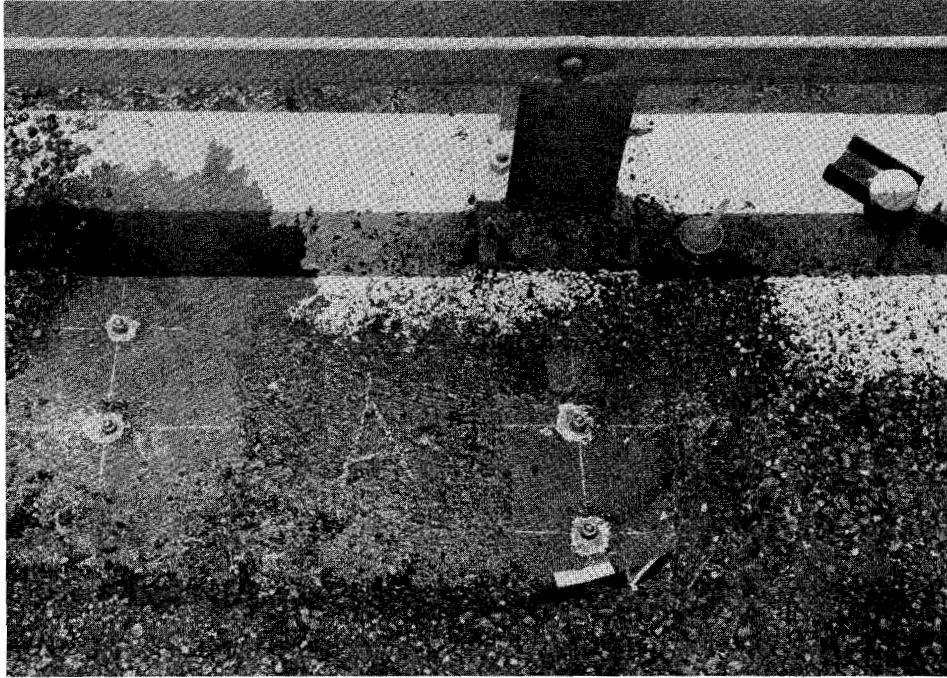


Fig 18. Slope station A.

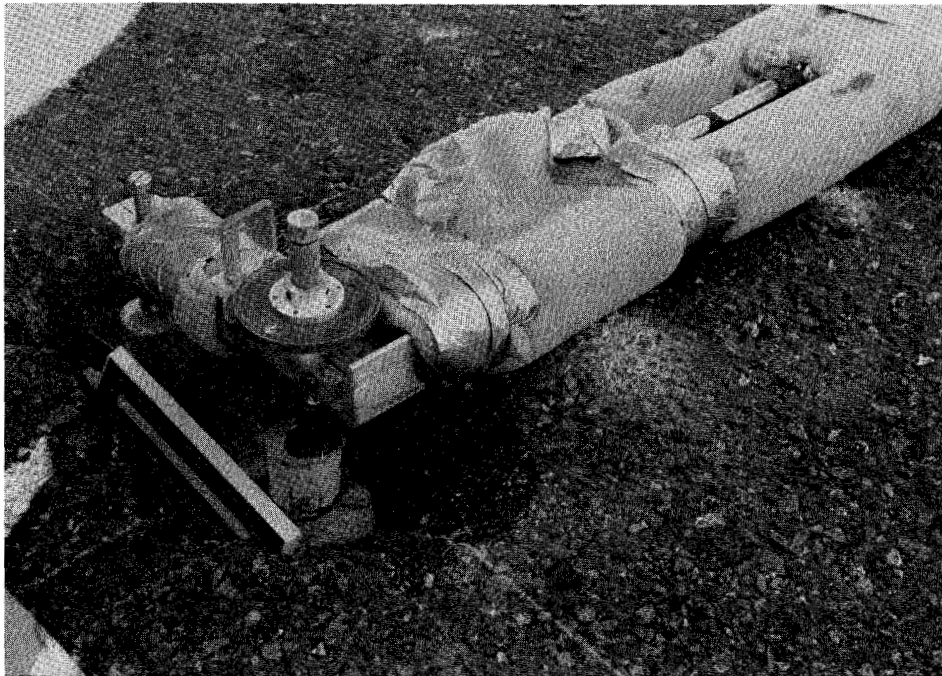


Fig 19. The inclinometer at slope station E.

high were cemented on the boxes prior to placement of the ball points to bring them above the surrounding asphalt level. One of the center stations is shown in Fig 19.

Data Recording

Each slope observation was taken with two readings of the micrometer screw, with the inclinometer reversed end-for-end between the two readings. This procedure serves to cancel instrument errors as well as to provide the possibility of self-checking the readings in the field. Only instrument errors that occur between the two readings are not canceled by this procedure. The inclinometer (insulated to retard sudden temperature changes) is shown in Fig 19 for the direct reading at Station E. Data from the inclinometer readings is summarized in Appendix A for each slope station. The inclinometer was shaded by an umbrella at all times.

A form for recording the inclinometer readings and a typical numerical example are shown in the table below. Regardless of the slope station or relative ball point elevations, the sum of any direct and reverse readings remains almost constant. Any significant variation was used as a basis for an immediate re-check of the slope readings.

Slope Station	A
Time	11:10
Direct (in)	.5750
	.0128
Reverse (in)	.9500
	.0111
Sum	1.5489
-2 x Reverse	-1.9222
=2 x Diff Elev	-0.3733

Direct readings were taken with the micrometer screw seated at the north reference points with the reverse readings at the south reference points. The

two entries for each direct and reverse reading are the two readings made of the micrometer screw. The first is the coarse reading of the screw barrel which has 0.025 inch divisions. The second is the vernier reading to 0.0001 inch divisions. This procedure eliminated the need to make a mental addition in the field and thus many errors were avoided. The sum of all four readings was performed in the field, and as can be seen in Appendix A, the sums remained almost constant at 1.55. The slight variation in sums is thought to be due to a gradual heating of the inclinometer through the test day with a shift occurring after noon due to mishandling. The actual elevation differences are still considered accurate to about 0.0002 inch. Any error from temperature or mishandling would have had to occur between the direct and reverse readings; this would have been immediately apparent resulting in a new set of readings.

Summary of Measurements

Six slope stations were located on the second span of the test structure. Each station had four measurement points of which two were the primary points and two the outrigger points for re-positioning purposes. The points were cemented near the ends of the two outside boxes and the centerline box. The primary points were located on the box centerlines at 12 inches and 36 inches from the bent centerline as shown in Fig 16.

Plots of the differences in elevation for the six slope stations are shown in Figs 20 and 21. The solid line between points shows the change in the slope readings as heating of the top surface took place. This line was then the no load reference. The dashed lines connect the slope readings taken with each of the three load positions (Fig 16). As can be seen, the response was almost antisymmetrical; that is, the slopes for Station A were like those for Station F, Station B was like E, and C was similar to D. Much greater slope changes occurred in the exterior boxes with the exterior loadings than for the center box with center loading. This does not imply that the exterior boxes should

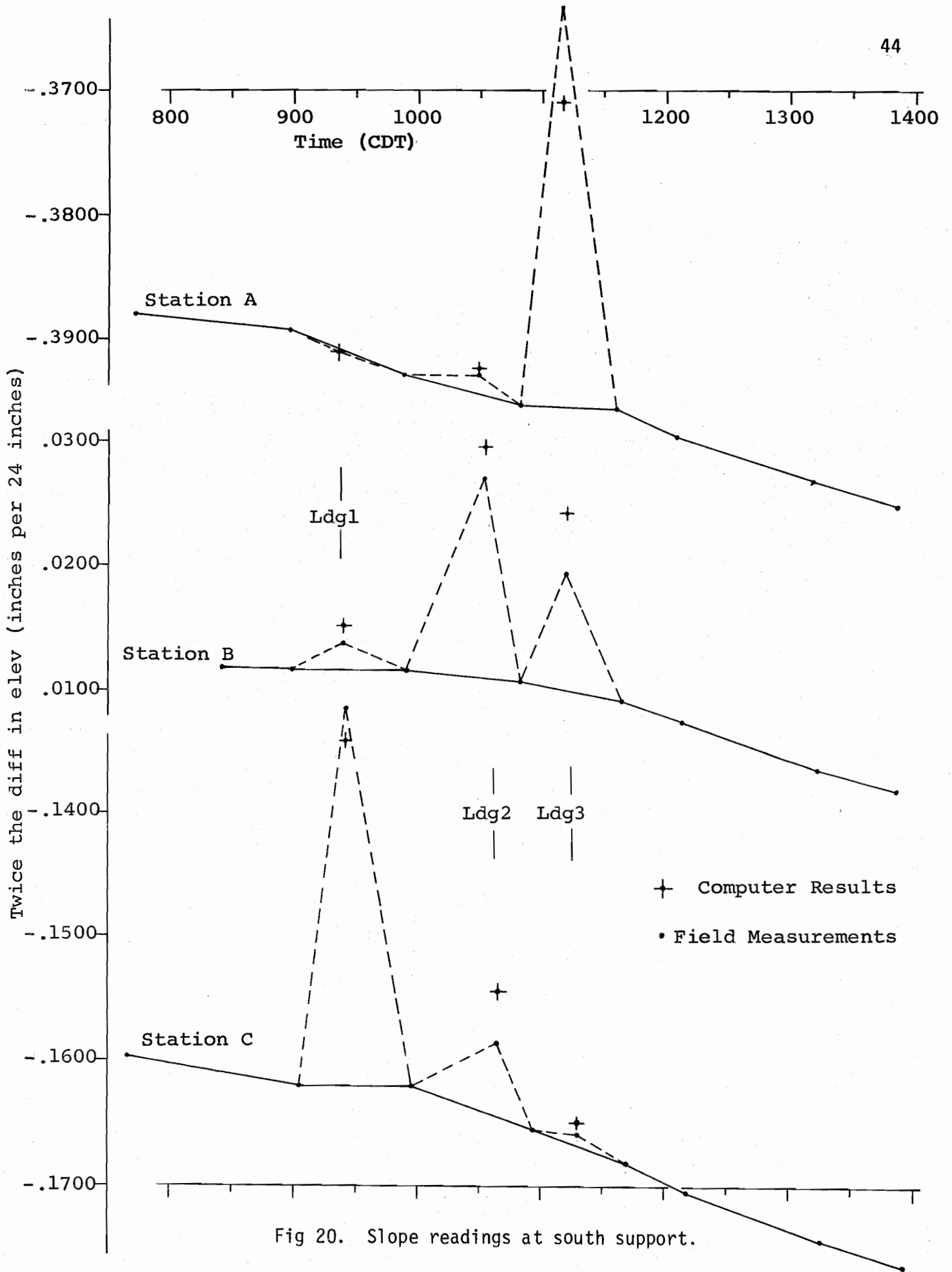


Fig 20. Slope readings at south support.

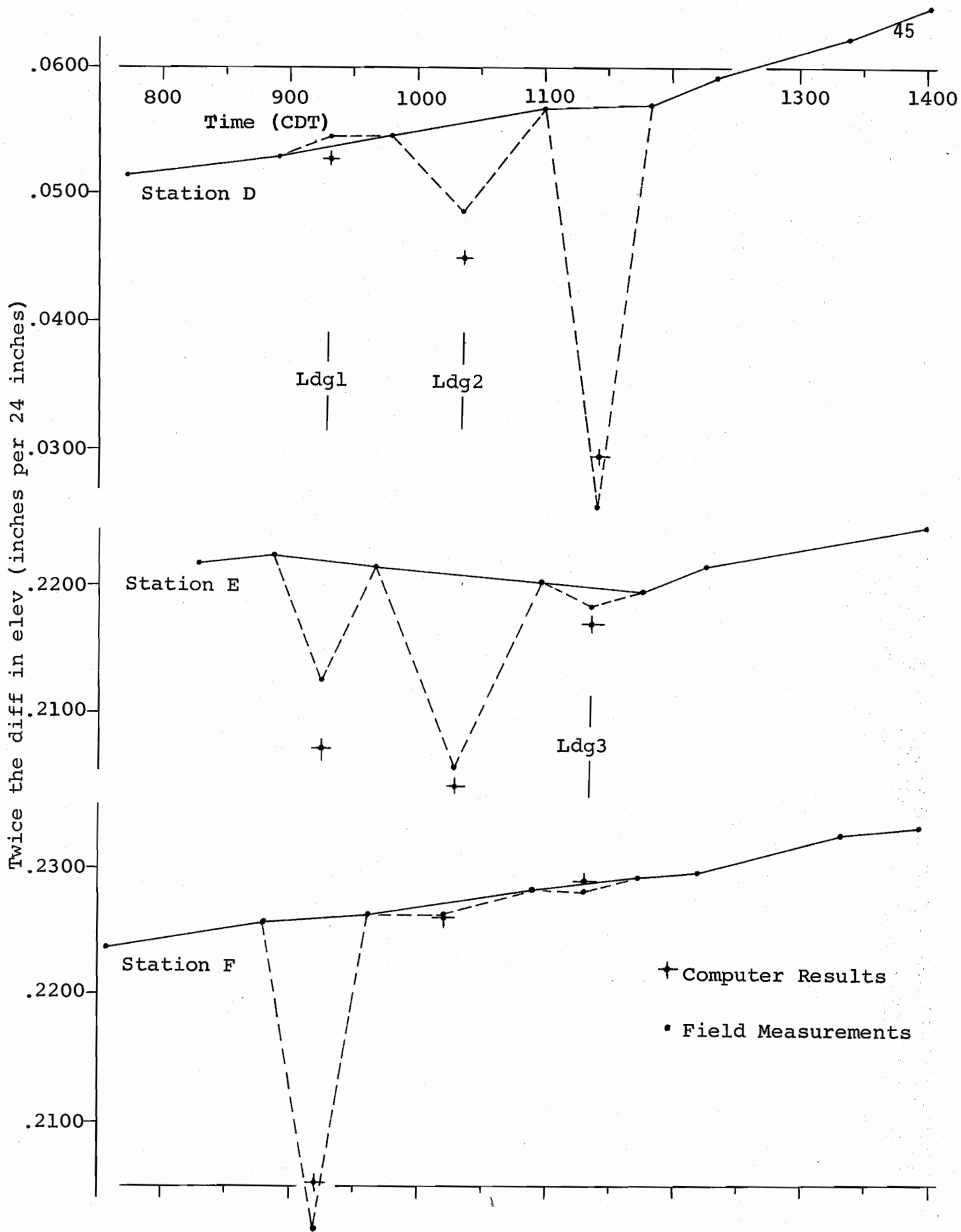


Fig 21. Slope readings at north support.

be designed for more load. Had two or three trucks been placed on the structure, then the maximum response of each box would be more nearly the same.

Computer Analysis Comparisons

Program SLBDG 2 was used to analyze the structure after the test to determine if the measured values could be predicted analytically. As can be seen in Figs 20 and 21, the computer values are in generally good agreement with the measured slopes. The computed slopes are about 15 percent less than the measured for the outside boxes with outside loadings and about 10 percent more for the center box with center loading. A number of computer solutions were made with various assumptions for concrete modulus of elasticity and transverse bending stiffnesses. It was found that a concrete modulus of about 6200 ksi gave the closest average match between measured and calculated. A zero shear key stiffness factor was also found to be the best model. It is hypothesized however, that due to the general heating of the structure, the central boxes may tend to prestress themselves together across the shear keys thus giving more distribution for the center boxes than for the outside boxes. It is impossible at this time to validate this hypothesis, but it seems reasonable in view of the smaller measured center box slopes.

The concrete used for the box beams had been tested at time of release and at 14 days. This data is listed in Appendix A. The average concrete strength at 14 days was approximately 7000 psi. Based on this, a concrete strength in excess of 8000 psi can be estimated to be present at the time of test loading. This strength correlates well with the concrete modulus of 6200 ksi which was used to obtain the computed results.

CHAPTER 7. CONCLUSIONS AND RECOMMENDATIONS

The problem of accurate design of skewed multi-beam structures is greatly complicated when the spans are skewed. The primary object of this study was to determine the appropriate design bending moments for multi-beam precast box members of the types currently in use in Texas.

Providing design charts and tabulations for all the variations of box width, skew, structure width, structure span, and individual structural variations such as twisting stiffness and transverse shear key stiffness was not economically justified. Development of such tabulations is possible, but it was found during the course of the project, that use of the developed data generator computer program was quite simple and it could be easily applied to specific design cases.

Verification of Analysis Methods

The analysis technique has been adequately verified by comparisons to other methods and to various field and laboratory tests by a number of investigators. A field test of a full-scale skewed structure was performed on this project and provided exceptionally good correlations between predicted and measured response. This correlation adds significant confidence to the user of the developed analysis technique.

Impact on Construction Costs

It has been demonstrated that a significant reduction in design live load bending moment can be expected by considering the skew angle and span aspect ratio for precast box beam bridges of the type currently in use in Texas. The reduction will vary from about 10 per cent for rectangular structures to about 50 percent for heavily skewed structures. The design live load reduction results in requiring fewer prestressing strands or allowing the maximum span length to be

increased for a particular box size. This second effect has a much greater impact on costs since the strand cost differential is usually small. Reducing the box size by one increment will usually reduce the cost in the range of \$1.00 to \$3.00 per square foot of structure. For instance, a 70 foot span square structure would require 28 inch deep boxes while the same span skewed 45 degrees would only require 20 inch deep boxes for the same concrete strengths.

The final conclusion and result of this project is to recommend that the SLBDG 2 computer program (which is essentially a data generator for the SLAB 49 program) be used for future design of skewed multi-beam box structures in Texas. It should also be used for more accurate analysis of these structures when subjected to planned overloads.

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APPENDIX A
FIELD TEST LOAD DATA

FIELD TEST LOAD DATA

Date: 7 July 1977 Bee County, US 181, Control 101-1-41

Dry Creek Bridge, Northbound Structure, 5.5 Miles S. of Beeville

Note: The direct readings are with the micrometer end of the slope indicator toward Beeville (to the north). Times are CDT. Results are twice the actual difference in elevation in inches per 24 inches between the reference points. A positive difference indicates that the northern most point is higher.

Slope Station A

Load No.	-	-	One	-	Two	-	Three
Time	7:41	8:57	9:21	9:51	10:28	10:48	11:10
Direct	.5750 .0016	.5750 .0005	.5500 .0243	.5500 .0232	.5500 .0230	.5500 .0215	.5750 .0128
Reverse	.9500 .0246	.9500 .0248	.9750 .0003	.9750 .0011	.9750 .0010	.9750 .0020	.9500 .0111
Sum	1.5512	1.5503	1.5496	1.5493	1.5490	1.5485	1.5489
-2XRev.	-1.9492	-1.9496	-1.9506	-1.9522	-1.9520	-1.9540	-1.9222
=2X D.E.	-0.3980	-0.3993	-0.4010	-0.4029	-0.4030	-0.4055	-0.3733

Load No	-	-	-	-			
Time	11:35	12:05	1:10	1:51			
Direct	.5500 .0218	.5500 .0192	.5500 .0177	.5500 .0164			
Reverse	.9750 .0026	.9750 .0022	.9750 .0045	.9750 .0051			
Sum	1.5494	1.5464	1.5472	1.5465			
-2XRev.	-1.9552	-1.9544	-1.9590	-1.9602			
=2X D.E.	-0.4058	-0.4080	-0.4118	-0.4137			

FIELD TEST LOAD DATA

Date: 7 July 1977 Bee County, US 181, Control 101-1-41

Dry Creek Bridge, Northbound Structure, 5.5 Miles S. of Beeville

Note: The direct readings are with the micrometer end of the slope indicator toward Beeville (to the north). Times are CDT. Results are twice the actual difference in elevation in inches per 24 inches between the reference points. A positive difference indicates that the northern most point is higher.

Slope Station B

Load No.	-	-	One	-	Two	-	Three
Time	8:25	8:59	9:23	9:55	10:32	10:50	11:12
Direct	.7750 .0070	.7750 .0062	.7750 .0071	.7750 .0059	.7750 .0133	.7750 .0051	.7750 .0093
Reverse	.7500 .0202	.7500 .0195	.7500 .0183	.7500 .0193	.7500 .0112	.7500 .0194	.7500 .0149
Sum	1.5522	1.5507	1.5504	1.5502	1.5495	1.5495	1.5492
-2XRev.	-1.5404	-1.5390	-1.5366	-1.5386	-1.5224	-1.5388	-1.5298
=2X D.E.	0.0118	0.0117	0.0138	0.0116	0.0271	0.0107	0.0194

Load No	-	-	-	-			
Time	11:39	12:08	1:14	1:52			
Direct	.7750 .0048	.7750 .0026	.7750 .0002	.7500 .0244			
Reverse	.7500 .0206	.7500 .0201	.7500 .0215	.7500 .0224			
Sum	1.5504	1.5477	1.5467	1.5468			
-2XRev.	-1.5412	-1.5402	-1.5430	-1.5448			
=2X D.E.	0.0092	0.0075	0.0037	0.0020			

FIELD TEST LOAD DATA

Date: 7 July 1977 Bee County, US 181, Control 101-1-41

Dry Creek Bridge, Northbound Structure, 5.5 Miles S. of Beeville

Note: The direct readings are with the micrometer end of the slope indicator toward Beeville (to the north). Times are CDT. Results are twice the actual difference in elevation in inches per 24 inches between the reference points. A positive difference indicates that the northern most point is higher.

Slope Station C

Load No.	-	-	One	-	Two	-	-
Time	7:39	9:03	9:25	9:57	10:38	10:52	10:57
Direct	.6750 .0211	.6750 .0197	.7000 .0105	.6750 .0192	.6750 .0208	.6750 .0171	.6750 .0174
Reverse	.8500 .0059	.8500 .0066	.8250 .0168	.8500 .0062	.8500 .0044	.8500 .0077	.8500 .0078
Sum	1.5520	1.5513	1.5523	1.5504	1.5502	1.5498	1.5502
-2XRev.	-1.7118	-1.7132	-1.6836	-1.7124	-1.7088	-1.7154	-1.7156
=2X D.E.	-0.1598	-0.1619	-0.1313	-0.1620	-0.1586	-0.1656	-0.1654

Load No	Three	-	-	-	-		
Time	11:15	11:41	12:10	1:16	1:55		
Direct	.6750 .0171	.6750 .0160	.6750 .0140	.6750 .0117	.6750 .0106		
Reverse	.8500 .0080	.8500 .0092	.8500 .0094	.8500 .0110	.8500 .0118		
Sum	1.5501	1.5502	1.5484	1.5477	1.5474		
-2XRev.	-1.7160	-1.7184	-1.7188	-1.7220	-1.7236		
=2X D.E.	-0.1659	-0.1682	-0.1704	-0.1743	-0.1762		

FIELD TEST LOAD DATA

Date: 7 July 1977 Bee County, US 181, Control 101-1-41

Dry Creek Bridge, Northbound Structure, 5.5 Miles S. of Beeville

Note: The direct readings are with the micrometer end of the slope indicator toward Beeville (to the north). Times are CDT. Results are twice the actual difference in elevation in inches per 24 inches between the reference points. A positive difference indicates that the northern most point is higher.

Slope Station D

Load No.	-	-	One	-	-	Two	Two
Time	7:44	8:55	9:19	9:43	9:47	10:21	10:24
Direct	.8000 .0017	.8000 .0020	.8000 .0031	.8000 .0024	.8000 .0023	.7750 .0237	.7750 .0237
Reverse	.7500 .0003	.7250 .0241	.7250 .0237	.7250 .0227	.7250 .0229	.7500 .0000	.7500 .0001
Sum	1.5520	1.5511	1.5518	1.5501	1.5502	1.5487	1.5488
-2XRev.	-1.5006	-1.4982	-1.4974	-1.4954	-1.4958	-1.5000	-1.5002
=2X D.E.	0.0514	0.0529	0.0544	0.0547	0.0544	0.0487	0.0486

Load No	-	Three	-	-	-	-	
Time	11:00	11:24	11:49	12:19	1:23	2:01	
Direct	.8000 .0032	.7750 .0124	.8000 .0032	.8000 .0038	.8000 .0047	.8000 .0061	
Reverse	.7250 .0214	.7500 .0118	.7250 .0212	.7250 .0197	.7250 .0176	.7250 .0165	
Sum	1.5496	1.5492	1.5494	1.5485	1.5473	1.5476	
-2XRev.	-1.4928	-1.5236	-1.4924	-1.4894	-1.4852	-1.4830	
=2X D.E.	0.0568	0.0256	0.0570	0.0591	0.0621	0.0646	

FIELD TEST LOAD DATA

Date: 7 July 1977 Bee County, US 181, Control 101-1-41

Dry Creek Bridge, Northbound Structure, 5.5 Miles S. of Beeville

Note: The direct readings are with the micrometer end of the slope indicator toward Beeville (to the north). Times are CDT. Results are twice the actual difference in elevation in inches per 24 inches between the reference points. A positive difference indicates that the northern most point is higher.

Slope Station E

Load No.	-	-	One	-	Two	-	Three
Time	8:21	8:52	9:16	9:40	10:17	10:58	11:22
Direct	.8750 .0112	.8750 .0110	.8750 .0062	.8750 .0103	.8750 .0023	.8750 .0095	.8750 .0084
Reverse	.6500 .0144	.6500 .0138	.6500 .0185	.6500 .0140	.6500 .0214	.6500 .0144	.6500 .0150
Sum	1.5506	1.5498	1.5497	1.5493	1.5487	1.5489	1.5484
-2XRev.	-1.3288	-1.3276	-1.3370	-1.3280	-1.3428	-1.3288	-1.3300
=2X D.E.	0.2218	0.2222	0.2127	0.2213	0.2059	0.2201	0.2184

Load No	-	-	-	-			
Time	11:47	12:15	1:21	1:59			
Direct	.8750 .0091	.8750 .0091	.8750 .0096	.8750 .0108			
Reverse	.6500 .0145	.6500 .0127	.6500 .0112	.6500 .0109			
Sum	1.5486	1.5468	1.5458	1.5467			
-2XRev.	-1.3290	-1.3254	-1.3224	-1.3218			
=2X D.E.	0.2196	0.2214	0.2234	0.2249			

FIELD TEST LOAD DATA

Date: 7 July 1977 Bee County, US 181, Control 101-1-41

Dry Creek Bridge, Northbound Structure, 5.5 Miles S. of Beeville

Note: The direct readings are with the micrometer end of the slope indicator toward Beeville (to the north). Times are CDT. Results are twice the actual difference in elevation in inches per 24 inches between the reference points. A positive difference indicates that the northern most point is higher.

Slope Station F

Load No.	-	-	One	-	Two	-	Three
Time	7:35	8:48	9:13	9:37	10:13	10:55	11:19
Direct	.8750 .0128	.8750 .0135	.8750 .0015	.8750 .0135	.8750 .0132	.8750 .0145	.8750 .0144
Reverse	.6500 .0140	.6500 .0127	.6500 .0249	.6500 .0123	.6500 .0120	.6500 .0112	.6500 .0111
Sum	1.5518	1.5512	1.5514	1.5508	1.5502	1.5507	1.5505
-2XRev.	-1.3280	-1.3254	-1.3498	-1.3246	-1.3240	-1.3224	-1.3222
=2X D.E.	0.2238	0.2258	0.2016	0.2262	0.2262	0.2283	0.2283

Load No	-	-	-	-			
Time	11:44	12:13	1:19	1:57			
Direct	.8750 .0147	.8750 .0141	.8750 .0151	.8750 .0154			
Reverse	.6500 .0105	.6500 .0095	.6500 .0076	.6500 .0074			
Sum	1.5502	1.5486	1.5477	1.5478			
-2XRev.	-1.3210	-1.3190	-1.3152	-1.3148			
=2X D.E.	0.2292	0.2296	0.2325	0.2330			

FIELD TEST LOAD DATA

Chronological sequence of slope indicator sums

Load	Slope	Time	Sum	Avg	Load	Slope	Time	Sum	Avg
	Sta	CDT				Sta	CDT		
NL	F	7:35	1.5518		THREE	A	11:10	1.5489	
	C	7:39	1.5520			B	11:12	1.5492	
	A	7:41	1.5512			C	11:15	1.5501	
	D	7:44	1.5520			F	11:19	1.5505	
	E	8:21	1.5506			E	11:22	1.5484	
	B	8:25	1.5522	1.5516		D	11:24	1.5492	1.5494
NL	F	8:48	1.5512		NL	A	11:35	1.5494	
	E	8:52	1.5498			B	11:39	1.5504	
	D	8:55	1.5511			C	11:41	1.5502	
	A	8:57	1.5503			F	11:44	1.5502	
	B	8:59	1.5507			E	11:47	1.5486	
	C	9:03	1.5513	1.5507		D	11:49	1.5494	1.5497
									*
ONE	F	9:13	1.5514		NL	A	12:05	1.5464	
	E	9:16	1.5497			B	12:08	1.5477	
	D	9:19	1.5518			C	12:10	1.5484	
	A	9:21	1.5496			F	12:13	1.5486	
	B	9:23	1.5504			E	12:15	1.5468	
	C	9:25	1.5523	1.5509		D	12:19	1.5485	1.5477
NL	F	9:37	1.5508		NL	A	1:10	1.5472	
	E	9:40	1.5493			B	1:14	1.5467	
	D	9:43	1.5501			C	1:16	1.5477	
	D	9:47	1.5502			F	1:19	1.5477	
	A	9:51	1.5493			E	1:21	1.5458	
	B	9:55	1.5502			D	1:23	1.5473	1.5470
	C	9:57	1.5504	1.5500					
					NL	A	1:51	1.5465	
TWO	F	10:13	1.5502			B	1:52	1.5468	
	E	10:17	1.5487			C	1:55	1.5474	
	D	10:21	1.5487			F	1:57	1.5478	
	D	10:24	1.5488			E	1:59	1.5467	
	A	10:28	1.5490			D	2:01	1.5476	1.5471
	B	10:32	1.5495						
	C	10:38	1.5502	1.5493					
NL	A	10:48	1.5485						
	B	10:50	1.5495						
	C	10:52	1.5498						
	F	10:55	1.5507						
	C	10:57	1.5502						
	E	10:58	1.5489						
	D	11:00	1.5496	1.5498					

* Probable instrument shift from mishandling

FIELD TEST LOAD DATA

Date: 7 July 1977

Bee County, US 181, Control 101-1-41

Dry Creek Bridge, Northbound Structure, 5.5 Miles S. of Beeville

At Station E		Bottom of Structure		Wind
Time	Temp (°F)	Time	Temp (°F)	
0847	90	0847	87	None
0915	92			
0939	93	0939	89	Approx 5 mph from SE
1012	94	1012	88	Approx 10 mph from SE
1058	98			Approx 20 mph from S
1122	100			
1146	102	1143	91	
1214	103	1222	93	
1320	108	1324	96	
1358	108			Approx 20 mph from S

The structure was in tree shade until about 0800 hours. Small clouds periodically shaded the bridge not more than 5 percent of the time.

Shade Air temperature at 1340 hours was 97°.

The temperature at Station E was within the asphalt excavation for the ball points (See Figure 19) which was shaded continuously by a cardboard covering except when inclinometer readings were made.

The temperature at the bottom was taken with a thermometer taped to the bottom surface near the SE corner of the structure.

APPENDIX B
GUIDE FOR DATA INPUT
PROGRAM SLBDG 2

SKEWED MULTI-BEAM BRIDGE ANALYSIS - SLBDG 2

This program acts as a data generator for the SLAB49 program and should be used for the analysis of multi-beam bridges composed of prestressed box cross-sections. It is especially useful for skewed structures since the live load design moment is significantly less than for square structures. The spans are assumed simply supported along the skewed support. Square structures may also be analyzed.

IDENTIFICATION OF RUN (two cards per run)

These first two cards identify the run. The Problem Series File No. (PSF No.) is for data filing purposes. The first card indicates the recommended information that should be included to identify the run; however, the complete card may be used for any descriptive data as indicated by the dashed lines. A consistent system of units must be used for all input data. Kips and inches are usually the most convenient.

Problem Series		County		Hwy No.		PD/ IPE		Control-Sec-Job			Coded By		Date		Units Force - Length	
1	5	8	20	23	28	31	34	37	47	50	52	55	66	69	80	

Structure Name or other descriptive run information (do not leave blank)

1	80
---	----

Check to ensure that each run has the above two header cards and that each problem in the run (which may encompass a series of problems) has a Prob No. card as follows. Each individual problem in the run may have any convenient descriptive Prob No. such as LDG 5. A final card in the run with CEASE as the Prob No. indicates the end of the data.

IDENTIFICATION OF PROBLEM (one card each problem)

Prob No.	Description of problem
1	5
11	80

Each page in this input guide is for a specific data table. If the blank general coding forms are used, which match the spacing on these forms, then each page can be placed over or adjacent to the blank form to act as the input heading for each data table.

All integer numbers must be right justified if in a 2 space field. All 5 or 10 space fields may be entered at the user's convenience. For example -123.4 or -1.234E+02 define the same number; a decimal point must be used. Blanks are interpreted as zeros.

TABLE 1. CONTROL DATA (two cards for each problem)

Any data may be kept from a prior problem unless if the problems are within a Parent-Offspring series. An Offspring problem must hold Tables 2, 3, and 4 from the previous problem. Tables 5, 7, or 8 may also be kept if desired. A new Table 6 is required for each Offspring problem.

The multiple-load option may be blank or zero in which case the problem is assumed to be independent with only a single load condition. A Parent problem is specified by entering a +1. An Offspring problem is specified by entering a -1. Specification of a Parent problem implies that there are 1 or more following Offspring problems for exactly the same span with different load patterns or placements.

The complete generated SLAB49 data may be printed if desired by entering a 1 for the option.

The skew angle is the angle between a perpendicular to the longitudinal centerline and the support centerline. A left forward skew is positive.

Enter 1 to keep data from prior TABLE								Multiple Load Option	Enter 1 to Print SLAB49 Generated Data
2	3	4	5	6	7	8	9		
5	10	15	20	25	30	35	40	50	75
Number of cards added for TABLE								Skew Angle Degrees	
2	3	4	5	6	7	8	9		
5	10	15	20	25	30	35	40	51	60

TABLE 2. CONSTANTS (one card - none if kept)

This table must be omitted for Offspring problems.

The number of increments are the totals along the span from c-c of assumed bearing and out to out transversely. A skewed span has the same input number of span X increments as a square span with the same increment length. The generated SLAB49 model will have more X increments for the skewed span however.

The increment lengths may be any fractional length. They should be chosen so there is at least 4 to 5 increments transversely in each box beam and 12 to 14 increments in the span.

Poisson's Ratio is usually specified equal to 0.15 or 0.20 for concrete.

The Modulus of Elasticity may be any conventional value for concrete such as 5000 kips per inch. The modulus only affects the computed deflections.

No. of Increments		Increment Lengths		Poisson's Ratio	Modulus of Elasticity
X	Y	X-Direction hx	Y-Direction hy		E
5	10	21	30	40	50
					60

TABLE 3. BOX STIFFNESS DATA (one card each type - none if kept)

This table must be omitted for Offspring problems.

The different box types used to make up the structure are input with this table beginning with type 1. Up to 10 types may be used.

The number of transverse Y increments used for each box defines the total box width. Therefore, this table must be consistent with the transverse increment length used in Table 1 and the box arrangement specified in Table 4.

The box depth, internal void width, and top and bottom slab thicknesses are input in F5.0 format. That is, a 5 inch slab may be entered as a right justified 5 or as 5.0. The decimal form is recommended. Solid cross-sections may be input by specifying a 0.0 void width and 0.0 slab thicknesses. A inverted U cross-section could also be input by specifying the void width as the distance between the webs with only a top slab.

The shear key stiffness factor is used to define the degree of remaining transverse stiffness at the shear key between beams. A value of 0.0 is usually best. A solid slab (not articulated) could be solved with a key stiffness factor of 1.0.

The longitudinal moment of inertia is the conventional total value computed about the horizontal neutral axis of the box. The torsional inertia is the value usually known as the St. Venant torsion constant and may be estimated for a box cross-section by the following formula taken from Ref 4.

$$J = \frac{4 (l_h l_v)^2}{\frac{2l_v}{t_w} + \frac{l_h}{t_t} \left(\frac{t_t + t_b}{t_b} \right)}$$

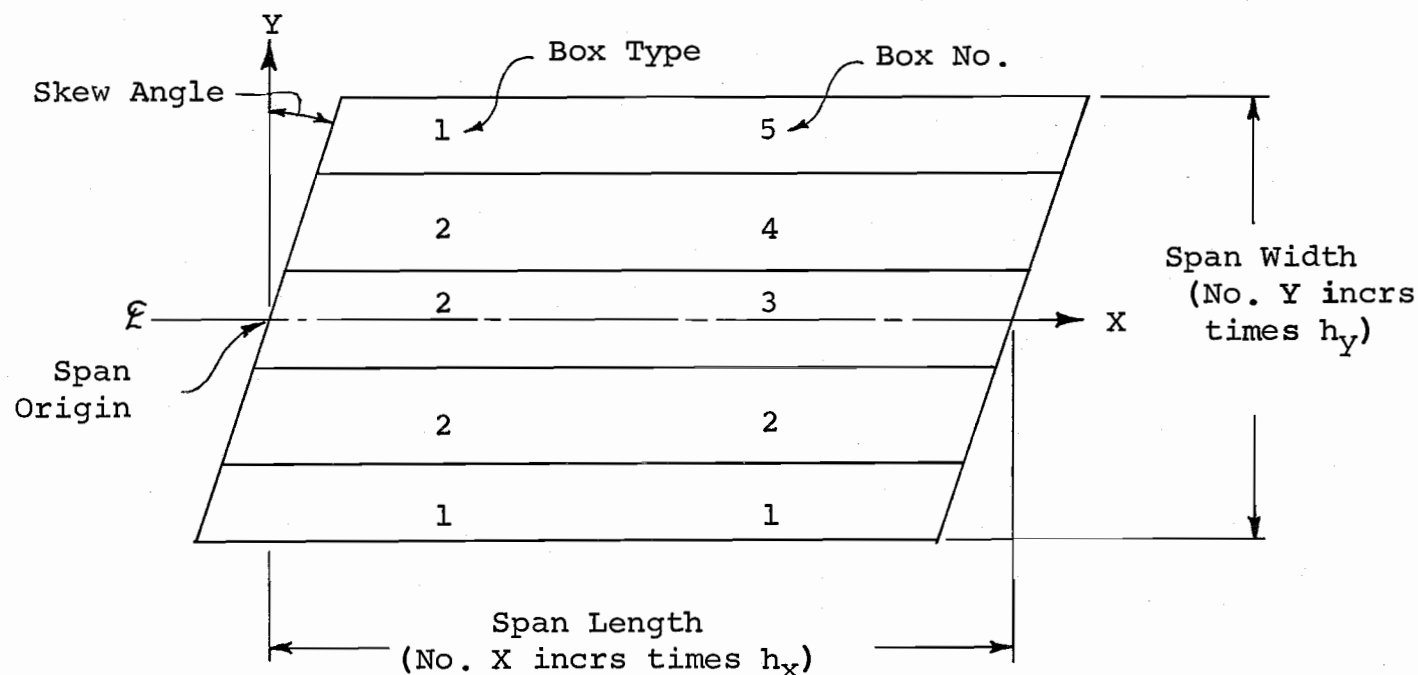
where l_h is the width between web centers, l_v is the depth between slab centers and t_w , t_t , and t_b are web, top, and bottom slab thicknesses.

Type No.	Increments in width	Box Depth	Void Width	Top Slab	Bot Slab	Shear Key Stiffness Factor	Moment Inertia	Torsional Inertia
	5	16	20	25	30	35	41	45
							51	60
								70

TABLE 4. BOX ARRANGEMENT (one card - none if kept)

This table must be omitted for Offspring problems.

The span is assumed symmetrical about the longitudinal centerline. The arrangement is input by type number beginning at the right most box up to and including the centerline if it straddles the centerline. The following sketch illustrates a typical box arrangement and box numbering scheme. Note for this example that there are 5 boxes. Only the first 3 are specified in this table. Any or all of the 5 boxes may be referenced in Table 7 for sums.



Box Arrangement by Type No. up to and including centerline

Box No.	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80

TABLE 5. LOAD PATTERNS (two cards per pattern - none if kept)

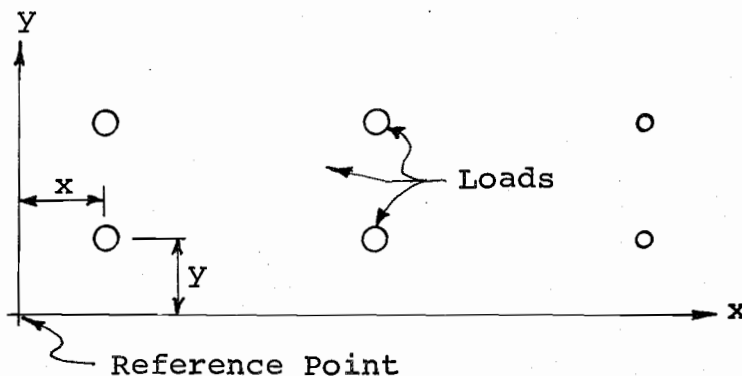
This table may be input or kept for Offspring problems. For a series of Offspring problems with different numbers of patterns, all the anticipated patterns may be input in the Parent problem and kept from problem to problem with only the desired patterns used by the Table 6 placements.

Each pattern is identified by a number (beginning with 1) up to a maximum of 10 different patterns. Each pattern may have from 1 to 6 loads. If the desired pattern has more than 6 loads, then multiple patterns may be specified which can be overlapped by the Table 6 placements.

Each load in the pattern is referenced to a local pattern coordinate system the origin of which may be one of the loads or any other convenient point. This local pattern reference point is the point by which the pattern is placed by Table 6. An example is shown below.

The coordinate distances are input in F5.0 format and may be negative.

Negative loads are downward.



No.	Pattern of	These decimal distances are from the load pattern reference point												
No.	Loads	x	y	x	y	x	y	x	y	x	y	x	y	
5	10	15	20	25	Magnitude of Loads				45	50	55	60	65	70
	11	20	30	40	50	60	70							

TABLE 6. PLACEMENTS OF LOAD PATTERNS (maximum of 10 cards - none if kept)

This table may not be kept for Offspring problems. Each offspring problem must have a new Table 6.

Each pattern may be placed at as many as 6 different locations on the span. This allows for instance, definition of one truck pattern by Table 5 with up to 6 like trucks placed simultaneously.

The placement is by pattern number and coordinate distances. The distances are in reference to the span geometry wherein the intersection of the longitudinal centerline and the first support line is considered to be the span origin. This is shown in the sketch on page 5.

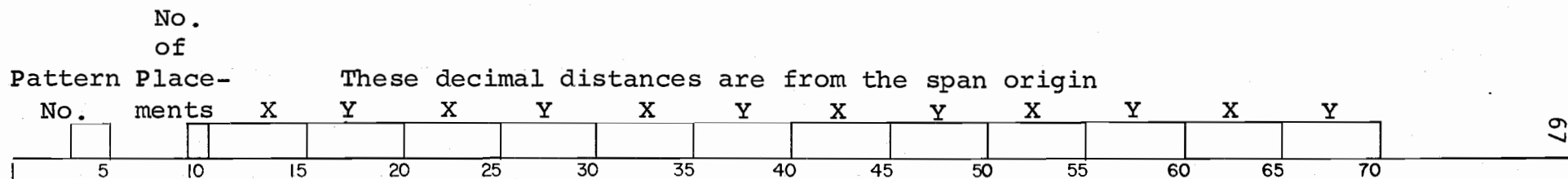


TABLE 7. SUMMATION OF BOX BENDING MOMENTS (maximum of 10 cards - none if kept)

This table may be kept for any type of problem or input for each problem.

The locations at which the longitudinal box bending moments will be summed are defined by this table. The summations are the total bending moment in the box and may be used directly for design purposes.

Sums may be for any or all boxes within the span. For instance, if the span has 5 boxes as in the sketch on page 5, the first 3 were specified by Table 4 but any of the 5 boxes may be summed. Any location along the span of the box may be summed.

Distances to the summation locations are in reference to the span coordinate system as shown on page 5.

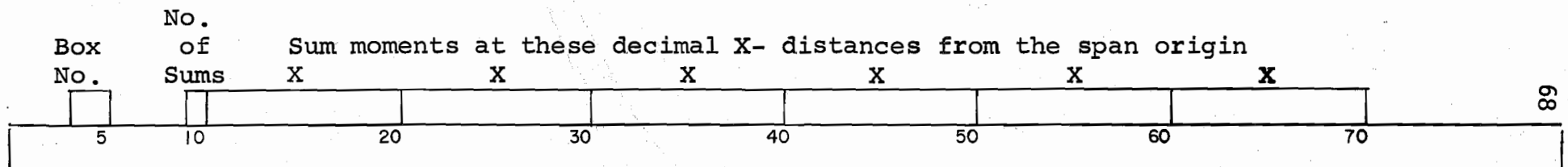


TABLE 8. PROFILE OUTPUT AREAS (maximum of 10 cards - none if kept)

This table may be kept for any type of problem or input for each problem.

Profile output is helpful in displaying the variation of deflections or moments along a beam line or transversely across the span. Deflection, bending moment in the X or Y direction, and maximum principal moment may be displayed. The profiles are presented in the form of crude printer plots for each area specified. The plots have a width of 20 print characters. Each plot has no scale or zero reference. The values are relative to one another, increasing positively to the right.

The profile output area limits are those SLAB49 stations which are nearest these input values of from and thru distance coordinates. The coordinates are with reference to the span origin.

Distances are input in F5.0 format. Thus 12345 is interpreted as 12345.0 and 12.34 is interpreted as input.

From		Thru		Enter 1 for				Principal Moment	The from and thru decimal distances refer to the span origin.
X	Y	X	Y	Defl	X-Mom	Y-Mom			
1	5	10	15	20	25	30	35	40	

TABLE 9. PRINTED OUTPUT LIMITS (maximum of 10 cards - none if kept)

This table may be kept for any type of problem or input for each problem.

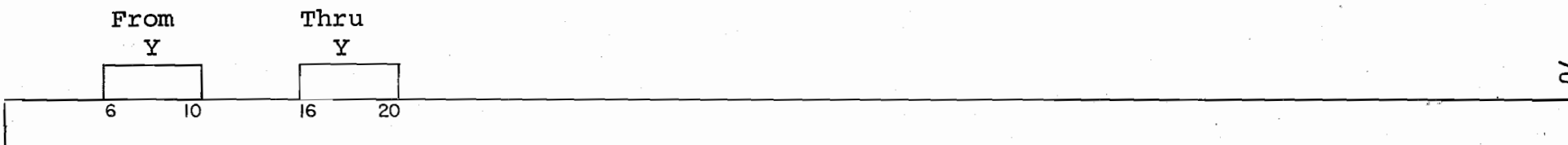
This table may be omitted in which case the complete detailed SLAB49 output is obtained.

Partial SLAB49 detailed output may be obtained by specifying Y-bounded limits designated on each card. The nearest SLAB49 Y-station to the Y-distances specified here is used for the limit of the section.

Y-bounded areas may overlap or be contiguous.

For most multi-beam box solutions, it is best to print the Y-bounded area for the box at or adjacent to the structure centerline unless an edge box is under investigation.

Distances to the Y-bounded areas are in reference to Right (negative) or Left (positive) distances from the span centerline as shown on page 5.



APPENDIX C
NOTATION AND LISTING
PROGRAM SLBDG 2

C	-----NOTATION FOR PROGRAM SLBDG2	10AG77	C	HXS	NUM OF X INCREMENTS IN SPAN OF STRUCTURE	01MY77
C	SLBDG2 IS DEPENDENT ON PROGRAM SLAB49 FOR SOLUTION, THE SLAB49	10AG77	C	MY	TOTAL NUM OF Y INCRS ACROSS WIDTH OF STRUCTURE	01MY77
C	NOTATION SHOULD ALSO BE USED WHEN REFERRING TO THIS NOTATION.	10AG77	C	MYINCR	SUM OF Y INCREMENTS IN SPECIFIED BOXES	01MY77
C	ADIST X DIST BETH SUPPORT LINE AND FIRST SUPPORT SPRING	01MY77	C	MYPI	NUMBER OF Y INCREMENTS IN Y DIRECTION PLUS 1	01MY77
C	AN1(), AN2() ALPHANUMERIC PRINT HEADING IDENTIFIERS	01MY77	C	N	TEMPORARY INDEX	01MY77
C	ATEST() TWO BLANKS USED IN TESTING PRINT HEADING TYPE	01MY77	C	NB	NUMBER OF BOXES SPECIFIED UP TO CENTERLINE	01MY77
C	BD() BOX DEPTH	01MY77	C	NBP1	NUM BOXES SPECIFIED UP TO CENTERLINE PLUS 1	10AG77
C	BO1ST X DIST BETH SUPPORT LINE AND SECOND SUPPORT SPRING	01MY77	C	NBSM	NUMBER OF BOXES WHICH HAVE MOMENTS SUMMED	01MY77
C	BME BENDING MODULUS OF ELASTICITY	01MY77	C	NBT	TOTAL NUMBER OF BOXES ACROSS STRUCTURE	01MY77
C	BM1() BOX MOMENT OF INERTIA	01MY77	C	NCDT	SUM OF NCD2 THROUGH NCD9	01MY77
C	BT1() BOX TORSIONAL INERTIA (2 X EQUIV SLAB TWIST STIFF)	01MY77	C	NCD2 - NCD9	NUM OF CARDS INPUT IN TABLE 2 THRU 9	01MY77
C	CH() TWISTING STIFFNESS OF EQUIV SLAB	01MY77	C	NCG2 THRU NCG9	GENERATED NUM OF SLAB49 CARD IMAGES TABLES 2 THRU 9	01MY77
C	CHN1() GENERATED SLAB49 TWISTING STIFFNESS	01MY77	C	NCG3T	TEMPORARY VALUE OF NCG3	01MY77
C	DISTRB SUBROUTINE NAME - DISTRIBUTES LOAD TO SURROUND JTS	01MY77	C	NCT3 - NCT9	TOTAL NUMBER OF CARDS IN TABLE 3 THRU 9	01MY77
C	DX() X STIFFNESS IN LONGIT DIRECTION	01MY77	C	NDES	ACCUMULATED DATA ERRORS	01MY77
C	DXN1() GENERATED SLAB49 SLAB STIFFNESS IN X DIRECTION	01MY77	C	NDE1 - NDE9	ACCUMULATED NUM OF DATA ERRORS IN TABLE 1 THRU 9	01MY77
C	DY1() HALF OF WEB Y STIFFNESS TIMES SHEAR KEY STIFF FACT	01MY77	C	NJ	NUMBER OF JOINTS TO WHICH LOAD IS ALLOCATED	01MY77
C	DY21() WEB Y STIFFNESS	01MY77	C	NLP	TEMPORARY VALUE OF NUM LOADS IN THE PATTERN	01MY77
C	DY31() SUM OF TOP AND BOTTOM SLAB Y STIFFNESSES	01MY77	C	NPN	TOTAL NUMBER OF LOAD PATTERNS	01MY77
C	DYN1() GENERATED SLAB49 SLAB STIFFNESS IN Y DIRECTION	01MY77	C	NPP	TEMPORARY VALUE OF NUM OF PLACEMENTS OF THE PATTERN	01MY77
C	FXN1(), FYN1() GENERATED SLAB49 BEAM STIFFNESSES (SET TO 0.0)	01MY77	C	NPROB1()	PROBLEM NUMBER	01MY77
C	HX INCREMENT LENGTH IN X DIRECTION (SPAN DIRECTION)	01MY77	C	NXSD	NUMBER OF X STATIONS OFF THE STRUCTURE	01MY77
C	HY INCREMENT LENGTH IN Y DIRECTION (TRANSVERSE)	01MY77	C	NYM	NUMBER OF Y INCREMENTS ACROSS THE BOX	01MY77
C	I TEMPORARY INDEX	01MY77	C	NYS	NUMBER OF Y STATIONS ACROSS THE BOX	01MY77
C	IBN1() BOX NUMBER TO HAVE X MOMENT SUMMED	01MY77	C	OMPR2	1 MINUS POISSONS RATIO SQUARED	01MY77
C	IHDG PRINT HEADING SWITCH	01MY77	C	OPPR	1 PLUS POISSONS RATIO	01MY77
C	II TEMPORARY VALUE NUM OF SUMS FOR THE BOX	01MY77	C	PBXN1(), PBYN1()	GENERATED SLAB49 BEAM THRUSTS (SET TO 0.0)	01MY77
C	IN13() THRU IN181() IN SLAB49 TABLES 3 THRU 8	01MY77	C	PCX(,)	X DISTANCE FROM PATTERN REFERENCE TO LOAD POINT	01MY77
C	IN23() THRU IN281() IN SLAB49 TABLES 3 THRU 8	01MY77	C	PCY(,)	Y DISTANCE FROM PATTERN REFERENCE TO LOAD POINT	01MY77
C	INCRM1() NUM OF Y INCRS IN WIDTH OF BOX	01MY77	C	PI	CIRCULAR CONSTANT = 3.1415927	01MY77
C	INLP() NUMBER OF LOADS IN THE PATTERN	01MY77	C	PR	POISSONS RATIO	01MY77
C	INP INPUT UNIT DESIGNATION = 5	01MY77	C	PSS	AXIAL THRUST BETH S1 AND S2 TO FORM SUPPORT SYSTEM	10AG77
C	INPP() NUM OF PLACEMENTS OF THE PATTERN	01MY77	C	PXN1()	GENERATED SLAB49 AXIAL THRUST FOR SUPPORT SYSTEM	01MY77
C	INS() NUM OF SUMS FOR THE BOX	01MY77	C	PYN1()	GENERATED SLAB49 AXIAL THRUST IN Y-DIR (SET TO 0.0)	01MY77
C	IOP OUTPUT UNIT DESIGNATION = 6	01MY77	C	QMN1()	GENERATED SLAB49 LOAD	01MY77
C	IPGND OPTION TO PRINT GENERATED SLAB49 DATA IF = 1	01MY77	C	QN()	GENERATED SLAB49 LOAD IN TABLE 4 (SET TO 0.0)	01MY77
C	IPN1() LOAD PATTERN NUMBER	01MY77	C	QOHXY	LOAD DIVIDED BY HX AND HY	01MY77
C	IPNP() LOAD PATTERN NUM PLACEMENT	01MY77	C	QP	CONCENTRATED LOAD IN THE LOAD PATTERN	10AG77
C	IR1 X-STA OF FIRST SUPPORT SPRING PRIOR TO SUPPORT LINE	01MY77	C	QPA1()	ARRAY OF 4 LOADS AT JOINTS SURROUNDING LOAD	01MY77
C	ISFA1() ARRAY OF 4 X-STAS OF JOINTS SURROUNDING LOAD	01MY77	C	QPOSUM	SUM OF LOADS PLACED OFF THE SLAB	01MY77
C	ITB1() ARRAY OF BOX ARRANGEMENT BY TYPE NUM UP TO CNTR L	01MY77	C	QPSUM	SUM OF LOADS PLACED ON THE SLAB	01MY77
C	ITEST1() RUN TERMINATOR = CEASE OR BLANKS	01MY77	C	RJYO	HALF WIDTH OF STRUCTURE IN DECIMAL Y STATIONS	01MY77
C	ITYPB1() TYPE NUMBER OF BOX	01MY77	C	RML1(,)	CONCENTRATED LOAD IN THE PATTERN	01MY77
C	IXD NO. OF X INCRS FROM ORIGIN TO CL AT BENT	10AG77	C	RXN1(), RYN1()	GENERATED SLAB49 ROTATIONAL STIFFNESS (SET TO 0.0)	01MY77
C	J TEMPORARY INDEX	01MY77	C	RXO	X DIST FROM STRUCTURE ORIGIN TO MODEL ORIGIN	01MY77
C	JJ TEMPORARY INDEX	01MY77	C	RYO	Y DIST FROM STRUCTURE ORIGIN TO MODEL ORIGIN	01MY77
C	JN13() THRU JN191() IN SLAB49 TABLES 3 THRU 8	01MY77	C	SA	COMPUTED SKEW ANGLE IN RADIAN	01MY77
C	JN23() THRU JN291() IN SLAB49 TABLES 3 THRU 8	01MY77	C	SAI	SKEW ANGLE INPUT	01MY77
C	JNT TEMPORARY INDEX FOR JOINT LOAD ALLOCATION	01MY77	C	SCX1(,)	X DIST FROM STRUCTURE ORIGIN TO PATTERN REF POINT	01MY77
C	JSTA1() ARRAY OF 4 Y-STAS OF JOINTS SURROUNDING LOAD	01MY77	C	SCY1(,)	Y DIST FROM STRUCTURE ORIGIN TO PATTERN REF POINT	01MY77
C	JYT CLOCK PARAMETER	01MY77	C	SKSF1()	SHEAR KEY TRANSVERSE STIFFNESS FACTOR (1.0 = FULL)	01MY77
C	JYS ACCUMULATED NUMBER OF Y STATIONS	01MY77	C	SLAB49	SUBROUTINE NAME - THIS IS THE MAJOR ROUTINE USED	01MY77
C	K TEMPORARY INDEX, USUALLY BOX TYPE NUM	01MY77	C	SMX1(,)	X DIST FROM STRUCTURE ORIGIN TO X MOM SUM LOCATION	10AG77
C	KASEP1() TABLE 8 PRINCIPAL MOMENT OPTION	01MY77	C	SN1()	GENERATED SLAB49 SUPPORT SPRING STIFFNESS	01MY77
C	KASEW1() TABLE 8 DEFLECTION OPTION	01MY77	C	SS	SUPPORT SPRING (1.0E6 USED TO AVOID ROUND OFF)	01MY77
C	KASEX1() TABLE 8 X BENDING MOMENT OPTION	01MY77	C	S1	FIRST SUPPORT SPRING	01MY77
C	KASEY1() TABLE 8 Y BENDING MOMENT OPTION	01MY77	C	S2	SECOND SUPPORT SPRING	01MY77
C	KEEP2 - OPTIONS TO KEEP DATA FROM PRIOR PROBLEM	01MY77	C	TAHSA	TANGENT OF THE SKEW ANGLE	01MY77
C	KEEP9 IN TABLES 2 THRU 9	01MY77	C	THKB1()	THICKNESS OF BOTTOM SLAB	01MY77
C	KEEP1() SUM OF KEEP2 THRU KEEP9	01MY77	C	THKT1()	THICKNESS OF TOP SLAB	01MY77
C	KK TEMPORARY INDEX	01MY77	C	TICTOC	SUBROUTINE NAME - PRINTS COMPUTATION TIME	01MY77
C	KLO INDEX = 1 IF LOAD IS ON STRUCTURE, = 0 IF OFF	01MY77	C	TXN1(), TYN1()	GENERATED SLAB49 APPLIED MOMENTS (SET TO 0.0)	01MY77
C	KPG2 THRU KPG9 GENERATED KEEP OPTIONS FOR SLAB49 TABLES 2 THRU 9	01MY77	C	VW1()	VOID WIDTH OF BOX	01MY77
C	KPN TEMPORARY INDEX FOR PATTERN NUMBER	01MY77	C	WEB	BOX WEB WIDTH FROM EDGE OF VOID TO CL JOINT	01MY77
C	KTBA1() ARRAY OF COMPLETE BOX TYPE ARRANGEMENT	01MY77	C	XD	X DISTANCE FROM STRUCTURE ORIGIN	01MY77
C	L TEMPORARY INDEX, USUALLY FOR REVERSE ORDER	01MY77	C	XDT	TOTAL X DIST FROM STRUCTURE ORIGIN	01MY77
C	LOCATE SUBROUTINE NAME - DETERMINES IF ON OR OFF STRUCTURE	01MY77	C	XLNGTH	X LENGTH OF STRUCTURE ALONG CENTERLINE	01MY77
C	ML MULTIPLE LOADING SWITCH	01MY77	C	XN1B1()	-FROM- X DIST FOR TABLE 8 PROFILE OUTPUT AREAS	01MY77
C	MODEV EQUALS 1 IF ODD NUMBER OF BOXES, 0 IF EVEN	01MY77	C	XN2B1()	-TO- X DIST FOR TABLE 8 PROFILE OUTPUT AREAS	01MY77
C	MX TOTAL NUM OF X INCRS FOR SLAB49	01MY77	C	XO	X DISTANCE FROM STA JUST PRIOR TO LOAD TO THE LOAD	01MY77
			C	XT	TANGENT OF SKEW ANGLE TIMES YO	10AG77
			C	YO	Y DISTANCE FROM STRUCTURE ORIGIN	01MY77
			C	YN1B1()	-FROM- Y DIST FOR TABLE 8 PROFILE OUTPUT AREAS	01MY77

C	YN19()	-FROM- Y DIST FOR TABLE 9 SELECTED OUTPUT	01MY77
C	YN28()	-TO- Y DIST FOR TABLE 8 PROFILE OUTPUT AREAS	01MY77
C	YN29()	-TO- Y DIST FOR TABLE 9 SELECTED OUTPUT	01MY77
C	YD	Y DISTANCE FROM STA JUST PRIOR TO LOAD TO THE LOAD	01MY77
C	YWIDTH	Y WIDTH OF STRUCTURE	01MY77

The additional variables added to the SLAB49 and SB49S routines (Ref 8) to allow selective summation of individual box beam bending moments are shown below. Some variables defined above for SLBDG2 are also used in that same area of SB49S. All remaining variables in SLAB49 and its associated routines are unchanged. Their definitions may be found in Ref 8.

C	----NOTATION ADDED TO SLAB 49		10AG77
C	SOME OF THE ABOVE VARIABLES ARE ALSO USED IN SLAB49 FOR THE BOX		10AG77
C	MOMENT SUMMATIONS.		10AG77
C	IDUM	DUMMY INTEGER FOR BOUNDARY ALIGNMENT	10AG77
C	SUM(,)	BOX MOMENT SUM	10AG77
C	SUMT(,)	BOX MOMENT SUM TEMPORARY	10AG77
C	ISM(,)	CLOSEST X STATION TO MOMENT SUM LOCATION	10AG77
C	JSM()	BOX NO. TO HAVE MOMENTS SUMMED	10AG77
C	JY1	Y STATION AT RT OF BOX TO BE SUMMED	10AG77
C	JY2	Y STATION AT LT OF BOX TO BE SUMMED	10AG77
C	JP2	TEMPORARY INDEX EQUAL TO J + 2	10AG77
C	II	X STATION INDEX FOR BOX MOMENT SUM	10AG77
C	IIT	X STATION INDEX FOR BOX MOMENT SUM TEMPORARY	10AG77

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C PROGRAM SLBDG2 -- DATA GENERATOR FOR SKEWED MULTI-BEAM PRECAST
C BOXES. DATA IS PASSED TO SLAB49 AS A SUBROUTINE FOR ANALYSIS.
IMPLICIT REAL*8 (A-H, O-Z)
DIMENSION ATEST(2), ITEST(4)
DIMENSION ITPB(10), BD(10), VM(10), THKT(10),
1 THKB(10), SKSF(10), BMI(10), BTI(10), ITB(16),
2 IPN(10), INLP(10), PCX(10,6), PCY(10,6), RMLL(10,6),
3 IPNP(10), INPP(10), SCX(10,6), SCY(10,6),
4 XN1B(10), YN1B(10), XN2B(10), YN2B(10),
5 YN19(10), YN19(10), YN29(10)
DIMENSION CH(10)
DIMENSION DY1(10), DY2(10), DY3(10), DX(10)
COMMON / BLK1 / HX, HY, TANSA, MXS, MY, IXO, ML, MK, IPGND, PR
COMMON / BLK2 / ISTA(4), JSTA(4), QPA(4)
COMMON / BLK3 / KPG2, KPG3, KPG4, KPG5, KPG6, KPG7, KPG8, KPG9,
1 NCG2, NCG3, NCG4, NCG5, NCG6, NCG7, NCG8, NCG9
COMMON / BLK4 / IBNI(10), INSI(10), SMX(10,6),
1 KTBA(32), INCRW(10), NBT, NBSM
COMMON /CARDS/ AN1(46), AN2(18), NPROB(2),
1 IN13(300), JN13(300), IN23(300), JN23(300), OXN(300),
2 DYN(300), FXN(300), FYN(300), QN(300), SN(300),
3 IN14( 50), JN14( 50), IN24( 50), JN24( 50),
4 RXN( 50), RYN( 50), TXN( 50), TYN( 50),
5 IN15(100), JN15(100), IN25(100), JN25(100), CHN(100),
6 IN16(150), JN16(150), IN26(150), JN26(150),
7 PXN(150), PYN(150), PBXN(150), PBYN(150),
8 IN17(100), JN17(100), IN27(100), JN27(100), QMN(100),
9 IN18( 10), JN18( 10), IN28( 10), JN28( 10),
A KASEW( 10), KASEX( 10), KASEY( 10), KASEP( 10),
B JN19( 10), JN29( 10)
DATA ITEST(1), ITEST(2), ITEST(3), ITEST(4), ATEST(1), ATEST(2)
1 / 1MC , 4HEASE , 1H , 4H , 2H , 4H /
2 FORMAT ( 50H PROGRAM SLBDG2 BRIDGE DIVISION, TEXAS DOHPT
35H PANAK REVISION DATE 10 AUG 77 / )
2 FORMAT ( 10H PSF , 15X, 25H HIGHWAY PD- CONTROL-,
1 10H CODED
2 / 50H NO COUNTY NO IPE SECTION-JO
3 35HB BY DATE )
6 FORMAT ( )
11 FORMAT ( 5H1 ,80X 10H1-----TRIM )
12 FORMAT ( 5(A2,A3), A3, 2(A2,A4), 2(3A4,A2), 3A4 / 20A4 )
13 FORMAT ( 5X, 5(A2,A3), A3, 2(A2,A4), 2(3A4,A2), 3A4 // 5X, 20A4 )
14 FORMAT ( A1, A4, 5X, 17A4, A2 )
15 FORMAT ( /10H PROB , /5X, A1, A4, 5X, 17A4, A2 )
16 FORMAT ( /17H PROB (CONTD) , /5X, A1, A4, 5X, 17A4, A2, // )
18 FORMAT ( // 50H ***** ERROR - BLANK PROB NO. IS NOT ALLOWED,
1 25HRUN IS TERMINATED ***** )
20 FORMAT ( 815, 5X, 15, 20X, 15, 5X, / , 815, 10X, E10.3 )
21 FORMAT ( 215, 10X, 4E10.3 )
33 FORMAT ( 215, 5X, 4F5.2, 5X, F5.2, 5X, 2E10.3 )
43 FORMAT ( 16 ( 15 ) )
53 FORMAT ( 215, 12F5.0 )
54 FORMAT ( 10X, 7E10.4 )
63 FORMAT ( 215, 12F5.0 )
73 FORMAT ( 215, 7F5.0 )
83 FORMAT ( 4F5.0, 415 )
93 FORMAT ( 2F10.0 )
100 FORMAT ( //30H TABLE 1. CONTROL DATA ,
1 / 55X, 20H TABLE NUMBER ,
2 / 40X, 45H 2 3 4 5 6 7 8 9
3 // 5X, 40H KEEP FROM PRECEDING PROBLEM (1=YES) , 815,
4 / 5X, 40H NUM CARDS INPUT THIS PROBLEM , 815
5 // 5X, 25H MULTIPLE LOAD OPTION , 15X, 15,
6 / 5X, 25H PRINT GENERATED DATA , 15X, 15,
7 / 5X, 25H SKEW ANGLE , 15X, E10.3 )
200 FORMAT ( //24H TABLE 2. CONSTANTS )
201 FORMAT ( / 50H NUMBER OF SPAN INCREMENTS IN X DIRECTION,
1 30X, 15,
2 / 10X, 35HNUMBER OF INCREMENTS IN Y DIRECTION , 35X, 15,
3 / 10X, 35HINCREMENT LENGTH IN X DIRECTION (AL
A 20HONG SPAN CENTERLINE) , 10X, 1PE10.3,
4 / 10X, 35HINCREMENT LENGTH IN Y DIRECTION , 30X, 1PE10.3,
5 / 10X, 35HPOISSONS RATIO , 30X, 1PE10.3,
6 / 10X, 35HMODULUS OF ELASTICITY , 30X, 1PE10.3 )

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300 FORMAT ( /34H TABLE 3. BOX STIFFNESS DATA / )
301 FORMAT ( 52H TYPE NO. INCR BOX VOID SLAB THICK SHEA09SE76
2 33HR KEY LONGIT. BOX INERTIAS O9SE76
3 / 52H NO. IN WIDTH DEP WIDTH TOP BOT STIF09SE76
4 33HF FACTOR MOMENT TORSIONAL ) O9SE76
311 FORMAT ( 8X, 12, 4X, 16, 2( 2X, F5.2, 1X, F5.2 ), 3X, E11.3, 3X,
1 IP2E11.3 ) O2JJ77
400 FORMAT ( /43H TABLE 4. BOX ARRANGEMENT BY TYPE NO. / ) O9SE76
411 FORMAT ( 5X, 16( 3X, 12 ) ) O9SE76
500 FORMAT ( /29H TABLE 5. LOAD PATTERNS ) O9SE76
501 FORMAT ( /50H PAT X AND Y DISTANCES (OF THE O2JJ77
1 14H LOAD PATTERN) X AND Y DISTANCES (OF THE O9SE76
2 /51H LDS LOAD MAGNITUDE) O9SE76
511 FORMAT ( 4X, 12, 2X, 12F5.0 ) O8FE77
512 FORMAT ( 5X, 12, 1X, IPTE11.3 ) O9SE76
600 FORMAT ( //42H TABLE 6. PLACEMENTS OF LOAD PATTERNS / ) 10AG77
601 FORMAT ( 50H PAT-PLMTS X AND Y DISTANCES (OF THE O2JJ77
1 11H STRUCTURE) ) O9SE76
611 FORMAT ( 5X, 12, 3X, 12, 3X, 12F5.0 ) O8FE77
700 FORMAT ( //50H TABLE 7. SUMMATION OF BOX BENDING MOMENTS 10AG77
1 // 50H BOX SUMS SUM AT X-STATION NEAREST O4AP77
2 20HTHESE X-DISTANCES ) 11MR77
712 FORMAT ( 5X, 215, 7F10.0 ) 10MR77
800 FORMAT ( //35H TABLE 8. PROFILE OUTPUT AREAS , O9SE76
1 // 45H DIST FROM - THRU DEFL X MOM O2JJ77
2 20H Y MOM PRIN MOM 10MR77
3 / 50H X Y X Y (1=YES) ) 10MR77
811 FORMAT ( 5X, 4F5.0, 15, 3110 ) 10MR77
900 FORMAT ( //40H TABLE 9. PRINTED OUTPUT LIMITS , O9SE76
1 // 25H Y-DIST FROM - THRU O2JJ77
2 / 25H Y ) 17MR77
903 FORMAT ( / 25H NONE ) O9SE76
905 FORMAT ( 46H USING DATA FROM THE PREVIOUS PROBLEM ) O9SE76
910 FORMAT ( 43H ADDITIONAL DATA FOR THIS PROBLEM ) 10MR77
911 FORMAT ( 5X, 2F10.0 )
980 FORMAT ( //40H ***** UNDESIGNATED ERROR STOP ***** ) 25JL751D
991 FORMAT ( /10H ***** , [4, 10SE76
1 33H DATA ERRORS IN THIS TABLE ***** ) 10SE76
C-----DEFINE READ AND WRITE UNITS 27AG751D
INP = 5 25JL751D
IOP = 6 25JL751D
C-----INITIALIZE CLOCK AND SET PSM MASK FOR UNDERFLOWS 25JL751D
JTT = 1 213C751D
CALL TICTOC(JTT) 210C751D
JTT = 2 213C751D
C-----BEGIN EXECUTION AND INITIALIZE CONSTANTS 13SE751D
READ (INP,12) ( AN1(N),N = 1, 46 ) 10AG771D
C----- - RETURN HERE TO READ NEW PROBLEM 25JL751D
1010 READ (INP,14) NPROB(1), NPROB(2), ( AN2(II), II = 1, 18 ) 25JL751D
C----- - IF NPROB = CEASE, TERMINATE RUN 25JL751D
IF (NPROB(1) .EQ. ITEST(1) .AND. NPROB(2) .EQ. ITEST(2)) 26AP761D
2 GO TO 9990 25JL751D
1 IF ( NPROB(1) .EQ. ITEST(3) .AND. NPROB(2) .EQ. ITEST(4) ) 26AP761D
GO TO 1015 02JA761D
1015 WRITE (IOP, 18) 02JA761D
GO TO 9990 02JA761D
1020 IF ( AN1( 3) .NE. ATEST(1) ) GO TO 1024 02JA761D
IF ( AN1( 9) .NE. ATEST(1) ) GO TO 1024 26AP761D
IF ( AN1(12) .NE. ATEST(1) ) GO TO 1024 26AP761D
IF ( AN1(14) .NE. ATEST(1) ) GO TO 1024 26AP761D
IHOG = 0 25JL751D
GO TO 1030 25JL751D
1024 IHOG = 1 25JL751D
1030 WRITE (IOP,11) 25JL751D
WRITE (IOP, 1) 25JL751D
IF ( IHOG .EQ. 0 ) GO TO 1032 25JL751D
GO TO 1034 25JL751D
1032 WRITE (IOP, 2) 25JL751D
IF ( AN1(20) .EQ. ATEST(2) ) CALL GETDAY ( AN1(20) ) 26AP761D
1034 WRITE (IOP,13) ( AN1(N), N = 1, 46 ) 10AG771D
WRITE (IOP,15) NPROB(1), NPROB(2), ( AN2(N), N = 1, 18 ) 25JL751D
C INPUT TABLE 1 CONTROL DATA 10SE76
C-----INPUT TABLE 1 CONTROL DATA O9SE76

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C-----ENSURE THAT A JOINT OCCURS AT CENTER LINE IF THERE IS AN EVEN NO. 02FE77
C   OF Y INCRS 02FE77
      IF ( ( MY - (MY/2)*2 ) .GT. 0 ) GO TO 2230 02FE77
      JJ = 0 02FE77
C-----FIND CENTERLINE 02FE77
      DO 2217 J = 1, NBT 02FE77
        NYS = INCRWIKTBA(J) + 1 02FE77
      DO 2215 K = 1, NYS 02FE77
        JJ = JJ + 1 02FE77
      IF ( ( JN13(JJ) - MY / 2 ) .EQ. 0 ) GO TO 2213 02FE77
      GO TO 2215 02FE77
2210 IF ( K .EQ. NYS ) GO TO 2215 02FE77
      GO TO 2220 02FE77
2215 CONTINUE 02FE77
2217 CONTINUE 02FE77
      GO TO 9980 02FE77
2220 IF ( ( IN13(JJ) + IXD ) .EQ. 0 ) GO TO 2230 02FE77
      IN13(JJ) = IN13(JJ) + 1 02FE77
C-----ENSURE THAT 2ND LINE IS CORRECTED IF FIRST WAS 02FE77
      IF ( ( IN13(JJ + 1) - IN13(JJ) ) .EQ. 0 ) GO TO 2230 02FE77
      IN13( JJ + 1 ) = IN13( JJ + 1 ) + 1 02FE77
C-----DETERMINE X STA OF FAR END OF LAST CARD IMAGE 02FE77
2230 IN23(NCG3) = IN13(1) + MX 10MR77
      DO 2250 J = 2, NCG3 02FE77
        L = NCG3 + 1 - J 02FE77
        IN23(L) = IN23(L+1) - IN13(J) + IN13(J-1) 02FE77
2250 CONTINUE 02FE77
C-----ZERO REMAINDER OF CARD IMAGE DATA 14MR77
      DO 2300 J = 1, NCG3 14MR77
        FXN(J) = 0.0 14MR77
        FYN(J) = 0.0 14MR77
        QNI(J) = 0.0 14MR77
        SN(J) = 0.0 14MR77
2300 CONTINUE 14MR77
C-----GENERATE TORSIONAL STIFFNESS CARD IMAGES 02FE77
      NCG5 = 0 02FE77
      JYS = 0 02FE77
      NYS = 1 02FE77
      DO 2270 J = 1, NBT 02FE77
        JYS = JYS + NYS 02FE77
        NYM = INCRWIKTBA(J) 02FE77
        NYS = NYM + 1 02FE77
      DO 2260 K = 1, NYM 02FE77
        NCG5 = NCG5 + 1 02FE77
        IN15(NCG5) = IN13(JYS + K) + 1 17FE77
        JN15(NCG5) = JN13(JYS + K) 02FE77
        IN25(NCG5) = IN23(JYS + K - 1) 22FE77
        JN25(NCG5) = JN15(NCG5) 02FE77
        CHN(NCG5) = CHN(KTBA(J)) 16FE77
2260 CONTINUE 02FE77
2270 CONTINUE 02FE77
C-----GENERATE SPRING AND AXIAL THRUST SUPPORT SYSTEMS 10AG77
      THE SPRING VALUE IS SET AS A MULTIPLE OF BME SO THAT ALL RESULTS 10AG77
      ARE LINEAR WITH A CHANGE IN BME. 02FE77
      SS = 1000.0 * BME 02FE77
      NCG6 = 0 02FE77
      MYP1 = MY + 1 02FE77
      NCG3T = NCG3 + 1 14MR77
      DO 2340 J = 1, MYP1 02FE77
        YD = RYO * HY + (J-1) * HY 02FE77
        XD = YD * TANSX 02FE77
C-----FIND FIRST X STA PRECEEDING SUPPORT LINE INTERSECT WITH OS EDGE 02J77
      IR1 = XD / HX - IXD 02FE77
      IF ( (IXD + IR1) .EQ. ( XD / HX ) ) GO TO 2320 02FE77
      ADIST = XD - ( IR1 + IXD ) * HX 02FE77
      BDIST = HX - ADIST 02FE77
      S1 = BDIST / HX * SS 02FE77
      S2 = ADIST / HX * SS 02FE77
      PSS = - ADIST * BDIST * SS / HX 22MR77
C-----GENERATE SECOND SPRINGS PAST FIRST SUPPORT LINE 02FE77
      NCG3 = NCG3 + 1 02FE77
      IN13(NCG3) = IR1 + 1 02FE77
      JN13(NCG3) = J - 1 02FE77
      IN23(NCG3) = IN13(NCG3) 02FE77

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      JN23(NCG3) = JN13(NCG3) 02FE77
      SN(NCG3) = S2 02FE77
C-----GENERATE AXIAL THRUSTS AT FIRST SUPPORT LINE 02FE77
      NCG6 = NCG6 + 1 02FE77
      IN16(NCG6) = IN13(NCG3) 02FE77
      JN16(NCG6) = JN13(NCG3) 02FE77
      IN26(NCG6) = IN13(NCG3) 02FE77
      JN26(NCG6) = JN13(NCG3) 10FE77
      PXN(NCG6) = PSS 02FE77
C-----GENERATE SECOND SPRINGS PAST SECOND SUPPORT LINE 02FE77
      NCG3 = NCG3 + 1 02FE77
      IN13(NCG3) = IR1 + 1 + MXS 02FE77
      JN13(NCG3) = J - 1 02FE77
      IN23(NCG3) = IN13(NCG3) 02FE77
      JN23(NCG3) = JN13(NCG3) 02FE77
      SN(NCG3) = S2 02FE77
C-----GENERATE AXIAL THRUSTS AT SECOND SUPPORT LINE 02FE77
      NCG6 = NCG6 + 1 02FE77
      IN16(NCG6) = IN13(NCG3) 02FE77
      JN16(NCG6) = JN13(NCG3) 02FE77
      IN26(NCG6) = IN13(NCG3) 02FE77
      JN26(NCG6) = JN13(NCG3) 02FE77
      PXN(NCG6) = PSS 02FE77
      GO TO 2330 02FE77
C-----SET SINGLE SPRING VALUE - USED BELOW AS FIRST SPRINGS 02FE77
2320 S1 = SS 02FE77
C-----GENERATE FIRST SPRINGS PRIOR TO FIRST SUPPORT LINE 02FE77
2330 NCG3 = NCG3 + 1 02FE77
      IN13(NCG3) = IR1 02FE77
      JN13(NCG3) = J - 1 02FE77
      IN23(NCG3) = IN13(NCG3) 02FE77
      JN23(NCG3) = JN13(NCG3) 02FE77
      SN(NCG3) = S1 02FE77
C-----GENERATE FIRST SPRINGS PRIOR TO SECOND SUPPORT LINE 02FE77
      NCG3 = NCG3 + 1 02FE77
      IN13(NCG3) = IR1 + MXS 02FE77
      JN13(NCG3) = J - 1 02FE77
      IN23(NCG3) = IN13(NCG3) 02FE77
      JN23(NCG3) = JN13(NCG3) 02FE77
      SN(NCG3) = S1 02FE77
2340 CONTINUE 02FE77
C-----ZERO REMAINDER OF CARD IMAGE DATA 14MR77
      DO 2350 J = NCG3T, NCG3 14MR77
        DXN(J) = 0.0 14MR77
        DYN(J) = 0.0 14MR77
        FXN(J) = 0.0 14MR77
        FYN(J) = 0.0 14MR77
        QNI(J) = 0.0 14MR77
2350 CONTINUE 14MR77
      DO 2360 J = 1, NCG6 14MR77
        PYN(J) = 0.0 14MR77
        PBXN(IJ) = 0.0 14MR77
        PBYN(IJ) = 0.0 14MR77
2360 CONTINUE 14MR77
C-----GENERATE LOAD DATA CARD IMAGES 02FE77
2370 NCG7 = 0 16MR77
      QPDSUM = 0.0 02FE77
      QPSUM = 0.0 02FE77
C-----DO FOR EACH PATTERN SPECIFIED BY TABLE 6 02FE77
      DO 3060 KK = 1, NCD6 02FE77
        NPP = INPP(KK) 02FE77
C-----DO FOR NUM PLACEMENTS EACH PATTERN 02FE77
      DO 3050 J = 1, NPP 02FE77
        KPN = IPNI(IPNP(KK)) 02FE77
        NLP = INLPI(IPNP(KK)) 02FE77
C-----DO FOR NUM LOADS IN EACH PATTERN 02FE77
      DO 3040 K = 1, NLP 02FE77
        XD = SCX(KK, J) + PCX(KPN, K) 02FE77
        YD = SCY(KK, J) + PCY(KPN, K) 02FE77
C-----DETERMINE IF LOAD IS WITHIN BOUNDARIES OF STRUCTURE 02FE77
      CALL LOCATE (KLO, XD, YD) 02FE77
      IF ( KLO .EQ. 0 ) GO TO 3010 02FE77
      QPSUM = QPSUM + RMLL(KK, K) 23FE77
C-----ALLOCATE LOAD TO SURROUNDING JOINTS 02FE77

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COMMON /TABLE/NC13, NCT3, NC14, NCT4, NC15, NCT5, NC16, NCT6, 100C9
1 NC17, NCT7, NC18, NCT8, NC19, NCT9 17FE1
COMMON /STIFF/PDHHXY, HYDHX3, ODHX3, ODHX2, ODHX, 100C9
1 ODHXHY, HXDHY3, ODHY3, ODHY2, ODHY, CRD(5) 24MY1
COMMON /R1 /MXP3, MYP3, NF, ITHPP 23DE0
COMMON /BLK1 /HX, HY, TANSA, MXS, MY, IXO, ML, MX, IPGNO, PR 14MR77
COMMON /BLK3 /KEEP2,KEEP3,KEEP4,KEEP5,KEEP6,KEEP7,KEEP8,KEEP9, 04AP77
1 NCO2, NCO3, NCO4, NCO5, NCO6, NCO7, NCO8, NCO9 04AP77
COMMON /BLK4 / IBN(10), INSI(10), SMX(10,6), 04AP77
1 KT8A(32), INCRW(10), NBT, NBSM
DATA SUP / 7HSUPPORT /, STAT / 7HSTATICS /, STRS / 6HSTRESS /, 08FE1
1 RCT / 8HREACTION/, CHK / 8H CHECK /, BMDM / 6HMOMENT /, 08FE1
2 SLBX / 6HSLAB X /, SLBY / 6HSLAB Y /, BLNK / 6H /, 08FE1
3 BEMX / 6HBEAM X /, BEMY / 6HBEAM Y / 11MR77
DATA ITEST(1), ITEST(2), ITEST(3), ITEST(4), ATEST(1), ATEST(2) 26AP76ID
1 / 1HC , 4HEASE , 1H , 4H , 2H , 4H / 26AP76ID
DATA ID1, ID2, ID3, ID4 /BHDFLECTN, 8HBN MOM X, 15JE0CDC
1 8HBN MOM Y, BH SIGO / 15JE0CDC
6 FORMAT ( ) 04MY3
11 FORMAT ( 5H1 , 80X, 10HI-----TRIM ) 17FE1
12 FORMAT ( 5(A2,A3), A3, 2(A2,A4), 2(3A4,A2), 3A4 / 20A4 ) 10AG77ID
13 FORMAT ( 5X, 5(A2,A3), A3, 2(A2,A4), 2(3A4,A2), 3A4 // 5X, 20A4 ) 10AG77ID
14 FORMAT ( A1, A4, 5X, 17A4, A2 ) 03MR77ID
15 FORMAT ( /10H PROB , /5X, A1, A4, 5X, 17A4, A2 ) 03MR77ID
16 FORMAT ( //17H PROB (CONTO), /5X, A5, 5X, 17A4, A2 ) 07DE0CDC
17 FORMAT ( /17H PROB (CONTO), /5X, A1, A4, 5X, 17A4, A2, // ) 03MR77ID
19 FORMAT ( //50H KEEP RUN TIME RECORDS FOR FUTURE ESTIMATES OF 21JA1
1 31H PARENT AND OFFSPRING RUN TIMES )
20 FORMAT ( 815, 5X, 15, 25X, 15, /, 815, 5X, 315, 12, 13, 3F5.0 ) 21JA1
21 FORMAT ( 215, 10X, 4E10.3 ) 06DE1
33 FORMAT ( 4( 2X, I3 ) , 6E10.3 ) 21JA1
43 FORMAT ( 4( 2X, I3 ) , 20X, 4E10.3 ) 21JA1
53 FORMAT ( 4( 2X, I3 ) , E10.3 ) 21JA1
63 FORMAT ( 4( 2X, I3 ) , 20X, 4E10.3 ) 21JA1
73 FORMAT ( 4( 2X, I3 ) , 40X, E10.3 ) 21JA1
83 FORMAT ( 4( 2X, I3 ) , 4( 4X, I1 ) ) 03FE1
93 FORMAT ( 2110 ) 060C0
100 FORMAT ( //30H TABLE 1. CONTROL DATA , 21JA1CDC
100 FORMAT ( //30H TABLE 1. CONTROL DATA , 06DE118M
1 / 55X, 20H TABLE NUMBER , 21JA1
2 / 40X, 45H 2 3 4 5 6 7 8 9 , 21JA1
3 // 5X, 40H KEEP FROM PRECEDING PROBLEM (1=YES) , 815, 21JA1CDC
4 // 5X, 40H KEEP FROM PRECEDING PROBLEM (1=YES) , 815, 06DE118M
5 // 5X, 40H NUM CARDS INPUT THIS PROBLEM , 815, 21JA1
6 // 5X, 25H MULTIPLE LOAD OPTION , 15X, 15, 21JA1
7 // 5X, 25H STATICS CHECK OPTION , 15X, 15, 21JA1
8 // 5X, 25H PRIN STRESS OPTION , 15X, 15, 21JA1
9 // 5X, 25H PROFILE PLOT OPTION , 15X, 15, 21JA1
9 // 5X, 25H 3-D PLOT OPTION , 15X, 15 ) 21JA1CDC
9 // 5X, 25H 3-D PLOT OPTION , 15X, 15 ) 06DE118M
200 FORMAT ( //24H TABLE 2. CONSTANTS ) 16AG8
201 FORMAT ( / 45H NUMBER OF INCREMENTS IN X DIRECTION , 21JA1
1 35X, 15, 21JA1
2 / 10X, 35HNUMBER OF INCREMENTS IN Y DIRECTION , 35X, 15, 21JA1
3 / 10X, 35HINCREMENT LENGTH IN X DIRECTION , 30X, 1PE10.3, 21JA118M
3 / 10X, 35HINCREMENT LENGTH IN X DIRECTION , 30X, E10.3, 21JA1CDC
4 / 10X, 35HINCREMENT LENGTH IN Y DIRECTION , 30X, 1PE10.3, 21JA118M
4 / 10X, 35HINCREMENT LENGTH IN Y DIRECTION , 30X, E10.3, 21JA1CDC
5 / 10X, 35HPOISSONS RATIO , 30X, 1PE10.3, 21JA118M
5 / 10X, 35HPOISSONS RATIO , 30X, E10.3, 21JA1CDC
6 / 10X, 35HSLAB THICKNESS , 30X, 1PE10.3 ) 21JA118M
6 / 10X, 35HSLAB THICKNESS , 30X, E10.3 ) 21JA1CDC
300 FORMAT ( //49H TABLE 3. JOINT BENDING STIFFNESSES, LOADS, 10DE1
+ 15HAND SUPPORTS 10DE1
1 // 50H FROM THRU DX DY FX , 30AG8
2 35H FY Q S , 30AG8
3 / 20H JOINT JOINT , / ) 30AG8
311 FORMAT ( 5X, 2( 1X, I2, 1X, I3 ) , 1P6E11.3 ) 21JA118M
311 FORMAT ( 5X, 2( 1X, I2, 1X, I3 ) , 6E11.3 ) 21JA1CDC
400 FORMAT ( //51H TABLE 4. JOINT RESTRAINTS AND APPLIED MOMENTS 10DE1
1 // 50H FROM THRU RX , 30AG8
2 35H RY TX TY , 30AG8

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3 / 20H JOINT JOINT , / ) 30AG8
411 FORMAT ( 5X, 2( 1X, I2, 1X, I3 ) , 22X, 1P4E11.3 ) 21JA118M
411 FORMAT ( 5X, 2( 1X, I2, 1X, I3 ) , 22X, 4E11.3 ) 21JA1CDC
500 FORMAT ( //50H TABLE 5. MESH TWISTING STIFFNESSES 10DE1
1 // 30H FROM THRU C , 30AG8
2 / 20H MESH MESH , / ) 30AG8
511 FORMAT ( 5X, 2( 1X, I2, 1X, I3 ) , 1PE11.3 ) 21JA118M
511 FORMAT ( 5X, 2( 1X, I2, 1X, I3 ) , E11.3 ) 21JA1CDC
600 FORMAT ( //40H TABLE 6. BAR AXIAL THRUSTS 10DE1
1 // 50H FROM THRU PX , 30AG8
2 35H PY PBX PBY , 30AG8
3 / 20H BAR BAR , / ) 30AG8
611 FORMAT ( 5X, 2( 1X, I2, 1X, I3 ) , 22X, 1P4E11.3 ) 21JA118M
611 FORMAT ( 5X, 2( 1X, I2, 1X, I3 ) , 22X, 4E11.3 ) 21JA1CDC
700 FORMAT ( //35H TABLE 7. MULTIPLE LOADS 10DE1
1 // 50H FROM THRU , 30AG8
2 20H QM, 30AG8
3 / 20H JOINT JOINT , / ) 30AG8
711 FORMAT ( 5X, 2( 1X, I2, 1X, I3 ) , 44X, 1PE11.3 ) 21JA118M
711 FORMAT ( 5X, 2( 1X, I2, 1X, I3 ) , 44X, E11.3 ) 21JA1CDC
800 FORMAT ( //35H TABLE 8. PROFILE OUTPUT AREAS , 21JA1CDC
800 FORMAT ( //35H TABLE 8. PROFILE OUTPUT AREAS , 06DE118M
1 // 40H FROM THRU DEFL X MOMENT 21JA1
2 35H Y MOMENT PRIN MOM OR STRESS , 21JA1
3 / 42H JOINT JOINT (1=YES) (1=SLAB, 24FE1CDC
4 35H2=BEAM) (1=YES) ) 24FE1CDC
3 / 42H JOINT JOINT (1=YES) (1=SLAB, 06DE118M
4 35H2=BEAM) (1=YES) ) 06DE118M
811 FORMAT ( 5X, 2(1X,I2,1X,I3), 4X, I2, 9X, I2, 10X, I2, 14X, I2 ) 16AP1
812 FORMAT ( / 45H BEAM MOMENTS ARE TOTAL PER BEAM ) 22JA1
813 FORMAT ( / 25X, A6, 5X, A6, 13X, 20H LARGEST BETA 22JA1
1 / 25X, A6, 5X, A6, 5X, A6, 2X, 20H PRINCIPAL X TO 10DE1
2 / 25X, A6, 5X, A6, 33H TWISTING SLAB LARGEST , 10DE1
1 A7, 22JA1
2 / 25H X, Y DEFL , A6, 5X, A6, 5X, A6, 4X, 22JA1
3 A6, 4X, A6, 2X, AB ) 22JA1
814 FORMAT ( 25X A6, 5X A6, 33X A7, 10DE1
+ / 25H X, Y DEFL , A6, 5X A6, 33X A8 ) 20DE1
C 815 FORMAT ( / 50H SLAB X MOMENT AND X TWISTING MOMENT ACT 12FE1CDC
C 1 35HIN THE X DIRECTION (ABOUT Y AXIS) , 22JA1CDC
C 2 / 10X, 50HY TWISTING MOMENT = -X TWISTING MOMENT, COUNTERCLD 22JA1CDC
C 815 FORMAT ( / 50H SLAB X MOMENT AND X TWISTING MOMENT ACT 06DE118M
1 35HIN THE X DIRECTION (ABOUT Y AXIS) , 06DE118M
2 / 10X, 50HY TWISTING MOMENT = -X TWISTING MOMENT, COUNTERCLD 06DE118M
3 25HCKWISE BETA ANGLES ARE 22JA1
4 / 10X, 50HPOSITIVE FROM THE X AXIS TO THE DIRECTION OF THE L 22JA1
5 17HARGEST PRINCIPAL , A6 , 22JA1
C 6 / 10X, 35HSLAB MOMENTS ARE PER UNIT WIDTH ) 22JA1CDC
6 / 10X, 35HSLAB MOMENTS ARE PER UNIT WIDTH ) 06DE118M
851 FORMAT ( 5X, I2, 1X, I3, 1P3E11.3, 28X, 1PE11.3 ) 21JA118M
C 851 FORMAT ( 5X, I2, 1X, I3, 3E11.3, 28X, E11.3 ) 21JA1CDC
C 852 FORMAT ( 5X, I2, 1X, I3, 1P5E11.3, 0PF6.1, 1PE11.3 ) 21JA118M
C 852 FORMAT ( 5X, I2, 1X, I3, 5E11.3, F6.1, E11.3 ) 21JA1CDC
C 853 FORMAT ( 22X, 1P2E11.3 ) 21JA118M
C 853 FORMAT ( 22X, 2E11.3 ) 21JA1CDC
C 860 FORMAT ( //50H STATICS CHECK. SUMMATION OF REACTION, 135E8CDC
C 1 6HS = , E10.3 ) 21JA1CDC
C 860 FORMAT ( //50H STATICS CHECK. SUMMATION OF REACTION, 06DE118M
1 6HS = , 1PE10.3 ) 06DE118M
C 861 FORMAT ( / 29X 35HMAXIMUM STATICS CHECK ERROR AT STA ,213, 09FE1CDC
C 1 2H =, E11.3 ) 16AP1CDC
C 861 FORMAT ( / 29X 35HMAXIMUM STATICS CHECK ERROR AT STA ,213, 06DE118M
1 2H =, 1PE11.3 ) 06DE118M
864 FORMAT ( // 25H PROFILE OUTPUT AREAS ) 15FE1
C 865 FORMAT ( 50H X MOMENTS ACT IN X DIRECTION (ABOUT Y AX 15FE1CDC
C 1 3HIS),/10X, 35HTHE PLOTTED RESULTS INDICATE THE RE 15MR1CDC
C 2 40HLATIVE VALUE EACH HAS WITHIN THAT LIST ) 15FE1CDC
C 865 FORMAT ( 50H X MOMENTS ACT IN X DIRECTION (ABOUT Y AX 06DE118M
C 1 3HIS),/10X, 35HTHE PLOTTED RESULTS INDICATE THE RE 06DE118M
C 2 40HLATIVE VALUE EACH HAS WITHIN THAT LIST ) 06DE118M
C 866 FORMAT ( // 42H BETWEEN 15FE1CDC
C 1 2H (, I3, 2H ,, I3, 8H ) AND (, I3, 2H ,, I3, 2H ) ) 06DE1CDC
C 866 FORMAT ( // 42H BETWEEN 29FE218M
1 2H (, I3, 2H ,, I3, 8H ) AND (, I3, 2H ,, I3, 2H ) ) 29FE218M

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867 FORMAT ( 1HL , 17X, 11HDEFLECTIONS 10DE1IBM
I // 25H X , Y DEFLECTION , / ) 10DE1IBM
C 867 FORMAT ( 1H+ , 17X, 11HDEFLECTIONS 10DE1COC
C 1 // 25H X , Y DEFLECTION , / ) 10DE1COC
868 FORMAT ( 1HL , 9X, A6, 5H AND ,A6, 1X, A6, 5H MOM , / ) 10DE1IBM
1 // 12H X , Y , A6, 5H MOM , 27X A6, 5H MOM , / ) 10DE1IBM
C 868 FORMAT ( 1H+ , 9X, A6, 5H AND ,A6, 1X, A6, 10DE1COC
C 1 // 12H X , Y , A6, 5H MOM , 27X A6, 5H MOM , / ) 10DE1COC
869 FORMAT ( 1HL , 20X, A6, 1X, A6, 10DE1IBM
1 // 12H X , Y , A6,5H MOM , / ) 10DE1IBM
C 869 FORMAT ( 1H+ , 20X, A6, 1X, A6, 10DE1COC
C 1 // 12H X , Y , A6,5H MOM , / ) 10DE1COC
870 FORMAT ( 1HL , 17X, 10HPRINCIPAL , A6, 10DE1IBM
1 // 17H X , Y PRIN , A6, / ) 10DE1IBM
C 870 66RMT ( 1H+ 8 17X, 10HPRINCIPAL , A6, 10DE1COC
C 1 // 17H X , Y PRIN , A6, / ) 10DE1COC
900 FORMAT ( //40H TABLE 9. PRINTED OUTPUT LIMITS 24FE1
1 // 20H FROM THRU , 21JA1
2 // 20H Y STA Y STA ) 14AP77
903 FORMAT ( / 25H NONE )
905 FORMAT ( 46H USING DATA FROM THE PREVIOUS PROBLEM )
910 FORMAT ( 43H ADDITIONAL DATA FOR THIS PROBLEM )
911 FORMAT (215X, I5)
920 FORMAT ( //15H RESULTS )
921 FORMAT ( //35H RESULTS--USING STIFFNESS DATA ,
1 22HFROM PREVIOUS PROBLEM , A5 )
C 1 22HFROM PREVIOUS PROBLEM , A1, A4 ) 14AP77
980 FORMAT ( //40H **** UNDESIGNATED ERROR STOP **** ) 19JEB
981 FORMAT ( //51H **** CAUTION. MULTIPLE LOADING OPTION MISUSED 02FE1
1 35H FOR THIS OR PRIOR PROBLEM **** ) 02FE1
982 FORMAT ( //50H **** MISUSE OF MULTIPLE LOADING OPTION **** ) 02FE1
983 FORMAT ( //50H **** IMPROPER NO OF CARDS INPUT OR KEPT **** ) 02FE1
984 FORMAT ( /50H **** X INCREMENTS EXCEED Y INCREMENTS **** ) 02FE1
985 FORMAT ( 35H **** ERRONEOUS DATA INPUT **** ) 02FE1
986 FORMAT ( /50H **** THE DIMENSIONED STORAGE IS TOO SMALL FOR 02FE1
1 30H THIS SIZE OF PROBLEM **** ) 12FE1
990 FORMAT ( 35H **** TOTAL NUMBER OF SPECIFIED 03FE1
1 10H POINTS IS , 15, 12FE1
2 25H , 300 IS MAX **** ) 24FE1
991 FORMAT ( /10H **** , I4, 19AG8
1 33H DATA ERRORS IN THIS TABLE **** ) 23AG8
992 FORMAT ( //30H **** PROBLEM TERMINATED , I4 , 23AG8
1 20H DATA ERRORS **** )
C
C-----DEFINE READ AND WRITE UNITS
INP = 5
IOP = 6
ID1 = 0
ID2 = 0
ID3 = 0
ID4 = 0
I4D = 0
C-----PROGRAM AND PROBLEM IDENTIFICATION
C
C-----INITIALIZE DATA NORMALLY READ IN TABLE 1
IF ( ML .EQ. -1 ) GO TO 1005
KML = 0
IPLFLG = 0
1005 THK = 0.0
ITMPP = 0
KROPT = 0
IOPPS = 0
IPDP = 0
IGSW = 0
I3D = 0
VEF = 0
RDF = 0
SLOPE = 0
C-----THESE VALUES ARE ADDED TO ALLOW ECHO PRINT OF THE DATA GENERATED
C BY SLAB49M TO BE PRINTED JUST AS IF IT HAD BEEN CODED.
KML = ML
SWS = 1.0
SWB = 0.0
IBOS = 1

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IBOST = 1
NDE1 = 0
NDE2 = 0
NDE3 = 0
NDE4 = 0
NDE5 = 0
NDE6 = 0
NDE7 = 0
NDE8 = 0
NDE9 = 0
IF ( KEEP3 .EQ. 0 ) NCT3 = NCD3
IF ( KEEP4 .EQ. 0 ) NCT4 = NCD4
IF ( KEEP5 .EQ. 0 ) NCT5 = NCD5
IF ( KEEP6 .EQ. 0 ) NCT6 = NCD6
IF ( KEEP7 .EQ. 0 ) NCT7 = NCD7
IF ( KEEP8 .EQ. 0 ) NCT8 = NCD8
IF ( KEEP9 .EQ. 0 ) NCT9 = NCD9
C-----OMIT PRINT OF GENERATED DATA
21JA1
21JA1 IF ( IPGND .EQ. 0 ) GO TO 4870
17FE1
C READ 12, (AN1(N), N = 1, 40)
C1010 READ 14, NPR0B, (AN2(N), N = 1, 18)
1020 IF ( NPR0B(2) - ITEST(2) ) 1021, 9990, 1021
1021 PRINT 11
PRINT 1
PRINT 13, AN1
PRINT 15, NPR0B, AN2
C-----INPUT TABLE 1 CONTROL DATA
C
C READ 20, KEEP2, KEEP3, KEEP4, KEEP5, KEEP6, KEEP7, KEEP8, 03MR77
C 1 KEEP9, ML, ITMPP, NCD2, NCD3, NCD4, NCD5, NCD6, NCD7, 03MR77
C 2 NCD8, NCD9, KROPT, IOPPS, IPOP, IGSW, I3D, VEF, RDF, SLOPE 03MR77
I4D = I4D + I3D
PRINT 100, KEEP2, KEEP3, KEEP4, KEEP5, KEEP6, KEEP7, KEEP8, 18MY0
KEEP9, NCD2, NCD3, NCD4, NCD5, NCD6, NCD7, 06JCO
NCD8, NCD9, ML, KROPT, IOPPS, IPOP, I3D 02FE1
NDE1 = 0
IF ( KML ) 1174, 1170, 1172
1170 IF ( ML ) 1171, 1175, 1175
1171 NOE1 = NDE1 + 1
PRINT 982
GO TO 1176
1172 IF ( ML ) 1176, 1173, 1173
1173 PRINT 981
GO TO 1175
1174 IF ( ML ) 1176, 1175, 1175
1175 IF ( KEEP2 ) 1179, 1177, 1178
1176 IF ( KEEP2 ) 1179, 1179, 1178
1177 IF ( NCD2 ) 1179, 1179, 1180
1178 IF ( NCD2 ) 1179, 1180, 1179
1179 NOE1 = NDE1 + 1
PRINT 983
1180 KML = ML
IF ( NDE1 ) 9980, 1200, 1182
1182 PRINT 991, NDE1
C-----INPUT TABLE 2 -- CONSTANTS
C
1200 PRINT 200 02FE1
IF ( KEEP2 ) 9980, 1201, 1240 24FE1
1201 NDE2 = 0 26AG8
IF ( NCD2 - 1 ) 1203, 1205, 1203 28AG8
1203 NDE2 = NDE2 + 1 26AG8
PRINT 985 23FE1
C1205 READ 21, MX, MY, HX, HY, PR, THK 03MR77
1205 PRINT 201, MX, MY, HX, HY, PR, THK 03MR77
IF ( MX - MY ) 1211, 1211, 1210 19AG8
1210 NDE2 = NDE2 + 1 16AG8

```

The statements which originally read the data cards in Tables 2 through 9 have been nulled with a C in column 1. The remainder of the data input and print logic is the same as in the original SB49S routine (Ref 8).


```

8480      K = INS(JJ)
DO 8485 II = 1, K
IF ( ISM(IJ,II) .EQ. ISTA ) GO TO 8490
CONTINUE
IF ( JSM(JP2) .EQ. JSM(JP2-1) ) GO TO 8550
GO TO 8495
8490      IF ( JSM(JP2+1) .EQ. 0 ) GO TO 8500
IF ( JSM(JP2) .EQ. JSM(JP2-1) ) GO TO 8505
8495      IF ( JSM(JP2-1) .EQ. 0 ) GO TO 8525
GO TO 8510
8500      IF ( JP2 .EQ. 2 .OR. JP2 .EQ. MYP2 ) GO TO 8502
SUM(IJ,II) = BMX(I) * HY * 0.5
GO TO 8550
8502      SUM(IJ,II) = BMX(I) * HY
GO TO 8550
8505      SUM(IJ,II) = SUM(JJ,II) + BMX(I) * HY
IF ( JSM(JP2) .EQ. JSM(JP2+1) ) GO TO 8550
SUM(JJ,II) = SUM(JJ,II) + SUMT(II)
GO TO 8550
8510      K = INS(JJ-1)
DO 8515 IIT = 1, K
IF ( ISM(IJ-1,IIT) .EQ. ISTA ) GO TO 8520
CONTINUE
GO TO 8525
8520      IF ( JSM(JP2) .EQ. JSM(JP2-1) ) GO TO 8525
SUM(IIT) = BMX(I) * HY * 0.5
8525      IF ( ISM(IJ,II) .EQ. ISTA ) GO TO 8530
GO TO 8550
8530      IF ( JP2 .EQ. 2 .OR. JP2 .EQ. MYP2 ) GO TO 8532
SUM(JJ,II) = SUM(JJ,II) + BMX(I) * HY * 0.5
GO TO 8550
8532      SUM(JJ,II) = SUM(JJ,II) + BMX(I) * HY
8550      CONTINUE
IF ( ISW ) 9980, 8600, 8650
8600      JN = J - 1
CALL DATA ( LI, JN, I, MXP3,
1          DX, DYMI, DY, DYPL, FX, FYMI,
2          FY, FYP1, Q, S,
3          RX, RYMI, RYPL, TX, TYMI, TYP1,
4          CH, CHP1, DXPL, DXML,
5          PX, PY, PYP1, PBX, PBY, PBYPL,
6          QM, IPRINT )
IF ( J - 1 ) 9980, 8610, 8650
8610      ISW = 1
DO 8620 I = 1, MXP3
WP2(I) = WP1(I)
WP1(I) = W(I)
W(I) = WM1(I)
WM1(I) = WM2(I)
WM2(I) = 0.0
8620      CONTINUE
DO 8630 I = 2, MXP2
CRD(2) = DMINI ( DXII, DYII )
BMYPL(I) = BMY(I)
BMY(I) = BMYMI(I)
BMYMI(I) = 0.0
BMBYPL(I) = BMBY(I)
BMBY(I) = BMBYMI(I)
BMBYMI(I) = 0.0
WSUM1 = ODHX2 * ( W(I-1) - 2.0 * W(I) + W(I+1) )
WSUM2 = ODHY2 * ( WM1(I) - 2.0 * W(I) + WP1(I) )
BMX(I) = DX(I) * WSUM1 + CRD(2) * PR * WSUM2
BMBX(I) = FX(I) * WSUM1
C-----BMX AND BMY ARE AT STATION 0, PBXM AND PBMY ARE STORED AT J = 2.
IF ( IBOS .EQ. 2 ) GO TO 8625
PBXM(I, 2) = BMX(I)
PBMY(I, 2) = BMY(I)
GO TO 8630
8625      PBXM(I, 2) = BMBX(I)
PBMY(I, 2) = BMBY(I)
8630      CONTINUE
JSTA = 0
J = 0
GO TO 7300

```

```

04AP77      CONTINUE
04AP77      PRINT 860, SUMR
04AP77      PRINT 861, ITEMP, JTEMP, STEMP
C-----PRINT BOX MOMENT SUMS
IF ( NBSM .EQ. 0 ) GO TO 8658
WRITE (IOP,820)
820 FORMAT (///45H BOX MOMENT SUMMATIONS (TOTAL ACROSS BOX)
1 // 51H BOX NO. X-STA X-DIST TOTAL BMX)
DO 8656 JJ = 1, NBSM
K = INS(IJJ)
DO 8654 II = 1, K
XD = ( ISM(IJ,II) + IXO ) * HX
WRITE (IOP,825) IBN(IJJ), ISM(IJ,II), XD, SUM(IJ,II)
825 FORMAT ( 10X, 2( 14, 5X ), 1PE10.3, 1PE13.3 )
8654      CONTINUE
WRITE (IOP,6)
8656      CONTINUE
GO TO 8659
8658      WRITE (IOP,828)
828 FORMAT (///30H NO BOX SUMS SPECIFIED )
8659      CONTINUE
C ***** NO ZOT, SPLOT ONLY, FOR IBM *****
IPDP = 0
C IF (IPOP.GT.0 .OR. I3D .EQ. 1) CALL PLOTS( IBUF,500,10)
C----- 3D PLOT CAPABILITY OMITTED FOR THIS VERSION *****

```

The following 154 cards are exactly the same as in SB49S (Ref 8) and are thus omitted from this listing.

```

C-----LINE PLOTS NOT AVAILABLE THIS VERSION
NP2 = NENO + 2
DO 8872 I = 1, NP2
8872      XX(I) = I - 1
IF ( IPOP.EQ.1 ) GO TO 8865
8884      CONTINUE
8885      CONTINUE
8950      CONTINUE
IF ( I3D.EQ.1 )
IPRINT 9099
9099 FORMAT ( //50H **** 3D PLOT NOT AVAILABLE THIS VERSION **** )
C CALL TIC TOC (4)
C GO TO 1010
C-----RETURN TO DATA GENERATOR
RETURN
9980 PRINT 980
9990 CONTINUE
9999 CONTINUE
C----- 3D PLOTS NOT AVAILABLE THIS VERSION
C IF ( IPOP.GT.0 .OR. I3D.EQ.1 ) CALL ENOPLT
PRINT 11
PRINT 1
PRINT 13, AN1
CALL TIC TOC (2)
PRINT 19
RETURN
END

```

The remainder of the subroutines are exactly the same as those used for SLAB 49 (Ref 8). The only changes are total double precisioning and the increase in the axial thrust array sizes in common block /CARDS/ of subroutine STIFF. 1198 cards are thus omitted from this listing.

APPENDIX D
TYPICAL OUTPUT
PROGRAM SLBDG 2

PROGRAM SLBDG2 BRIDGE DIVISION, TEXAS DOT PANAK REVISION DATE 10 AUG 77

POST TEST ANALYSIS OF BEEVILLE STRUCTURE

14 SPAN INCRS, 44 TRANSVERSE INCREMENTS RUN AS CHECK FOR 10 AUG 77 VERSION

PROB 170A LOADING NO. ONE EAST EDGE

TABLE 1. CONTROL DATA

	2	3	4	5	6	7	8	9
KEEP FROM PRECEDING PROBLEM (1=YES)	0	0	0	0	0	0	0	0
NUM CARDS INPUT THIS PROBLEM	1	1	1	2	1	2	3	3
MULTIPLE LOAD OPTION	1							
PRINT GENERATED DATA	0							
SKEW ANGLE	0.3740	02						

TABLE 2. CONSTANTS

NUMBER OF SPAN INCREMENTS IN X DIRECTION	14
NUMBER OF INCREMENTS IN Y DIRECTION	44
INCREMENT LENGTH IN X DIRECTION (ALONG SPAN CENTERLINE)	3.7390 01
INCREMENT LENGTH IN Y DIRECTION	1.2160 01
POISSONS RATIO	1.5000-01
MODULUS OF ELASTICITY	6.2000 03

TABLE 3. BOX STIFFNESS DATA

TYPE NO.	INCR NO.	BOX WIDTH	VOID DEP	SLAB THICK TOP	SLAB THICK BOT	SHEAR KEY STIFF FACTOR	LONGIT. BOX MOMENT	BOX INERTIAS TORSIONAL
1	4	20.00	23.75	5.50	5.00	0.1000-05	3.0990 04	7.1980 04

TABLE 4. BOX ARRANGEMENT BY TYPE NO.

1 1 1 1 1 1

TABLE 5. LOAD PATTERNS

PAT	X AND Y DISTANCES (OF THE LOAD PATTERN)										
LDS	LOAD MAGNITUDE										
1	-170.	-39.	-170.	39.	-25.	-36.	-25.	36.	25.	-36.	25.
6	-5.9300 00	-5.9300 00	-1.4550 01	-1.4550 01	-1.4550 01	-1.4550 01	-1.4550 01	-1.4550 01	-1.4550 01	-1.4550 01	

TABLE 6. PLACEMENTS OF LOAD PATTERNS

PAT-PLMTS	X AND Y DISTANCES (OF THE STRUCTURE)	
1	1	402. 181.

TABLE 7. SUMMATION OF BOX BENDING MOMENTS

BOX	SUMS	SUM AT X-STATION NEAREST THESE X-DISTANCES				
10	5	370.	406.	442.	478.	514.
11	5	370.	406.	442.	478.	514.

TABLE 8. PROFILE OUTPUT AREAS

DISTS FROM - THRU				DEFL	X MOM	Y MOM	PRIN MOM
X	Y	X	Y		(1=YES)		
160.	242.	727.	242.	1	0	0	0
-38.	0.	562.	0.	1	0	0	0
-204.	-242.	360.	-242.	1	0	0	0

TABLE 9. PRINTED OUTPUT LIMITS

I-----TRIM Y-DIST FROM - THRU
Y
-268. -243.
-12. 12.
243. 268.

SUMMATION OF LOADS ON STRUCTURE -7.0060 01
SUMMATION OF LOADS OFF STRUCTURE 0.0

RESULTS FROM SLAB49 PROGRAM USING DATA GENERATED BY SLBDG2

POST TEST ANALYSIS OF BEEVILLE STRUCTURE

14 SPAN INCRS, 44 TRANSVERSE INCREMENTS RUN AS CHECK FOR 10 AUG 77 VERSION

PROB (CONTD) 170A LOADING NO. ONE EAST EDGE

RESULTS

SLAB X MOMENT AND X TWISTING MOMENT ACT IN THE X DIRECTION (ABOUT Y AXIS)
Y TWISTING MOMENT = -X TWISTING MOMENT, COUNTERCLOCKWISE BETA ANGLES ARE POSITIVE FROM THE X AXIS TO THE DIRECTION OF THE LARGEST PRINCIPAL MOMENT
SLAB MOMENTS ARE PER UNIT WIDTH

X , Y	DEFL	SLAB X MOMENT	SLAB Y MOMENT	SLAB X TWISTING MOMENT	LARGEST PRINCIPAL SLAB MOMENT	BETA X TO LARGEST MOMENT	SUPPORT REACTION
0 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
8 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
10 44	2.9930-02	0.0	0.0	0.0	0.0	0.0	0.0
11 44	1.0200-02	-3.1260 00	7.1020-16	1.9220 01	-2.0850 01	-42.7	-3.3770 04
12 44	-1.1700-02	-1.8360 00	-2.8920-16	-1.8460 00	-2.9800 00	31.8	3.3790 04
13 44	-3.4870-02	2.2140 00	-1.3680-15	-2.8100 00	4.1270 00	-34.2	0.0
14 44	-5.6500-02	4.6900 00	-2.3760-15	-2.7610 00	5.9670 00	-24.8	0.0
15 44	-7.4890-02	7.0780 00	-3.2340-15	-2.7390 00	8.0150 00	-18.9	0.0
16 44	-8.8390-02	9.3640 00	-3.8720-15	-2.8640 00	1.0170 01	-15.7	0.0

17	44	-9.5410-02	9.7330 00	-4.2200-15	-3.0950 00	1.0630 01	-16.2	0.0
18	44	-9.5690-02	8.4930 00	-4.2660-15	-3.2640 00	9.6020 00	-18.8	0.0
19	44	-9.0100-02	6.4210 00	-4.0470-15	-3.3130 00	7.8230 00	-22.9	0.0
20	44	-8.0060-02	4.4500 00	-3.6280-15	-3.2740 00	5.1840 00	-27.9	0.0
21	44	-6.6950-02	2.6520 00	-3.0670-15	-3.2000 00	4.7900 00	-33.7	0.0
22	44	-5.2000-02	1.0050 00	-2.4250-15	-3.0820 00	3.6260 00	-40.4	0.0
23	44	-3.6360-02	-4.1430-01	-1.7480-15	-2.8850 00	-3.0990 00	42.9	0.0
24	44	-2.1000-02	-1.2490 00	-1.0820-15	-2.5100 00	-3.2100 00	38.0	0.0
25	44	-6.5110-03	-7.4490-01	-4.5000-16	-1.4180 00	-1.8390 00	37.6	2.1560 04
26	44	7.4660-03	1.2560-14	1.7590-16	-2.3200-31	2.3200-01	-45.0	-2.1560 04

0	43	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	43	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2	43	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3	43	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4	43	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5	43	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6	43	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7	43	0.0	0.0	0.0	0.0	0.0	0.0	0.0
8	43	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9	43	4.4170-02	0.0	0.0	0.0	0.0	0.0	0.0
10	43	2.4460-02	-7.9170-11	3.3060-01	2.7090 01	2.7250 01	45.2	0.0
11	43	4.7220-03	-5.1400 00	3.1110 00	-6.9870-01	-5.1990 00	4.8	-2.2910 04
12	43	-1.6990-02	-2.1780 00	1.2860 00	-3.2220 00	-4.1050 00	30.9	2.2890 04
13	43	-3.9530-02	4.3530 00	1.7880-01	-5.2340 00	7.9010 00	-34.1	0.0
14	43	-6.0540-02	9.1570 00	7.1460-02	-5.2790 00	1.1580 01	-24.6	0.0
15	43	-7.8320-02	1.3860 01	2.3130-06	-5.2370 00	1.5610 01	-18.5	0.0
16	43	-9.1210-02	1.8490 01	-1.7340-01	-5.5870 00	2.0030 01	-15.5	0.0
17	43	-9.7540-02	1.9190 01	-1.6700-01	-6.2390 00	2.1030 01	-16.4	0.0
18	43	-9.7090-02	1.6700 01	-8.8930-02	-6.6990 00	1.9040 01	-19.3	0.0
19	43	-9.0730-02	1.2550 01	1.5770-02	-6.8290 00	1.5560 01	-23.7	0.0
20	43	-7.9930-02	8.6780 00	6.3170-02	-6.7340 00	1.2360 01	-28.7	0.0
21	43	-6.6070-02	5.1450 00	1.0680-01	-6.5710 00	9.6630 00	-34.5	0.0
22	43	-5.0390-02	1.8950 00	1.7080-01	-6.3240 00	7.4150 00	-41.1	0.0
23	43	-3.4060-02	-9.4120-01	2.8300-01	-5.9070 00	-5.2680 00	42.0	0.0
24	43	-1.8070-02	-2.2070 00	5.4960-01	-5.2010 00	-6.5290 00	36.3	0.0
25	43	-3.0630-03	-2.2150 00	2.3160 00	-2.8040 00	3.6560 00	-64.5	1.4880 04
26	43	1.1040-02	-1.0890-11	9.3110-01	-3.9870-01	1.0780 00	-69.7	-1.4880 04

0	42	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	42	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2	42	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3	42	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4	42	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5	42	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6	42	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7	42	0.0	0.0	0.0	0.0	0.0	0.0	0.0
8	42	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9	42	3.8580-02	0.0	0.0	0.0	0.0	0.0	0.0
10	42	1.8990-02	-1.0440-11	4.7640-01	-7.4040-01	1.0160 00	-53.9	-3.6610 03
11	42	-6.1200-04	-5.6850 00	5.1150-01	-1.0470 00	-5.8570 00	9.3	3.6760 03
12	42	-2.2210-02	-1.0660 00	4.7740-01	-2.2770 00	-2.6980 00	35.6	0.0
13	42	-4.4210-02	4.5850 00	2.8360-01	-4.1760 00	7.1320 00	-31.4	0.0
14	42	-6.4630-02	9.3350 00	1.4340-01	-4.4960 00	1.1170 01	-22.2	0.0
15	42	-8.1830-02	1.4150 01	2.6240-02	-4.5010 00	1.5460 01	-16.3	0.0
16	42	-9.4130-02	1.9160 01	-8.8590-02	-5.1600 00	2.0450 01	-14.1	0.0
17	42	-9.9790-02	1.9830 01	-1.0810-01	-6.3520 00	2.1680 01	-16.3	0.0
18	42	-9.8580-02	1.7170 01	-5.9560-02	-7.1530 00	1.9750 01	-19.9	0.0
19	42	-9.1430-02	1.2800 01	2.3340-02	-7.3750 00	1.6170 01	-24.6	0.0
20	42	-7.9840-02	8.8200 00	1.0130-01	-7.2670 00	1.2930 01	-29.5	0.0
21	42	-6.5210-02	5.2010 00	1.8800-01	-7.1630 00	1.0280 01	-35.4	0.0
22	42	-4.8790-02	1.8550 00	3.0080-01	-7.0460 00	8.1660 00	-41.9	0.0
23	42	-3.1740-02	-1.1090 00	4.5400-01	-6.7300 00	-7.1030 00	41.7	0.0
24	42	-1.5100-02	-2.9810 00	6.2350-01	-5.4050 00	-6.8760 00	35.8	2.9100 03
25	42	4.7990-04	-3.9970 00	4.5120-01	-1.7890 00	-4.6270 00	19.4	-2.8830 03
26	42	1.4650-02	-8.8450-13	4.4230-01	1.7900 01	1.8120 01	45.4	0.0

0	41	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	41	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2	41	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3	41	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4	41	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5	41	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6	41	0.0	0.0	0.0	0.0	0.0	0.0	0.0

7	41	0.0	0.0	0.0	0.0	0.0	0.0	0.0
8	41	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9	41	3.3350-02	0.0	0.0	0.0	0.0	0.0	0.0
10	41	1.3990-02	-1.3990-10	1.0110 00	-4.8190 00	5.3510 00	-48.0	-2.4250 04
11	41	-5.4240-03	-5.6930 00	2.4790 00	-1.0220 00	-5.8190 00	7.0	2.4230 04
12	41	-2.6980-02	-2.4810-01	4.6760-01	-1.3680 00	1.5240 00	-52.3	0.0
13	41	-4.8640-02	4.7530 00	1.6170-01	-3.2060 00	5.4000 00	-27.2	0.0
14	41	-6.8630-02	9.1690 00	8.3900-02	-3.7210 00	1.0500 01	-19.7	0.0
15	41	-8.5380-02	1.3860 01	2.5500-02	-3.7510 00	1.4810 01	-14.2	0.0
16	41	-9.7240-02	1.9340 01	-2.0370-01	-4.7220 00	2.0420 01	-12.9	0.0
17	41	-1.0230-01	1.9870 01	-1.7480-01	-6.4730 00	2.1780 01	-16.4	0.0
18	41	-1.0020-01	1.7050 01	-9.1040-02	-7.6180 00	1.9940 01	-23.8	0.0
19	41	-9.2180-02	1.2490 01	2.8400-02	-7.9250 00	1.6340 01	-25.9	0.0
20	41	-7.9710-02	8.6050 00	6.4980-02	-7.7980 00	1.3230 01	-30.6	0.0
21	41	-6.4200-02	5.0850 00	1.0630-01	-7.7570 00	1.0740 01	-36.1	0.0
22	41	-4.6910-02	1.8040 00	1.8010-01	-7.7840 00	8.8190 00	-42.0	0.0
23	41	-2.8980-02	-1.2320 00	4.5470-01	-7.7380 00	-3.1720 00	41.9	0.0
24	41	-1.1510-02	-3.7800 00	2.7810 00	-5.9060 00	-7.2550 00	33.5	1.9950 04
25	41	4.4780-03	-4.1820 00	6.0420 00	-3.6150 00	7.1910 00	-72.4	-2.0000 04
26	41	1.8680-02	0.0	0.0	0.0	0.0	0.0	0.0

0	23	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	23	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2	23	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3	23	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4	23	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5	23	3.8190-03	0.0	0.0	0.0	0.0	0.0	0.0
6	23	7.9680-04	-1.1370-11	3.3010 00	-2.2950 00	4.4770 00	-62.9	-3.7130 03
7	23	-2.3920-03	-2.1460 00	1.4620 00	-2.8310 00	-3.6990 00	28.7	3.6850 03
8	23	-6.4140-03	-2.1570 00	2.6560-01	-3.8070 00	-4.9410 00	36.2	0.0
9	23	-1.1210-02	-1.7040 00	1.2400-01	-4.3650 00	-5.2490 00	39.1	0.0
10	23	-1.6620-02	-7.4390-01	7.5860-02	-4.9070 00	-5.2570 00	42.6	0.0
11	23	-2.2290-02	6.2980-01	4.0110-02	-5.2590 00	5.6020 00	-43.4	0.0
12	23	-2.7740-02	2.2460 00	5.7350-03	-5.3150 00	6.5570 00	-39.0	0.0
13	23	-3.2400-02	3.9050 00	-2.5660-02	-5.0430 00	7.3520 00	-34.4	0.0
14	23	-3.5680-02	5.5070 00	-6.1110-02	-4.4420 00	7.9650 00	-29.0	0.0
15	23	-3.7010-02	6.9300 00	-1.0750-01	-3.4920 00	8.3680 00	-22.4	0.0
16	23	-3.5880-02	7.9320 00	-1.6710-01	-2.2110 00	8.4970 00	-14.3	0.0
17	23	-3.1940-02	8.1860 00	-2.3360-01	-7.8190-01	8.2580 00	-5.3	0.0
18	23	-2.5090-02	7.6460 00	-3.1810-01	5.2160-01	7.6800 00	3.7	0.0
19	23	-1.5530-02	6.5130 00	-4.5230-01	1.7950 00	5.9480 00	13.6	0.0
20	23	-3.6390-03	7.5130 00	-1.8250 00	1.5900 00	7.7770 00	9.4	1.6960 04
21	23	1.1000-02	6.0460-13	-8.7070-01	1.6320 01	-1.6760 01	-65.8	-1.6940 04
22	23	2.5680-02	0.0	0.0	0.0	0.0	0.0	0.0
23	23	0.0	0.0	0.0	0.0	0.0	0.0	0.0
24	23	0.0	0.0	0.0	0.0	0.0	0.0	0.0
25	23	0.0	0.0	0.0	0.0	0.0	0.0	0.0
26	23	0.0	0.0	0.0	0.0	0.0	0.0	0.0

0	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5	22	1.8440-03	0.0	0.0	0.0	0.0	0.0	0.0
6	22	-1.9490-06	-3.2090 00	3.5470-01	-1.8550 00	-3.9990 00	23.1	1.2080 01
7	22	-2.9770-03	-1.8570 00	3.8930-01	-2.4490 00	-3.4280 00	32.7	0.0
8	22	-6.6160-03	-1.9790 00	2.8110-01	-3.2750 00	-4.3130 00	35.5	0.0
9	22	-1.0950-02	-1					

25	22	0.0	0.0	0.0	0.0	3.0	0.0	0.0	0.0
26	22	0.0	0.0	0.0	0.0	3.0	0.0	0.0	0.0
0	21	0.0	0.0	0.0	0.0	3.0	0.0	0.0	0.0
1	21	0.0	0.0	0.0	0.0	3.0	0.0	0.0	0.0
2	21	0.0	0.0	0.0	0.0	3.0	0.0	0.0	0.0
3	21	0.0	0.0	0.0	0.0	3.0	0.0	0.0	0.0
4	21	3.1080-03	0.0	0.0	0.0	3.0	0.0	0.0	0.0
5	21	1.3410-03	-6.1900-12	3.3940-01	1.8570 00	2.0350 00	47.6	-2.0660 03	
6	21	-4.4230-04	-2.5930 00	7.0880-01	-1.0010 00	-2.8720 00	15.6	2.0610 03	
7	21	-3.1780-03	-1.7310 00	2.0900-01	-2.2250 00	-3.1880 00	33.2	0.0	
8	21	-6.5370-03	-1.7020 00	1.5650-01	-2.8390 00	-3.7600 00	35.9	0.0	
9	21	-1.0510-02	-1.2690 00	1.1190-01	-3.6010 00	-4.2450 00	39.6	0.0	
10	21	-1.4930-02	-4.1370-01	7.3330-02	-4.2430 00	-4.4210 00	43.4	0.0	
11	21	-1.9500-02	7.7990-01	3.8460-02	-4.6380 00	5.0620 00	-42.7	0.0	
12	21	-2.3800-02	2.1730 00	5.5520-03	-4.7030 00	5.9160 00	-38.5	0.0	
13	21	-2.7330-02	3.6100 00	-2.6050-02	-4.4050 00	6.5570 00	-33.8	0.0	
14	21	-2.9580-02	4.9780 00	-6.1440-02	-3.7280 00	5.9580 00	-28.0	0.0	
15	21	-3.0070-02	6.1530 00	-1.0600-01	-2.6530 00	7.1260 00	-20.1	0.0	
16	21	-2.8380-02	6.9470 00	-1.6290-01	-1.1960 00	7.1430 00	-9.3	0.0	
17	21	-2.4230-02	7.1470 00	-2.4230-01	5.3000-01	7.1850 00	4.1	0.0	
18	21	-1.7540-02	6.7350 00	-4.9240-01	2.3550 00	7.4340 00	16.5	0.0	
19	21	-8.4390-03	5.6450 00	-2.4710 00	3.3810 00	5.8690 00	19.9	1.3000 04	
20	21	2.7800-03	3.1370-11	-5.3650 00	4.9210 00	-8.2870 00	-59.3	-1.2950 04	
21	21	1.4270-02	0.0	0.0	0.0	3.0	0.0	0.0	
22	21	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
23	21	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
24	21	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
25	21	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
26	21	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
0	20	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
1	20	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
2	20	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
3	20	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
4	20	2.5450-03	0.0	0.0	0.0	3.0	0.0	0.0	
5	20	8.5080-04	-2.3850-14	1.2120-06	-1.0900 00	1.0900 00	-45.0	-2.6210 03	
6	20	-8.4340-04	-2.3850 00	1.7040-06	-1.4650 00	-3.0810 00	25.4	2.6310 03	
7	20	-3.3630-03	-1.6240 00	2.5200-06	-2.1860 00	-3.1440 00	34.8	0.0	
8	20	-6.4430-03	-1.5070 00	2.0400-06	-2.6560 00	-3.5140 00	37.1	0.0	
9	20	-1.0050-02	-1.0730 00	3.7970-07	-3.2400 00	-3.8210 00	40.3	0.0	
10	20	-1.4020-02	-2.5670-01	-2.1880-06	-3.7500 00	-3.8800 00	44.0	0.0	
11	20	-1.8080-02	8.7060-01	-5.3630-06	-4.0630 00	4.5220 00	-41.9	0.0	
12	20	-2.1840-02	2.1830 00	-8.8340-06	-4.1030 00	5.3370 00	-37.6	0.0	
13	20	-2.4850-02	3.5340 00	-1.2280-05	-3.8340 00	5.9880 00	-32.6	0.0	
14	20	-2.6630-02	4.8100 00	-1.5370-05	-3.2430 00	5.4430 00	-26.7	0.0	
15	20	-2.6750-02	5.8850 00	-1.7750-05	-2.3270 00	5.6940 00	-19.2	0.0	
16	20	-2.4830-02	6.5890 00	-1.8940-05	-1.1190 00	5.7740 00	-9.4	0.0	
17	20	-2.0640-02	6.7370 00	-1.8330-05	2.5450-01	6.7470 00	2.2	0.0	
18	20	-1.4110-02	6.2670 00	-1.5100-05	1.6330 00	5.6570 00	13.8	0.0	
19	20	-5.4190-03	6.0460 00	-6.9780-06	2.7400 00	7.1030 00	21.1	1.6700 04	
20	20	5.3660-03	-1.5330-14	-3.7650-06	1.8170 00	-1.8170 00	-45.0	-1.6740 04	
21	20	1.6150-02	0.0	0.0	0.0	3.0	0.0	0.0	
22	20	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
23	20	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
24	20	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
25	20	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
26	20	0.0	0.0	0.0	0.0	3.0	0.0	0.0	
0	2	-2.4960-04	1.2550-14	1.8050-02	-2.9720-01	3.0640-01	-45.9	0.0	
1	2	-5.9410-06	-1.6350-01	1.7990-02	-6.8160-02	-1.8630-01	18.5	3.5690 01	
2	2	1.8020-04	-2.4250-01	2.3480-02	-1.8990-01	-3.4130-01	27.5	-3.4730 01	
3	2	2.8120-04	-3.6930-01	1.8480-02	-2.2490-01	-4.7230-01	24.6	0.0	
4	2	2.5330-04	-4.6430-01	1.4640-02	-2.3990-01	-5.6380-01	22.5	0.0	
5	2	6.4000-05	-5.1440-01	1.2120-02	-2.5900-01	-6.2050-01	22.3	0.0	
6	2	-3.0410-04	-4.9220-01	9.8490-03	-2.8620-01	-6.2190-01	24.4	0.0	
7	2	-8.4300-04	-3.7270-01	6.7710-03	-3.1910-01	-5.5420-01	29.6	0.0	
8	2	-1.5110-03	-1.4500-01	2.0550-03	-3.5290-01	-4.3200-01	39.1	0.0	
9	2	-2.2300-03	1.8710-01	-5.0410-03	-3.8290-01	4.8580-01	-38.0	0.0	
10	2	-2.8840-03	6.0340-01	-1.5250-02	-4.0320-01	8.0220-01	-26.3	0.0	
11	2	-3.3280-03	1.0640 00	-2.9340-02	-4.0720-01	1.1990 00	-18.3	0.0	
12	2	-3.4020-03	1.5090 00	-4.8210-02	-3.8780-01	1.6000 00	-13.2	0.0	
13	2	-2.9510-03	1.8530 00	-7.2920-02	-3.3580-01	1.9090 00	-9.6	0.0	
14	2	-1.8550-03	1.9680 00	-1.0360-01	-2.2810-01	1.9930 00	-6.2	0.0	

15	2	-7.2100-05	1.5500 00	-1.2690-01	8.8670-02	1.5550 00	3.0	4.3310 02	
16	2	2.2540-03	-4.1180-14	-1.2020-01	1.7700-01	-2.4700-01	-54.4	-4.3440 02	
17	2	4.5860-03	0.0	0.0	0.0	0.0	0.0	0.0	
18	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
19	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
20	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
21	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
22	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0	1	-1.9270-04	7.2790-14	3.8340-02	-1.5950-02	4.4110-02	-70.1	2.5960 02	
1	1	5.3660-05	-9.6510-02	5.2930-02	-1.0650-01	-1.4430-01	24.2	-2.6040 02	
2	1	2.6120-04	-2.3430-01	1.9040-02	-1.9000-01	-3.3600-01	28.2	0.0	
3	1	3.8490-04	-3.5670-01	1.0990-02	-2.1070-01	-4.5240-01	24.4	0.0	
4	1	3.8190-04	-4.4930-01	9.1510-03	-2.2970-01	-5.4460-01	22.5	0.0	
5	1	2.1970-04	-4.9700-01	8.7240-03	-2.4950-01	-5.9940-01	22.3	0.0	
6	1	-1.1870-04	-4.7410-01	8.3820-03	-2.7150-01	-5.9610-01	24.2	0.0	
7	1	-6.2510-04	-3.5690-01	7.1830-03	-2.9420-01	-5.2080-01	29.1	0.0	
8	1	-1.2580-03	-1.3500-01	4.6300-03	-3.1440-01	-3.8720-01	38.7	0.0	
9	1	-1.9390-03	1.8770-01	2.8050-04	-3.2800-01	4.3520-01	-37.0	0.0	
10	1	-2.5530-03	5.9110-01	-6.3400-03	-3.3020-01	7.3760-01	-23.9	0.0	
11	1	-2.9590-03	1.0360 00	-1.5710-02	-3.1480-01	1.1230 00	-15.5	0.0	
12	1	-2.9970-03	1.4630 00	-2.8510-02	-2.7510-01	1.5120 00	-10.1	0.0	
13	1	-2.5160-03	1.7890 00	-4.7520-02	-2.0320-01	1.8110 00	-6.2	0.0	
14	1	-1.4010-03	1.8970 00	-9.4520-02	-1.1720-01	1.9030 00	-3.4	1.8880 03	
15	1	3.8910-04	1.4120 00	-1.1450-01	5.6890-02	1.4140 00	2.1	-1.8880 03	
16	1	2.6850-03	-1.1050-14	-7.2460-02	3.0150 00	-3.3510 00	-45.3	0.0	
17	1	4.9840-03	0.0	0.0	0.0	0.0	0.0	0.0	
18	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
19	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
20	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
21	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
22	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0	0	-1.3430-04	3.3300-16	7.3060-22	-9.2780-03	9.2780-03	-45.0	3.8790 02	
1	0	1.1710-04	-3.5930-02	-4.0950-22	-5.5260-02	-7.5720-02	36.1	-3.8790 02	
2	0	3.4400-04	-1.1590-01	-1.1460-21	-9.5070-02	-1.6930-01	29.3	0.0	
3	0	4.9080-04	-1.7980-01	-1.7550-21	-1.0760-01	-2.3010-01	25.1	0.0	
4	0	5.1320-04	-2.2730-01	-2.5390-21	-1.1640-01	-2.7640-01	22.8	0.0	
5	0	3.7840-04	-2.5140-01	-2.2490-21	-1.2500-01	-3.0300-01	22.4	0.0	
6	0	6.9540-05	-2.3950-01	-8.7430-22	-1.3400-01	-2.9940-01	24.1	0.0	
7	0	-4.0490-04	-1.7970-01	2.4120-21	-1.4290-01	-2.5860-01	28.9	0.0	
8	0	-1.0040-03	-6.6760-02	3.7870-21	-1.5040-01	-1.8740-01	38.7	0.0	
9	0	-1.6490-03	9.7070-02	6.4810-21	-1.5490-01	2.1380-01	-36.3	0.0	
10	0	-2.2260-03	3.0150-01	1.1800-20	-1.5430-01	3.6640-01	-22.8	0.0	
11	0	-2.5960-03	5.2630-01	5.9690-21	-1.4610-01	5.6420-01	-14.5	0.0	
12	0	-2.6010-03	7.4090-01	1.5020-20	-1.2730-01	7.6220-01	-9.5	0.0	
13	0	-2.0930-03	9.0290-01	2.5010-20	-9.3750-02	9.1250-01	-		

BOX MOMENT SUMMATIONS (TOTAL ACROSS BOX)

BOX NO.	X-STA	X-DIST	TOTAL BMX
10	16	3.739D 02	9.226D 02
10	17	4.113D 02	9.475D 02
10	18	4.487D 02	8.112D 02
10	19	4.861D 02	5.937D 02
10	20	5.234D 02	4.097D 02
11	16	3.739D 02	9.313D 02
11	17	4.113D 02	9.612D 02
11	18	4.487D 02	8.299D 02
11	19	4.861D 02	6.154D 02
11	20	5.234D 02	4.247D 02

19	41	-9.218D-02
20	41	-7.971D-02
21	41	-6.420D-02
22	41	-4.691D-02
23	41	-2.898D-02
24	41	-1.151D-02
25	41	4.478D-03
10	42	1.899D-02
11	42	-6.120D-04
12	42	-2.221D-02
13	42	-4.421D-02
14	42	-6.463D-02
15	42	-8.183D-02
16	42	-9.413D-02
17	42	-9.979D-02
18	42	-9.858D-02
19	42	-9.143D-02
20	42	-7.984D-02
21	42	-6.521D-02
22	42	-4.879D-02
23	42	-3.174D-02
24	42	-1.510D-02
25	42	4.799D-04



BETWEEN (4 , 22) AND (21 , 22)

DEFLECTIONS

X , Y DEFLECTION

4	22	0.0
5	22	1.844D-03
6	22	-1.949D-06
7	22	-2.977D-03
8	22	-6.616D-03
9	22	-1.095D-02
10	22	-1.583D-02
11	22	-2.092D-02
12	22	-2.577D-02
13	22	-2.983D-02
14	22	-3.257D-02
15	22	-3.344D-02
16	22	-3.198D-02
17	22	-2.787D-02
18	22	-2.102D-02
19	22	-1.158D-02
20	22	3.119D-06
21	22	1.448D-02

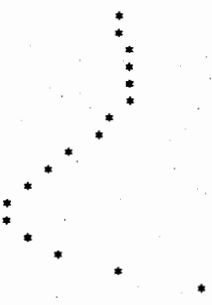


BETWEEN (0 , 2) AND (16 , 2)

DEFLECTIONS

X , Y DEFLECTION

0	2	-2.496D-04
1	2	-5.941D-06
2	2	1.802D-04
3	2	2.812D-04
4	2	2.533D-04
5	2	6.400D-05
6	2	-3.041D-04
7	2	-8.430D-04
8	2	-1.511D-03
9	2	-2.230D-03
10	2	-2.884D-03
11	2	-3.328D-03
12	2	-3.402D-03
13	2	-2.951D-03
14	2	-1.855D-03
15	2	-7.210D-05
16	2	2.254D-03



CPU TIME SINCE LAST CALL = 183.363 SECONDS, TOTAL ELAPSED TIME = 183.363 SECONDS

RESULTS FROM SLAB49 PROGRAM USING DATA GENERATED BY SLBDG2

POST TEST ANALYSIS OF BEEVILLE STRUCTURE

14 SPAN INCRS, 44 TRANSVERSE INCREMENTS RUN AS CHECK FOR 10 AUG 77 VERSIQV

PROB (CONTD)

170A LOADING NO. ONE EAST EDGE

PROFILE OUTPUT AREAS

X MOMENTS ACT IN X DIRECTION (ABOUT Y AXIS)

THE PLOTTED RESULTS INDICATE THE RELATIVE VALUE EACH HAS WITHIN THAT LIST

BETWEEN (10 , 41) AND (25 , 42)

DEFLECTIONS

X , Y DEFLECTION

10	41	1.399D-02
11	41	-5.424D-03
12	41	-2.698D-02
13	41	-4.864D-02
14	41	-6.863D-02
15	41	-8.538D-02
16	41	-9.724D-02
17	41	-1.023D-01
18	41	-1.002D-01



PROGRAM SLBDG2 BRIDGE DIVISION, TEXAS DQHPT PANAK REVISION DATE 10 AUG 77
 POST TEST ANALYSIS OF BEEVILLE STRUCTURE
 14 SPAN INCRS, 44 TRANSVERSE INCREMENTS RUN AS CHECK FOR 10 AUG 77 VERSIDV
 PROB
 171A LOADING NO. TWO CENTERLINE

TABLE 1. CONTROL DATA

	2	3	4	5	6	7	8	9
KEEP FROM PRECEDING PROBLEM (1=YES)	1	1	1	1	0	0	0	1
NUM CARDS INPUT THIS PROBLEM	0	0	0	0	1	2	3	0
MULTIPLE LOAD OPTION	-1							
PRINT GENERATED DATA	0							
SKEW ANGLE	0.3740	02						

TABLE 2. CONSTANTS

USING DATA FROM THE PREVIOUS PROBLEM

NUMBER OF SPAN INCREMENTS IN X DIRECTION	14
NUMBER OF INCREMENTS IN Y DIRECTION	44
INCREMENT LENGTH IN X DIRECTION (ALONG SPAN CENTERLINE)	3.7390 01
INCREMENT LENGTH IN Y DIRECTION	1.2160 01
POISSON'S RATIO	1.5000-01
MODULUS OF ELASTICITY	6.2000 03

TABLE 3. BOX STIFFNESS DATA

USING DATA FROM THE PREVIOUS PROBLEM

TYPE NO.	INCR	BOX NO.	VOID	SLAB THICK	SHEAR KEY	LONGIT. BOX INERTIAS		
NO.	IN	WIDTH	DEP	WIDTH	TOP BOT	STIFF FACTOR	MOMENT	TORSIONAL
1	4	20.00	23.75	5.50	5.00	0.1000-05	3.0990 04	7.1980 04

TABLE 4. BOX ARRANGEMENT BY TYPE NO.

USING DATA FROM THE PREVIOUS PROBLEM

1	1	1	1	1	1
---	---	---	---	---	---

TABLE 5. LOAD PATTERNS

USING DATA FROM THE PREVIOUS PROBLEM

PAT	X AND Y DISTANCES (OF THE LOAD PATTERN)									
LDS	LOAD MAGNITUDE									
1	-170.	-39.	-170.	39.	-25.	-36.	-25.	36.	25.	36.
6	-5.9300 00	-5.9300 00	-1.4550 01	-1.4550 01	-1.4550 01	-1.4550 01	-1.4550 01	-1.4550 01	-1.4550 01	-1.4550 01

TABLE 6. PLACEMENTS OF LOAD PATTERNS

PAT-PLMTS	X AND Y DISTANCES (OF THE STRUCTURE)	
1	1	262. -1.

TABLE 7. SUMMATION OF BOX BENDING MOMENTS

BOX	SUMS	SUM AT X-STATION NEAREST THESE X-DISTANCES				
5	5	190.	226.	262.	298.	334.
6	5	190.	226.	262.	298.	334.

TABLE 8. PROFILE OUTPUT AREAS

DISTS FROM - THRU				DEFL	X MOM	Y MOM	PRIN MOM
X	Y	X	Y		(1=YES)		
160.	242.	727.	242.	1	0	0	0
-38.	0.	562.	0.	1	0	0	0
-204.	-242.	360.	-242.	1	0	0	0

TABLE 9. PRINTED OUTPUT LIMITS

Y-DIST FROM - THRU	Y	Y
	USING DATA	FROM THE PREVIOUS PROBLEM
	-268.	-243.
	-12.	12.
	243.	268.

SUMMATION OF LOADS ON STRUCTURE -7.0060 01
 SUMMATION OF LOADS OFF STRUCTURE 0.0

RESULTS FROM SLAB49 PROGRAM USING DATA GENERATED BY SLBDG2

POST TEST ANALYSIS OF BEEVILLE STRUCTURE

14 SPAN INCRS, 44 TRANSVERSE INCREMENTS RUN AS CHECK FOR 10 AUG 77 VERSIDV

PROB (CONTD)
 171A LOADING NO. TWO CENTERLINE

RESULTS--USING STIFFNESS DATA FROM PREVIOUS PROBLEM 170A

SLAB X MOMENT AND X TWISTING MOMENT ACT IN THE X DIRECTION (ABOUT Y AXIS)
 Y TWISTING MOMENT = -X TWISTING MOMENT, COUNTERCLOCKWISE BETA ANGLES ARE
 POSITIVE FROM THE X AXIS TO THE DIRECTION OF THE LARGEST PRINCIPAL MOMENT
 SLAB MOMENTS ARE PER UNIT WIDTH

X, Y	DEFL	SLAB X MOMENT	SLAB Y MOMENT	SLAB X TWISTING MOMENT	LARGEST PRINCIPAL SLAB MOMENT	BETA X TO LARGEST MOMENT	SUPPRT REACTION
0 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
8 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
10 44	1.2610-02	0.0	0.0	0.0	0.0	0.0	0.0
11 44	3.7130-03	1.3440 00	2.5720-16	7.9740 00	8.6740 00	42.6	-1.2300 04
12 44	-4.2580-03	2.3140 00	-1.0580-16	-3.7790-01	2.3740 00	-9.0	1.2300 04
13 44	-1.0630-02	2.8950 00	-3.9940-16	-5.8270-01	3.0080 00	-11.0	0.0
14 44	-1.5000-02	2.9480 00	-6.0380-16	-6.8220-01	3.0980 00	-12.4	0.0
15 44	-1.7320-02	2.6620 00	-7.1650-16	-7.5370-01	2.8600 00	-14.8	0.0
16 44	-1.7810-02	2.1540 00	-7.4650-16	-8.0000-01	2.4190 00	-18.3	0.0

17 44 -1.6810-02 1.5290 00 -7.0930-16 -8.2380-01 1.8880 00 -23.6 0.0
18 44 -1.4750-02 8.7410-01 -6.2430-16 -8.2810-01 1.3730 00 -31.1 0.0
19 44 -1.2080-02 2.6540-01 -5.1210-16 -8.1530-01 9.5870-01 -40.4 0.0
20 44 -9.2330-03 -2.3380-01 -3.9140-16 -7.8800-01 -9.1350-01 40.8 0.0
21 44 -6.5460-03 -5.7640-01 -2.7760-16 -7.4910-01 -1.0910 00 34.5 0.0
22 44 -4.2580-03 -7.4300-01 -1.8160-16 -7.0130-01 -1.1650 00 31.0 0.0
23 44 -2.4830-03 -7.3820-01 -1.0840-16 -6.4350-01 -1.1110 00 30.1 0.0
24 44 -1.2190-03 -5.6210-01 -5.7630-17 -5.6080-01 -9.0830-01 31.7 0.0
25 44 -3.4450-04 -1.9580-01 -2.3480-17 -3.2200-01 -4.3450-01 36.5 1.1410 03
26 44 3.9500-04 -3.3680-15 8.4510-18 -5.3640-02 -5.3640-02 45.0 -1.1410 03

0 43 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
1 43 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
2 43 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
3 43 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
4 43 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
5 43 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
6 43 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
7 43 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
8 43 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
9 43 1.9640-02 0.0 0.0 0.0 0.0 0.0 0.0 0.0
10 43 1.0680-02 -3.9510-11 -1.4220-01 1.1930 01 -1.2010 01 -45.2 0.0
11 43 1.7230-03 2.9300 00 9.0120-02 5.9990-02 2.9310 00 1.2 -8.3580 03
12 43 -6.2010-03 4.7140 00 -7.6370-02 -6.7000-01 4.8060 00 -7.8 8.3570 03
13 43 -1.2450-02 5.7310 00 -1.0930-01 -1.1360 00 5.9450 00 -10.6 0.0
14 43 -1.6680-02 5.8160 00 -8.5080-02 -1.3800 00 5.1230 00 -12.5 0.0
15 43 -1.8840-02 5.2420 00 -5.9290-02 -1.5490 00 5.6610 00 -15.2 0.0
16 43 -1.9140-02 4.2340 00 -3.5810-02 -1.6570 00 4.8020 00 -18.9 0.0
17 43 -1.7950-02 2.9980 00 -1.5180-02 -1.7110 00 3.7710 00 -24.3 0.0
18 43 -1.5700-02 1.7070 00 2.8780-03 -1.7180 00 2.7730 00 -31.8 0.0
19 43 -1.2840-02 5.0710-01 1.8790-02 -1.6850 00 1.9660 00 -40.9 0.0
20 43 -9.8060-03 -4.7720-01 3.2500-02 -1.6180 00 -1.8600 00 40.5 0.0
21 43 -6.9410-03 -1.1530 00 4.3880-02 -1.5240 00 -2.1920 00 34.3 0.0
22 43 -4.4860-03 -1.4820 00 5.4600-02 -1.4120 00 -2.3220 00 30.7 0.0
23 43 -2.5580-03 -1.4780 00 7.0250-02 -1.2840 00 -2.2030 00 29.5 0.0
24 43 -1.1560-03 -1.1520 00 1.1800-01 -1.1340 00 -1.8160 00 30.4 0.0
25 43 -1.6640-04 -5.4590-01 5.3720-01 -6.2500-01 -8.3130-01 24.5 8.0750 02
26 43 6.0250-04 -6.8890-13 2.2000-01 -9.2180-02 2.5350-01 -70.0 -8.1190 02

0 42 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
1 42 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
2 42 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
3 42 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
4 42 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
5 42 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
6 42 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
7 42 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
8 42 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
9 42 1.7770-02 0.0 0.0 0.0 0.0 0.0 0.0 0.0
10 42 8.7360-03 -4.7990-12 -2.4410-01 3.5700-01 -4.9930-01 -54.4 -1.6840 03
11 42 -2.8010-04 3.2090 00 -2.5740-01 1.0800-01 3.2120 00 1.8 1.6830 03
12 42 -8.1720-03 5.0200 00 -2.0490-01 -7.9320-01 5.1380 00 -8.4 0.0
13 42 -1.4320-02 5.9480 00 -1.6020-01 -1.2880 00 5.2090 00 -11.4 0.0
14 42 -1.8390-02 6.0050 00 -1.2090-01 -1.5530 00 5.3760 00 -13.4 0.0
15 42 -2.0380-02 5.3960 00 -8.5570-02 -1.7330 00 5.8980 00 -16.1 0.0
16 42 -2.0500-02 4.3480 00 -5.3880-02 -1.8410 00 5.0170 00 -20.0 0.0
17 42 -1.9110-02 3.0700 00 -2.5690-02 -1.8880 00 3.9630 00 -25.3 0.0
18 42 -1.6660-02 1.7370 00 -6.4380-04 -1.8810 00 2.9410 00 -32.6 0.0
19 42 -1.3600-02 4.9940-01 2.1800-02 -1.8310 00 2.1070 00 -41.3 0.0
20 42 -1.0370-02 -5.1550-01 4.2270-02 -1.7440 00 -2.0030 00 40.5 0.0
21 42 -7.3280-03 -1.2110 00 6.1890-02 -1.6360 00 -2.3300 00 34.4 0.0
22 42 -4.7050-03 -1.5510 00 8.3140-02 -1.5230 00 -2.4620 00 30.9 0.0
23 42 -2.6220-03 -1.5500 00 1.0970-01 -1.3990 00 -2.3470 00 29.7 0.0
24 42 -1.0810-03 -1.2080 00 1.3930-01 -1.1400 00 -1.8580 00 29.7 2.0840 02
25 42 3.3750-05 -9.4030-01 1.0360-01 -3.9750-01 -1.0740 00 18.6 -2.0270 02
26 42 8.1790-04 -3.0380-14 1.0370-01 1.0230 00 1.0760 00 46.5 0.0

0 41 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
1 41 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
2 41 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
3 41 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
4 41 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
5 41 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
6 41 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

7 41 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
8 41 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
9 41 1.5700-02 0.0 0.0 0.0 0.0 0.0 0.0
10 41 6.5600-03 -6.8480-11 -5.2340-01 2.1310 00 -2.4090 00 -48.5 -1.1370 04
11 41 -2.5480-03 3.4700 00 -1.2140 00 1.2270-01 3.4730 00 1.5 1.1380 04
12 41 -1.0370-02 5.1280 00 -2.2450-01 -1.0220 00 5.3170 00 -10.4 0.0
13 41 -1.6360-02 5.9330 00 -1.1720-01 -1.4540 00 5.2640 00 -12.8 0.0
14 41 -2.0260-02 5.9560 00 -8.5300-02 -1.7290 00 5.4160 00 -14.9 0.0
15 41 -2.2040-02 5.3330 00 -5.9160-02 -1.9180 00 5.9660 00 -17.7 0.0
16 41 -2.1930-02 4.2820 00 -5.5570-02 -2.0260 00 5.0840 00 -21.6 0.0
17 41 -2.0310-02 3.0100 00 -1.4970-02 -2.0650 00 4.0570 00 -26.9 0.0
18 41 -1.7630-02 1.6880 00 3.0560-03 -2.0450 00 3.0570 00 -33.8 0.0
19 41 -1.4340-02 4.6120-01 1.9050-02 -1.9770 00 2.2290 00 -41.8 0.0
20 41 -1.0900-02 -5.4280-01 3.2940-02 -1.8710 00 -2.1480 00 40.6 0.0
21 41 -7.6500-03 -1.2260 00 4.4470-02 -1.7490 00 -2.4510 00 35.0 0.0
22 41 -4.8350-03 -1.5540 00 5.7370-02 -1.6370 00 -2.5730 00 31.9 0.0
23 41 -2.5720-03 -1.5610 00 1.0700-01 -1.5520 00 -2.4890 00 30.9 0.0
24 41 -8.6670-04 -1.3320 00 5.8110-01 -1.2040 00 -1.9130 00 25.8 1.5020 03
25 41 3.3860-04 -9.8060-01 1.2700 00 -8.0880-01 1.5310 00 -72.1 -1.5130 03
26 41 1.1330-03 0.0 0.0 0.0 0.0 0.0 0.0

0 23 0.0 0.0 0.0 0.0 0.0 0.0 0.0
1 23 0.0 0.0 0.0 0.0 0.0 0.0 0.0
2 23 0.0 0.0 0.0 0.0 0.0 0.0 0.0
3 23 0.0 0.0 0.0 0.0 0.0 0.0 0.0
4 23 0.0 0.0 0.0 0.0 0.0 0.0 0.0
5 23 1.8190-02 0.0 0.0 0.0 0.0 0.0
6 23 3.6740-03 -5.0040-11 4.8210 00 -4.1700 00 7.2260 00 -60.0 -1.7120 04
7 23 -1.1090-02 -1.7420 00 2.2120 00 -4.0920 00 4.7800 00 -57.9 1.7080 04
8 23 -2.6580-02 2.1990 00 3.4450-01 -5.0270 00 5.3840 00 -39.8 0.0
9 23 -4.1310-02 5.0040 00 1.2490-01 -4.8300 00 7.9750 00 -31.6 0.0
10 23 -5.4280-02 6.9540 00 1.0800-01 -4.7660 00 9.3990 00 -27.0 0.0
11 23 -6.4790-02 9.5340 00 7.2270-02 -4.9000 00 1.1610 01 -23.0 0.0
12 23 -7.1940-02 1.3200 01 -8.3520-02 -4.8020 00 1.6750 01 -17.9 0.0
13 23 -7.4420-02 1.4340 01 -1.0940-01 -4.3950 00 1.5570 01 -15.7 0.0
14 23 -7.1820-02 1.2900 01 -8.6060-02 -3.9340 00 1.3990 01 -15.6 0.0
15 23 -6.4660-02 8.7940 00 7.4100-02 -3.6950 00 1.0150 01 -20.1 0.0
16 23 -5.4400-02 5.6710 00 1.1640-01 -3.6240 00 7.4600 00 -26.3 0.0
17 23 -4.2130-02 3.0200 00 1.6580-01 -3.3710 00 5.2530 00 -33.5 0.0
18 23 -2.8810-02 4.9360-01 2.4920-01 -2.8880 00 3.2520 00 -43.8 0.0
19 23 -1.5330-02 -2.0330 00 3.9270-01 -2.3310 00 -3.4480 00 31.3 0.0
20 23 -2.5790-03 -6.4420 00 1.5930 00 -1.3750 00 -5.6710 00 9.4 1.2020 04
21 23 7.8090-03 7.1890-13 7.4600-01 1.1160 01 1.1540 01 46.0 -1.2030 04
22 23 1.8160-02 0.0 0.0 0.0 0.0 0.0
23 23 0.0 0.0 0.0 0.0 0.0
24 23 0.0 0.0 0.0 0.0 0.0
25 23 0.0 0.0 0.0 0.0 0.0
26 23 0.0 0.0 0.0 0.0 0.0

0 22 0.0 0.0 0.0 0.0 0.0 0.0 0.0
1 22 0.0 0.0 0.0 0.0 0.0 0.0 0.0
2 22 0.0 0.0 0.0 0.0 0.0 0.0 0.0
3 22 0.0 0.0 0.0 0.0 0.0 0.0 0.0
4 22 0.0 0.0 0.0 0.0 0.0 0.0 0.0
5 22 1.2050-02 0.0 0.0 0.0 0.0 0.0
6 22 -2.9140-06 -6.8420 00 7.1460-01 -3.4680 00 -8.1920 00 21.3 1.8070 01
7 22 -1.4460-02 -1.3280 00 6.7680-01 -3.0230 00 -3.5100 00 35.8 0.0
8 22 -2.9410-02 2.2920 00 4.3810-01 -3.8550 00 5.3290 00 -38.2 0.0
9 22 -4.3590-02 5.0820 00 2.7250-01 -4.1370 00 7.4520 00 -29.9 0.0
10 22 -5.6020-02 7.1300 00 7.7240-01 -4.2620 00 9.1530 00 -25.4 0.0
11 22 -6.0000-02 9.7860 00 8.7540-02 -4.3630 00 1.1460 01 -21.0 0.0
12 22 -7.2590-02 1.3390 01 -4.3080-03 -4.4170 00 1.4710 01 -16.7 0.0
13 22 -7.4550-02 1.4540 01 -3.4130-02 -4.4100 00 1.5770 01 -15.6 0.0
14 22 -7.1470-02 1.3040 01 -6.6180-04 -4.3610 00 1.4370 01 -16.9 0.0
15 22 -6.3870-02 8.9760 00 9.8710-02 -4.2980 00 1.0720 01 -22.0 0.0
16 22 -5.3180-02 5.7480 00 1.9580-01 -4.2280 00 8.0300 00 -28.4 0.0
17 22 -4.0500-02 2.9920 00 3.1530-01 -4.1190 00 5.9850 00 -36.0 0.0
18 22 -2.6810-02 3.4110-01 4.8600-01 -3.8610 00 4.2750 00 -45.5 0.0
19 22 -1.3020-02 -2.1210 00 7.1130-01 -3.1390 00 -4.1490 00 32.9 0.0
20 22 -3.0330-06 -7.0540 00 7.3830-01 -3.6100 00 -8.4690 00 21.4 1.8610 01
21 22 1.0530-02 0.0 0.0 0.0 0.0 0.0
22 22 0.0 0.0 0.0 0.0 0.0
23 22 0.0 0.0 0.0 0.0 0.0
24 22 0.0 0.0 0.0 0.0 0.0

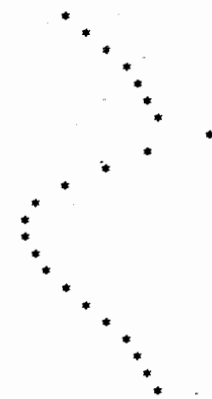
25	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
26	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
0	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4	21	2.0820-02	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5	21	8.9480-03	-4.3090-11	7.2360-01	1.2830 01	1.3190 01	45.8	-1.3780 04	
6	21	-2.9560-03	-6.3140 00	1.5580 00	-1.2610 00	-5.5110 00	8.9	1.3770 04	
7	21	-1.7170-02	-1.2030 00	3.8390-01	-2.1200 00	-2.6730 00	34.7	0.0	
8	21	-3.1830-02	2.4490 00	1.9290-01	-2.8300 00	4.3680 00	-34.1	0.0	
9	21	-4.5630-02	5.0940 00	1.1630-01	-3.4580 00	6.8660 00	-27.1	0.0	
10	21	-5.7640-02	7.0240 00	1.0750-01	-3.7590 00	8.6740 00	-23.7	0.0	
11	21	-6.7170-02	9.5940 00	7.1040-02	-3.8280 00	1.0940 01	-19.4	0.0	
12	21	-7.3320-02	1.3210 01	-8.2910-02	-4.0340 00	1.4340 01	-15.6	0.0	
13	21	-7.4790-02	1.4320 01	-1.0920-01	-4.4260 00	1.5570 01	-15.8	0.0	
14	21	-7.1190-02	1.2850 01	-8.4960-02	-4.7870 00	1.4430 01	-18.3	0.0	
15	21	-6.3040-02	8.7590 00	7.3360-02	-4.9000 00	1.0960 01	-24.2	0.0	
16	21	-5.1800-02	5.6170 00	1.1680-01	-4.8330 00	8.4280 00	-30.2	0.0	
17	21	-3.8590-02	2.9340 00	1.7620-01	-4.8820 00	5.5280 00	-37.1	0.0	
18	21	-2.4340-02	2.7130-01	4.1000-01	-4.9870 00	5.3280 00	-45.4	0.0	
19	21	-1.0020-02	-2.4450 00	2.3010 00	-4.1250 00	-4.8310 00	30.0	1.5430 04	
20	21	3.3210-03	3.7300-11	5.0350 00	-4.3260 00	7.5230 00	-60.1	-1.5470 04	
21	21	1.6410-02	0.0	0.0	0.0	0.0	0.0	0.0	
22	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0	20	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
1	20	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
2	20	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
3	20	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
4	20	1.7570-02	0.0	0.0	0.0	0.0	0.0	0.0	
5	20	5.8760-03	-2.8910-14	1.1800-06	-4.6440 00	4.6440 00	-45.0	-1.8100 04	
6	20	-5.8190-03	-6.7920 00	9.9610-06	-4.1760 00	-8.7780 00	25.4	1.8150 04	
7	20	-1.9860-02	-1.0160 00	3.3710-05	-5.1370 00	-5.6700 00	42.2	0.0	
8	20	-3.4260-02	2.7490 00	5.3540-05	-5.4210 00	6.9670 00	-37.9	0.0	
9	20	-4.7700-02	5.3510 00	6.9710-05	-5.5880 00	8.8710 00	-32.2	0.0	
10	20	-5.9300-02	7.2020 00	8.2800-05	-5.4660 00	1.0150 01	-28.3	0.0	
11	20	-6.8400-02	9.7790 00	9.3400-05	-5.0490 00	1.1920 01	-23.0	0.0	
12	20	-7.4120-02	1.3670 01	1.0090-04	-4.5430 00	1.5040 01	-16.8	0.0	
13	20	-7.5110-02	1.4800 01	1.0170-04	-3.9960 00	1.5810 01	-14.2	0.0	
14	20	-7.0980-02	1.3290 01	9.5150-05	-3.4280 00	1.4120 01	-13.6	0.0	
15	20	-6.2250-02	8.8860 00	8.2050-05	-2.8700 00	3.7320 00	-16.4	0.0	
16	20	-5.0460-02	5.7180 00	6.6180-05	-2.4160 00	5.6020 00	-23.1	0.0	
17	20	-3.6680-02	3.0160 00	4.8610-05	-2.2210 00	4.1930 00	-27.9	0.0	
18	20	-2.1860-02	3.3790-01	2.9530-05	-2.1780 00	2.3530 00	-42.8	0.0	
19	20	-6.9240-03	-3.4350 00	7.9740-06	-2.2470 00	-4.5460 00	26.3	2.1330 04	
20	20	6.8250-03	-1.0630-14	2.2260-07	-1.4150 00	1.4150 00	-45.0	-2.1290 04	
21	20	2.0570-02	0.0	0.0	0.0	0.0	0.0	0.0	
22	20	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	20	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	20	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	20	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	20	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0	2	1.2590-03	-7.7580-15	1.1730-01	1.5620 00	1.6220 00	46.1	0.0	
1	2	4.8220-05	-1.0630 00	1.1720-01	-4.5010-01	-1.2150 00	18.7	-2.8970 02	
2	2	-1.5370-03	-1.3400 00	1.5810-01	-1.2950 00	-2.0870 00	30.0	2.9610 02	
3	2	-3.5930-03	-1.6710 00	1.2460-01	-1.5980 00	-2.6060 00	39.3	0.0	
4	2	-6.2360-03	-1.5890 00	9.3870-02	-1.7450 00	-2.6850 00	32.1	0.0	
5	2	-9.4330-03	-1.1020 00	6.8590-02	-1.8760 00	-2.4820 00	36.3	0.0	
6	2	-1.3010-02	-2.1350-01	4.4960-02	-1.9920 00	-2.0800 00	43.1	0.0	
7	2	-1.6670-02	9.9740-01	2.0880-02	-2.0710 00	2.6370 00	-38.4	0.0	
8	2	-1.9990-02	2.3810 00	-4.0300-03	-2.0990 00	3.6030 00	-30.2	0.0	
9	2	-2.2480-02	3.7650 00	-2.9660-02	-2.0730 00	4.6780 00	-23.8	0.0	
10	2	-2.3660-02	4.9890 00	-5.6370-02	-1.9960 00	5.6840 00	-19.2	0.0	
11	2	-2.3120-02	5.8980 00	-8.4790-02	-1.8650 00	6.4320 00	-16.0	0.0	
12	2	-2.0520-02	6.3100 00	-1.1520-01	-1.6710 00	6.7190 00	-13.7	0.0	
13	2	-1.5740-02	6.0240 00	-1.4780-01	-1.3930 00	5.3240 00	-12.1	0.0	
14	2	-8.8660-03	4.8580 00	-1.8390-01	-8.5520-01	5.0000 00	-9.4	0.0	

15	2	-2.9850-04	2.9600 00	-2.3550-01	6.5660-02	2.9610 00	1.2	1.7930 03	
16	2	9.3060-03	-1.5720-13	-2.2330-01	3.2600-01	-4.5630-01	-54.5	-1.7930 03	
17	2	1.8920-02	0.0	0.0	0.0	0.0	0.0	0.0	
18	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
19	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
20	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
21	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
22	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0	1	9.3480-04	-3.3850-13	2.4850-01	-1.0430-01	2.8660-01	-73.0	-1.2600 03	
1	1	-2.5860-04	-6.1600-01	6.0730-01	-7.0850-01	-9.4020-01	24.6	1.2550 03	
2	1	-1.7000-03	-1.2750 00	1.3470-01	-1.2880 00	-2.0390 00	30.7	0.0	
3	1	-3.6000-03	-1.5900 00	8.0650-02	-1.4620 00	-2.4390 00	30.1	0.0	
4	1	-6.0660-03	-1.5140 00	6.2310-02	-1.6120 00	-2.5200 00	32.0	0.0	
5	1	-9.0700-03	-1.0420 00	4.8770-02	-1.7390 00	-2.3190 00	36.3	0.0	
6	1	-1.2440-02	-1.8080-01	3.3920-02	-1.8400 00	-1.9160 00	43.3	0.0	
7	1	-1.5890-02	9.9150-01	1.6610-02	-1.9040 00	2.4700 00	-37.8	0.0	
8	1	-1.8980-02	2.3310 00	-2.0070-03	-1.9240 00	3.4150 00	-29.4	0.0	
9	1	-2.1240-02	3.6720 00	-2.0770-02	-1.8990 00	4.4740 00	-22.9	0.0	
10	1	-2.2210-02	4.8560 00	-4.0030-02	-1.8270 00	5.4630 00	-18.4	0.0	
11	1	-2.1460-02	5.7290 00	-6.0910-02	-1.7070 00	5.1950 00	-15.3	0.0	
12	1	-1.8680-02	6.1140 00	-6.3260-02	-1.5330 00	5.4720 00	-13.2	0.0	
13	1	-1.3730-02	5.8030 00	-1.0080-01	-1.2890 00	5.0720 00	-11.8	0.0	
14	1	-6.7320-03	4.5250 00	-2.6230-02	-7.6730-01	4.6510 00	-9.3	9.0730 03	
15	1	1.8700-03	2.7040 00	1.9920-01	3.3810-02	2.7040 00	0.8	-9.0750 03	
16	1	1.1420-02	3.6040-15	-1.2830-01	1.2750 01	-1.2820 01	-45.1	0.0	
17	1	2.0970-02	0.0	0.0	0.0	0.0	0.0	0.0	
18	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
19	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
20	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
21	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
22	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0	0	6.1960-04	-1.0190-15	-2.4860-21	-6.0690-02	-5.0590-02	45.0	-1.7890 03	
1	0	-5.4040-04	-2.2030-01	1.6450-21	-3.6470-01	-4.9120-01	36.6	-1.7900 03	
2	0	-1.8530-03	-6.2110-01	7.1870-21	-6.3610-01	-1.0180 00	32.0	0.0	
3	0	-3.5950-03	-7.9260-01	3.7970-20	-7.3140-01	-1.2280 00	30.8	0.0	
4	0	-5.8850-03	-7.5700-01	1.3180-20	-7.9810-01	-1.2520 00	32.3	0.0	
5	0	-8.6990-03	-5.1830-01	3.2190-20	-8.5220-01	-1.1500 00	36.5	0.0	
6	0	-1.1870-02	-8.2030-02	4.1660-20	-8.9420-01	-9.3610-01	43.7	0.0	
7	0	-1.5100-02	5.1190-01	5.3510-20	-9.2050-01	1.7210 00	-37.2	0.0	
8	0	-1.7980-02	1.1910 00	6.8230-20	-9.2860-01	1.6990 00	-28.7	0.0	
9	0	-2.0030-02	1.8700 00	8.2960-20	-9.1760-01	2.2450 00	-22.2	0.0	
10	0	-2.0790-02	2.4700 00	8.5890-20	-8.8730-01	2.7560 00	-17.8	0.0	
11	0	-1.9830-02	2.9100 00	7.9590-20	-8.3680-01	3.1330 00	-15.0	0.0	
12	0	-1.6870-02	3.1000 00	6.5300-20	-7.6400-01	3.2780 00	-13.1	0.0	
13	0	-1.1760-02	2.9320 00	4.1690-20	-6.6570-01	3.0770 0			

BOX MOMENT SUMMATIONS (TOTAL ACROSS BOX)

BOX NO.	X-STA	X-DIST	TOTAL BMX
5	11	1.8690 02	4.4700 02
5	12	2.2430 02	6.1060 02
5	13	2.6170 02	6.5620 02
5	14	2.9910 02	5.9000 02
5	15	3.3650 02	4.0180 02
6	11	1.8690 02	4.6970 02
6	12	2.2430 02	6.5000 02
6	13	2.6170 02	7.0550 02
6	14	2.9910 02	6.3380 02
6	15	3.3650 02	4.3100 02

19	41	-1.4340-02
20	41	-1.0900-02
21	41	-7.6500-03
22	41	-4.8350-03
23	41	-2.5720-03
24	41	-8.6670-04
25	41	3.3860-04
10	42	8.7360-03
11	42	-2.8010-04
12	42	-8.1720-03
13	42	-1.4320-02
14	42	-1.8390-02
15	42	-2.0380-02
16	42	-2.0500-02
17	42	-1.9110-02
18	42	-1.6660-02
19	42	-1.3600-02
20	42	-1.0370-02
21	42	-7.3280-03
22	42	-4.7050-03
23	42	-2.6220-03
24	42	-1.0810-03
25	42	3.3750-05



DEFLECTIONS BETWEEN (4 , 22) AND (21 , 22)

X , Y DEFLECTION

4	22	0.0
5	22	1.2050-02
6	22	-2.9140-06
7	22	-1.4460-02
8	22	-2.9410-02
9	22	-4.3590-02
10	22	-5.6020-02
11	22	-6.6000-02
12	22	-7.2590-02
13	22	-7.4550-02
14	22	-7.1470-02
15	22	-6.3870-02
16	22	-5.3180-02
17	22	-4.0500-02
18	22	-2.6810-02
19	22	-1.3020-02
20	22	-3.0330-06
21	22	1.0530-02



DEFLECTIONS BETWEEN (0 , 2) AND (15 , 2)

X , Y DEFLECTION

0	2	1.2590-03
1	2	4.8220-05
2	2	-1.5370-03
3	2	-3.5930-03
4	2	-6.2360-03
5	2	-9.4330-03
6	2	-1.3010-02
7	2	-1.6670-02
8	2	-1.9990-02
9	2	-2.2480-02
10	2	-2.3660-02
11	2	-2.3120-02
12	2	-2.0520-02
13	2	-1.5740-02
14	2	-8.8660-03
15	2	-2.9850-04
16	2	9.3060-03



DEFLECTIONS BETWEEN (10 , 41) AND (25 , 42)

X , Y DEFLECTION

10	41	6.5600-03
11	41	-2.5480-03
12	41	-1.0370-02
13	41	-1.6360-02
14	41	-2.0260-02
15	41	-2.2040-02
16	41	-2.1930-02
17	41	-2.0310-02
18	41	-1.7630-02

CPU TIME SINCE LAST CALL = 27.739 SECONDS, TOTAL ELAPSED TIME = 211.102 SECONDS

PROGRAM SLBDG2 BRIDGE DIVISION, TEXAS DDHPT PANAK REVISION DATE 10 AUG 77
 POST TEST ANALYSIS OF BEEVILLE STRUCTURE
 14 SPAN INCRS, 44 TRANSVERSE INCREMENTS RUN AS CHECK FOR 10 AUG 77 VERSION
 PROB
 172A LOADING NO. THREE WEST EDGE

TABLE 1. CONTROL DATA

	TABLE NUMBER							
	2	3	4	5	6	7	8	9
KEEP FROM PRECEDING PROBLEM (1=YES)	1	1	1	1	0	0	0	1
NUM CARDS INPUT THIS PROBLEM	0	0	0	0	1	2	3	0
MULTIPLE LOAD OPTION	-1							
PRINT GENERATED DATA	0							
SKREW ANGLE	0.3740	02						

TABLE 2. CONSTANTS

USING DATA FROM THE PREVIOUS PROBLEM

NUMBER OF SPAN INCREMENTS IN X DIRECTION	14
NUMBER OF INCREMENTS IN Y DIRECTION	44
INCREMENT LENGTH IN X DIRECTION (ALONG SPAN CENTERLINE)	3.739D 01
INCREMENT LENGTH IN Y DIRECTION	1.216D 01
POISSONS RATIO	1.500D-01
MODULUS OF ELASTICITY	6.200D 03

TABLE 3. BOX STIFFNESS DATA

USING DATA FROM THE PREVIOUS PROBLEM									
TYPE NO.	INCR	BOX WIDTH	VOID DEP	SLAB THICK	SHEAR KEY STIFF FACTOR	LONGIT. BOX MOMENT	BOX INERTIAS	TORSIONAL	
NO.	IN WIDTH	TOP	BOT	TOP	BOT	MOMENT	TORSIONAL		
1	4	20.00	23.75	5.50	5.00	0.100D-05	3.099D 04	7.198D 04	

TABLE 4. BOX ARRANGEMENT BY TYPE NO.

USING DATA FROM THE PREVIOUS PROBLEM

1	1	1	1	1	1
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TABLE 5. LOAD PATTERNS

USING DATA FROM THE PREVIOUS PROBLEM

PAT	X AND Y DISTANCES (OF THE LOAD PATTERN)					LOAD MAGNITUDE				
LDS	X	Y	X	Y	X	Y	X	Y	X	Y
1	-170.	-39.	-170.	39.	-25.	-36.	-25.	36.	25.	-36.
6	-5.930D 00	-5.930D 00	-1.455D 01	-1.455D 01	-1.455D 01	-1.455D 01	-1.455D 01	-1.455D 01	-1.455D 01	-1.455D 01

TABLE 6. PLACEMENTS OF LOAD PATTERNS

PAT-PLMTS X AND Y DISTANCES (OF THE STRUCTURE)

1	1	117.-187.
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TABLE 7. SUMMATION OF BOX BENDING MOMENTS

BOX	SUMS	SUM AT X-STATION NEAREST THESE X-DISTANCES				
		60.	96.	132.	168.	
1	5	24.	60.	96.	132.	168.
2	5	24.	60.	96.	132.	168.

TABLE 8. PROFILE OUTPUT AREAS

DISTS FROM - THRU	DEFL	X MOM	Y MOM	PRIN MOM
X Y X Y		(1=YES)		
160. 242. 727. 242.	1	0	0	0
-38. 0. 562. 0.	1	0	0	0
-204.-242. 360.-242.	1	0	0	0

I-----TRIM TABLE 9. PRINTED OUTPUT LIMITS

Y-DIST FROM - THRU	Y	Y
	USING DATA	FROM THE PREVIOUS PROBLEM
-268.	-243.	
-12.	12.	
243.	268.	

SUMMATION OF LOADS ON STRUCTURE -7.006D 01
 SUMMATION OF LOADS OFF STRUCTURE 0.0

RESULTS FROM SLAB49 PROGRAM USING DATA GENERATED BY SLBDG2

POST TEST ANALYSIS OF BEEVILLE STRUCTURE

14 SPAN INCRS, 44 TRANSVERSE INCREMENTS RUN AS CHECK FOR 10 AUG 77 VERSION
 PROB (CONTD)
 172A LOADING NO. THREE WEST EDGE

RESULTS--USING STIFFNESS DATA FROM PREVIOUS PROBLEM 170A

SLAB X MOMENT AND X TWISTING MOMENT ACT IN THE X DIRECTION (ABOUT Y AXIS)
 Y TWISTING MOMENT = -X TWISTING MOMENT, COUNTERCLOCKWISE BETA ANGLES ARE POSITIVE FROM THE X AXIS TO THE DIRECTION OF THE LARGEST PRINCIPAL MOMENT
 SLAB MOMENTS ARE PER UNIT WIDTH

X, Y	DEFL	SLAB X MOMENT	SLAB Y MOMENT	SLAB X TWISTING MOMENT	LARGEST PRINCIPAL SLAB MOMENT	BETA X TO LARGEST MOMENT	SUPPORT REACTION
0 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
8 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9 44	0.0	0.0	0.0	0.0	0.0	0.0	0.0
10 44	2.508D-03	0.0	0.0	0.0	0.0	0.0	0.0
11 44	6.611D-04	6.188D-01	4.563D-17	1.567D 00	1.907D 00	39.4	-2.189D 03
12 44	-7.579D-04	8.504D-01	-1.901D-17	-4.357D-02	3.527D-01	-2.9	2.189D 03
13 44	-1.588D-03	7.598D-01	-5.709D-17	-6.702D-02	7.657D-01	-5.0	0.0
14 44	-1.893D-03	5.955D-01	-7.174D-17	-9.557D-02	5.104D-01	-8.9	0.0
15 44	-1.786D-03	3.961D-01	-6.794D-17	-1.107D-01	4.249D-01	-14.6	0.0
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7 22 -1.1000-02 6.5680 00 -8.8370-01 2.4130 00 7.2820 00 16.5 0.0
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1	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
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5	21	1.0830-02	-5.6260-11	-9.7080-01	1.6110 01	-1.6600 01	-45.9	-1.6680 04	
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7	21	-1.4890-02	7.1050 00	-5.0060-01	1.5900 00	7.4250 00	11.3	0.0	
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15	21	-1.8440-02	-4.7030-02	3.8970-02	-4.6520 00	-4.6560 03	44.7	0.0	
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17	21	-8.9860-03	-1.7400 00	1.0390-01	-3.6650 00	-4.5970 00	37.9	0.0	
18	21	-5.0720-03	-1.9750 00	2.2120-01	-3.1710 00	-4.2330 00	35.4	0.0	
19	21	-1.8670-03	-1.8500 00	1.2230 00	-2.3630 00	-3.1320 00	28.5	2.8760 03	
20	21	6.2210-04	6.9360-12	2.7660 00	-1.9180 00	3.7480 00	-62.9	-2.8990 03	
21	21	2.9710-03	0.0	0.0	0.0	0.0	0.0	0.0	
22	21	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
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4	20	2.2070-02	0.0	0.0	0.0	0.0	0.0	0.0	
5	20	7.3910-03	-6.0730-14	-3.0660-06	4.6760 00	-4.6760 00	-45.0	-2.2770 04	
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8	20	-2.7700-02	8.5780 00	-1.8850-05	5.6130-01	8.6150 00	3.7	0.0	
9	20	-3.3590-02	8.7930 00	-2.0130-05	-1.2380 00	8.9640 00	-7.9	0.0	
10	20	-3.6450-02	8.1420 00	-1.9000-05	-2.9660 00	9.1070 00	-18.0	0.0	
11	20	-3.6480-02	6.7200 00	-1.6090-05	-4.3540 00	8.8600 00	-25.2	0.0	
12	20	-3.4190-02	4.9230 00	-1.2160-05	-5.2450 00	9.2550 00	-32.4	0.0	
13	20	-3.0200-02	3.0510 00	-7.7890-06	-5.6470 00	7.3740 00	-37.4	0.0	
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15	20	-1.9650-02	-1.8060-01	5.0790-07	-5.2620 00	-5.3530 00	44.5	0.0	
16	20	-1.4220-02	-1.2990 00	3.6870-06	-4.6480 00	-5.3430 00	41.0	0.0	
17	20	-9.2310-03	-1.9740 00	5.7110-06	-3.8800 00	-4.9900 00	37.9	0.0	
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19	20	-1.3690-03	-2.4450 00	3.5240-06	-2.2460 00	-3.7800 00	30.7	4.2160 03	
20	20	1.3450-03	-1.0500-14	2.5750-06	-9.9040-01	9.9040-01	-45.0	-4.1940 03	
21	20	4.0580-03	0.0	0.0	0.0	0.0	0.0	0.0	
22	20	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	20	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
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2	2	-1.8730-02	-2.1960 00	6.3880-01	-5.4010 00	-6.3620 00	37.6	3.6100 03	
3	2	-3.8850-02	8.4870-01	4.9910-01	-6.8010 00	7.4780 00	-44.3	0.0	
4	2	-5.8710-02	5.2310 00	3.7590-01	-7.0980 00	1.0310 01	-35.6	0.0	
5	2	-7.6770-02	9.0820 00	2.9070-01	-7.0630 00	1.3010 01	-29.1	0.0	
6	2	-9.1700-02	1.1810 01	2.3140-01	-7.1100 00	1.5190 01	-25.4	0.0	
7	2	-1.0260-01	1.5150 01	1.9430-01	-7.3160 00	1.8130 01	-22.2	0.0	
8	2	-1.0820-01	1.9200 01	1.6930-01	-7.2010 00	2.1620 01	-18.6	0.0	
9	2	-1.0720-01	2.0830 01	1.6590-01	-6.6260 00	2.2770 01	-16.3	0.0	
10	2	-9.8950-02	1.8940 01	1.8700-01	-5.8550 00	2.0620 01	-16.0	0.0	
11	2	-8.4180-02	1.2850 01	2.4480-01	-5.4920 00	1.4910 01	-20.5	0.0	
12	2	-6.4980-02	7.1940 00	3.2990-01	-5.5350 00	1.0270 01	-29.1	0.0	
13	2	-4.3310-02	1.5450 00	4.7170-01	-5.2200 00	5.2560 00	-42.1	0.0	
14	2	-2.1120-02	-4.5990 00	6.8730-01	-3.2210 00	-5.1220 00	25.3	0.0	

15	2	-5.6350-04	-7.2370 00	6.5730-01	-1.6470 00	-7.5670 00	11.3	3.3850 03	
16	2	1.7450-02	-2.6870-13	6.1920-01	-9.3750-01	1.2970 00	-54.1	-3.3640 03	
17	2	3.5440-02	0.0	0.0	0.0	0.0	0.0	0.0	
18	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
19	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
20	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
21	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
22	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0	1	1.3990-02	-5.0780-12	9.1630-01	-3.9440-01	1.0630 00	-69.6	-1.8860 04	
1	1	-3.8820-03	-2.1530 00	2.2960 00	-2.8140 00	3.5580 00	-64.2	1.8840 04	
2	1	-2.2630-02	-1.9080 00	5.6900-01	-5.2690 00	-5.0820 00	38.4	0.0	
3	1	-4.2090-02	9.9680-01	3.0980-01	-6.0440 00	6.7070 00	-43.4	0.0	
4	1	-6.1210-02	5.1580 00	1.7860-01	-6.4520 00	9.5840 00	-34.5	0.0	
5	1	-7.8510-02	8.8810 00	1.1690-01	-6.5990 00	1.2420 01	-28.2	0.0	
6	1	-9.2680-02	1.1640 01	1.3070-01	-6.7620 00	1.4770 01	-24.8	0.0	
7	1	-1.0270-01	1.4900 01	1.0840-01	-7.0040 00	1.7690 01	-21.7	0.0	
8	1	-1.0750-01	1.8680 01	5.6200-03	-6.9990 00	2.1010 01	-18.4	0.0	
9	1	-1.0570-01	2.0200 01	-3.9440-02	-6.6670 00	2.2200 01	-16.7	0.0	
10	1	-9.6790-02	1.8330 01	-3.5050-02	-6.2180 00	2.0240 01	-17.1	0.0	
11	1	-8.1360-02	1.2640 01	1.1760-01	-6.0830 00	1.5100 01	-22.1	0.0	
12	1	-6.1470-02	7.0890 00	1.6930-01	-6.2970 00	1.0810 01	-30.6	0.0	
13	1	-3.9080-02	1.4000 00	3.1630-01	-6.3950 00	7.2770 00	-42.6	0.0	
14	1	-1.6210-02	-5.8020 00	1.8100 00	-4.1100 00	-7.5970 00	23.6	2.1850 04	
15	1	4.5100-03	-6.6010 00	4.1190 00	-9.5340-01	-5.6850 00	5.0	-2.1880 04	
16	1	2.2690-02	1.7080-14	4.2780-01	2.5100 01	2.5320 01	45.2	0.0	
17	1	4.0850-02	0.0	0.0	0.0	0.0	0.0	0.0	
18	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
19	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
20	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
21	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
22	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
23	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
24	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
25	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
26	1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
0	0	9.4810-03	-1.2540-14	-3.9180-20	-2.2960-01	-2.2960-01	45.0	-2.7380 04	
1	0	-8.2670-03	-7.0720-01	5.6050-20	-1.4150 00	-1.8120 00	38.0	2.7380 04	
2	0	-2.6500-02	-8.3130-01	1.1210-19	-2.5220 00	-2.9720 00	40.3	0.0	
3	0	-4.5320-02	5.8290-01	2.8350-19	-2.9230 00	3.2290 00	-42.2	0.0	
4	0	-6.3730-02	2.6620 00	5.2050-19	-3.1240 00	4.7270 00	-33.5	0.0	
5	0	-8.0290-02	4.5470 00	4.0800-19	-3.2190 00	5.2150 00	-27.4	0.0	
6	0	-9.3710-02	5.9790 00	5.1800-19	-3.3080 00	7.4480 00	-23.9	0.0	
7	0	-1.0300-01	7.6400 00	3.4980-19	-3.4130 00	8.9430 00	-20.9	0.0	
8	0	-1.0700-01	9.5170 00	4.4220-19	-3.4310 00	1.0520 01	-17.9	0.0	
9	0	-1.0440-01	1.0270 01	3.3290-19	-3.3370 00	1.1260 01	-16.5	0.0	
10	0	-9.4730-02	5.3100 00	3.7990-19	-3.2050 00	1.0310 01	-17.3	0.0	
11	0	-7.8600-02	6.4780 00	4.4530-19	-3.1840 00	7.7810 00	-22.3	0.0	
12	0	-5.7990-02	3.6500 00	4.1400-19	-3.2910 00	5.5890 00	-30.5	0.0	
13	0	-3.4850-02	7.2820-01	3.2670-19	-3.4280 00	3.8110 00			

BOX MOMENT SUMMATIONS (TOTAL ACROSS BOX)

BOX NO.	X-STA	X-DIST	TOTAL BMX
1	7	3.739D 01	7.292D 02
1	8	7.478D 01	9.268D 02
1	9	1.122D 02	1.006D 03
1	10	1.496D 02	9.165D 02
1	10	1.496D 02	0.0
2	7	3.739D 01	6.992D 02
2	8	7.478D 01	8.925D 02
2	9	1.122D 02	9.710D 02
2	10	1.496D 02	8.864D 02
2	10	1.496D 02	0.0

RESULTS FROM SLAB49 PROGRAM USING DATA GENERATED BY SLBDG2

POST TEST ANALYSIS OF BEEVILLE STRUCTURE

14 SPAN INCRS, 44 TRANSVERSE INCREMENTS RUN AS CHECK FOR 10 AUG 77 VERSION

PROB (CONTD)

172A LOADING NO. THREE WEST EDGE

PROFILE OUTPUT AREAS

X MOMENTS ACT IN X DIRECTION (ABOUT Y AXIS)

THE PLOTTED RESULTS INDICATE THE RELATIVE VALUE EACH HAS WITHIN THAT LIST

DEFLECTIONS BETWEEN (10 , 41) AND (25 , 42)

X , Y DEFLECTION

10	41	1.393D-03
11	41	-5.414D-04
12	41	-1.922D-03
13	41	-2.684D-03
14	41	-2.900D-03
15	41	-2.693D-03
16	41	-2.211D-03
17	41	-1.595D-03
18	41	-9.673D-04

19	41	-4.183D-04
20	41	-5.458D-06
21	41	2.465D-04
22	41	3.409D-04
23	41	3.003D-04
24	41	1.564D-04
25	41	-6.038D-05
10	42	1.835D-03
11	42	-5.866D-05
12	42	-1.464D-03
13	42	-2.269D-03
14	42	-2.530D-03
15	42	-2.371D-03
16	42	-1.932D-03
17	42	-1.357D-03
18	42	-7.638D-04
19	42	-2.452D-04
20	42	1.415D-04
21	42	3.707D-04
22	42	4.446D-04
23	42	3.841D-04
24	42	2.210D-04
25	42	-7.207D-06

DEFLECTIONS BETWEEN (4 , 22) AND (21 , 22)

X , Y DEFLECTION

4	22	0.0
5	22	1.423D-02
6	22	3.268D-06
7	22	-1.100D-02
8	22	-1.968D-02
9	22	-2.569D-02
10	22	-2.899D-02
11	22	-2.977D-02
12	22	-2.844D-02
13	22	-2.554D-02
14	22	-2.163D-02
15	22	-1.723D-02
16	22	-1.282D-02
17	22	-8.728D-03
18	22	-5.197D-03
19	22	-2.311D-03
20	22	-1.629D-06
21	22	1.367D-03

DEFLECTIONS BETWEEN (0 , 2) AND (16 , 2)

X , Y DEFLECTION

0	2	1.854D-02
1	2	5.965D-04
2	2	-1.873D-02
3	2	-3.885D-02
4	2	-5.871D-02
5	2	-7.677D-02
6	2	-9.170D-02
7	2	-1.026D-01
8	2	-1.082D-01
9	2	-1.072D-01
10	2	-9.895D-02
11	2	-8.418D-02
12	2	-6.498D-02
13	2	-4.331D-02
14	2	-2.112D-02
15	2	-5.635D-04
16	2	1.745D-02

CPU TIME SINCE LAST CALL = 27.513 SECONDS, TOTAL ELAPSED TIME = 238.614 SECONDS