

A DISCRETE-ELEMENT METHOD OF ANALYSIS FOR ORTHOGONAL SLAB
AND GRID BRIDGE FLOOR SYSTEMS

by

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Development of Methods for Computer Simulation
of Beam-Columns and Grid-Beam and Slab Systems

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PREFACE

This report describes a numerical method for the analysis of orthogonal plates with grid-beams. The method is particularly suited for application to highway bridge structures composed of slabs with supporting beam and diaphragm systems. A typical Texas Highway Department bridge is analyzed for various load and stiffness configurations and presented as a series of examples.

The computer program described and included in this report culminates one phase of the effort expended over several years of this research project. The program is compatible with most computers.

This work was supported by the Texas Highway Department in cooperation with the U. S. Department of Transportation Federal Highway Administration, under Research Project 3-5-63-56.

The continued assistance and advice of the project contact representatives and others of the Bridge Division of the Texas Highway Department is deeply appreciated.

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LIST OF REPORTS

Report No. 56-1, "A Finite-Element Method of Solution for Linearly Elastic Beam-Columns" by Hudson Matlock and T. Allan Haliburton, presents a solution for beam-columns that is a basic tool in subsequent reports. September 1966.

Report No. 56-2, "A Computer Program to Analyze Bending of Bent Caps" by Hudson Matlock and Wayne B. Ingram, describes the application of the beam-column solution to the particular problem of bridge bent caps. October 1966.

Report No. 56-3, "A Finite-Element Method of Solution for Structural Frames" by Hudson Matlock and Berry Ray Grubbs, describes a solution for frames with no sway. May 1967.

Report No. 56-4, "A Computer Program to Analyze Beam-Columns under Movable Loads" by Hudson Matlcock and Thomas P. Taylor, describes the application of the beam-column solution to problems with any configuration of movable non-dynamic loads. June 1968.

Report No. 56-5, "A Finite-Element Method for Bending Analysis of Layered Structural Systems" by Wayne B. Ingram and Hudson Matlock, describes an alternating-direction iteration method for solving two-dimensional systems of layered grids-over-beams and plates-over-beams. June 1967.

Report No. 56-6, "Discontinuous Orthotropic Plates and Pavement Slabs" by W. Ronald Hudson and Hudson Matlock, describes an alternating-direction iteration method for solving complex two-dimensional plate and slab problems with emphasis on pavement slabs. May 1966.

Report No. 56-7, "A Finite-Element Analysis of Structural Frames" by T. Allan Haliburton and Hudson Matlock, describes a method of analysis for rectangular plane frames with three degrees of freedom at each joint. July 1967.

Report No. 56-8, "A Finite-Element Method for Transverse Vibrations of Beams and Plates" by Harold Salani and Hudson Matlock, describes an implicit procedure for determining the transient and steady-state vibrations of beams and plates, including pavement slabs. June 1968.

Report No. 56-9, "A Direct Computer Solution for Plates and Pavement Slabs" by C. Fred Stelzer, Jr., and W. Ronald Hudson, describes a direct method for solving complex two-dimensional plate and slab problems. October 1967.

Report No. 56-10, "A Finite-Element Method of Analysis for Composite Beams" by Thomas P. Taylor and Hudson Matlock, describes a method of analysis for composite beams with any degree of horizontal shear interaction. January 1968.

Report No. 56-11, "A Discrete-Element Solution of Plates and Pavement Slabs Using a Variable-Increment-Length Model" by Charles M. Pearre, III, and W. Ronald Hudson, presents a method for solving freely discontinuous plates and pavement slabs subjected to a variety of loads. April 1969.

Report No. 56-12, "A Discrete-Element Method of Analysis for Combined Bending and Shear Deformations of a Beam" by David F. Tankersley and William P. Dawkins, presents a method of analysis for the combined effects of bending and shear deformations. December 1969.

Report No. 56-13, "A Discrete-Element Method of Multiple-Loading Analysis for Two-Way Bridge Floor Slabs" by John J. Panak and Hudson Matlock, includes a procedure for analysis of two-way bridge floor slabs continuous over many supports. January 1970.

Report No. 56-14, "A Direct Computer Solution for Plane Frames" by William P. Dawkins and John R. Ruser, Jr., presents a direct method of solution for the computer analysis of plane frame structures. May 1969.

Report No. 56-15, "Experimental Verification of Discrete-Element Solutions for Plates and Slabs" by Sohan L. Agarwal and W. Ronald Hudson, presents a comparison of discrete-element solutions with small-dimension test results for plates and slabs, including some cyclic data. April 1970.

Report No. 56-16, "Experimental Evaluation of Subgrade Modulus and Its Application in Model Slab Studies" by Qaiser S. Siddiqi and W. Ronald Hudson, describes a series of laboratory experiments to evaluate layered foundation coefficients of subgrade reaction for use in the discrete-element method. January 1970.

Report No. 56-17, "Dynamic Analysis of Discrete-Element Plates on Nonlinear Foundations" by Allen E. Kelly and Hudson Matlock, presents a numerical method for the dynamic analysis of plates on nonlinear foundations. July 1970.

Report No. 56-18, "A Discrete-Element Analysis for Anisotropic Skew Plates and Grids" by Mahendrakumar R. Vora and Hudson Matlock, describes a tridirectional model and a computer program for the analysis of anisotropic skew plates or slabs with grid-beams. August 1970.

Report No. 56-19, "An Algebraic Equation Solution Process Formulated in Anticipation of Banded Linear Equations" by Frank L. Endres and Hudson Matlock, describes a system of equation-solving routines that may be applied to a wide variety of problems by using them within appropriate programs. January 1971.

Report No. 56-20, "Finite-Element Method of Analysis for Plane Curved Girders" by William P. Dawkins, presents a method of analysis that may be applied to plane-curved highway bridge girders and other structural members composed of straight and curved sections. June 1971.

Report No. 56-21, "Linearly Elastic Analysis of Plane Frames Subjected to Complex Loading Conditions" by Clifford O. Hays and Hudson Matlock, presents a design-oriented computer solution for plane frames structures and trusses that can analyze with a large number of loading conditions. June 1971.

Report No. 56-22, "Analysis of Bending Stiffness Variation at Cracks in Continuous Pavements," by Adnan Abou-Ayyash and W. Ronald Hudson, describes an evaluation of the effect of transverse cracks on the longitudinal bending rigidity of continuously reinforced concrete pavements. April 1972.

Report No. 56-23, "A Nonlinear Analysis of Statically Loaded Plane Frames Using a Discrete Element Model" by Clifford O. Hays and Hudson Matlock, describes a method of analysis which considers support, material, and geometric nonlinearities for plane frames subjected to complex loads and restraints. May 1972.

Report No. 56-24, "A Discrete-Element Method for Transverse Vibrations of Beam-Columns Resting on Linearly Elastic or Inelastic Supports" by Jack Hsiao-Chieh Chan and Hudson Matlock, presents a new approach to predict the hysteretic behavior of inelastic supports in dynamic problems. June 1972.

(P) indicates Preliminary Report.

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ABSTRACT

A method is presented for the discrete-element analysis of isotropic or orthotropic slabs and plates on elastic supports. An integral grid-beam system can also be included which makes it especially useful for highway bridge structures. Other structures such as highway and airfield pavements on elastic foundations, flat or haunched building slabs, two-way mat foundations, stiffened plates, or any grid-type subassemblage of more complex structures can also be investigated.

The developed computer program allows for the free variation of stiffnesses, supports, and loads. In-plane axial thrusts, if their distribution is known, are included and coupled with the stiffnesses of the structure. Inputs to the program may be retained and used for a series of problems for the convenience of the user. Several plotted output options are available including a three-dimensional display of the exaggerated deflected shape of the structure.

Research investigators in other areas have applied preliminary versions of the program to a variety of structures including experimental comparisons and it has been shown to yield satisfactory results.

Four example problems are presented of a typical highway bridge presently in use by the Texas Highway Department. The problems include and discuss most of the brief input calculations and assumptions necessary.

KEY WORDS: bridge decks, orthotropic plates, discrete-element analysis, grid systems, computers.

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SUMMARY

This study presents a method for the analysis of highway bridge structures which can be used to supplement and extend existing methods of analysis for the highway bridge investigator. Discrete-element modeling techniques are used to represent the actual structure. The associated computer program with formulations based on the model is used to solve the system of resulting equations by high-speed digital computer. The method can also be directly used for highway pavement analysis in which the pavement is represented as a discretized model on elastic supports representing the subgrade.

Input to the program requires simple engineering judgment in providing appropriate values to represent the stiffnesses, loads, and restraints of the actual structure. Output is arranged in tabular and graphical form to allow ease of interpretation by the engineer-user. Three-dimensional plotted displays of the deformed structure can also be obtained.

This report includes complete documentation for the computer program including

- (1) summarized derivation of equations based on the discrete-element model,
- (2) computer program listings with brief flow charts, and
- (3) detailed program input instructions.

The engineer-user, if he so desires, can apply the program with little study by using the simplified guide for data input and associating his particular structure with the included example problems. The example is a standard Texas Highway Department bridge solved for dead load (non-composite) and three variations of live load (composite).

The computer program allows any number of problems to be run at the same time and utilizes the most recent solution and multiple-loading techniques. Preliminary versions of the program have been used in Research Project 3-5-68-115, "Experimental Verification of Computer Simulation Methods for Slab and Girder Bridge Systems," and the method was shown to give extremely good correlations with experimental data for the various structures considered.

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IMPLEMENTATION STATEMENT

This program can be immediately applied by highway bridge and pavement designers to their analysis problems which are now difficult or impossible by conventional methods. It is particularly suited for use on bridge structures since an interconnected grid-beam network is included which can represent beams and diaphragms. Composite action of the beam and slab can be considered by appropriate equivalent beam representations. Extensive experimental comparisons by Research Project 3-5-68-115, "Experimental Verification of Computer Simulation Methods for Slab and Girder Bridge Systems," with versions of this program have been shown to give surprisingly good results for a wide range of bridge types. That project has also shown that even nonlinear or post-cracking behavior under overloads may be predicted by the program by using appropriate elastic estimates of the nonlinear stiffnesses of the structure.

No additional program development is needed to allow immediate use of the program by the sponsors. User-oriented versions of the program could be made which would make application easier for specific classes of problems. These could be data-generation routines for constant thickness slab structures, or automatic computation of effective bending stiffnesses for beam and slab structures.

The availability of this program will make feasible the study of various design options and their overall effect on highway bridge structural capabilities. Among these parameter studies could be the effect of beam spacing as it relates to span-width aspect ratios to better define the present live load distribution coefficients. Diaphragm spacing and positioning could be also studied to aid in defining their effectiveness.

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NOMENCLATURE

<u>Symbol</u>	<u>Typical Units</u>	<u>Definition</u>
a	-	Term in stiffness matrix
[a]	-	Submatrix of stiffness matrix
{A}	-	Recursion coefficient vector
[b]	-	Submatrix of stiffness matrix
b ¹ , b ² , b ³	-	Terms in stiffness matrix
[c]	-	Submatrix of stiffness matrix
c ¹ , c ² , c ³ , c ⁴ , c ⁵	-	Terms in stiffness matrix
[C]	-	Recursion coefficient matrix
c ^t	lb-in ² /in/rad	Plate twisting stiffness
[d]	-	Submatrix of stiffness matrix
d ¹ , d ² , d ³	-	Terms in stiffness matrix
[D]	-	Recursion coefficient multiplier matrix
D ^{cr}	lb-in ² /in	Smaller of D ^x or D ^y at a joint
D ^x , D ^y	-in ² /in	Plate bending stiffnesses
e		Term in stiffness matrix
[e]	-	Submatrix of stiffness matrix
E	lb/in ²	Modulus of elasticity
[E]	-	Recursion coefficient multiplier matrix
f	lb	Load term

<u>Symbol</u>	<u>Typical Units</u>	<u>Definition</u>
$\{f\}$	-	Load vector
f^s	lb/in ²	Stress in slab
F^x, F^y	lb-in ²	Grid-beam bending stiffnesses
h_x, h_y	-	Discrete-element widths
i	-	Numbering associated with x-direction
j	-	Numbering associated with y-direction
k	lb/in	Subgrade modulus
M^s	in-lb	Slab bending moment
M^x, M^y	in-lb/in	Bending moments in plate
\bar{M}^x, \bar{M}^y	in-lb	Bending moments in grid-beams
P^x, P^y	lb	Axial thrusts in plate
\bar{P}^x, \bar{P}^y	lb	Axial thrusts in grid-beams
Q	lb	Applied load
Q^{Rx}, Q^{Ry}	lb	Joint forces from rotational restraints
Q^{Tx}, Q^{Ty}	lb	Joint forces from applied moments
R^x, R^y	lb-in/rad	Rotational restraints
S	lb/in	Support spring
t	in.	Plate or slab thickness
T^x, T^y	lb-in	Applied moments
V^t	lb	Joint force due to twisting
V^x, V^y	lb	Shear forces in plate
\bar{V}^x, \bar{V}^y	lb	Shear forces in grid-beams

<u>Symbol</u>	<u>Typical Units</u>	<u>Definition</u>
w	in.	Deflection
{w}	-	Deflection vector
x	-	Coordinate in short direction
y	-	Coordinate in long direction
y ^t	in.	Distance from n.a. to top of slab
α	rad	Unit angular mesh rotation
θ^x, θ^y	rad	Mesh angles of rotation
θ^{Jx}, θ^{Jy}	rad	Joint angles of rotation
v	-	Poisson's ratio
ϕ	rad	Any curvature

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CHAPTER 1. INTRODUCTION

This report presents a method for the analysis of isotropic or orthotropic plates on elastic supports which has the added capability of also considering an integral or separate grid system. The major application and emphasis is for use on highway bridge structures, but the method can be used for any system which can be represented as a grid or plate.

Problem Definition

Analysis of highway bridge structures has become a much more complex problem in recent years due to the increasing availability of better materials, more complex structural configurations, and the need for greater economy. Presently, most highway bridge structures are one of three distinct types: (1) concrete slab spans, which can be of variable thickness, be post-tensioned, and have various added sections that participate structurally, such as sidewalks and parapet railings; (2) slab and girder arrangements, which consist of steel girders or pre-tensioned concrete girders, with both usually including cross bracing in the form of diaphragms; or (3) a type that might be considered a special case of the second, with the primary difference the use of two-way reinforced decks over widely spaced girders and floor beams. All of these structures have one characteristic in common, the stiffening effect of the basic structure from outstanding members. These outstanding members are the girders on slab and girder-type structures or sidewalks and parapets on slab-type structures. The stiffening contributes significantly to the over-all structural action and must be included for realistic analyses. Thus, a method of analysis which can effectively solve these general structure types would apply to the majority of highway bridge structures and therefore be most useful to the bridge design engineer.

The computer program which is described here is the culmination of several years of development by this research project. It provides an excellent general solution for all of the above structure types, but is not limited to highway bridge structures. Other structures such as highway or airfield pavements on elastic foundations, flat or haunched building slabs, one-way or two-way mat foundations, stiffened steel or aluminum plates, and

any plate or grid-type substructure of more complex configurations can also be investigated.

The Analysis Procedure

The basic solution procedure for line members was originally presented in this project for several beam-column applications (Refs 8, 10, 11, and 16). The procedure was extended in various forms and applied to a variety of problems including static, dynamic, and nonlinear solutions of beams and plates (Refs 4, 6, 7, 12, 13, 14, and 15). All of these developments laid the groundwork for the discrete-element solution procedure which is described in Chapter 2.

The mathematical analysis process of the solution has recently been extended to consider all general banded equation systems and at the same time was streamlined for efficient computer operations (Ref 3). The addition of beam-type stiffness members to the basic slab or plate solution was originally presented in this project in 1967 (Ref 6). A beginning direct solution formulation was outlined in 1968 and incorporated in 1969 in a preliminary version of the computer program described in Chapter 3. During the same time, a parallel procedure for skewed, anisotropic plate structures was developed (Ref 18) which also includes the capability of added beam-stiffnesses.

Application to Bridge Structures

Chapter 4 presents example problems involving a typical highway bridge structure to demonstrate use of the computer program. The problems include and discuss most of the basic calculations and assumptions which are made by the engineer in applying the analysis procedure. It is felt that this use of an actual structure expedites effective implementation of the program. The structure is of a type now being used by the Texas Highway Department.

Program Features

This program is the only documented two-dimensional slab or plate program developed on this project, with the exception of the skewed anisotropic program (Ref 18), which stores the image of the read-in data cards for successive use throughout a series of problems, by means of keep options. This

data holding feature was first shown to be extremely useful when it was applied to one-dimensional problems in the moving-load beam-column program (Ref 11). The data card images are also proving to be very helpful in making this program useful for special applications in which a user-oriented data-generation routine developed for specific problem types creates card images which are accepted and used by this program to obtain a solution.

Another very useful feature of this program is the ability to selectively profile plot the output values in a variety of ways. In addition, a pseudo three-dimensional display of the deflected shape can also be obtained.

Program Documentation

The computer program is described in Chapter 3 and is documented in Appendix 1. As has been the practice in previous research reports in this project, program flow charts and extensive comment cards in the program listing are included to aid in future developments and modifications by other programmers and researchers.

The program is compatible with IBM 360, UNIVAC 1108, and CDC 6600 computer systems.

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CHAPTER 2. THE DISCRETE-ELEMENT MODEL

The Computer Model

The basic computer model for orthotropic plates, which is used to model slabs and bridge decks, is essentially the same as that used in previous developments on this project (Refs 4, 6, 7, 12, 13, 14, and 15). The slab torsional stiffness is represented as a twisting element connected directly to the joints instead of a pair of torsion rods which frame into the bars. The twist reaction forces which result at the joints are the same as in the previous developments, but the model presented in Fig 1 is felt to be simpler to visualize.

Figure 2 presents the discrete-element model of the grid system which acts in conjunction with the plate element shown in Fig 1. Both models are connected at the joints by what may be thought of as ball and socket connections which ensure that the deflection will be the same at the common joint locations.

Figure 3 shows a schematic of the deformed slab model and indicates the general numbering system used.

Equations of Equilibrium

Figures 4 and 5 depict the free-body of a typical joint i, j with all appropriate forces and reactions shown. Forces with a line over the letter (such as $\bar{M}_{i, j}^y$) are related to the grid system. Summation of vertical forces at joint i, j gives

$$\begin{aligned} Q_{i,j} - v_{i,j}^t + v_{i,j+1}^t + v_{i+1,j}^t - v_{i+1,j+1}^t \\ + v_{i,j}^x - v_{i+1,j}^x + v_{i,j}^y - v_{i,j+1}^y \\ + \bar{v}_{i,j}^x - \bar{v}_{i+1,j}^x + \bar{v}_{i,j}^y - \bar{v}_{i,j+1}^y \\ + Q_{i,j}^{Rx} + Q_{i,j}^{Ry} + Q_{i,j}^{Tx} + Q_{i,j}^{Ty} - s_{i,j} w_{i,j} = 0 \end{aligned} \quad (2.1)$$

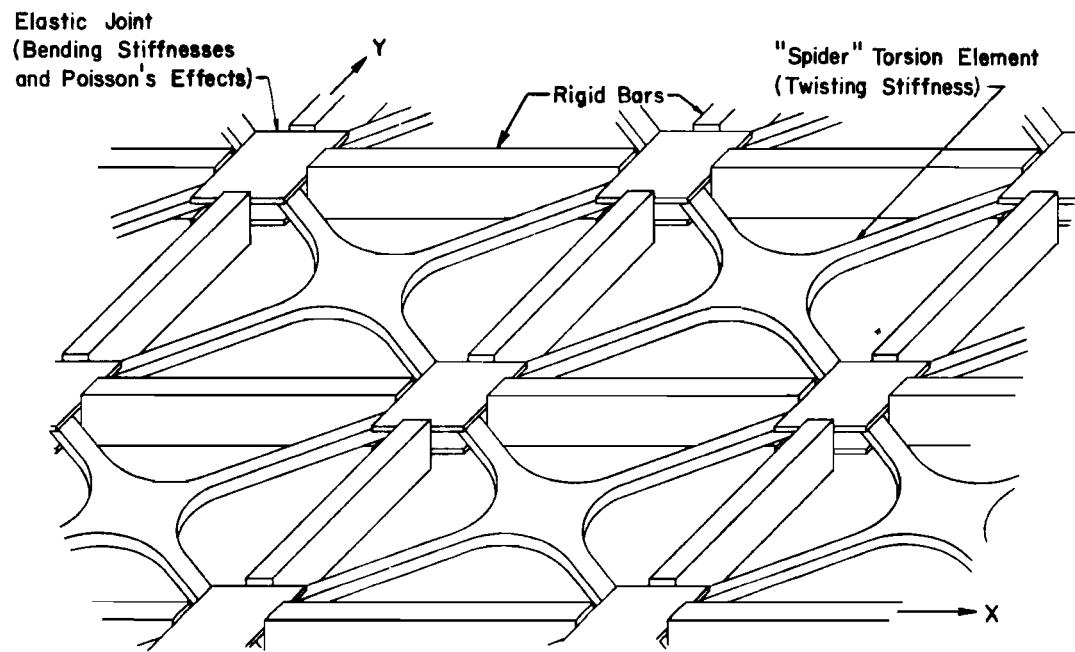


Fig 1. Discrete-element model of slab or plate.

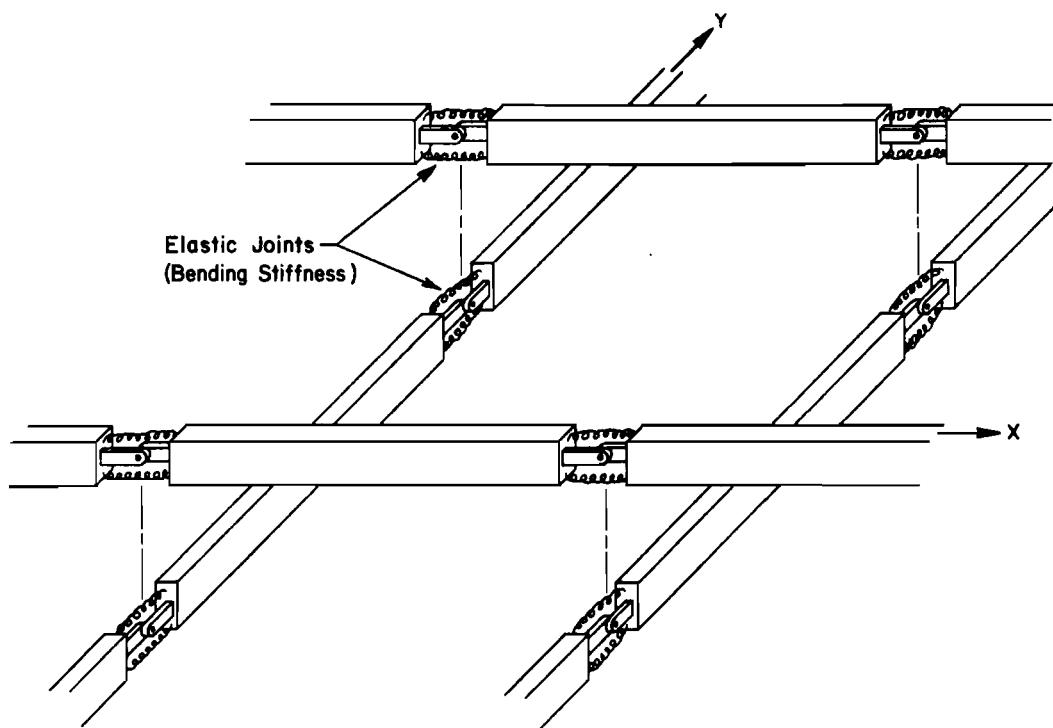


Fig 2. Discrete-element model of grid system.

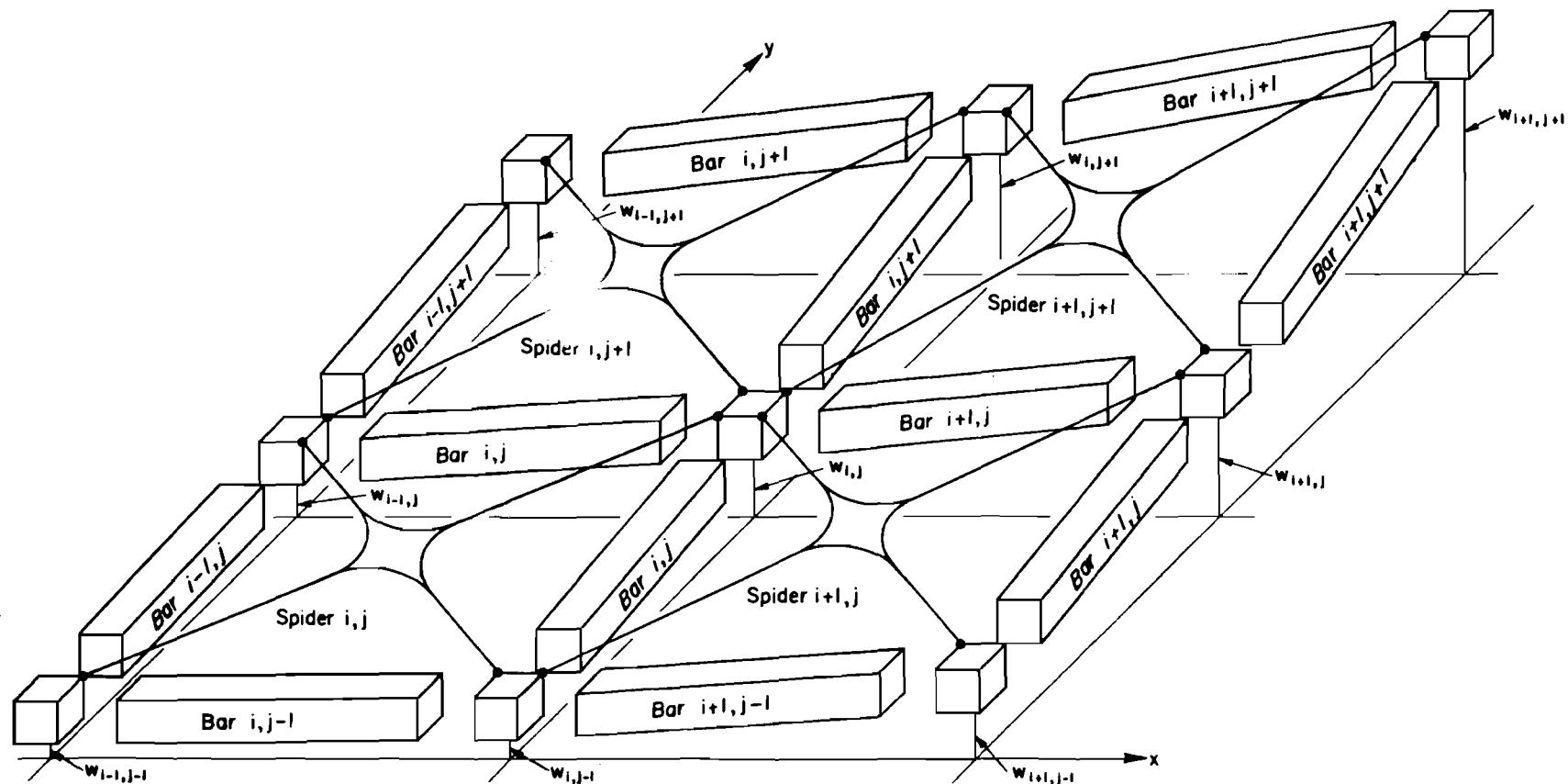


Fig 3. The deformed slab model showing general numbering system.

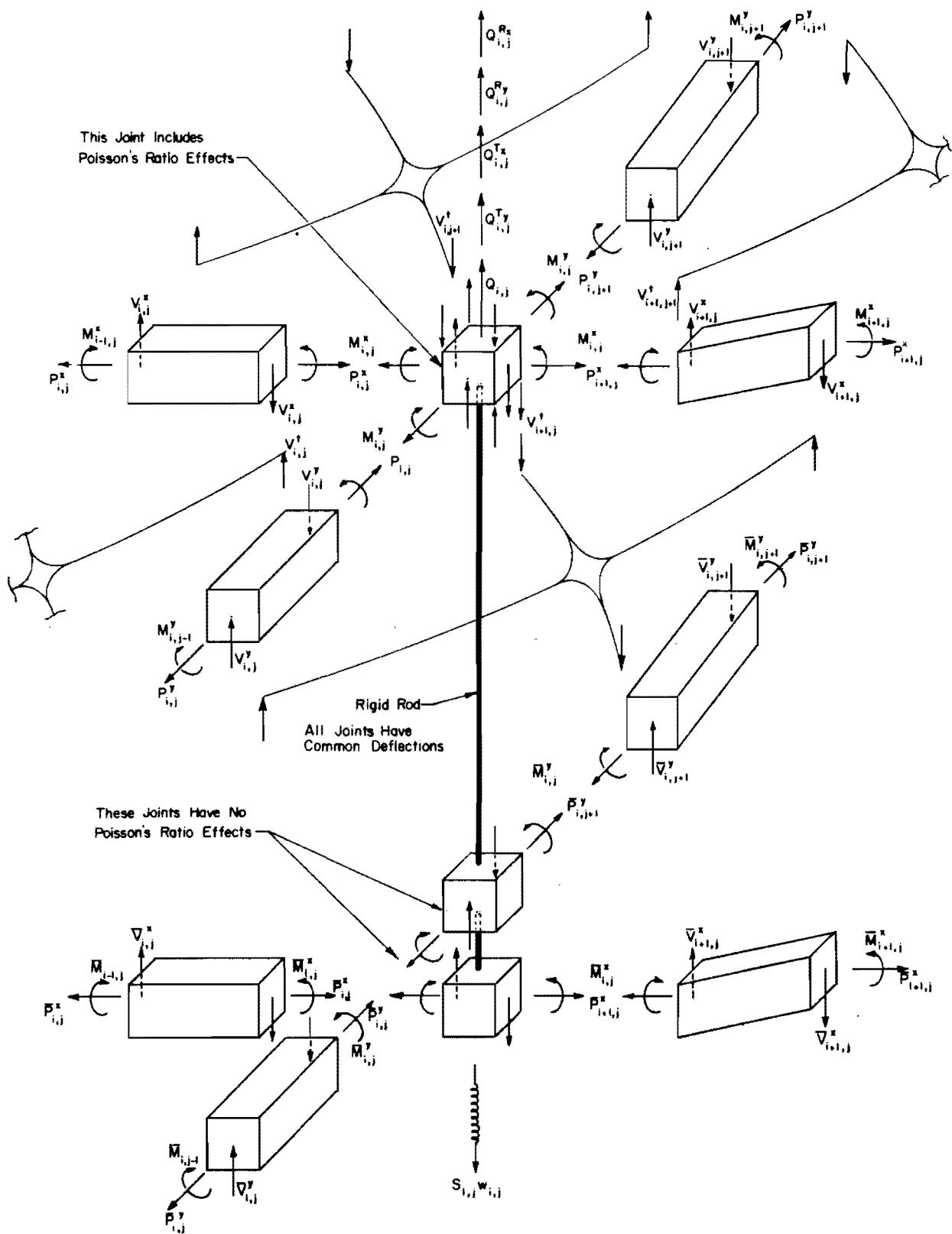


Fig 4. Free-body diagram.

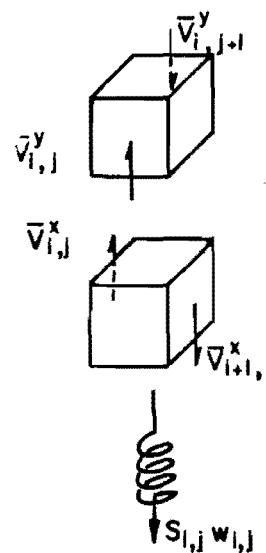
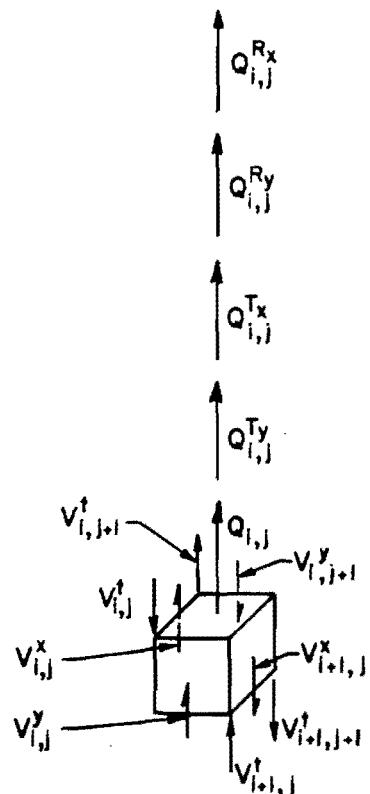


Fig 5. Free body of joint i,j .

The V^t forces are those transferred to joint i,j through the adjacent torsion elements or "spiders" as they are hereafter referred to. A typical spider is shown in Fig 6 for mesh $i+1, j+1$ which is located in the area in the increasing x- and y-directions from joint i,j . The spider shown in Fig 7 is deformed in a positive sense; that is, the angle $\theta_{i+1,j+1}^x$ is greater than angle $\theta_{i+1,j}^x$, and angle $\theta_{i+1,j+1}^y$ is greater than angle $\theta_{i,j+1}^y$. The unit angle change $\alpha_{i,j}$ for spider i,j is then seen to be equal to

$$\alpha_{i,j} = \frac{1}{h_y} (\theta_{i+1,j+1}^x - \theta_{i+1,j}^x)$$

or

$$\alpha_{i,j} = \frac{1}{h_x} (\theta_{i+1,j+1}^y - \theta_{i,j+1}^y) \quad (2.2)$$

By substituting the appropriate deflection difference relationships for the angles θ , the expression for $\alpha_{i,j}$ in either case is

$$\alpha_{i,j} = \frac{1}{h_x h_y} (w_{i+1,j+1} - w_{i,j+1} - w_{i+1,j} + w_{i,j}) \quad (2.3)$$

The equivalence of the application of a force $V_{i,j}^t = 2\beta$ at the corners of the spider i,j to the application of unit twisting moments β along the edges of a plate element is clearly presented in a previous report (Ref 4). Considering $\beta = C_{i,j}^t \alpha_{i,j}$, with $C_{i,j}^t$ being the torsional stiffness per unit of plate width and $\alpha_{i,j}$ the unit angular rotation of mesh i,j , the following expressions are obtained:

$$V_{i,j}^t = \frac{2C_{i,j}^t}{h_x h_y} (w_{i-1,j-1} - w_{i-1,j} - w_{i,j-1} + w_{i,j})$$

$$V_{i,j+1}^t = \frac{2C_{i,j+1}^t}{h_x h_y} (w_{i-1,j} - w_{i-1,j+1} - w_{i,j} + w_{i,j+1})$$

$$V_{i+1,j}^t = \frac{2C_{i+1,j}^t}{h_x h_y} (w_{i,j-1} - w_{i,j} - w_{i+1,j-1} + w_{i+1,j})$$

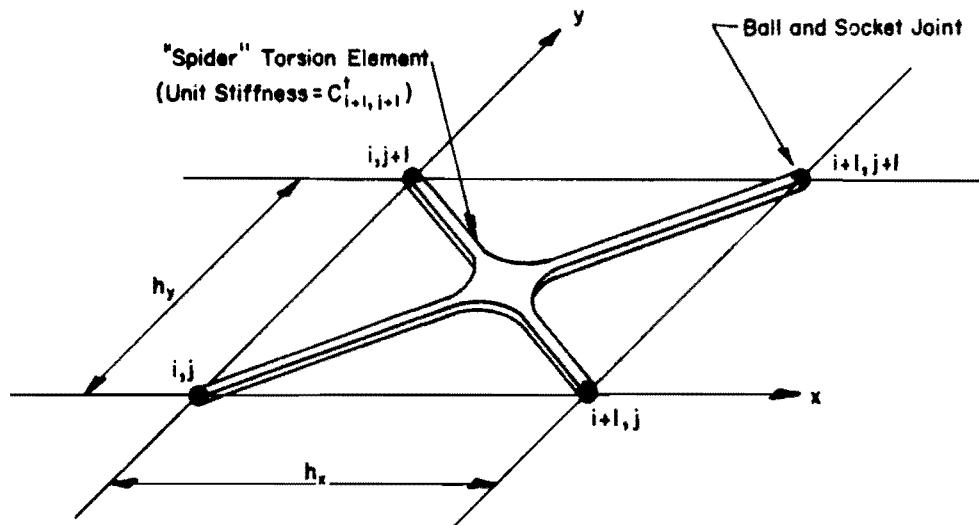


Fig 6. Torsional stiffness model, mesh $i+1,j+1$.

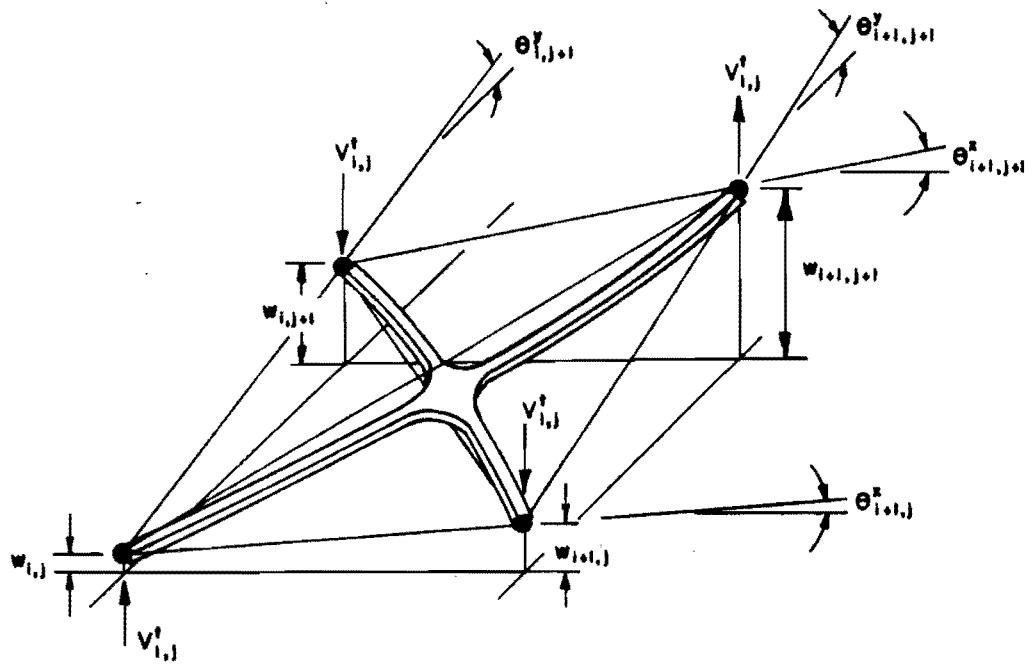


Fig 7. Free body of deformed spider element.

$$V_{i+1,j+1}^t = \frac{2C_{i+1,j+1}^t}{h_x h_y} (w_{i,j} - w_{i,j+1} - w_{i+1,j} + w_{i+1,j+1}) \quad (2.4)$$

Now, considering the bending moments and axial thrusts acting in the slab or plate and summing moments about each bar, the following expressions are obtained for the vertical shear forces.

$$\begin{aligned} V_{i,j}^x &= \frac{1}{h_x} \left[-M_{i-1,j}^x + M_{i,j}^x - P_{i,j}^x (-w_{i-1,j} + w_{i,j}) \right] \\ V_{i+1,j}^x &= \frac{1}{h_x} \left[-M_{i,j}^x + M_{i+1,j}^x - P_{i+1,j}^x (-w_{i,j} + w_{i+1,j}) \right] \\ V_{i,j}^y &= \frac{1}{h_y} \left[-M_{i,j-1}^y + M_{i,j}^y - P_{i,j}^y (-w_{i,j-1} + w_{i,j}) \right] \\ V_{i,j+1}^y &= \frac{1}{h_y} \left[-M_{i,j}^y + M_{i,j+1}^y - P_{i,j+1}^y (-w_{i,j} + w_{i,j+1}) \right] \end{aligned} \quad (2.5)$$

Similarly, summation of moments about each bar of the supporting grid-work gives identical equations for the beam shear forces.

$$\begin{aligned} \bar{V}_{i,j}^x &= \frac{1}{h_x} \left[-\bar{M}_{i-1,j}^x + \bar{M}_{i,j}^x - \bar{P}_{i,j}^x (-w_{i-1,j} + w_{i,j}) \right] \\ \bar{V}_{i+1,j}^x &= \frac{1}{h_x} \left[-\bar{M}_{i,j}^x + \bar{M}_{i+1,j}^x - \bar{P}_{i+1,j}^x (-w_{i,j} + w_{i+1,j}) \right] \\ \bar{V}_{i,j}^y &= \frac{1}{h_y} \left[-\bar{M}_{i,j-1}^y + \bar{M}_{i,j}^y - \bar{P}_{i,j}^y (-w_{i,j-1} + w_{i,j}) \right] \\ \bar{V}_{i,j+1}^y &= \frac{1}{h_y} \left[-\bar{M}_{i,j}^y + \bar{M}_{i,j+1}^y - \bar{P}_{i,j+1}^y (-w_{i,j} + w_{i,j+1}) \right] \end{aligned} \quad (2.6)$$

The axial thrusts P^x , P^y , \bar{P}^x , and \bar{P}^y are the concentrated values of axial thrust that act in the bars of either the slab system or the supporting grid system. It will be noted that since the complete assembly has a common deflection reference plane, the axial thrusts in the slab or gridwork have

exactly the same effect. They are input as separate parameters for simplicity and also to allow for independent solution of either a grid or slab.

The bending moments in Eqs 2.5 and 2.6 may be expressed in a central difference form. A complete derivation of the slab bending relationships is presented in a previous report (Ref 4).

$$\begin{aligned}
 M_{i,j}^x &= D_{i,j}^x h_y \left[\frac{w_{i-1,j} - 2w_{i,j} + w_{i+1,j}}{h_x^2} \right] \\
 &\quad + D_{i,j}^{cr} h_y v \left[\frac{w_{i,j-1} - 2w_{i,j} + w_{i,j+1}}{h_y^2} \right] \\
 M_{i,j}^y &= D_{i,j}^y h_x \left[\frac{w_{i,j-1} - 2w_{i,j} + w_{i,j+1}}{h_y^2} \right] \\
 &\quad + D_{i,j}^{cr} h_x v \left[\frac{w_{i-1,j} - 2w_{i,j} + w_{i+1,j}}{h_x^2} \right]
 \end{aligned} \tag{2.7}$$

The plate bending stiffnesses $D_{i,j}^x$ and $D_{i,j}^y$ are defined per unit of plate width. $D_{i,j}^{cr}$ is the smaller of either $D_{i,j}^x$ or $D_{i,j}^y$ so that the Poisson's ratio effect is felt only in the parent plate of a stiffened plate area (Ref 12).

$$\begin{aligned}
 \bar{M}_{i,j}^x &= F_{i,j}^x \left[\frac{w_{i-1,j} - 2w_{i,j} + w_{i+1,j}}{h_x^2} \right] \\
 \bar{M}_{i,j}^y &= F_{i,j}^y \left[\frac{w_{i,j-1} - 2w_{i,j} + w_{i,j+1}}{h_y^2} \right]
 \end{aligned} \tag{2.8}$$

The beam bending stiffnesses $F_{i,j}^x$ and $F_{i,j}^y$ are the concentrated values of beam bending stiffness and are exactly equivalent to the stiffness terms used in previous beam-column programs (Refs 8, 10, and 11).

Expressions similar to Eqs 2.7 and 2.8 for $M_{i-1,j}^x$, $M_{i+1,j}^x$, $M_{i,j-1}^y$, $M_{i,j+1}^y$, $\bar{M}_{i-1,j}^x$, $\bar{M}_{i+1,j}^x$, $\bar{M}_{i,j-1}^y$, and $\bar{M}_{i,j+1}^y$ are obtained by cyclic permutation of the subscripts i and j .

$Q_{i,j}^{Rx}$, $Q_{i,j}^{Ry}$, $Q_{i,j}^{Tx}$, and $Q_{i,j}^{Ty}$ are the transverse loads at joint i,j due to rotational restraints and applied moments. It should be noted that a rotational restraint or applied moment contributes to the transverse load or restraint one station from where they are placed, as is also true in previous beam-column work (Refs 8 and 11).

$$\begin{aligned} Q_{i,j}^{Rx} &= \frac{1}{2h_x} \left[-R_{i-1,j}^{Jx} \theta_{i-1,j}^{Jx} + R_{i+1,j}^{Jx} \theta_{i+1,j}^{Jx} \right] \\ Q_{i,j}^{Ry} &= \frac{1}{2h_y} \left[-R_{i,j-1}^{Jy} \theta_{i,j-1}^{Jy} + R_{i,j+1}^{Jy} \theta_{i,j+1}^{Jy} \right] \end{aligned} \quad (2.9)$$

$$\begin{aligned} Q_{i,j}^{Tx} &= \frac{1}{2h_x} \left[-T_{i-1,j}^{Jx} + T_{i+1,j}^{Jx} \right] \\ Q_{i,j}^{Ty} &= \frac{1}{2h_y} \left[-T_{i,j-1}^{Jy} + T_{i,j+1}^{Jy} \right] \end{aligned} \quad (2.10)$$

where,

$$\begin{aligned} \theta_{i-1,j}^{Jx} &= \frac{1}{2h_x} (-w_{i-2,j} + w_{i,j}) \\ \theta_{i+1,j}^{Jx} &= \frac{1}{2h_x} (-w_{i,j} + w_{i+2,j}) \\ \theta_{i,j-1}^{Jy} &= \frac{1}{2h_y} (-w_{i,j-2} + w_{i,j}) \\ \theta_{i,j+1}^{Jy} &= \frac{1}{2h_y} (-w_{i,j} + w_{i,j+2}) \end{aligned} \quad (2.11)$$

are the slopes at respective joints and are the average of the slopes in the adjacent bars, which was defined as the central difference slope in the moving-load beam-column report (Ref 11).

Substituting Eqs 2.9 through 2.11 into Eq 2.1, rearranging, and collecting the terms associated with each unknown deflection, gives

$$\begin{aligned}
& a_{i,j} w_{i,j-2} + b_{i,j}^1 w_{i-1,j-1} + b_{i,j}^2 w_{i,j-1} + b_{i,j}^3 w_{i+1,j-1} \\
& + c_{i,j}^1 w_{i-2,j} + c_{i,j}^2 w_{i-1,j} + c_{i,j}^3 w_{i,j} + c_{i,j}^4 w_{i+1,j} + c_{i,j}^5 w_{i+2,j} \\
& + d_{i,j}^1 w_{i-1,j+1} + d_{i,j}^2 w_{i,j+1} + d_{i,j}^3 w_{i+1,j+1} + e_{i,j} w_{i,j+2} \\
= & f_{i,j} \tag{2.12}
\end{aligned}$$

where

$$\begin{aligned}
a_{i,j} &= \frac{1}{h_y^3} \left[h_x D_{i,j-1}^y + F_{i,j-1}^y \right] - \frac{1}{4h_x^2} R_{i,j-1}^y \\
b_{i,j}^1 &= \frac{1}{h_x h_y} \left[v(D_{i-1,j}^{cr} + D_{i,j-1}^{cr}) + 2c_{i,j}^t \right] \\
b_{i,j}^2 &= -\frac{2}{h_y^3} \left[h_x (D_{i,j-1}^y + D_{i,j}^y) + F_{i,j-1}^y + F_{i,j}^y \right] \\
&- \frac{2}{h_x h_y} \left[v(D_{i,j-1}^{cr} + D_{i,j}^{cr}) + c_{i,j}^t + c_{i+1,j}^t \right] \\
&- \frac{1}{h_y} \left[p_{i,j}^y + \bar{p}_{i,j}^y \right] \\
b_{i,j}^3 &= \frac{1}{h_x h_y} \left[v(D_{i,j-1}^{cr} + D_{i+1,j}^{cr}) + 2c_{i+1,j}^t \right] \\
c_{i,j}^1 &= \frac{1}{h_x^3} \left[h_y D_{i-1,j}^x + F_{i-1,j}^x \right] - \frac{1}{4h_x^2} R_{i-1,j}^x \\
c_{i,j}^2 &= -\frac{2}{h_x^3} \left[h_y (D_{i-1,j}^x + D_{i,j}^x) + F_{i-1,j}^x + F_{i,j}^x \right] \\
&- \frac{2}{h_x h_y} \left[v(D_{i-1,j}^{cr} + D_{i,j}^{cr}) + c_{i,j}^t + c_{i,j+1}^t \right]
\end{aligned}$$

$$- \frac{1}{h_x} \left[P_{i,j}^x + \bar{P}_{i,j}^x \right]$$

$$c_{i,j}^3 = \frac{1}{h_x^3} \left[h_y (D_{i-1,j}^x + 4D_{i,j}^x + D_{i+1,j}^x) + F_{i-1,j}^x + 4F_{i,j}^x + F_{i+1,j}^x \right]$$

$$+ \frac{1}{h_y^3} \left[h_x (D_{i,j-1}^y + 4D_{i,j}^y + D_{i,j+1}^y) + F_{i,j-1}^y + 4F_{i,j}^y + F_{i,j+1}^y \right]$$

$$+ \frac{2}{h_x h_y} \left[4 v D_{i,j}^{cr} + C_{i,j}^t + C_{i,j+1}^t + C_{i+1,j}^t + C_{i+1,j+1}^t \right]$$

$$+ \frac{1}{h_x} \left[P_{i,j}^x + P_{i+1,j}^x + \bar{P}_{i,j}^x + \bar{P}_{i+1,j}^x \right]$$

$$+ \frac{1}{h_y} \left[P_{i,j}^y + P_{i,j+1}^y + \bar{P}_{i,j}^y + \bar{P}_{i,j+1}^y \right]$$

$$+ \frac{1}{4h_x^2} \left[R_{i-1,j}^x + R_{i+1,j}^x \right] + \frac{1}{4h_y^2} \left[R_{i,j-1}^y + R_{i,j+1}^y \right] + S_{i,j}$$

$$c_{i,j}^4 = - \frac{2}{h_x^3} \left[h_y (D_{i,j}^x + D_{i+1,j}^x) + F_{i,j}^x + F_{i+1,j}^x \right]$$

$$- \frac{2}{h_x h_y} \left[v (D_{i,j}^{cr} + D_{i+1,j}^{cr}) + C_{i+1,j}^t + C_{i+1,j+1}^t \right]$$

$$- \frac{1}{h_x} \left[P_{i+1,j}^x + \bar{P}_{i+1,j}^x \right]$$

$$c_{i,j}^5 = \frac{1}{h_x^3} \left[h_y D_{i+1,j}^x + F_{i+1,j}^x \right] - \frac{1}{4h_x^2} R_{i+1,j}^x$$

$$d_{i,j}^1 = \frac{1}{h_x h_y} \left[v (D_{i-1,j}^{cr} + D_{i,j+1}^{cr}) + 2C_{i,j+1}^t \right]$$

$$d_{i,j}^2 = - \frac{2}{h_y^3} \left[h_x (D_{i,j}^y + D_{i,j+1}^y) + F_{i,j}^y + F_{i,j+1}^y \right]$$

$$\begin{aligned}
 & - \frac{2}{h_x h_y} \left[v(D_{i,j}^{cr} + D_{i,j+1}^{cr}) + C_{i,j+1}^t + C_{i+1,j+1}^t \right] \\
 & - \frac{1}{h_y} \left[P_{i,j+1}^y + \bar{P}_{i,j+1}^y \right] \\
 d_{i,j}^3 = & \frac{1}{h_x h_y} \left[v(D_{i,j+1}^{cr} + D_{i+1,j}^{cr}) + 2C_{i+1,j+1}^t \right] \\
 e_{i,j} = & \frac{1}{h_y^3} \left[h_x D_{i,j+1}^y + F_{i,j+1}^y \right] - \frac{1}{4h_y^2} R_{i,j+1}^y \quad (2.13)
 \end{aligned}$$

and

$$f_{i,j} = Q_{i,j} + \frac{1}{2h_x} \left[-T_{i-1,j}^x + T_{i+1,j}^x \right] + \frac{1}{2h_y} \left[-T_{i,j-1}^y + T_{i,j+1}^y \right] \quad (2.14)$$

The Combined Stiffness Matrix

The thirteen coefficients (Eqs 2.13) of the unknown deflections form a pattern which composes the customary thirteen point operator for orthogonal plate systems. If there are no twisting stiffness or Poisson's ratio effects, the coefficients b^1 , b^3 , d^1 , and d^3 would be eliminated, thus degenerating the operator to nine elements which is appropriate for grid systems.

The ordered system of equations forms a diagonally banded and symmetrical stiffness matrix as shown in Fig 8 which is taken from Ref 12. The coefficient matrix is partitioned into banded submatrices where each horizontal partition represents the equilibrium equations for joints along a constant j -level.

Recursion-Inversion Solution Procedure

A recursion-inversion method developed and presented in previous reports is used to solve the system of equations for unknown displacements. An analogous procedure is the recursive process described for solution of beam-columns (Ref 8). The similarity between the recursive equations developed for slabs and grids and those derived for the recursive solution of beam-columns has been shown (Refs 12 and 15). A restatement of the recursion-inversion method is included here.

GENERAL EQUATION:

$$a_{i,j}w_{i,j-2} + b_{i,j}w_{i,j-1} + b^2_{i,j}w_{i,j} + b^3_{i,j}w_{i,j+1} + c_{i,j}w_{i-2,j} + c^2_{i,j}w_{i-1,j} + c^3_{i,j}w_{i,j} + c^4_{i,j}w_{i+1,j} + c^5_{i,j}w_{i+2,j} + d_{i,j}w_{i-1,j+1} + d^2_{i,j}w_{i,j+1} + d^3_{i,j}w_{i+1,j+1} + e_{i,j}w_{i,j+2} = f_{i,j}$$

OR IN MATRIX FORM:

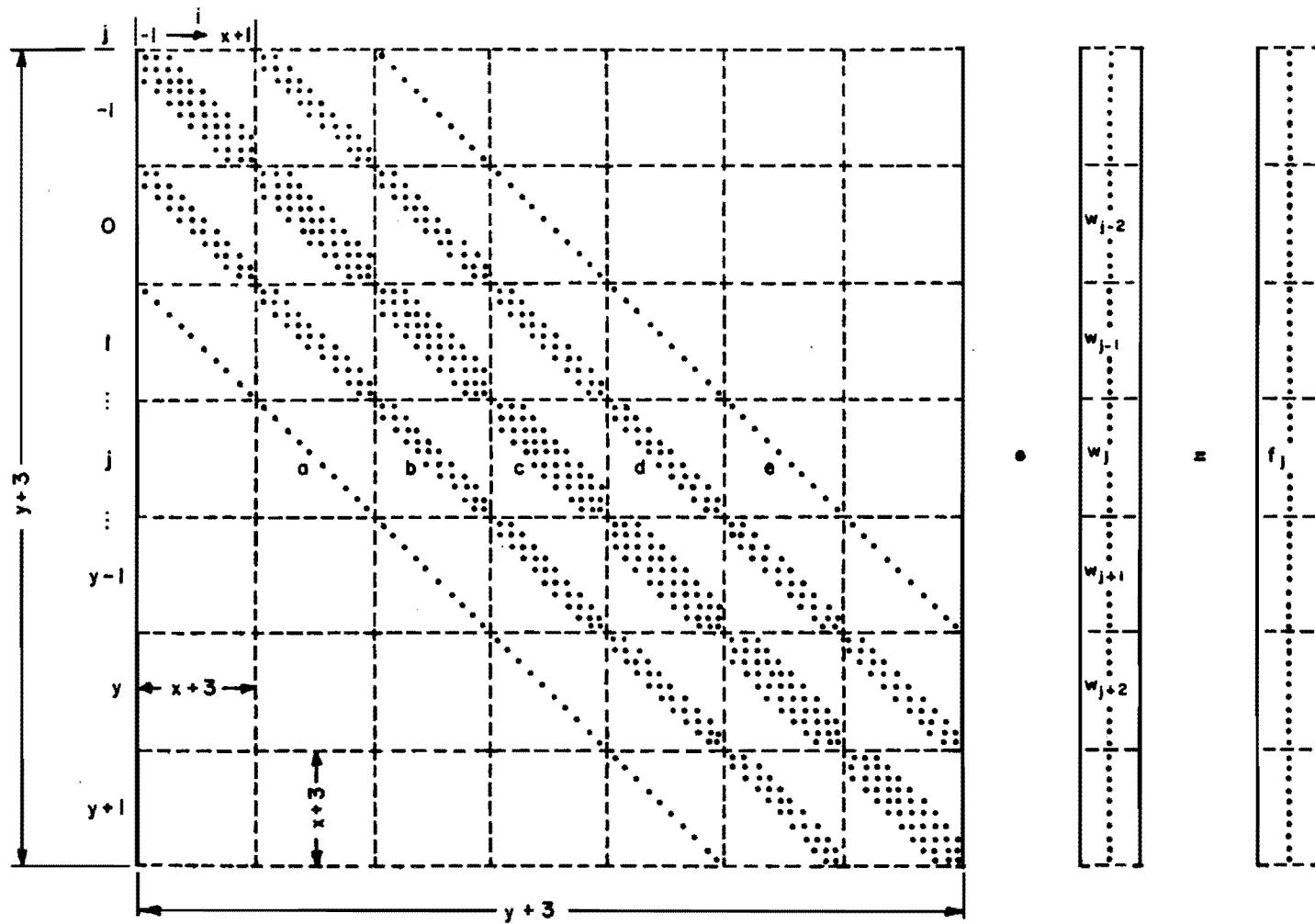


Fig 8. Form of the equations showing partitioned stiffness matrix (Ref 12).

Considering the horizontal partition at j of the equations of equilibrium shown in Fig 8,

$$\begin{aligned} & \left[\begin{array}{c} \diagup \\ a_j \\ \diagdown \end{array} \right] \{w_{j-2}\} + \left[\begin{array}{c} \diagup \\ b_j \\ \diagdown \end{array} \right] \{w_{j-1}\} + \left[\begin{array}{c} \diagup \\ c_j \\ \diagdown \end{array} \right] \{w_j\} \\ & + \left[\begin{array}{c} \diagup \\ d_j \\ \diagdown \end{array} \right] \{w_{j+1}\} + \left[\begin{array}{c} \diagup \\ e_j \\ \diagdown \end{array} \right] \{w_{j+2}\} = \{f_j\} \end{aligned} \quad (2.15)$$

the recursive equations may be written as

$$\begin{aligned} \{A_j\} &= \left[\begin{array}{c} D_j \\ E_j \end{array} \right] \left[\begin{array}{c} A_{j-1} \\ C_{j-1} \end{array} \right] + \left[\begin{array}{c} \diagup \\ a_j \\ \diagdown \end{array} \right] \{A_{j-2}\} - \{f_j\} \\ \{B_j\} &= \left[\begin{array}{c} D_j \\ E_j \end{array} \right] \left[\begin{array}{c} B_{j-1} \\ C_{j-1} \end{array} \right] + \left[\begin{array}{c} \diagup \\ d_j \\ \diagdown \end{array} \right] \\ \{C_j\} &= \left[\begin{array}{c} D_j \\ E_j \end{array} \right] \left[\begin{array}{c} \diagup \\ e_j \\ \diagdown \end{array} \right] \end{aligned} \quad (2.16)$$

where

$$\begin{aligned} \left[\begin{array}{c} D_j \\ E_j \end{array} \right] &= - \left[\begin{array}{c} \diagup \\ a_j \\ \diagdown \end{array} \right] \left[\begin{array}{c} C_{j-2} \\ B_{j-1} \end{array} \right] + \left[\begin{array}{c} \diagup \\ b_j \\ \diagdown \end{array} \right] \left[\begin{array}{c} B_{j-1} \\ C_j \end{array} \right]^{-1} \\ \left[\begin{array}{c} E_j \\ B_j \end{array} \right] &= \left[\begin{array}{c} \diagup \\ a_j \\ \diagdown \end{array} \right] \left[\begin{array}{c} B_{j-2} \\ C_j \end{array} \right] + \left[\begin{array}{c} \diagup \\ b_j \\ \diagdown \end{array} \right] \end{aligned} \quad (2.17)$$

and the back-substitution equation as

$$\begin{Bmatrix} w_j \end{Bmatrix} = \begin{Bmatrix} A_j \end{Bmatrix} + \begin{Bmatrix} B_j \\ \vdots \\ E_{j+1} \end{Bmatrix} \begin{Bmatrix} w_{j+1} \end{Bmatrix} + \begin{Bmatrix} C_j \\ \vdots \\ D_{j+2} \end{Bmatrix} \begin{Bmatrix} w_{j+2} \end{Bmatrix} \quad (2.18)$$

The above matrix equations are seen to be identical in form to those previously given for a beam-column solution (Ref 8). The long, or y , direction of a slab and grid or plate problem determines the number of partitions of the stiffness matrix and is directly analogous to the number of partitions in a beam-column solution.

For a symmetric stiffness matrix, it has been shown that the above set of recursive equations can be further modified (Ref 3):

$$\begin{aligned} \begin{Bmatrix} A_j \end{Bmatrix} &= \begin{Bmatrix} D_j \\ \vdots \\ E_j \end{Bmatrix} \begin{Bmatrix} A_{j-1} \end{Bmatrix} + \begin{Bmatrix} e_{j-2} \\ \vdots \\ e_{j-1} \end{Bmatrix}^t \begin{Bmatrix} A_{j-2} \end{Bmatrix} - \begin{Bmatrix} f_j \end{Bmatrix} \\ \begin{Bmatrix} B_j \\ \vdots \\ E_{j+1} \end{Bmatrix} &= \begin{Bmatrix} D_j \\ \vdots \\ E_j \end{Bmatrix}^t \\ \begin{Bmatrix} C_j \\ \vdots \\ D_{j+2} \end{Bmatrix} &= \begin{Bmatrix} D_j \\ \vdots \\ e_j \end{Bmatrix} \quad (2.19) \end{aligned}$$

where

$$\begin{aligned} \begin{Bmatrix} D_j \\ \vdots \\ e_j \end{Bmatrix} &= - \begin{Bmatrix} e_{j-2} \\ \vdots \\ e_{j-1} \end{Bmatrix}^t \begin{Bmatrix} C_{j-2} \end{Bmatrix} + \begin{Bmatrix} E_j \\ \vdots \\ B_{j-1} \end{Bmatrix} + \begin{Bmatrix} c_j \\ \vdots \\ d_{j-1} \end{Bmatrix}^{-1} \\ \begin{Bmatrix} E_j \\ \vdots \\ B_{j-2} \end{Bmatrix} &= \begin{Bmatrix} e_{j-2} \\ \vdots \\ e_{j-1} \end{Bmatrix}^t \begin{Bmatrix} B_{j-2} \end{Bmatrix} + \begin{Bmatrix} d_{j-1} \\ \vdots \\ d_{j-2} \end{Bmatrix}^t \quad (2.20) \end{aligned}$$

and

$$\begin{bmatrix} E_{j+1} \end{bmatrix} = \begin{bmatrix} e_{j-1} \\ \vdots \end{bmatrix}^t \begin{bmatrix} B_{j-1} \end{bmatrix} + \begin{bmatrix} d_j \\ \vdots \end{bmatrix}^t \quad (2.21)$$

The superscript t indicates the transpose of the appropriate submatrix. Since the stiffness matrix for the discrete element model is a symmetric one, Eqs 2.19 through 2.21 are used with Eq 2.18 in the solution procedure.

Multiple-Loading Technique

The multiple-loading recursive technique presented in the two-way bridge floor slab report (Ref 12) is briefly reviewed here. It is noted that the load vector $\{f_j\}$ appears in only the $\{A_j\}$ recursion coefficient vector of Eq 2.19. Since $\{A_j\}$ is not involved in either the remaining coefficient or multiplier matrices, the $[E_j]$, $[D_j]$, $[C_j]$, and $[B_j]$ coefficients are computed once and retained on disk, tape, or other auxiliary storage files and then recalled as needed for each successive problem. The $\{A_j\}$ recursion vector is simply modified or reformed for each successive loading.

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CHAPTER 3. THE COMPUTER PROGRAM

The computer program described in this report applies solution techniques developed during this project study in such fashion as to solve the numerical equations in the most efficient and direct manner. The basic solution procedure is directly analogous to that originally presented (Ref 8) and later extended to two-dimensional systems (Ref 15). The efficiency of multiple-loadings (Ref 12) has been included. A recently developed streamlined and efficient recursion-inversion procedure as applied to banded systems (Ref 3) is also used.

Data Card Images

A significant feature of the program is the technique through which all data input in a series of problems are retained and echo-printed for all problems. This procedure was first applied and found useful for the moving-load beam-column program (Ref 11). The procedure avoids the necessity of re-coding and including common data for a problem series. In previous two-dimensional programs written on this project, computer storage for the complete slab was set aside for each different type of input stiffness and load. This program simply stores the input data card images which are searched at each level in the solution process and only the necessary stiffness and load terms are generated. Thus, a significant saving in required computer storage is achieved for a slight increase in computational effort. The technique of storing data card images has also been shown to provide the most convenient method of transferring information from a data-generation routine written for certain specific problem types. The data card images are created by the data-generation routines and then the basic solution program utilizing the general slab and grid system is used for the solution of the structure. By this means, any number of special purpose programs can use this one basic orthogonal system computer solution technique.

The FORTRAN Program

The version of the orthogonal slab and grid program described here is called SLAB 49, which indicates that it is the 49th in the sequence of developments

for plate and grid-type structures. The program is written in FORTRAN and follows the basic guidelines given for ASA FORTRAN (Ref 2). The program is written for both CDC 6600 and IBM 360 computers and is easily made compatible with other similar computer systems.

The program listing presented in Appendix 3 is specifically for the IBM 360 computer. Those cards needed to operate on the CDC 6600 computer are included as companion cards and are nulled with a C in column one and have the symbols CDC in columns 78 through 80, following the card preparation date. All necessary cards for IBM operation such as the selective double precision statements are tagged with IBM in columns 78 through 80. Not all variables need to be double precisioned to yield correct solutions with the IBM 360 computer, which saves a significant amount of storage over the customary procedure of complete double precisioning. When converting from one computer system to another, it is recommended that the companion IBM cards be retained and nulled with an added C in column one at the same time the C is omitted from the CDC cards.

Storage Requirements

The storage requirements are variable, depending upon the size of the problem to be run. Cards which must be changed for different sized problems are specified at the beginning of the program and include only the dimension statements and two variables which define the number of increments in both directions. The largest dimensioned storage arrays are designated as R1 , R2 , R3 , PW , PBMX , PBMY , and PSIGO (Refer to the program listing, Appendix 3). The first three are doubly subscripted, with both subscripts equal to the number of increments plus 3 in the shorter slab or grid direction, and are required for the basic solution routine (Ref 3). The last four variables above are doubly subscripted, with subscripts equal to the number of increments plus 3 in both slab directions, and these variables are concerned with plotting of the output values of deflection, bending moments, and principal moments. A simplified version of the program without plots could be easily made which would then allow the length of the problem to be almost without limit as far as computer storage is concerned. Computer time considerations limit all large problems to those solutions that are economically justified.

A plot of the CDC 6600 storage requirements is shown in Fig 9. If it is necessary to gain more storage space, certain variables could be set equal

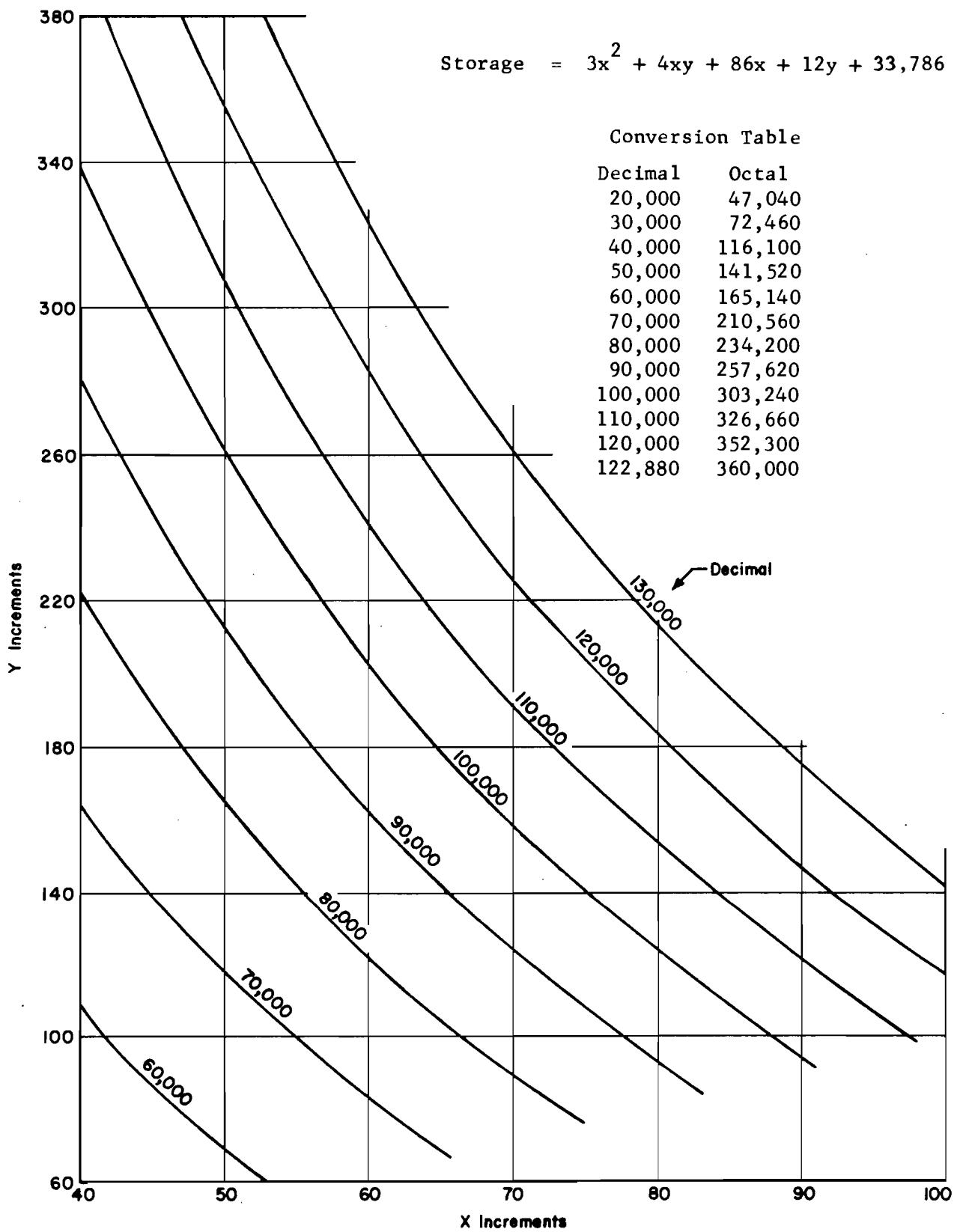


Fig 9. SLAB 49 storage requirements.

to each other in storage by an equivalence declaration. Layering is another technique to reduce storage requirements that could be used at a slight increase in computational time. These procedures have not been presented with the included program to avoid initial confusion when converting to other computer systems.

A summary flow diagram of the program is shown in Fig 10. A list of the variables used and their definitions is given in the Notation in Appendix 2. A complete listing of the program, including brief flowcharts of each routine, is shown in Appendix 3. All subroutines in the program are variably dimensioned as functions of the short X and long Y lengths, which are specified in the main driving program.

Data Input Tables

The general procedures to be followed for input of a problem are outlined in the Guide for Data Input, Appendix 1. The guide is designed so that additional copies may be made and used for routine reference. A parallel study of the guide will help the reader to understand the following discussion.

The first two cards of a problem series are for identification purposes. Any desired alphanumeric descriptive information can be entered by the user. It is suggested that the date of the run, the user's name, and the chosen units always be included on these two cards. The next card is the problem number card with a brief description of the particular problem. The problem number itself may contain alphabetical characters if desired. The problem series terminates when a blank problem number is encountered. Any number of problems may be run at the same time.

Table 1 is used to input the problem control data and is always comprised of two data cards that include the keep options, multiple load option, number of cards input for this problem, and other ouput options including plots.

The first card of Table 1 contains the keep options. Any data of the preceding problem may be retained if desired. If Table 2 is retained, it may not be added to or modified. Any of the other data tables may be retained and additional data cards may be input, up to the combined maximum total cards for each table. The Multiple Load Option in column 50 of the first Table 1 card is left blank if each successive problem is independent of the preceding problem. If a following problem is for the same slab-and-grid system, and if

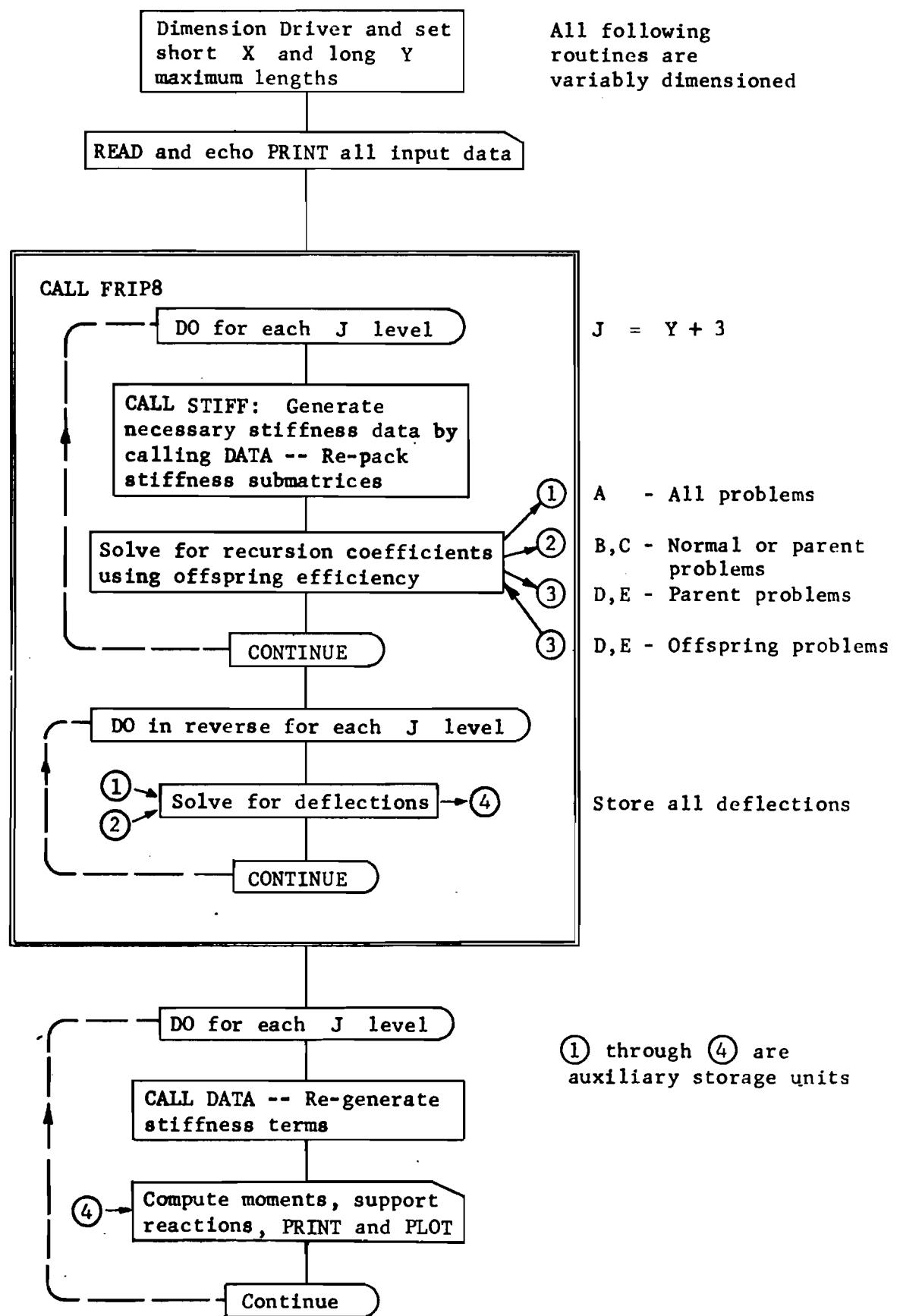


Fig 10. Summary flow diagram for SLAB 49.

only the load pattern and placement given by Table 7 are to change, the first problem in the loading series is specified with a +1 for the option. This will be the "parent" problem. Each successive loading must have a -1 for the option and it will be termed an "offspring" problem. When a blank option or another +1 is encountered, that problem is another independent problem or a new parent problem.

The second card of Table 1 is for the designation of the number of data cards to be input in this problem for Tables 2 through 9. The user should carefully check the card counts to avoid data errors. A number of common types of data errors are checked for by the input routines, but it is possible to create a false problem by incorrect card counts.

The four options in columns 50 through 65 of the second card of Table 1 are for output options. The statics check option may be exercised by entering a 1 in column 50. This is useful for determining if a solution inaccuracy exists, especially for computer systems which have required double precision operations. The statics check is computed internally in the program by reapplication of the governing equations (Eqs 2.12 through 2.14) to the computed values of deflection. Any applied loads and external couples are deducted and the remaining quantity represents the computational error at each joint of the system. This check depends on there being no error in the data or stiffness generation portion of the program since the same data and stiffnesses are used for the back-substitution. The statics check option will normally be left blank, in which case the output value printed will be the concentrated support reaction at each joint. The second output option, in column 55, is exercised by entering a 1 if the user desires that the computed value of principal moment in the slab be converted to an equivalent value of stress. The computed value of stress is correct only for slab areas of uniform thickness with no discontinuities. If this stress option is exercised, an appropriate value of slab thickness must then be available in Table 2. The principal moment is converted to a stress having the same sign by multiplication of the moment by the plate section modulus, which is internally computed from the Table 2 input value of thickness. Axial thrusts if present are not included as part of the stress calculation. The third output option, in column 60, is for control of the type of plotted output associated with the specified areas in Table 8. If the column is left blank or zero, a crude printer plot is obtained along with tabulated output designated

by Tab : 8. If set equal to 1, no tabulated output is obtained for the areas of Table 8 but the plots of those areas are on microfilm (assuming it is available on the particular computer system). If this option is set equal to 2, then a combination of 0 and 1 is obtained; that is, Table 8 areas are tabulated, a printer plot display is made, and a microfilm plot is also made. If the option is set equal to 3, only a line plot on paper is obtained for the Table 8 areas and no tabulation is printed. The fourth option, in column 65, is exercised by entering a 1 to create a pseudo-three-dimensional plotted display of all the computed deflections for the entire slab or grid. This display is discussed further under Computed Results.

Table 2 is used to specify the constants for the problem. These are the number of increments in the X and Y directions, the increment lengths in both directions, and Poisson's ratio. For efficient solution of the program, the number of Y increments must be equal to or greater than the number of X increments. Table 2 must be kept for offspring problems since the constants must be the same as in the parent problem. The constants specified for the parent are retained and used by all successive offspring problems. Table 2 also provides a space for entry of slab or plate pseudo-thickness. The thickness must be entered if the stress option of Table 1 is exercised. The use of the thickness is appropriate only for plates of a constant thickness. For most two-way concrete slabs, the principal moment is a more important design quantity which is automatically obtained if the stress option in Table 1 is left blank. At specified discontinuities in the slab, such as a crack or joint which might be modeled by means of a reduced bending stiffness (Ref 9), the output value of "stress" at that location may be misleading. A better estimate of stress at discontinuities may be obtained by inspecting the variation in computed stress at several stations adjacent to the discontinuity.

Table 3 has the number of cards specified in Table 1. Card counts should be carefully checked. It is recommended that a listing of the data cards be checked by the user prior to submission of the program for a run.

Joint stiffness and load data are entered by a coordinate system notation. The coordinates refer to the discrete-element model numbering system. A joint is defined as occurring at the intersection of the station lines in each X and Y direction. A mesh is defined as that area surrounded by four joints. A bar is defined as the discrete-element length between adjacent

joints. Figure 3 in Chapter 2 and Fig A1 of Appendix 1 summarize this notation. Note that mesh data cannot have either a zero X or zero Y coordinate; X-bar data cannot have a zero X-coordinate, and, similarly, the Y-bar cannot have a zero Y-coordinate. If the data occur only at one location (such as a concentrated load), the From and Through coordinates are simply repeated. If the data occur along a line, the coordinates will reflect this by having either both X or both Y coordinates the same. Data distributed over a rectangular area are specified by entering the lower left and upper right coordinates bounding the area.

The orthogonal slab bending stiffnesses D^x and D^y are entered in each direction and are specified on a per unit width basis. If the edge of the slab coincides with a station line, a half-value of stiffness should be input for both D^x and D^y along the edge. If the edge of the real slab is not on a station line, a proportionate value of full stiffness is entered. This is demonstrated by a sample input in Appendix 1, Fig A2. The stiffness apportionment may be thought of as a direct function of the plan area of real slab surrounding each joint.

The orthogonal slab stiffnesses may be varied by the user to provide for any degree of flexibility in both directions. For instance, a crack in a slab can be represented by an appropriate reduced value of stiffness at that location (Ref 9). The customary stiffness relationship that may be used for isotropic plates or slabs is given here for reference. Poisson's ratio for concrete is a difficult constant to define precisely, but a value of 0.15 is used by most investigators.

$$D^x = D^y = \frac{Et^3}{12(1 - \nu^2)} \quad (3.1)$$

The beam bending stiffnesses F^x and F^y are entered in each direction and are the total concentrated values of stiffness which represent beams or combination of beams and a composite slab. The input of beam stiffnesses is more completely discussed in the example problems of Chapter 4. The beam stiffness characterizations and inputs are identical with the models of previous beam-column developments (Refs 8, 10, 11, and 16).

Load Q is concentrated on a per joint basis and may be apportioned at each joint by the contributory area loaded around each joint. Positive loads

act upward. Loads that occur between joints may be fractionally proportioned to the adjacent joints. Support springs S are concentrated values input and apportioned exactly like loads. A rigid support may be specified by introducing a large value for the support spring. A maximum value of 1×10^{25} is suggested to avoid computational difficulties for some computers. A uniform subgrade of modulus k can be specified by application of this relationship

$$S = kh_{x,y} \quad (3.2)$$

Multi-valued subgrade moduli or foundations with voids may also be modeled by entering the appropriate spring stiffnesses.

Table 4 is for input of rotational restraints and applied couples. These are input as concentrated effects in either the X or Y direction. An effective rotational restraint could be computed for a slab supported by a column which is framed into the slab, or for a girder which is framed into its support members. Both restraints and applied moments are used and input the same as in previous beam-column programs (Refs 8, 10, 11, and 16).

Table 5 is for input of the twisting stiffness associated with the slab portion of the structure. Since it is a quantity that represents stiffness between joints of the model (Fig 1) it is input in a separate table from the bending stiffnesses constants.

The twisting stiffness C^t is input on a per unit width basis for each mesh surrounded by four joints. When the geometric edges of the actual slab do not fall on a station line, proportionate values of unit twisting stiffness may be input similar to bending stiffness apportionment. Computations of twisting stiffnesses for slabs or plates are at best still approximate procedures. This is due to uncertainty in the defining of the shearing modulus of rigidity. One procedure is to ascertain the twisting stiffness experimentally as outlined by Hudson (Ref 4). The formula shown below for reference is correct for uniformly thick isotropic plates.

$$C^t = \frac{Et^3}{12(1 + \nu)} \quad (3.3)$$

An approximate value for twisting stiffness for orthotropic slabs or stiffened plates may be obtained by using procedures outlined by Huffington (Ref 5) or

computations summarized by Troitsky (Ref 17). Fortunately, precise values of twisting stiffness are unnecessary to model a slab. The main load-carrying capabilities of a slab are due to its bending stiffness which is easier to define.

Table 6 is for input of the axial thrusts in either the slab or supporting grid-beam network. All the thrusts are concentrated values, and therefore any distributed axial thrusts in slabs must be concentrated over the appropriate increment width. As previously stated, since the slab and grid systems have a common deflection reference, the effect of slab or beam axial thrusts is the same. The provision for inputting them separately is made so that they are easier to visualize and also to allow the independent solution of either grid-beam assemblages or slabs. In either case, the output is shortened, as discussed later.

The beam and slab axial thrusts (Fig 4) have exactly the same effect as their counterparts in previous beam-column solutions (Refs 11 and 16).

Axial thrusts in bridge decks might be due to differential temperature between a slab and floor system, closing of an expansion joint, or traffic braking and acceleration forces. There is no provision in the program for automatic distribution of applied axial forces since no in-plane supports are used which would restrain them. The user must specify the distribution of the axial tensions (+) and compressions (-) in each X-bar and Y-bar of the model (Figs 1 and 4). Since these are bar forces, no data should be input which would represent forces outside the boundaries of the actual slab. A brief sample of data input is given in Fig A2 in Appendix 1.

Table 7 is used for input of loads if they are to change position or magnitude for two or more problems on the same structure. Its use is strictly for convenience. Loads appearing in Table 7 could also have been input as Table 3 loads, and that is why the data field for the load value is in columns 61 through 70 for both Tables 3 and 7. For offspring problems (multiple load option equal to -1), the loads must be input in Table 7, and loads or stiffnesses input in Table 3 are retained. For normal problems or parent problems (multiple load option equal to 0 or +1), loads may be input in either Table 3 or 7.

Table 8 is used to define the lines or areas of selected tabulated and plotted output for deflection, bending moments in the X and Y directions, and either the maximum principal moment or stress, depending on the stress

option in Table 1. Thus, concise printout for a specific location, such as near wheel loads and support points, can be obtained. The number of cards is as specified in Table 1 and may include up to a maximum of 10 cards. Each card can encompass up to a maximum of 300 points; for instance, coordinates from 11,11 through 20,40 or from 0,0 through 11,24 could be specified. If a larger area is required, another card covering the adjacent area can be added.

The major advantage of Table 8 is that a crude printer plot display can be obtained for each area specified, if the option in Table 1 is 0 or 2. This feature is discussed more completely under Computed Results. Table 8 is especially useful when the deflection and moment variations along a line or over a local area are being studied. Table 8 can be omitted, if desired, since all selected output values appear in the complete printout of results, but caution should be used; if all or part of the complete printout of results is suppressed in Table 9, discussed below, a significant amount of computer time will be used but no printout of results will be made. The type of printer or line plotted output depends on the plot control option in Table 1, as discussed above.

Table 9 allows the user to have only selected sections of the complete output printed. This is sometimes very desirable for a problem series in which local areas of the structure are under study for various positions of loads, supports, discontinuities, etc. When Table 9 is omitted the complete output is printed. A partial output is printed when the sections to be printed are specified within the Y-bounds designated on the Table 9 cards. Up to 10 different Y-bounded sections can be printed. The sections may overlap; the doubly defined areas are printed only once. This option is useful for study in areas near span centerlines and in areas near supports for continuous structures; the printout can be deleted for the noncritical areas. Caution is again advised, to assure that output which might be of interest is not suppressed.

General Data Input Comments

As previously stated, it is wise to obtain a listing of the data input for verification prior to program submission, especially for large and time-consuming problems with complex data input.

All data in Tables 3, 4, 5, 6, and 7 are algebraically accumulated to storage as needed and values therefore may be added or subtracted regardless of other values input or held from previous problems.

For offspring problems, only Tables 1 and 7 are required, but Tables 8 and 9 may be again specified if different output areas are desired. When solving an offspring problem, the user must hold Tables 2, 3, 4, 5, and 6, adding no cards. As discussed in Chapter 2, the multiple-loading recursive technique offers a considerable computer time advantage for a problem series in which only the load magnitude and position change. If the user adds any data to that in Tables 3 through 6, the problem must then be considered another parent or independent problem.

Data Errors

All data are checked for compatibility with the geometry of the specified slab and consistency of coordinate input. A count is made of the number of data errors in each table and the problem is then terminated with a message showing the number of data errors. Typical errors are (1) misusing the multiple-load option, such as inputting a -1 to follow a 0 in the preceding problem; (2) having the number of increments in the X-direction exceed those in the Y-direction, which would result in an inefficient and time consuming computer solution; (3) specifying a negative or zero increment length; (4) inputting a negative Poisson's ratio or thickness; (5) making the Through X or Y coordinate in a data specification numerically less than the From coordinate; (6) specifying data outside the geometric limits of the slab; (7) specifying a zero X or Y coordinate for a twisting stiffness; (8) using a zero X coordinate for X-bar axial thrusts or a zero Y coordinate for Y-bar thrusts; (9) specifying a number of increments greater than the dimensioned storage with which the program can operate; and (10) misusing the selected output option.

Computed Results

The computed results are headed by a line which includes the program title, specific version, and latest program revision date. Immediately below are the two alphanumeric information data header cards, followed by the problem number and description. The importance of using run dates and descriptive alphanumeric information for the problem series header cards

and problem number cards in order to avoid confusion when running a large number of problems cannot be overemphasized.

Data cards for each table, including those kept from previous problems, are echo printed with explanatory headings exactly as they were input. It is good practice to recheck all data for possible errors prior to inspection of the results.

Tabulated Results. The computed results are listed in Y-station groups, in reverse order because of the computation arrangement set up in the program and for convenience in correlating of results with the sketch of the problem input. The output is arranged to give the X and Y-joint coordinate, the transverse deflection at each joint (upward deflections are positive), the slab and beam bending moments, the slab twisting moment, the principal slab moment or stress and its direction, and either the concentrated value of support reaction or the statics check. Output values of slab moments are per unit width; beam moments are total per beam. Bending moments and stress are positive for compression in the top of the beams or slab. The X-bending moments act in the X-direction and the Y-bending moments in the Y-direction.

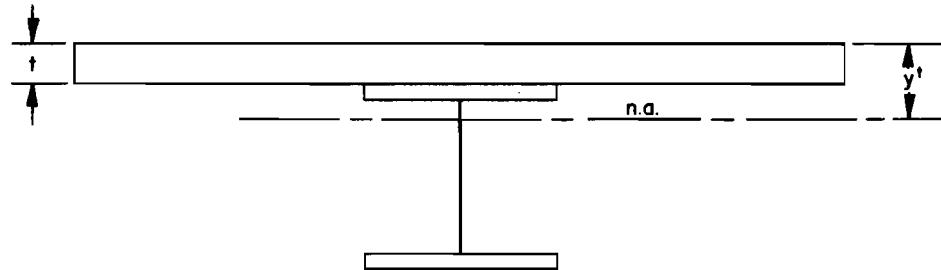
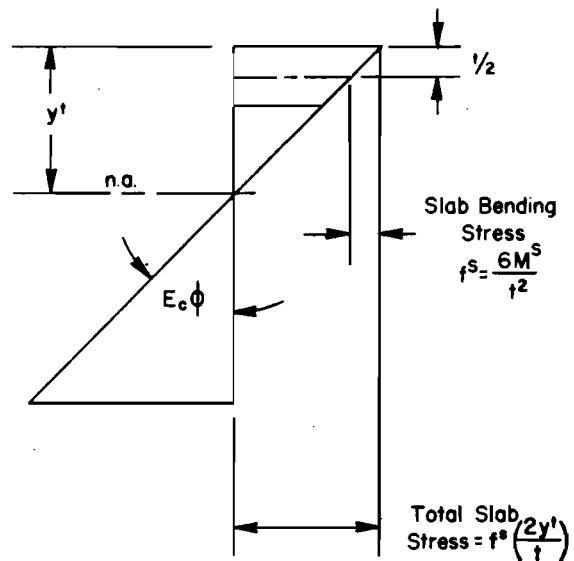
The output for the slabs is automatically given in a reduced form if the input data did not include any beam stiffnesses. The reverse is true if no slab stiffness data were input. The output is arranged so that the X and Y-beam moments are printed directly below each value of slab moment when both slab and beam data are present.

Twisting Moments. The per unit width X-twisting moments are tabulated and are exactly equal to the Y-twisting moments with opposite sign. The X-twisting moments act in the X-direction and are about the Y-axis. Even though the input values of twisting stiffnesses were specified for each mesh, the output values of twisting moment are the average of four adjacent mesh areas and are given at the joints. The user is cautioned that the output values of twisting moment along the edges or other discontinuities of a slab or plate reflect the average and may be a one-quarter, one-half, or some other proportionate value. The output values of largest principal moment or stress at edges are also affected by this averaging.

Principal Moments or Stresses. A Mohr's circle analysis is made at each joint, using the orthogonal slab bending moments and twisting moments to yield the larger numeric value (positive or negative) of principal moment per unit

width and the angle from the X-axis of the coordinate system to the acting direction of this larger value. Counterclockwise angles are positive. The principal moment values are converted to the larger numerical value of principal stress if the stress option was specified in Table 1 and a thickness is present in Table 2. A positive stress indicates tension in the bottom of the slab, which follows the same sign convention as do the bending moments. The stress option is properly used only for slabs or plates of constant thickness. A direct conversion can be made for principal stress from the principal moment for plates of variable stiffness and thickness. The output value of stress does not include any in-plane forces that may be present. The user must consider axial thrusts (tension or compression) input in Table 6 when interpreting stress results.

Stresses in Composite Slabs. Figure 11 presents a typical situation when interpreting combined beam and slab results. The input value of beam stiffness was computed from the complete composite section, using appropriate transformed areas, effective flange widths, etc., but excluding the stiffness of the slab about its own axis. The input value of slab stiffness was computed on the basis of the slab thickness and estimated elastic constants for the concrete. The output value of maximum slab stress, which was internally computed, was probably not in a major orthogonal direction due to the presence of loads between beams. Assuming it was nearly orthogonal, it is desired to compute an estimate of the slab stress in the direction of the beam from the results of the computer model. First, the bending stress in the slab in the beam direction is determined; the output value is taken or the stress is computed from the slab bending moment and slab section modulus in which M^s is the slab bending moment in the beam direction. This stress is shown as f^s in Fig 11. Second, this stress is modified by the ratio of twice the computed depth to the neutral axis of the composite cross section divided by the slab thickness, also shown in Fig 11. This total slab stress could then be further modified for added stresses due to axial thrusts. The output values of transverse slab bending moment and the slab twisting moment can then be converted to stress and a Mohr's circle analysis made to determine the maximum slab stress and its direction. All of these calculations could have been made in the computer, but that would require the input of estimated depths to the composite neutral axis at all locations in the slab-beam system or an even more complex input, with all areas, moduli, etc., at all points. A version of the program developed specifically for special

Composite Cross Section:**Stresses in Transformed Section:****Fig 11. Stress in composite slab.**

applications would be best for an automatic stress calculation. However, when local slab stresses have been computed, it has been found that they usually occur in directions which are transverse to the relatively stiff beams. That is, the beams act as almost rigid supports to the slab (Ref 12), and the calculations as shown in Fig 11 are not normally required.

Moments. The values of beam moment in the printout are concentrated values rather than distributed, as are those for slab moments. Slab moments are typically in kip-feet per foot, or in units consistent with the input; and beam moments are in kip-feet. The user can obtain predicted beam stresses by applying usual section modulus values consistent with the input stiffnesses.

Reactions. The last column of output lists the support reactions at each station if the statics check option in Table 1 is blank. The support reaction is the concentrated value of resistance to displacement offered by any support springs that are present. A subgrade modulus spring will reflect the concentrated value of pressure under the slab. If the spring is specified with a large value to represent a rigid support (which is unrealistic, since all structural supports have some degree of elastic response), then the value printed is the rigid support reaction.

A statics check is printed instead of support reaction if the Column 50 statics check option in Table 1 is exercised. This statics check is the summation of all the computed shears, twisting moment forces, restraint and applied moment forces, subgrade reaction, and applied external load at each joint, as shown in Fig 5. The value printed represents the amount of error at that joint which is inherent in the computer solution. This option has no practical application, but if it is suspected that there are computer inaccuracies which are being generated by roundoff, truncation, or errors, this option will help to determine their magnitude.

As a check on the back-substitution process in the computer solution, and as a check on the total load input to the grid-slab structure, a final result is printed at the end of the detailed output. This is the algebraic sum of all the reaction values and should be equal to the sum of all the applied loads. This check should always be inspected to verify that the desired load system was specified and that the problem was properly solved. Another value is printed following this, the maximum statics check error and the station at which it occurred. This value is always printed, whether the statics check option in Table 1 is exercised or not, and can act as an

immediate flag to the user if some error has occurred in the computer. The error would normally be expected to be less than about 10 orders of magnitude smaller than the largest load applied. The user should not concern himself with the significant digits of any remnant (approximately zero) values of output.

Profile Output. After the detailed output, areas of selected profile output designated by Table 8 are printed. No tabulated values will be printed if the plot option in Column 60 of Table 1 was 1 or 3. Profile tabulations are obtained if the option was 0 or 2. These tabulations are printed in consecutive groups associated with the largest number of increments designated in the Table 8 rectangular area. For instance, if the area desired is from 10,16 to 12,20 then there would be three X-groups, each with five values. If the area is square, the groups would be for consecutive Y values, which is the same arrangement as the normal output discussed above. Adjacent to the coordinates is the numerical value of the deflection, moment, or stress. Printed to the right of the output values is a series of asterisks whose placement relative to one another is based on the numerical values. Thus, a crude plot of the output values is obtained. The plot has a width of 20 printer characters and a length equal to the number of nodes encompassed by the area specified. The plot has no scale and no zero; the values are relative to one another, increasing positively to the right. The 20-character width is based on the minimum and maximum values in the area to be plotted. The user is cautioned not to misinterpret apparent changes in plot curvature which might be due to very slight numerical changes.

The printer plots have been found to be especially valuable because the user obtains them with the rest of his printed output; unnecessary time is not spent waiting for line plotter output or in hand plotting. They are also useful in understanding slab behavior for areas adjacent to concentrated wheel loads and supports. Deflection areas are printed and plotted first, followed by bending moment areas, which are tabulated and plotted adjacent to each other with a set of common coordinates if both X and Y-moments desired were in the same area. The final selected profile output is for the principal moments or stresses, again depending on the Table 1 option. Plots of principal moments or stresses along slab lines are somewhat misleading since the direction of the moment or stress usually varies along the line. They

are valuable, however, in pointing out maximum values which might be overlooked when inspecting a mass of numbers in the normal detailed output.

The final printed output is the computer time used for the problem and the total accumulated time for the problem series. The user should record run times for parent and offspring problems for each problem size run on his computer system to estimate run times required for future problems. For small problems, the offspring times will be from 20 to 50 percent of the parent problem times. Fortunately, the offspring problem time decreases to a very small proportion of the parent problem time as the problem size becomes large. A time as low as 4 percent is possible.

Three-dimensional deflection plots are obtainable with the program if the appropriate plotter routines and hardware are available. The subroutines which generate the three-dimensional plots have been written using standard routines available for Calcomp plot systems. Each plot is arranged to fit within a 7-inch by 10-inch area. The receding angle of the plot measured from the X-axis has a tangent of 0.9 (approximately a 42 degree angle). The receding Y-axis lengths are reduced by a factor of 0.75 of their true length relative to the X-axis lengths. The vertical deflections are distorted (magnified) so that the maximum deflection is 1 inch on the plot. These automatic values for these three parameters were fixed to yield what has been found to be a reasonably optimum implied position of the viewer's eye for most problems. These values can be changed if necessary since they are variables in the subroutines that generate the three-dimensional deflection plots. A typical plot is shown in Fig 12, which is taken from one of the problems in Chapter 4. The problem number is plotted to the left of the plot, for reference. It should be noted that on the plot some areas are hidden due to the magnification of the vertical deflections. The small tics superimposed around the edges of the plot are of the undeformed structure and thus comparisons of relative deflections can be made.

1003R

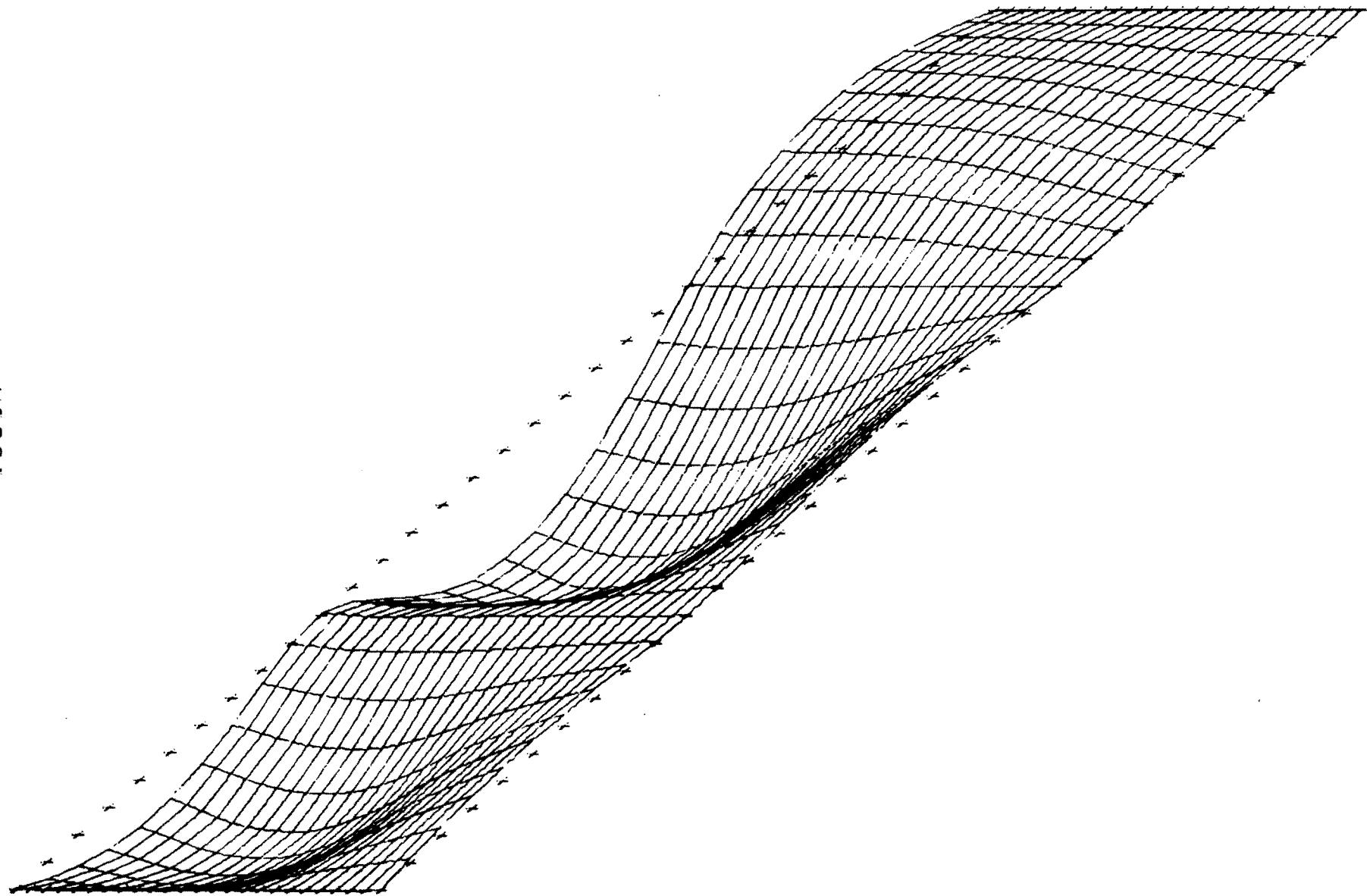


Fig 12. Three-dimensional plot of deflections, Example Problem 1003.

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CHAPTER 4. EXAMPLE PROBLEMS

Application of the SLAB 49 program to actual bridge structures is demonstrated by a series of problems which present some of the steps required for the analysis of a typical highway bridge structure. The structure is similar to those currently being designed by the Texas Highway Department and is shown in Fig 13. It consists of a concrete deck resting on a system of longitudinal main beams which are continuous over two intermediate supports with transverse diaphragms framing between the beams.

The structural system is analyzed for

- (1) the dead load of beams and concrete deck (PROB 1001).
- (2) the maximum positive moment in the center span of the main beams due to HS20 truck loading (PROB 1002).
- (3) the maximum negative moment due to HS20 lane loading (PROB 1003).
- (4) the maximum negative moment in an interior beam due to two HS20 truck loadings in adjacent lanes of two spans (PROB 1004).

The structure geometry as given in Fig 13 must be slightly modified for application of the SLAB 49 program. Slight changes in dimensions are required so the actual geometry may be fitted to suitable increment lengths in both directions. The number of increments and increment lengths in the transverse X-direction is primarily governed by the spacing of the main beams. The 7.25-foot beam spacing is subdivided into five equal increment lengths h_x of 1.45 feet. The concrete deck is extended two increments (2.9 feet) transversely over the outer beams whereas the actual deck overhang is 3.125 feet. The effect of the 0.225-foot excess can be included by using proportionate stiffness and load input data for the exterior stations. If an integral parapet or curb were present, this would also be included by adding its contributory stiffness to the exterior stations (Ref 12).

The selection of the increment length in the Y-direction is influenced by the diaphragm spacing and span lengths. The interior diaphragms are spaced uniformly at 22 feet whereas the end spacing is 24.646 feet. The main beams have a total length of 160 feet and rest on four supports with spans as indicated in Fig 13. The Y-direction increment length was adjusted to $h_y = 5$ feet by resetting the exterior span lengths to 50 feet and relocating the diaphragms at alternating 25-foot and 20-foot intervals. The schematic

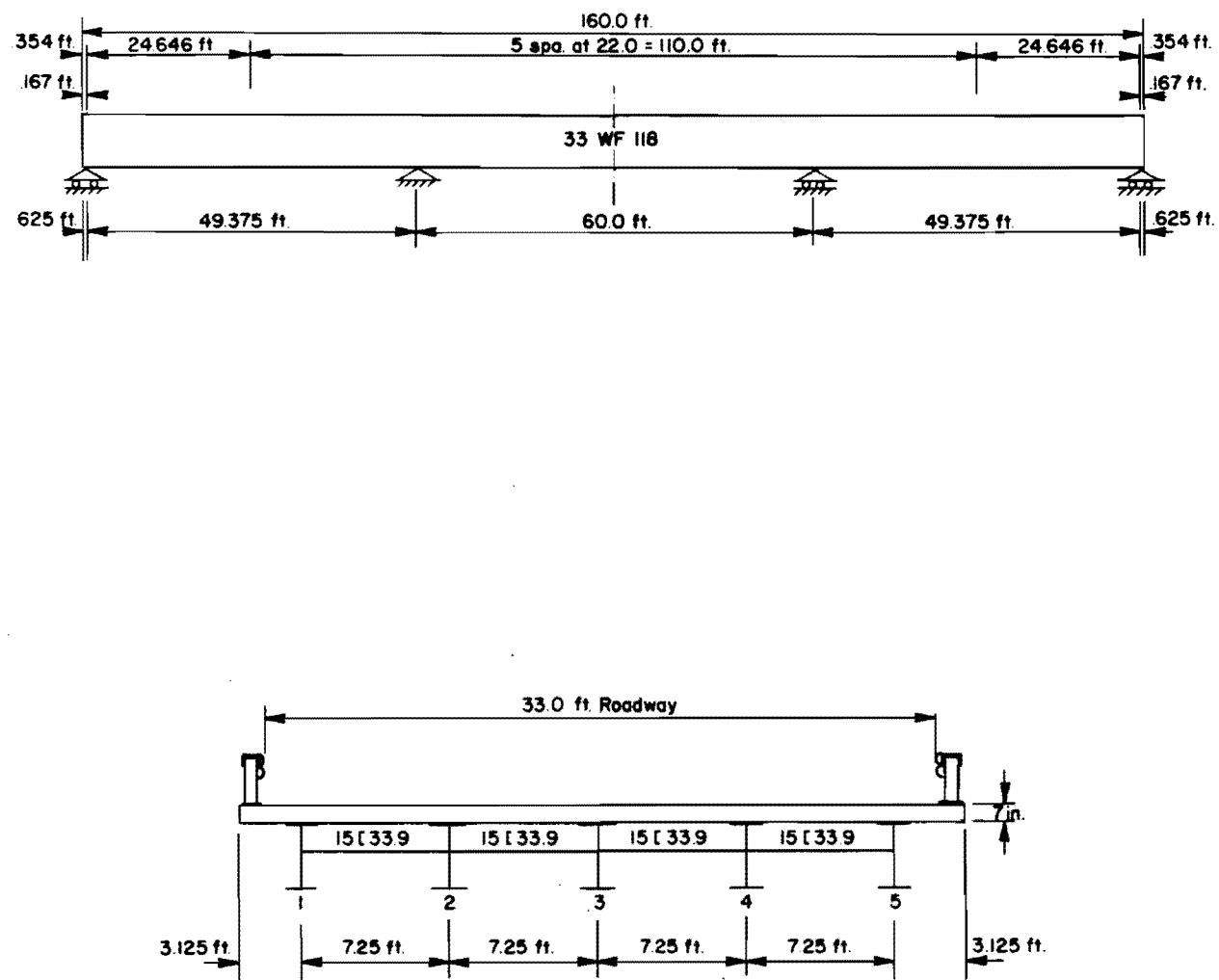


Fig 13. Three-span highway bridge.

plan of the structure as modeled is shown in Fig 14. The interior diaphragms are not offset from their original locations by more than 2 feet, which will not appreciably affect the final distribution of loads among adjacent beams. The exterior span lengths were increased by only 1 percent, which is also a negligible effect.

Stiffnesses are computed for the main beams, which are 33 WF 118, and the transverse diaphragms as well as for the 7-inch slab. Note that the end diaphragms are 14 WF 34 sections with cut bottom flanges whereas interior ones are 15 [33.9 sections. The slab bending and twisting stiffnesses are computed by the conventional formulas $D = \frac{Et^3}{12(1 - \nu^2)}$ and $C_t = \frac{Et^3}{12(1 + \nu)}$

with $\nu = 0.15$ and $E = 3 \times 10^3$ kips per sq. in. All stiffness values are given in Fig 14.

Supports are specified as large springs to effectively restrain the beams vertically. If a support were assumed to settle a given amount, the imposed settlement can be set by applying an appropriate, large downward load at the location of the large support spring. For instance, to set a 0.1-foot displacement, a load of -1×10^{14} kips is placed over the support, which has a fictitious large modulus of 1×10^{15} kips per foot. Thus, the load completely overpowers any other restraint offered by the structure and is resisted directly by the support at the equilibrium deflection of -0.1 foot, which counterbalances the load. A much more realistic simulation of a support settlement problem would be to input an elastic support spring which, when subjected to the actual structure's dead load, would deform to about the same location. If the support is drilled shafts or pilings, load-settlement curves can be used directly to estimate a support modulus. The elastic shortening of the columns can also be included as part of the support spring modulus.

Problem 1001. Dead Load, Non-Composite

The given structural system is first analyzed for the dead load of the 33 WF 118 beams and the overlying 7-inch concrete deck, by considering a grid system composed of the main beams and the transverse diaphragms. The beams carry dead weight of the slab and beams as line loads. It should be noted that the dead load carried by the outer beams is somewhat less than that carried by the interior ones, because the contributory area of the slab is smaller.

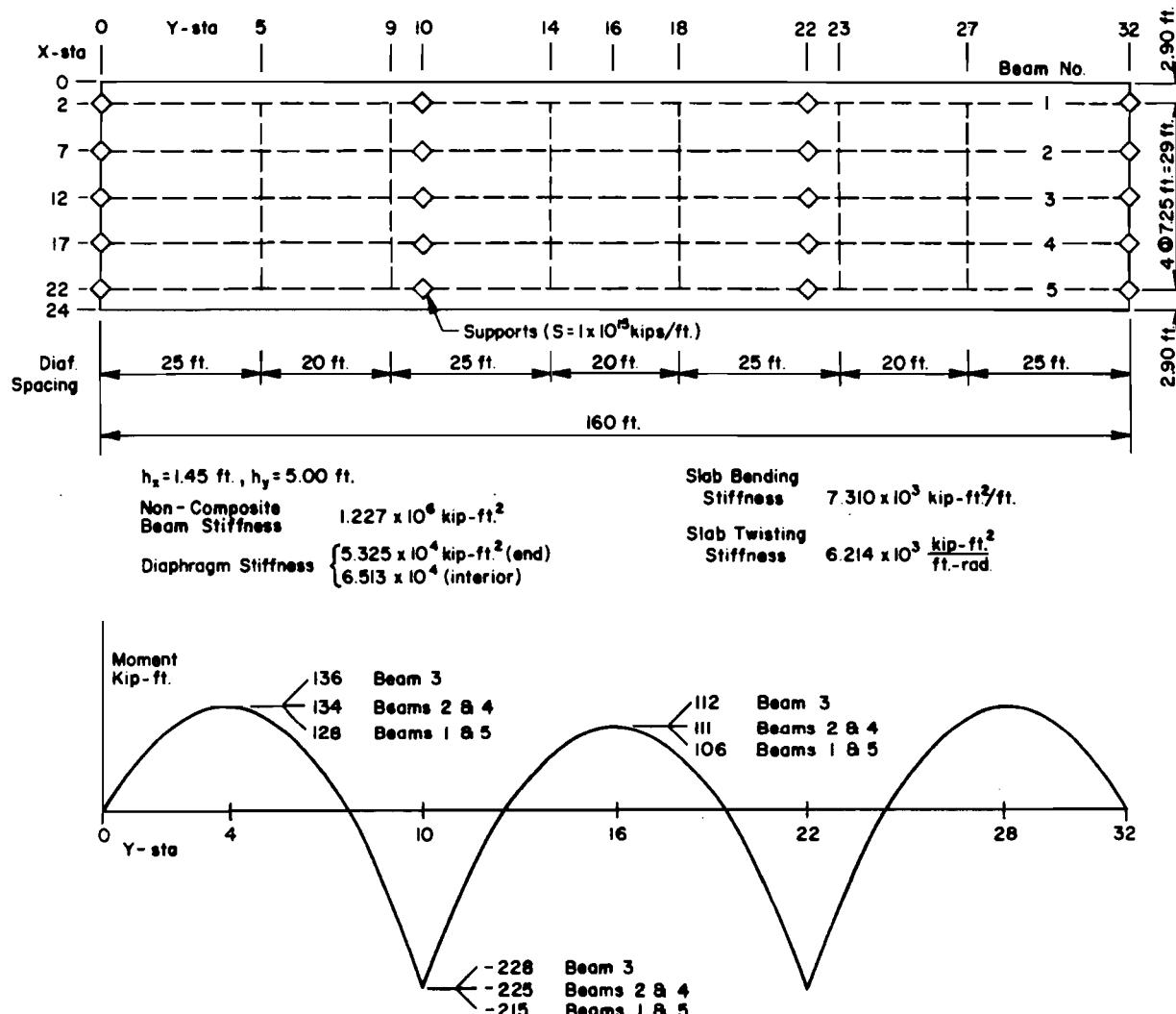


Fig 14. Problem 1001. Three-span structure as modeled, showing dead load beam moments.

Moment variations from the computed results are plotted along the main girders (Fig 14). For the center Beam No. 3, the maximum positive moments on center and exterior spans are 112 and 136 kip-feet, and the maximum negative support moment is -228 kip-feet. Note that the moment curves follow the same pattern for all the beams, with the maximum difference in ordinates not exceeding 3 percent. These moment values are in almost perfect agreement with those obtained from a conventional three-span continuous beam solution (Ref 11). Since the dead load is shared almost equally by all five beams, the diaphragms do not participate.

Problem 1002. Two HS20 Trucks, Maximum Positive Moment for Beam No. 2

Next the structural system is analyzed for the maximum positive moment in the center spans of the main beams, considering HS20 truck loadings (Ref 1). Two trucks are positioned in adjacent lanes with the center axle of the standard three-axle trucks placed at the center line of the middle span, as shown in Fig 15. The wheel loads are apportioned to adjacent stations, as depicted in Fig 15, since the wheel spacing and the lane boundaries do not directly fit the stationing and the increment lengths chosen for the analysis. This spreading or apportionment of the loads causes no appreciable error in the results for the bending moments in the main beams. Local slab results are affected and can be adjusted for actual loadings or another solution run with a fine mesh in the local slab area, considering only a portion of the slab (Ref 12). The wheel loads shown in Fig 15 are increased by an impact factor of 27.0 percent, which is computed using the length of the loaded span under consideration (Ref 1).

In addition to bending stiffnesses computed and input for the slab and diaphragms, a composite-beam stiffness is described for the main beams and the overlying slab. After the concrete deck has hardened, the beams and the slab within an appropriate effective width act compositely when subjected to positive bending. The effective slab width for composite action for this structure is taken to be the beam spacing of 7.25 feet (Ref 1). The composite stiffness is input at stations which fall within the approximate positive moment areas, which are from Y-station 12 through 20. Note that the composite stiffness along the exterior beams is less than for the interior ones, because the slab area on the overhangs is smaller.

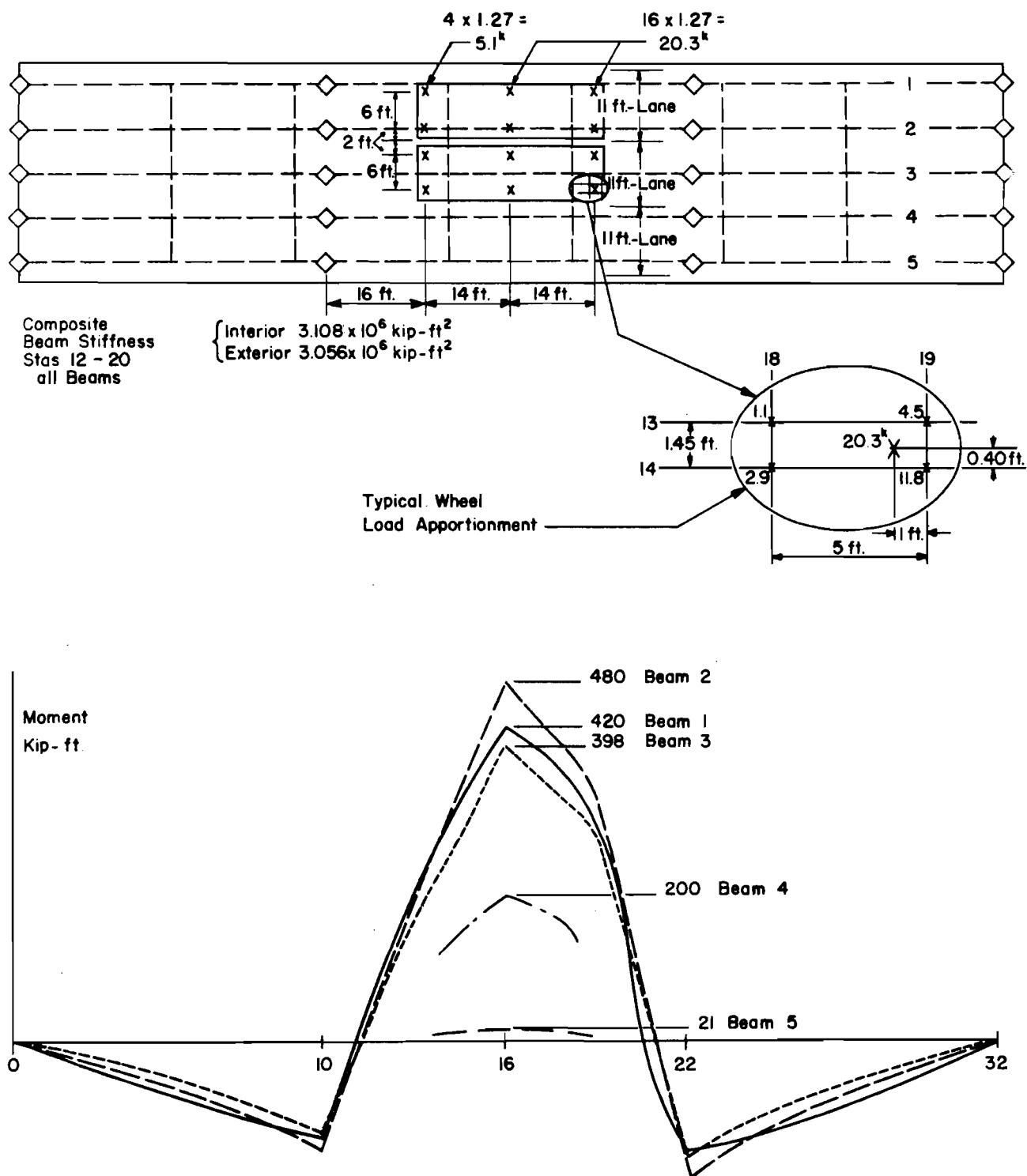


Fig 15. Problem 1002A. HS20 truck loading for maximum positive moment in center span.

The computer results of this solution are summarized by the plots of main beam moments in Fig 15. Dead load moments are not included. As might be expected from the chosen position of the loads, the maximum positive moment occurs in Beam No. 2 and is 480 kip-feet. This value was compared to the conventional line-member analysis obtained from a beam-column solution (Ref 11) using the standard AASHO distribution (Ref 1), which gave a slightly conservative moment value of 495 kip-feet. This demonstrates that the standard distribution formulas are indeed appropriate when applied to this type of structure with these spans and beam spacing.

Problem 1003. Lane Loading, Maximum Negative Moment

The system is next analyzed for the maximum negative moment in the main beams, considering an HS20 lane loading (Ref 1). Two spans are loaded with the uniform lane loading of 818 pounds per foot on two adjacent lanes, which includes an impact factor of 27.8 percent. In addition, two concentrated loads with impact of 23.0 kips are placed at the mid-spans. Both the concentrated and the uniform loads are distributed over a 10-foot lane width as shown in Fig 16.

Slab and diaphragm stiffnesses are the same as in the preceding problem except that the composite stiffnesses of the beams are now input at Y-stations along the center and first spans, which are within the approximate positive moment areas.

The computed moment variations are plotted for the first four main beams in Fig 16. The maximum value of -236 kip-feet can be compared to a conventional analysis value of -247 kip-feet, which was obtained from a beam-column solution considering the specified AASHO distribution (Refs 1 and 11).

Problem 1004. Alternate Truck Loading for Maximum Negative Moment

An alternate solution for possible maximum negative moment is illustrated by considering a two-truck HS20 loading condition in adjacent lanes of the first two spans, as shown in Fig 17.

Wheel loads of the two trucks moving opposite to each other in adjacent lanes are increased by the same impact factor, 27.8 percent, as in Prob 1003. The trucks are positioned to produce the maximum negative support moment for

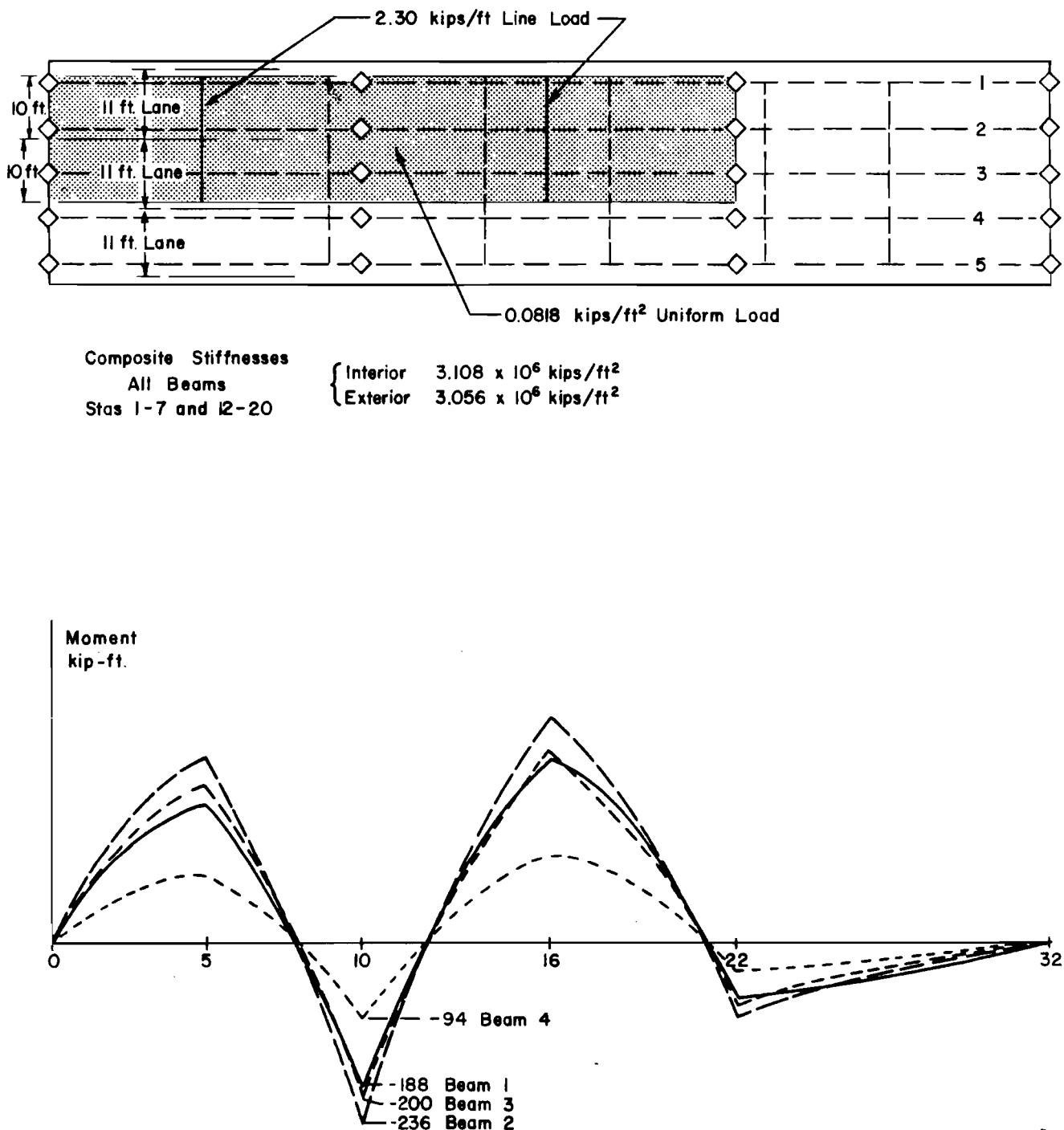


Fig 16. Problem 1003. HS20 lane loading for maximum negative moment.

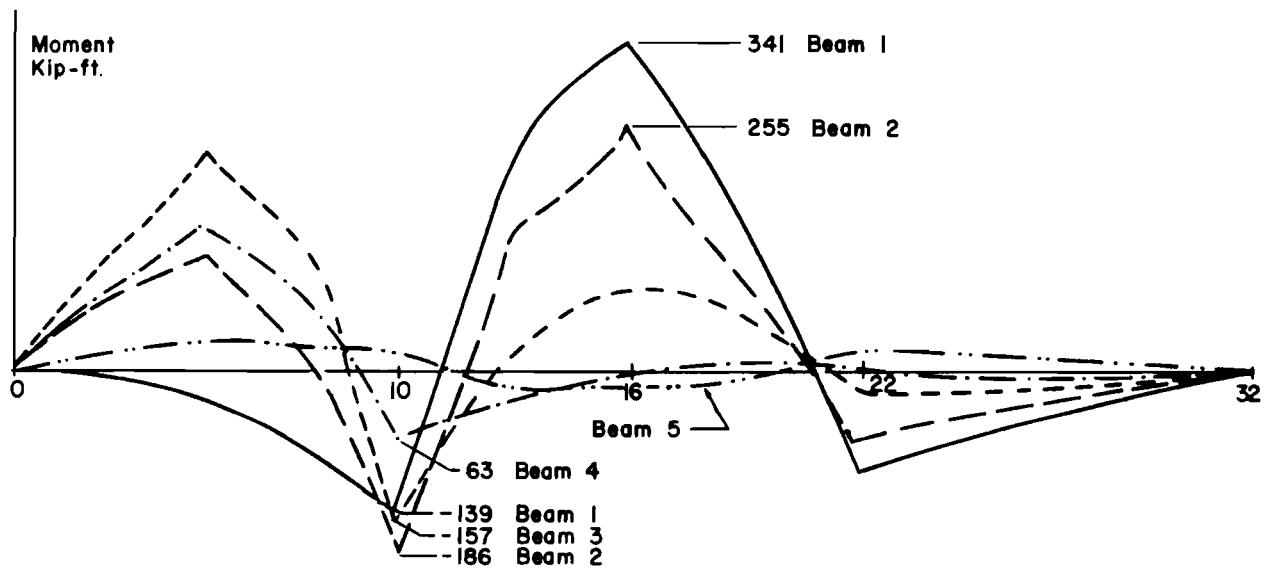
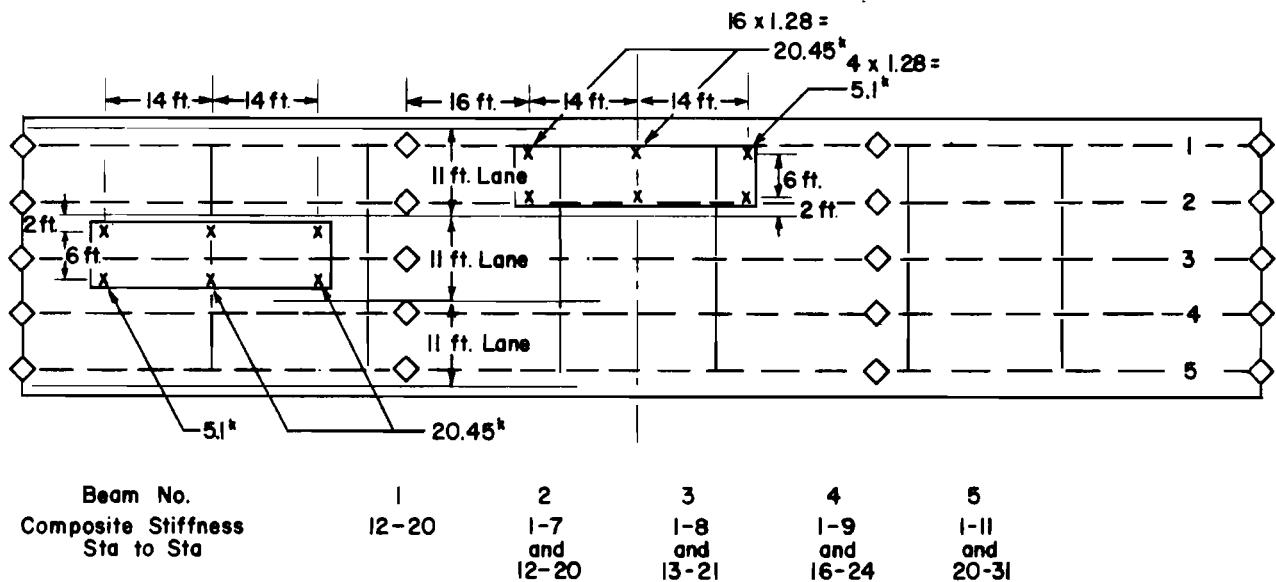


Fig 17. Problem 1004. Two HS20 trucks in adjacent lanes for maximum negative moment.

Beam No. 2. The beam composite stiffnesses are adjusted to include the stations indicated in Fig 17.

The computed moment variations in the beams are shown in Fig 17. The lane loading condition of Problem 1003 is seen to have created a larger maximum, but this arrangement of truck loadings might possibly control for other bridge configurations. In addition, it is a more realistic loading condition than the lane loading of Prob 1003 which is an approximation used for the convenience of designers (Footnote to Art 1.2.7, Ref 1).

Fig 18 shows the moment in the first interior diaphragm which has the central truck wheels of one load pattern placed over it (Fig 17). Note that even though the beams act as supports for the diaphragm, no negative moment occurs at Beam No. 3 since the adjacent beams are also deflecting. The maximum moment of 23.2 kip-feet corresponds to an approximate diaphragm stress of 6600 psi.

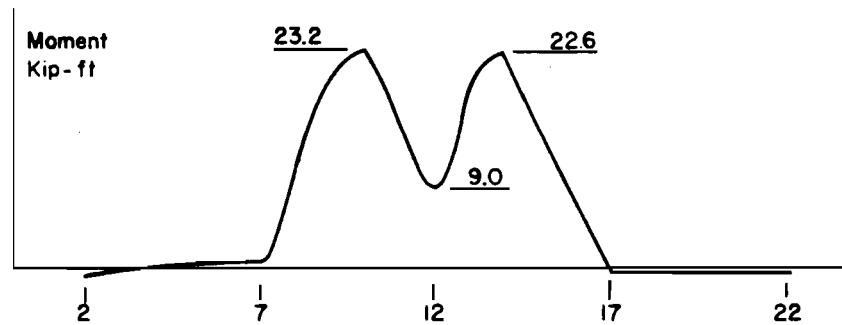
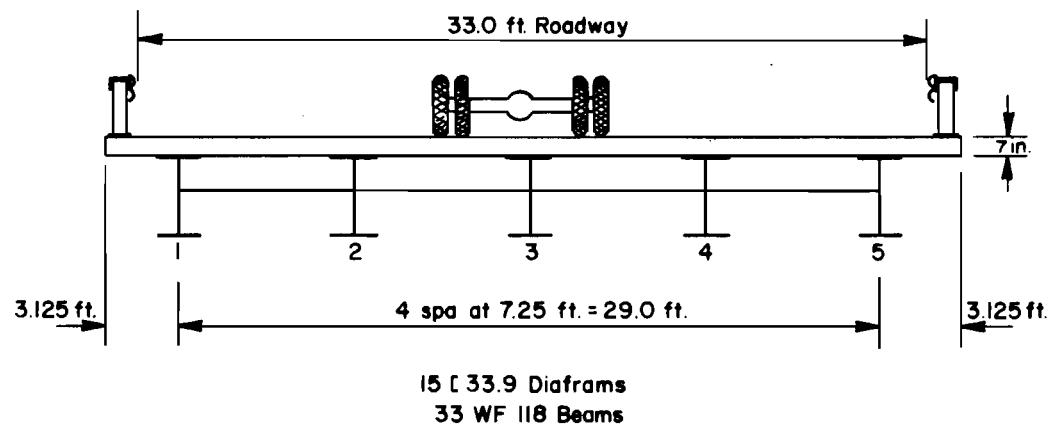


Fig 18. Problem 1004. Moment in first interior diaphragms.

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CHAPTER 5. SUMMARY AND RECOMMENDATIONS

The problem of analysis of orthogonal bridge floor systems has been presented. A computer program with documentation is included. This program could be considered the culmination of work on this research project in this area. The method is not limited to bridge floor systems, however. Pavement slabs, building slabs, stiffened and anchored bulkheads, and other two-dimensional plate or grid systems are among the structures to which the analysis can be applied.

The computer program is written and organized in a manner which will facilitate its modification and extension for specific applications. Of primary need is a series of user-oriented data-generation routines which will use this program as the basic solver. Among these would be data generation routines for constant thickness slab structures, and beam and slab systems such as presented in the included example problem.

The existing discrete-element model requires the user to know and specify the distribution of axial or in-plane thrust throughout the slab or plate. A valuable extension of this work would be the modification of the model to include axial deformations, and the development of the force-deformation equations for in-plane thrust. Not only could the axial and bending solutions be coupled for combined axial-bending analysis of plates, but the in-plane analysis could be applied to plane-stress problems. Furthermore, an in-plane solution would be required for the analysis of plates subjected to thermal gradients.

A further valuable extension of the program would be to automatically include the nonlinear stiffness properties of reinforced concrete beams. This would allow the user to make a more realistic determination of the ultimate capacity of the structure. Correlation with experimental data would be extremely useful. Carefully controlled model studies such as those currently being made in Research Study 3-5-71-158, "Diaphragm Requirements for Pre-stressed Concrete Bridges," could easily be compared to analytical solutions using the included computer program. Adjustments in estimated stiffnesses could then be made for application to prototype concrete structures.

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APPENDIX I

GUIDE FOR DATA INPUT

EX-1000000

109 110 111 112

GUIDE FOR DATA INPUT FOR SLAB 49

with supplementary notes

extract from

A DISCRETE-ELEMENT METHOD OF ANALYSIS FOR ORTHOGONAL SLAB
AND GRID BRIDGE FLOOR SYSTEMS

by

John J. Panak and Hudson Matlock

May 1972

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SLAB 49 GUIDE FOR DATA INPUT - CARD FORMS

IDENTIFICATION OF RUN (2 cards per run)

Page 1 of 9

Enter descriptive alphanumeric information -- date of run, user's name,

and the chosen units should always be included

IDENTIFICATION OF PROBLEM (1 card each problem; program stops if PROB NUM is left blank)

PROB NUM Alphanumeric problem description

	5 11
--	-----------

TABLE 1. CONTROL DATA (2 cards for each problem)

Enter "1" to KEEP prior TABLE

2	3	4	5	6	7	8	9
---	---	---	---	---	---	---	---

Multiple +1 for Parent Problems
Load -1 for Offspring Problems
Option

5	10	15	20	25	30	35	40
---	----	----	----	----	----	----	----

46	50
----	----

Enter "1" for Plot

Statics	Principal	Options
Check	Stress	Profiles 3-D

2	3	4	5	6	7	8	9
---	---	---	---	---	---	---	---

46	50	55	60	65
----	----	----	----	----

*							
---	--	--	--	--	--	--	--

**	***
----	-----

5	10	15	20	25	30	35	40
---	----	----	----	----	----	----	----

* Number of cards added must be zero if preceding table is kept or if this is an offspring problem.

** Profile plots are for areas specified by Table 8. If option is zero or blank, printer plot is made; if 1, microfilm; if 2, printer and microfilm; if 3, paper. The IBM 360 program version has only option zero.

*** Enter 1 to obtain exaggerated isometric (three-dimensional paper plot) display of deflections.

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TABLE 2. CONSTANTS (one card, none if Table 2 of preceding problem is kept)

Page 2 of 9

Num X	Incr Y	Increment Length in X-Direction		Length in Y-Direction		Poisson's Ratio ν	Slab Thickness t	
		h_x	h_y	21	30	40	50	60
		5	10					

TABLE 3. JOINT BENDING STIFFNESSES, LOADS, AND SUPPORTS (Maximum of 300 cards including those kept)

From X	Through Y	Slab Bending Stiffness D^x	Beam Bending Stiffness F^x	Load Q	Spring S						
X	Y	X	Y								
		5	10	15	20	30	40	50	60	70	80

TABLE 4. JOINT RESTRAINTS AND APPLIED MOMENTS (Maximum of 50 cards including those kept)

From X	Through Y	Rotational Restraint R^x		Applied Moment T^x	T^y					
X	Y	X	Y							
		5	10	15	20	41	50	60	70	80

TABLE 5. MESH TWISTING STIFFNESSES (Maximum of 100 cards including those kept)

From X	Through Y	Twisting Stiffness C^t				
X	Y	X	Y			
		5	10	15	20	30

TABLE 6. BAR AXIAL THRUSTS (Maximum of 50 cards including those kept)

From X	Through Y	Slab Axial Thrust P^x		Beam Axial Thrust P^y						
X	Y	X	Y							
		5	10	15	20	41	50	60	70	80

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TABLE 7. MULTIPLE LOADS (Maximum of 100 cards including those kept)

Page 3 of 9

From Through				Load	
X	Y	X	Y	Q	
5	10	15	20	61	70

TABLE 8. PROFILE OUTPUT AREAS (Maximum of 10 cards, including those kept)

From Through				Princ Mom or Stress			* Enter "1" for slab moments, "2" for beam moments	
X	Y	X	Y	Defl	X-Mom	Y-Mom		
5	10	15	20	25	30	35	*	*

TABLE 9. PRINTED OUTPUT LIMITS (Maximum of 10 cards including those kept)

From Through		If this table is omitted, all results will be printed. Each Y-bounded area specified includes the complete X-width.	
Y	Y		
6	10	16	20

TERMINATION OF RUN (one blank PROB NUM card)

5	Alphanumeric information may be punched here if desired such as "END OF DATA".
---	--

GENERAL PROGRAM NOTES

The data cards must be assembled in proper order for the program to run.

A consistent system of units must be used for all input data, for example, kips and feet.

All 2 to 5-space words are understood to be right-justified integers or whole decimal numbers. . . + 4 3 2 1

All 10-space words are floating-point decimal numbers + 4 . 3 2 1 E + 0 3

Any number of problems may be run together.

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TABLE 1. CONTROL DATA

Page 4 of 9

If the KEEP option for Table 2 is set equal to 1, there must be no card input for that table.

For Tables 3 through 9, any data from prior problems may be retained in card image storage by the KEEP options. The number of cards input for each table is independent of the KEEP options, except that the cumulative total of cards cannot exceed the specified amount for each table.

Card counts for Tables 3 through 9 should be carefully rechecked after coding is completed.

The multiple-load option is exercised for problem series in which only the load positions and magnitudes will vary. This is done by input of new loads in Table 7. Tables 2 through 6 must be held and no cards may be added to them. The first problem in a series is the Parent and is specified by entering +1; successive loadings are the Offspring and are specified by entering -1. If the option is left blank or zero, the problem is complete within itself. Tables 8 and 9 may be used as desired for all problems.

The options for Statics Check or Principal Stress may be exercised by entering 1. If the Principal Stress is to be used, then a slab or plate thickness must be available in Table 2. The option is useful only if the real slab is of uniform thickness with no discontinuities. The output value of principal stress has the same sign as the principal moment from which it is computed.

Profile plots may be designated for areas specified by Table 8. If the option is left blank or set equal to zero, the printer creates profile plots along with tabulated values; if the option is equal to 1, the plot is made on microfilm with no tabulation; if the option is set equal to 2, the combination of 0 and 1 is obtained; if the option is set equal to 3, only paper plots are made. The IBM version of the program has only option zero which yields printer plots.

A pseudo three-dimensional paper plot of the entire set of deflections may be obtained by exercising the 3-D option (IBM and CDC versions).

TABLE 2. CONSTANTS

Variables: h_x , h_y v t

Typical Input Units: in. none in.

This table is omitted for Offspring problems

Poisson's ratio will be taken as zero unless specified (always positive). It is not needed when running grid-beam type problems since no Poisson's effects are considered for the beam elements.

Slab or plate thickness must be entered if the Stress Option in Table 1 is used. The stress is computed directly from the value of principal moment and has the same sign.

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TABLES 3 and 4. JOINT BENDING STIFFNESSES, LOADS, SUPPORTS, RESTRAINTS, AND APPLIED MOMENTS DATA

Variables:	D^x, D^y	F^x, F^y	Q	S	R^x, R^y	T^x, T^y
Typical Input Units:	$\frac{lb \cdot in^2}{in}$	$lb \cdot in^2$	lb	$\frac{lb}{in}$	$\frac{in \cdot lb}{rad}$	in-lb

Unit stiffness values D^x and D^y for a slab or plate and concentrated stiffness values F^x and F^y for beams are input at all joints. The values are reduced proportionately for edges.

Customary relationships for isotropic slabs or plates and beams of known cross section are given here for reference:

$$D^x = D^y = \frac{Et^3}{12(1-\nu^2)} \quad C_t = \frac{Et^3}{12(1+\nu)} \quad F = EI$$

E is the Modulus of Elasticity, t, the plate or slab thickness, ν is Poisson's ratio, and I is the total beam cross section moment of inertia including composite effects if present.

Load values Q and support springs S for any joint are determined by multiplying the unit load or unit support value by the appropriate area of the real slab assigned to that joint. Hinged supports are provided by using large S values. Concentrated loads that occur between joints can be apportioned geometrically to adjacent joints.

All data are described with a coordinate system which is related to the X and Y-station numbers. To distribute data over a rectangular area, the lower left hand and the upper right hand coordinates must be specified. Figure A2 illustrates a sample data input.

To specify data at a single location, the same coordinates must be specified for both the "From" and "Through" coordinates.

The "Through" coordinates must always be equal to or numerically greater than the "From" coordinates.

The user may input values on the edges of the slab and the corners to represent the proportionate area desired as illustrated in Fig A2.

There are no restrictions on the order of cards. The values input are algebraically accumulated at each coordinate.

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TABLE 5. MESH TWISTING STIFFNESSES

Variable: C^t
 Typical Input Unit: $\frac{lb\cdot in^2}{in/rad}$

Unit twisting stiffness C^t is defined for the mesh of the plate or slab surrounded by four rigid bars and four joints. The mesh is numbered according to the joint number at the upper right corner of the mesh as shown in Fig A1.

The same general notes as listed for Tables 3 and 4 are applicable.

TABLE 6. BAR AXIAL THRUSTS

Variables: P^x, P^y \bar{P}^x, \bar{P}^y
 Typical Input Units: 1b 1b

All data in this table are concentrated. Distributed data must be summed over the width of the increment involved. Proportionate values can be used along edges.

All tension (+) or compression (-) values P^x are specified for each X-bar in the X-direction. Since it is a bar force, no coordinate should be used which would specify a P^x value in a bar outside the real plate or slab. The vars are numbered according to the joint number at the increasing station end of the bar as shown in Fig A1. P^y values are specified in the Y-direction.

The same general notes as listed for Tables 3 and 4 are applicable.

TABLE 7. MULTIPLE LOADS

Variable: Q
 Typical Input Unit: 1b

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When a problem is such that only the load changes from problem to problem, it is appropriate to enter it in this table and hold all other stiffness, load, and geometrical data of Tables 2, 3, 4, 5, and 6 from the previous problem, thus creating an Offspring problem. Any loads entered or held in Table 3 are added to the loads of Table 7.

The multiple-loading options are specified in Table 1. The greatest amount of computer time is needed for the first problem in a multiple-loading problem series and subsequent problems are then solved in a fraction of the solution time.

Each card may encompass up to a maximum of 300 points. For larger areas, additional cards may be used to the limit of 10, including those kept from previous problems.

If profile plot options in Table 1 were set to 1 or 3, no tabulated output of Table 8 areas is printed. A blank or 2 option will cause tabulated and printer display of the selected profiles.

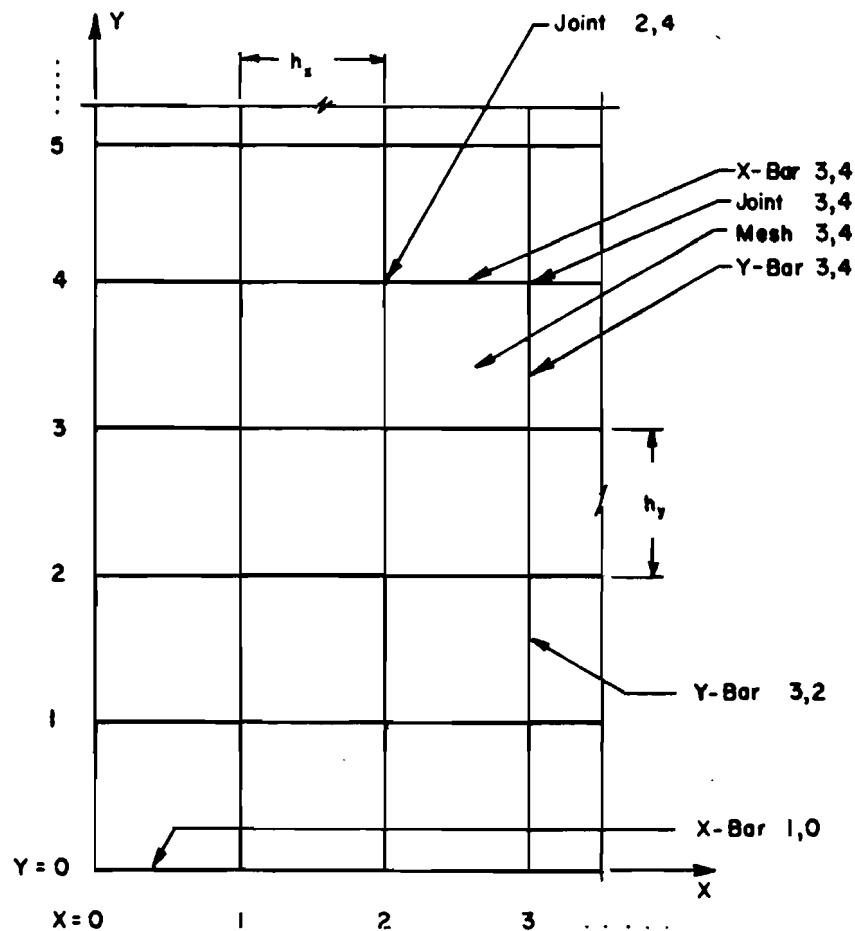
Any one or all four types of profile output may be selected by entering a 1 for those desired. Beam moments may be chosen by entering a 2. One limitation for the moment options is that all areas entered or kept from the previous problem must be either for slab or beam X and Y-moments. A mixture of slab and beam profile output within a problem is disallowed. Parent-offspring may have beam-slab options.

TABLE 9. PRINTED OUTPUT LIMITS

If this table is omitted, the complete printout of results is obtained. Partial output may be obtained by specifying the sections to be printed within the Y-bounded limits designated on each card. Up to 10 Y-bounded sections may be printed.

Y-bounded areas may overlap or be contiguous.

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Joint Data : D^x , D^y , F^x , F^y , Q , S , R^x , R^y , T^x , T^y

(D^x and D^y are per unit width, all others are concentrated values)

Mesh Data : C^\dagger

(C^\dagger is per unit width)

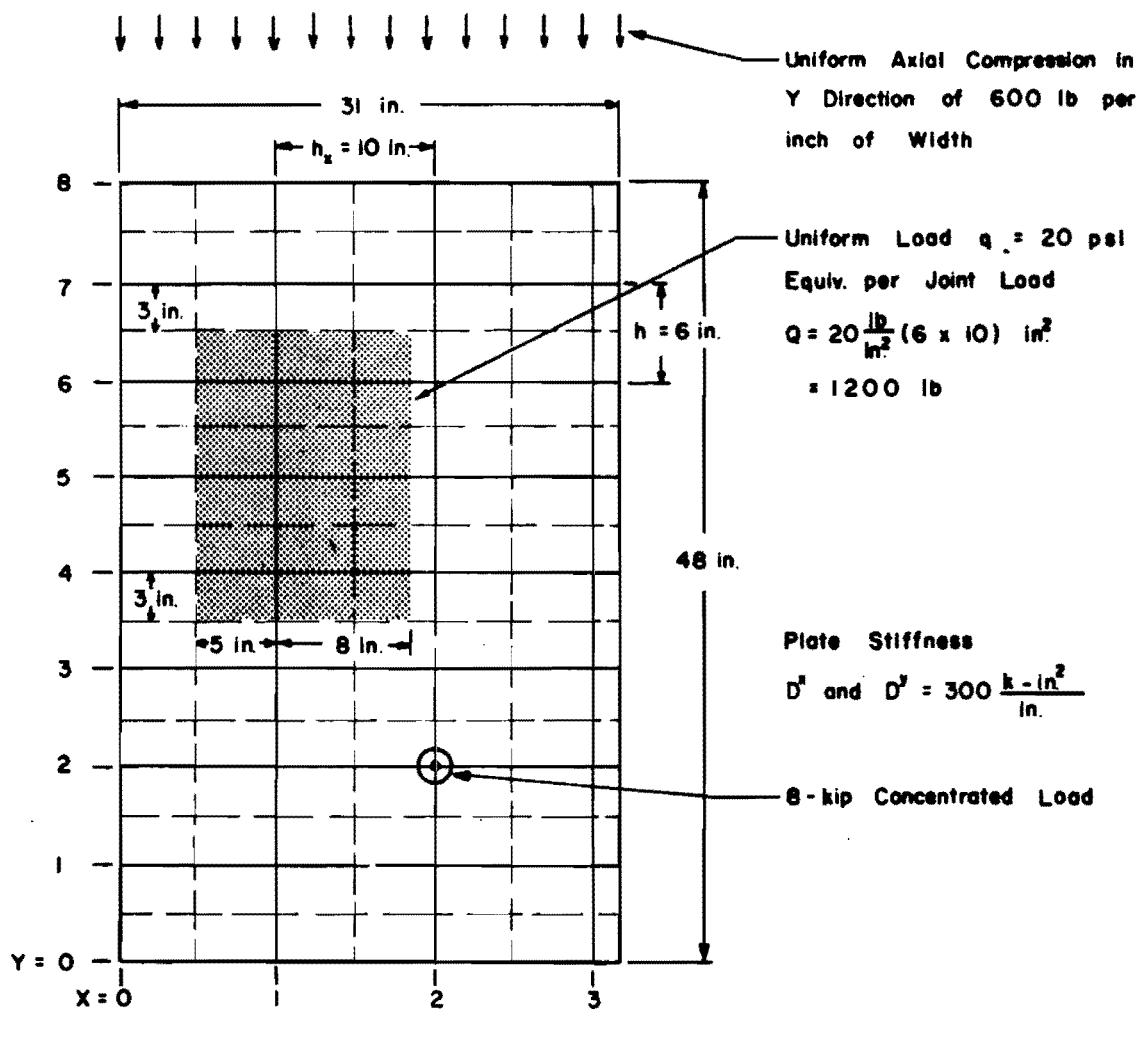
Bar Data : P^x , P^y , \bar{P}^x , \bar{P}^y

(all values are concentrated)

Fig Al. Data coordinate numbering system.

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From		Through		D^x and D^y	Q	P^y
X	Y	X	Y			
0	0	2	8	7.500E+04		
0	1	2	7	7.500E+04		
1	1	2	7	1.500E+05		
3	0	3	8	9.000E+04		
3	1	3	7	9.000E+04		
1	4	1	6		-1.200E+03	
2	4	2	6		-3.600E+02	
2	2	2	2		-8.000E+03	
0	1	0	8			-3.000E+03
1	1	2	8			-6.000E+03
3	1	3	8			-3.600E+03

data incomplete for this sample

Fig A2. Sample data input.

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APPENDIX 2

**GLOSSARY OF NOTATION FOR COMPUTER PROGRAM
SLAB 49**

STAGE 4
DEPARTMENT OF DEFENSE DRAFT SUBMITTER REPORT

C-----NOTATION FOR SLAB 49 AND 88498

```

C      AC( ,1)          CONTINUITY COEFFICIENT
C      AAC( ,1 )        COEFF IN STIFFNESS MATRIX
C      ALP               ANGLE ON MOHRS CIRCLE
C      AM1( ,1)          CONTINUITY COEFFICIENT A( ,1) AT J=1
C      AM2( ,1)          CONTINUITY COEFFICIENT A( ,1) AT J=2
C      AN1( ), AN2( )    IDENTIFICATION AND REMARKS ( ALPHA=NUM )
C      AT( ,1)           TEMP STORAGE FOR A( ,1) CONTINUITY COEFF
C      BBC( ,3)          COEFF IN STIFFNESS MATRIX
C      BMX( )            ALPHANUMERIC, = BEAM X, BEAM Y
C      BETA              HALF THETA ( COUNTER CLOCKWISE IS + )
C      BLNK              ALPHANUMERIC, = 6 BLANKS
C      BMA              AVERAGE OF X AND Y BENDING MOMENTS
C      BMX( )            BEAM BENDING MOMENT IN X DIRECTION
C      BMY( )            BEAM BENDING MOMENT IN Y DIRECTION
C      BMYH1( )          BMY AT J=1
C      BMYH1C( )         BMY AT J+1
C      BMOM              ALPHANUMERIC, = MOMENT
C      BMP              BMX MINUS BMA FOR MOHRS CIRCLE
C      BMD              FIRST PRINCIPAL BENDING MOMENT
C      BMR              RADIUS OF MOHRS CIRCLE
C      BMT              SECOND PRINCIPAL BENDING MOMENT
C      BMX( )            SLAB BENDING MOMENT IN THE X DIRECTION
C      BMY( )            SLAB BENDING MOMENT IN THE Y DIRECTION
C      BMYH( )           BMY AT J=1
C      BMYH1( )          BMY AT J+1
C      CC( ,5)           COEFP IN STIFFNESS MATRIX
C      CH( )             PLATE TWISTING STIFFNESS PER UNIT WIDTH
C      CHK              ALPHANUMERIC, = CHECK
C      CHN( )            INPUT VALUE OF CH IN TABLE S
C      CHP1( )           CH AT J+1
C      CRD( )            CROSS BENDING STIFFNESS FOR PR EFFECTS
C      DDC( ,3)          COEFP IN STIFFNESS MATRIX
C      DX( ), DY( )      SLAB BENDING STIFFNESSES PER UNIT WIDTH
C      DM1( )            DX AT J=1
C      DXP1( )           DX AT J+1
C      DXM( ), DYN( )    INPUT VALUES OF DX AND DY IN TABLE S
C      DYH( )            DY AT J=1
C      DYP1( )           DY AT J+1
C      EEC( ,1)          COEFF IN STIFFNESS MATRIX
C      FPC( ,1)          COEFP IN LOAD VECTOR
C      FX( ), FY( )      BEAM BENDING STIFFNESSES
C      FXM( ), FYN( )    INPUT VALUES OF FX AND FY IN TABLE S
C      FYM( )            FY AT J=1
C      FYP1( )           FY AT J+1
C      HX                INCREMENT LENGTH IN X DIRECTION
C      MXDHY             MX DIVIDED BY HY
C      MXDHY3             MX DIVIDED BY HY CUBED
C      HY                INCREMENT LENGTH IN Y DIRECTION
C      HYDHX             HY DIVIDED BY HX
C      HYDHX3             HY DIVIDED BY HY CUBED
C      I, II, IZ          X DIRECTION INDEXES (ISTA+2)
C      IBD0              BEAM OR SLAB MOMENT PROFILE PLOT SWITCH
C      IBDST              TEMP VALUE OF IBD0
C      IBSUP( )          PLOTTER BUFFER
C      ID                TEMP VALUE OF ID1 THRU ID4
C      ID1 THRU ID4     ALPHANUMERIC IDENTIFIERS FOR SUBROUTINE ZOT 1 CDC
C      ID1 THRU ID4     RANDOM INDEXES FOR FILES 1 THRU 4 IBM
C      IGBW              REF GRID TYPE SWITCH FOR 3-D PLOTS
C      IN13( )           -FRDH= X STATION USED IN
C      IN1B( )           TABLES 3 THRU 8
C      IN23( )           -THRU= X STATION USED IN
C      IN2B( )           TABLES 3 THRU 8
C      IOPFB             INPUT OPTION TO PRINT PRINCIPAL STRESS
C      IPOP              INPUT PLOT OPTION

```

```

C      IPRINT            PRINTING CONTROL AT EACH J STEP
C      IROLL             OPTION TO MOVE TO A NEW PLOT FRAME
C      ISTA              X STATION
C      ISB               B'NITCH DEPENDANT ON END VALUES OF J
C      ITEMP              ISTA AT MAX STATICS CHECK ERROR
C      ITEST              ALPHANUMERIC BLANKS USED TO TERMINATE
C      ITMP              TEMP PRINT B'NITCH FOR DEBUGGING
C      I3D               INPUT OPTION FOR 3D PLOT OF DEFLECTIONS
C      I4D               SWITC'H USED TO TERMINATE PLOT FILE
C      J, J1, J2          Y DIRECTION INDEXES (JSTA+2)
C      JN13( )           ACTUAL Y STA, EQUALS J=2
C      JN1C( )           -FROM= Y STATION USED IN
C      JN2( )             TABLES 3 THRU 9
C      JN2C( )           -THRU= Y STATION USED IN
C      JN29( )           TABLES 3 THRU 9
C      JSTA              Y STATION
C      JTEHP             JSTA AT MAX STATICS CHECK ERROR
C      K                 TEMY OR TEMY INDEX FOR SPLOTS OR SPLDT4
C      KASEP              TABLE 8 PRINCIPAL MOMENT OR STRESS OPTION
C      KASEW              TABLE 8 DEPICTION OPTION
C      KABEX, KABEY       KEEP2 THRU KEEP9
C      KLONG              KEEP OPTIONS FOR DATA TABLES 2 THRU 9
C      KML               MAXIMUM NUMBER OF Y STAS
C      KSHORT             MULTIPLE LOAD OPTION IN PRIOR PROBLEM
C      KSHORT3             MAXIMUM NUMBER OF X STAS
C      KPROB              PROBLEM NUMBER FROM PARENT PROBLEM
C      KROPT              INPUT OPTION TO PRINT STATIC CHECK
C      LL                DO LOOP INDEX FOR REVERSED J
C      LOP               LINE OR POINT PLOT OPTION
C      L1                KSHORT+3, USED FOR VARIABLE DIMENSIONING
C      L2                KLONG+3, USED FOR VARIABLE DIMENSIONING
C      MC                NUMBER OF CHARACTERS IN PLOT TITLE
C      ML                MULTIPLE LOADING SWITCH
C      MDP               MICROFILM (#0) OR PAPER (#1) PLOT OPTION
C      MX                NUMBER OF INCREMENTS IN X DIRECTION
C      MX1 THRU MX5      MX+1 THRU MX5
C      MY                NUMBER OF INCREMENTS IN Y DIRECTION
C      MY1 THRU MY5      MY+1 THRU MY5
C      NC02 THRU NC09    INDEX FOR READING CARDS
C      NC13 THRU NC19    NUM CARDS IN TABLES 2 THRU 9, THIS PROB
C      NC13 THRU NC19    TOTAL NUMBER OF CARDS IN TABLES 3 THRU 9
C      NC13 THRU NC19    STARTING VALUE FOR READING CARDS,
C      NC13 THRU NC19    TABLES 3 THRU 9
C      NDE8              TOTAL NUMBER OF DATA ERRORS
C      NDE1 THRU NDE9    NUMBER OF DATA ERRORS IN TABLES 1 THRU 9
C      NEND              MAX NUMBER OF POINTS, EACH TABLE 8 CARD
C      NPROB             INITIAL COUNTER, SOLUTION ROUTINES (=1)
C      NPROB             PROBLEM NUMBER (PROG STOPS IF BLANK)
C      NEND + 2           ONE DIVIDED BY MX
C      NEND + 2           ONE DIVIDED BY MX TIMES HY
C      NEND + 2           ONE DIVIDED BY HY SQUARED
C      NEND + 2           ONE DIVIDED BY MX CUBED
C      NEND + 2           ONE DIVIDED BY HY
C      NEND + 2           ONE DIVIDED BY HY SQUARED
C      NEND + 2           ONE DIVIDED BY MX CUBED
C      PBX( , )          PLOTTING ARRAY FOR BMX OR BMX
C      PBMY( , )          PLOTTING ARRAY FOR BMY OR BMY
C      PBX( )             AXIAL THRUST IN BEAM IN X DIRECTION
C      PBXN( ), PSYN( )  INPUT VALUES OF PBX AND PBY IN TABLE 6
C      PBYC( )            AXIAL THRUST IN BEAM IN Y DIRECTION
C      PBYP1( )           PBXHY
C      PBMAX              POISSONS RATIO DIVIDED BY HX TIMES HY
C      PR                LARGEST PRINCIPAL MOMENT (+ OR -)
C      PSIG0( , )          POISSONS RATIO
C      PWC( , )           PLOTTING ARRAY FOR SIG0
C      PWC( , )           PLOTTING ARRAY FOR W

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C      PX( )          AXIAL THRUST IN SLAB IN X DIRECTION
C      PXN( ), PYN( ) INPUT VALUES OF PX AND PY IN TABLE 6
C      PY( )          AXIAL THRUST IN SLAB IN Y DIRECTION
C      PYPI( )        PY AT J+1
C      Q( )          APPLIED LOAD PER JOINT
C      QBMX , QBMY    LOAD ABSORBED IN BEAM BENDING
C      QBMX , QBMY    LOAD ABSORBED IN SLAB BENDING
C      QM( )          APPLIED MULTIPLE LOAD PER JOINT
C      QMN( )        INPUT VALUES OF LOAD IN TABLE 7
C      QMX , QPY     INPUT VALUE OF TRANSVERSE LOAD IN TABLE 3
C      QPX , QPY     LOAD ABSORBED DUE TO BEAM AXIAL THRUSTS
C      QRX , QRY     LOAD ABSORBED DUE TO SLAB AXIAL THRUSTS
C      QTMX , QTHY   LOAD ABSORBED DUE TO ROTATIONAL RESTRAINTS
C      QTX , QTY     LOAD ABSORBED IN TWISTING
C      RCT           LOAD ABSORBED DUE TO APPLIED COUPLES
C      RDP           ALPHANUMERIC, = REACTION
C      REACT         REDUCTION FACTOR IN Y DIRECTION, 3D PLOT
C      ROCK          SUPPORT REACTION PER JOINT
C      RDC1 , RDC2    REACTION OR STATIC CHECK
C      RX( )          FORMAT CONTROLS (BASED ON KROPT)
C      RXN( ), RYN( ) ROTATIONAL RESTRAINT IN X DIRECTION
C      RYM( )          INPUT VALUES OF RX AND RY IN TABLE 4
C      RYP1( )        ROTATIONAL RESTRAINT IN Y DIRECTION AT J-1
C      RYP1( )        ROTATIONAL RESTRAINT IN Y DIRECTION AT J+1
C      RIC( ), THRU   RECURSION COEFFICIENTS AND MULTIPLIERS
C      RSC( )        USED IN SOLUTION PACKAGE
C      S( )          SPRING SUPPORT, VALUE PER JOINT
C      S0T2          MOMENT MULTIPLIER FOR PLATE STRESS
C      S1G0          LARGEST PRINCIPAL MOMENT OR STRESS
C      SBLX, SLY     ALPHANUMERIC, = SLAB X, SLAB Y
C      SLOPE         TAN OF GRID ANGLE, 3D PLOT
C      SNC( )        INPUT VALUES OF SUPPORT SPRINGS IN TABLE 3
C      S0M           STRESS OR MOMENT FORMAT CONTROL
C      STACH         STATIC CHECK
C      STAT          ALPHANUMERIC, = STATIC
C      STEMP         MAXIMUM VALUE OF STACH
C      STRS          ALPHANUMERIC, = STRESS
C      SUMR          SUMMATION OF SUPPORT REACTIONS
C      SUP           ALPHANUMERIC, = SUPPORT
C      SWS           SWITCH TO COMPUTE AND PRINT BEAM OUTPUT
C      SWB           SWITCH TO COMPUTE AND PRINT SLAB OUTPUT
C      TEMX( ), TEMY( ) TEMPORARY PLOT ARRAY FOR SUBROUTINE ZOT 1
C      THETA         HOURS CIRCLE ANGLE BETWEEN X DIRECTION
C                  AND PRINCIPAL MOMENT
C      THK           THICKNESS OF SLAB FOR STRESS CALCULATIONS
C      TMX           TWISTING MOMENT IN X DIRECTION (ABDUT Y)
C      TMY           NEGATIVE OF TMX
C      TX( )          APPLIED COUPLE IN X DIRECTION
C      TXN( ), TYN( ) INPUT VALUES OF TX AND TY IN TABLE 4
C      TYM( )          APPLIED COUPLE IN Y DIRECTION AT J-1
C      TYP1( )        APPLIED COUPLE IN Y DIRECTION AT J+1
C      U              MAKES FILES 1 THRU 4 RANDOM INDEXED WITH IBM
C                  VARIABLE LENGTH RECORDS IBM
C      VEF           VERTICAL EXPANSION FACTOR, 3D PLDT
C      W( )          DEFLLECTION AT EACH JOINT
C      WM1( )        W AT J=1
C      WM2( )        W AT J=2
C      WP1( )        W AT J+1
C      WP2( )        W AT J+2
C      WSUM1         WSUM1 AT J=1
C      WSUM2         WSUM2 AT J=2
C      WSUM1 , WSUM2 2ND DIFF OF DEFLECTIONS FOR MOMENTS
C      WSUM3         2ND DIFF OF DEFLECTIONS FOR TWIST
C      XX( )          INDEPENDENT VARIABLE FOR SUBROUTINE ZOT 1

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C-----NOTATION FOR SUBROUTINE DATA

```

C      AN1( ), AN2( ) IDENTIFICATION AND REMARKS ( ALPHA-NUM )
C      CH( )          PLATE TWISTING STIFFNESS PER UNIT WIDTH
C      CHN( )        INPUT VALUE OF CH IN TABLE 5
C      CHP1( )        CH AT J+1
C      DX( ), DY( )  SLAB BENDING STIFFNESSES PER UNIT WIDTH
C      DXM1( )        DX AT J-1
C      DXN( ), DYN( ) INPUT VALUES OF DX AND DY IN TABLE 3
C      OXP1( )        DX AT J+1
C      OYH1( )        DY AT J-1
C      DYP1( )        DY AT J+1
C      FX( ), FY( )  BEAM BENDING STIFFNESSES
C      FXN( ), FYN( ) INPUT VALUES OF FX AND FY IN TABLE 3
C      FYH1( )        FY AT J-1
C      FYP1( )        FY AT J+1
C      I , II , I2   X DIRECTION INDEXES (ISTA+2)
C      IN13( ) THRU  -FROM= X STATION USED IN
C      IN18( )          TABLES 3 THRU 8
C      IN23( ) THRU  -THRU= X STATION USED IN
C      IN28( )          TABLES 3 THRU 8
C      IPRINT         PRINTING CONTROL AT EACH J STEP
C      JN             ACTUAL Y STA, EQUALS J-2
C      JN13( ) THRU  -FROM= Y STATION USED IN
C      JN18( )          TABLES 3 THRU 8
C      JN23( ) THRU  -THRU= Y STATION USED IN
C      JN28( )          TABLES 3 THRU 8
C      KABEP          TABLE 8 PRINCIPAL MOMENT OR STRESS OPTION
C      KABEW          TABLE 8 DEFLECTION OPTION
C      KASEX , KASEY  TABLE 8 BENDING MOMENT OPTIONS
C      LI              KSHORT=3, USED FOR VARIABLE DIMENSIONING
C      ML              MULTIPLE LOADING SWITCH
C      MXP3            MX + 3
C      N              INDEX FOR READING CARDS
C      NC13 THRU    STARTING VALUE FOR READING CARDS,
C                  TABLES 3 THRU 9
C      NC19          TOTAL NUMBER OF CARDS IN TABLES 3 THRU 9
C      NCT3 THRU NCT9 NUMBER OF DATA ERRORS IN TABLES 1 THRU 9
C      NOEI THRU NDE9 AXIAL THRUST IN BEAM IN X DIRECTION
C      PSX( )        INPUT VALUES PBX AND PBY IN TABLE 6
C      PSXN( ), PYN( ) AXIAL THRUST IN BEAM IN Y DIRECTION
C      PSY( )        PBY AT J+1
C      PBYPI( )      AXIAL THRUST IN SLAB IN X DIRECTION
C      PX( )          INPUT VALUES OF PX AND PY IN TABLE 6
C      PXN( ), PYN( ) AXIAL THRUST IN SLAB IN Y DIRECTION
C      PY( )          PY AT J+1
C      PYPI( )        APPLIED LOAD PER JOINT
C      Q( )          APPLIED MULTIPLE LOAD PER JOINT
C      QM( )          INPUT VALUES OF LDAO IN TABLE 7
C      QMN( )        INPUT VALUE OF TRANSVERSE LOAD IN TABLE 3
C      QMX           ROTATIONAL RESTRAINT IN X DIRECTION
C      QPY           INPUT VALUES OF RX AND RY IN TABLE 4
C      QTMX          ROTATIONAL RESTRAINT IN Y DIRECTION AT J-1
C      QTHY           ROTATIONAL RESTRAINT IN Y DIRECTION AT J+1
C      QTX           SPRING SUPPORT, VALUE PER JOINT
C      QTY           INPUT VALUES OF SUPPORT SPRINGS IN TABLE 3
C      RCP1( )        APPLIED COUPLE IN X DIRECTION
C      TXN( ), TYN( ) INPUT VALUES OF TX AND TY IN TABLE 4
C      TYM( )          APPLIED COUPLE IN Y DIRECTION
C      TYP1( )        APPLIED COUPLE IN Y DIRECTION AT J+1

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C-----NOTATION FOR SUBROUTINES SPLOTS AND SPLOTS

```

C      BETA      TRANSFORMED VALUE OF PLOTTING ARRAY
C      I , K , L  DD LOOP INDICES
C      I8        GENERAL INDEX
C      IOTA     FIXED VALUE OF BETA
C      IR        REMAINDER
C      IS        INDEX OVER X
C      ISKP1 , ISKP2, FOUR-CHARACTER BLOCKS USED
C                  FOR SPACING
C      ISITA    X STATION
C      IW        WIDTH OF PLOT ( CHARACTERS )
C      IX        IW DIVIDED BY 4
C      II        IN18+2
C      IZ        IN28+2
C      J8        INDEX OVER Y
C      JSTA    Y STATION
C      JI        JN18+2
C      JS        JN28+2
C      NEND    LENGTH OF ARRAY TO BE PLOTTED
C      OMESA   MAXIMUM VALUE IN PLOT ARRAY
C      O2        OMEGA FOR SECOND PLOT
C      SIGMA   SCALE FACTOR
C      SPACE   EMPTY DATA BLOCK FOR SKIPPING CHARACTERS
C      SYMS    ARRAY OF FOUR LAST BLOCKS TO BE PLOTTED
C      SYMD    LAST BLOCK TO BE PLOTTED
C      SYMD2   SYMD FOR SECOND PLOT
C      S2        SIGNA FOR SECOND PLOT
C      TTHETA  MINIMUM VALUE IN PLOT ARRAY
C      T2        THETA FOR SECOND PLOT
C      WIDTHH  FLOATED VALUE OF IW
C      X( )    ARRAY TO BE PLOTTED
C      Y( )    SECOND ARRAY TO BE PLOTTED

```

C-----NOTATION FOR SUBROUTINE DIM3

```

C      H        HEIGHT OF PLOT
C      HX       INCREMENT LENGTH IN X DIRECTION
C      HY       INCREMENT LENGTH IN Y DIRECTION
C      I        GENERAL INDEX
C      IDEL    DISTANCE BETWEEN POINTS IN X DIRECTION
C      IGBW   REF GRID TYPE SWITCH FOR 3-D PLOTS
C      IN        NUMBER OF Y PARTITIONS
C      INIT    INITIALIZATION SWITCH
C      IOFX   OFFSET IN X DIRECTION
C      IOFY   OFFSET IN Y DIRECTION
C      IP0P    INPUT PLOT OPTION
C      IZ        THE MASK
C      J        GENERAL INDEX
C      JS        ROTATION SWITCH
C      KN        NUMBER OF X PARTITIONS
C      L1        KBHDT+3, USED FOR VARIABLE DIMENSIONING
C      L2        XLONG+3, USED FOR VARIABLE DIMENSIONING
C      MX        NUMBER OF INCREMENTS IN X DIRECTION
C      MXP2   MX+2
C      MY        NUMBER OF INCREMENTS IN Y DIRECTION
C      MYP2   MY+2
C      NPROB   PROBLEM NUMBER (PROG STOPS IF BLANK)
C      PLTHIN  MINIMUM Y COORD OF MASK
C      RDF     REDUCTION FACTOR IN Y DIRECTION, 3D PLOT
C      SLOPE   TAN OF GRID ANGLE, 3D PLOT
C      START  INITIALIZATION VARIABLE FOR PLOT FILE
C      VEF    VERTICAL EXPANSION FACTOR
C      VSF    VERTICAL SCALE FACTOR, 3D PLOT
C      W( )    WIDTH OF PLOT
C      X0      X COORD OF RELATIVE ORIGIN FOR GRID 1
C      XX      TEMP DISTANCE ALONG SLOPE
C      Y0      Y COORD OF RELATIVE ORIGIN FOR GRID 1
C      Z        FUNCTION TO BE PLOTTED

```

C-----NOTATION FOR SUBROUTINE ZOT1

```

C      IO        VARIABLE OR ARRAY CONTAINING TITLE OF PLOT
C      INC      STEP SIZE FOR SCANNING PLOT ARRAY
C      I0        PLOTTING SYMBOL SELECTOR
C      IROLL   PLOT SWITCH FOR FRAME POSITION CONTROL
C      IT1 , IT2  SWITCHES FOR POSITIONING PLOTTER
C      LOP      LINE OR POINT PLOT OPTION
C      NC        NUMBER OF CHARACTERS IN PLOT TABLES
C      NOP      MICROFILM OR PAPER PLOT OPTION
C      NC        TEMPORARY FOR NC
C      NP        NUMBER OF POINTS TO BE PLOTTED
C      X        X AXIS POSITIONER
C      XF        ARRAY CONTAINING THE X - COORDINATES
C      XF1 , XF2  TEMPORARIES FOR XF
C      XL        X AXIS LENGTH IN INCHES
C      XH        X AXIS POSITION
C      XQ        ORIENTATION AT X AXIS
C      Y        Y AXIS POSITIONER
C      YF        ARRAY CONTAINING THE Y - COORDINATES
C      YF1 , YF2  TEMPORARIES FOR YF
C      VL        Y AXIS LENGTH IN INCHES
C      YM        Y AXIS POSITION
C      YO        ORIENTATION AT Y AXIS

```

C-----NOTATION FOR SUBROUTINE GRO

```

C      IDEL    DISTANCE BETWEEN POINTS IN X DIRECTION
C      IGBW   REF GRID TYPE SWITCH FOR 3-D PLOTS
C      IOFX , IOFY  OFFSETS IN X AND Y DIRECTIONS
C      LOP    LINE OPTION ( 1 = SOLID, 0 = DASHED )
C      MX , MY  NUMBER OF INCREMENTS IN X AND Y DIRECTIONS
C      SLOPE  SLOPE OF Y DIRECTION ARM RELATIVE TO X
C      X0 , Y0  COORDS OF RELATIVE ORIGIN FOR GRID 1

```

C-----NOTATION FOR SUBROUTINE TD2

C AT ABSCISSA OF REAL POINT
 C L / 100
 C DEL IDEL / 100
 C DELX X SHIFT AT ORIGIN
 C DELY Y SHIFT AT ORIGIN
 C I GENERAL INDEX
 C IDEL DISTANCE BETWEEN POINTS IN X DIRECTION
 C IEND LENGTH OF MASK
 C II NU = 1
 C IN NUMBER OF Y PARTITIONS
 C INIT INITIALIZATION SWITCH
 C IOFX OFFSET IN X DIRECTION
 C IOFY OFFSET IN Y DIRECTION
 C IPEN PEN STATUS
 C IPOV POV + 100
 C IR DISTANCE ABOVE THE CURRENT HORIZON
 C IZ THE MASK
 C JS ROTATION SWITCH
 C K INDEX OVER KN
 C KL KN = 1
 C KMP K + MP (POINT CONTIGUOUS TO POINT K)
 C KN NUMBER OF X PARTITIONS
 C L ABSCISSA
 C LL INDEX OVER IDEL
 C MP OPTIMIZING SWEEP SWITCH
 C NU ROW NUMBER
 C OFX IOFX / 100
 C OFY IOFY / 100
 C PLTMIN MINIMUM Y COORD OF MASK
 C POV MINIMUM VALUE OF Z, SCALED, IN INCHES
 C SI SLOPE OF CURRENT X SLICE
 C VSF VERTICAL SCALE FACTOR
 C X X COORD
 C XH TEMP VECTOR FOR VERTICAL SLICES
 C Y Y COORD
 C YM TEMP VECTOR FOR VERTICAL SLICES
 C Z FUNCTION TO BE PLOTTED

C-----NOTATION FOR SUBROUTINE CRNRI

C IDEL DISTANCE BETWEEN POINTS IN X DIRECTION
 C IOFX, IOFY OFFSETS IN X AND Y DIRECTIONS
 C MX, MY NUMBER OF INCREMENTS IN X AND Y DIRECTIONS
 C SIZE TICK MARK SIZE CONTROL
 C X, Y COORDS OF CENTER OF TICK MARK (GLOBAL)
 C X0, Y0 COORDS OF RELATIVE ORIGIN FOR GRID 1

C-----NOTATION FOR SUBROUTINE MESH1

C DEL IDEL = 0.81
 C I, J DO LOOP INDEXES
 C IDEL DISTANCE BETWEEN POINTS IN X DIRECTION
 C IOFX, IOFY OFFSETS IN X AND Y DIRECTIONS
 C MP REVERSING SWITCH
 C MX, MY NUMBER OF INCREMENTS IN X AND Y DIRECTIONS
 C MXP1 MX PLUS ONE
 C MYP1 MY PLUS ONE
 C OFX, OFY HALF OF X AND Y TICK MARK LENGTHS
 C SIZE TICK MARK SIZE CONTROL
 C X, Y COORDS OF CENTER OF TICK MARK (GLOBAL)
 C XX X PLOTTER COORDINATE
 C X0, Y0 COORDS OF RELATIVE ORIGIN FOR GRID 1

C-----NOTATION FOR SUBROUTINE GRID1

C I GENERAL INDEX
 C IDEL DISTANCE BETWEEN POINTS IN X DIRECTION
 C IMI I - 1
 C IN NUMBER OF Y PARTITIONS
 C IN1 IN = 1
 C IOFX OFFSET IN X DIRECTION
 C IOFY OFFSET IN Y DIRECTION
 C KN NUMBER OF X PARTITIONS
 C KNS KN + 1
 C LOP LINE OPTION (1 = SOLID, 0 = DASHED)
 C MP OPTIMIZING SWEEP SWITCH
 C XX TEMP CONSTANT
 C X0, Y0 COORD OF RELATIVE ORIGIN
 C X1, Y1 COORD OF BEGINNING POINT
 C X2, Y2 COORD OF ENDING POINT

C-----NOTATION FOR SUBROUTINE EDGE1

C DEL IDEL = 0.81
 C I DO LOOP INDEX TO INCREMENT IN X DIRECTION
 C IDEL DISTANCE BETWEEN POINTS IN X DIRECTION
 C IOFX, IOFY OFFSETS IN X AND Y DIRECTIONS
 C MX, MY NUMBER OF INCREMENTS IN X AND Y DIRECTIONS
 C OFX, OFY HALF OF X AND Y TICK MARK LENGTHS
 C SIZE TICK MARK SIZE CONTROL
 C X, Y COORDS OF CENTER OF TICK MARK (GLOBAL)
 C X0, Y0 COORDS OF RELATIVE ORIGIN FOR GRID 1

C-----NOTATION FOR SUBROUTINE MARK2

```

C      ARM      LENGTH OF ARM OF TICK MARK
C      SIZE      TICK MARK SIZE CONTROL
C      SLOPE     SLOPE OF Y DIRECTION ARM RELATIVE TO X
C      STEP      8.2 IN.
C      X, Y      COORDS OF CENTER OF TICK MARK ( GLOBAL )
C      XX, YY    PLOTTER COORDINATES ( GLOBAL )
C      X1, Y1    COORDS OF TICK MARK ENDS, REL TO (X, Y)

```

C-----NOTATION FOR SUBROUTINE DASH 2

```

C      DIS      DIST BETWEEN POINTS
C      I       GENERAL INDEX
C      INC      NO OF INCREMENTS BETWEEN POINTS
C      INC2     INC+2
C      IP      PEN STATUS
C      IS      PEN STATUS SWITCH
C      LOP     LINE OPTION ( 1 = SOLID, 8 = DASHED )
C      X, Y      COORD OF PEN POSITION
C      XD, YD    PROJECTED LENGTH OF DASH
C      XT, YT    TEMP COORD FOR FIRST DASH
C      X1, Y1    COORD OF BEGINNING POINT
C      X2, Y2    COORD OF ENDING POINT

```

C-----NOTATION FOR SUBROUTINE FRIPS

```

C      AC( ,1)    CONTINUITY COEFFICIENT
C      AM1( ,1)    CONTINUITY COEFFICIENT AC( ,1) AT J=1
C      AM2( ,1)    CONTINUITY COEFFICIENT AC( ,1) AT J=2
C      ATM( ,1)    TEMP STORAGE FOR AC( ,1) RECURSION COEFF
C      CCC( ,5)    COEFFS IN STIFFNESS MATRIX
C      CH( )      PLATE TWISTING STIFFNESSES PER UNIT WIDTH
C      CHP1( )    CW AT J+1
C      DT      DD TRANSPOSE AT J
C      DXC( ), OYC( ) SLAB BENDING STIFFNESSES PER UNIT WIDTH
C      DXM1( )    DX AT J-1
C      DXP1( )    DX AT J+1
C      OYM1( )    DY AT J-1
C      OYP1( )    DY AT J+1
C      EE( ,1)    COEFF IN STIFFNESS MATRIX
C      ET1      EE TRANSPOSE AT J=1
C      ET2      EE TRANSPOSE AT J=2 OR AA AT J
C      FFC( ,1)    COEFF IN LOAD VECTOR
C      FXC( ), FYC( ) BEAM BENDING STIFFNESSES
C      FYM1( )    FY AT J-1
C      FYP1( )    FY AT J+1
C      I       X DIRECTION INDEX (JSTA+2)
C      ID1 THRU ID8 RANDOM INDEXES FOR FILES 1 THRU 4 IBM
C      IIII      TEMPORARY INDEX
C      ITKPP     TEMPORARY PRINT SWITCH FOR DEBUGGING
C      J       Y DIRECTION INDEX (JSTA+2)
C      JJ      J FOR SUBROUTINE MATRIX
C      J1      J2-1 IBM
C      J2      TWICE THE INDEX J IBM
C      K, L      DO LOOP INDEXES
C      L1      KBORT+3, USED FOR VARIABLE DIMENSIONING
C      L2      KLONG+3, USED FOR VARIABLE DIMENSIONING
C      ML      MULTIPLE LOADING SWITCH
C      NP      STARTING VALUE FOR MAIN DO LOOP
C      NK      ORDER OF SUBMATRICES
C      NKK     TEMPORARY INDEX FOR NK
C      NL      MATRIX ORDER OF OVERALL COEFFICIENT MATRIX
C      NM2     NL MINUS 2
C      N1      BAND WIDTH OF EE
C      N2      BAND WIDTH OF DD
C      N3      BAND WIDTH OF CC
C      PBX( )    AXIAL THRUST IN BEAM IN X DIRECTION
C      PBY( )    AXIAL THRUST IN BEAM IN Y DIRECTION
C      PBYP1    PBY AT J+1
C      PX( )      AXIAL THRUST IN SLAB IN X DIRECTION
C      PY( )      AXIAL THRUST IN SLAB IN Y DIRECTION
C      PYP1( )    PY AT J+1
C      Q( )      APPLIED LOAD PER JOINT
C      QMC( )    APPLIED MULTIPLE LOAD PER JOINT
C      RXC( )    ROTATIONAL RESTRAINT IN X DIRECTION
C      RYH1( )    ROTATIONAL RESTRAINT IN Y DIRECTION AT J-1
C      RYV1( )    ROTATIONAL RESTRAINT IN Y DIRECTION AT J+1
C      R1( , ) THRU RECURSION COEFFICIENTS AND MULTIPLIERS
C      R3( , )    USED IN SOLUTION PACKAGE
C      S( )      SPRING SUPPORT, VALUE PER JOINT
C      TX( )      APPLIED COUPLE IN X DIRECTION
C      TYM1( )    APPLIED COUPLE IN Y DIRECTION AT J-1
C      TYP1( )    APPLIED COUPLE IN Y DIRECTION AT J+1
C      WNC( , )    COMPLETE ARRAY OF W VECTORS NEEDED
C                           FOR PLOTTING

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APPENDIX 3

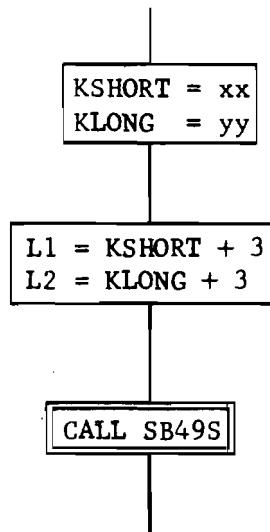
**FLOW DIAGRAMS AND LISTING OF COMPUTER PROGRAM
SLAB 49**

СОВЕТСКАЯ

МОСКОВСКАЯ ОБЛАСТЬ СОВЕТСКАЯ ГИДРОГЕОЛОГИЧЕСКАЯ
СЕТЬ

GENERAL FLOW DIAGRAM FOR SLAB 49

All variables are dimensioned in this main driving routine. For different sized problems only the cards labeled RE-DIMEN columns 73 through 80 must be changed. The main program and all subroutines are variably dimensioned.



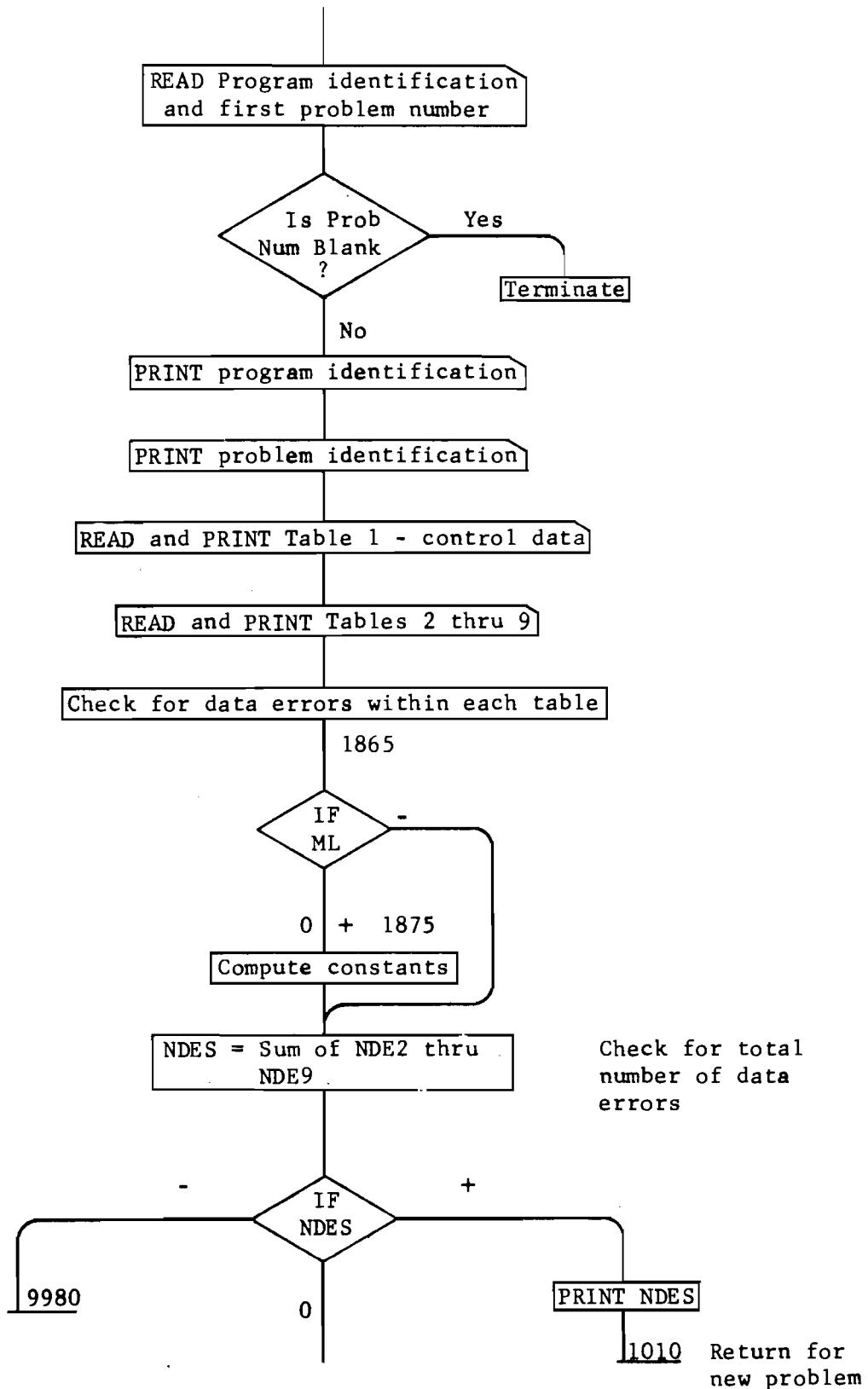
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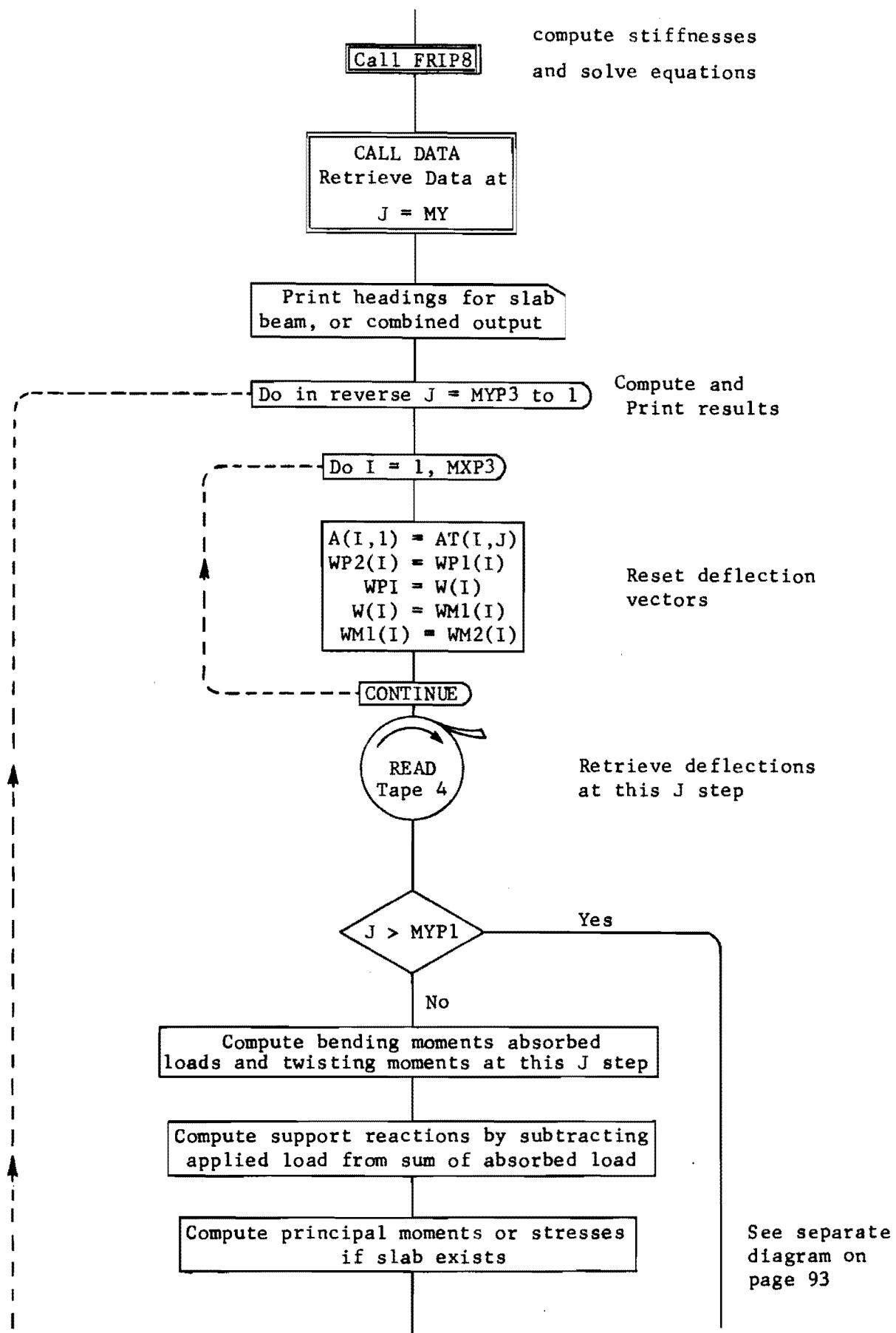
C PROGRAM SLAB49 * INPUT, OUTPUT, TAPE1, TAPE2, TAPE3, TAPE4 L 02N00CDC
C
C----FOR OTHER PROBLEM SIZES, ONLY THIS DRIVER NEED BE RE-DIMENSIONED.
C FOR EXAMPLE, AA(S+3,L), CC(S+3,5), DD(S+3,3), RI(S+3,S+3),
C EX(S+3), PN(S+3,L+3), KSHORT = S, AND KLONG = L WHERE S AND L
C ARE THE MAXIMUM X AND Y SIZES OF THE PROBLEM.
C
C----THIS PROGRAM IS NOW DIMENSIONED TO SOLVE A 24 BY 48 GRID.      RE-DIMEN
C
C----THIS PROGRAM WILL OPERATE ON CDC 6000 SERIES OR IBM G-LEVEL
C SYSTEMS. THE CARDS NEEDED FOR CDC OPERATION HAVE A C IN COLUMN
C 1 AND CDC IN COLUMN 78 THRU 80. THOSE CARDS NEEDED FOR IBM
C OPERATION ARE TAGGED WITH IBM. WHEN CONVERTING FROM ONE SYSTEM
C TO ANOTHER, THOSE CARDS NOT NEEDED SHOULD BE RETAINED AND NULLED
C WITH AN ACCED C. SOME CARDS NOW TAGGED IBM ARE PUNCHED IN THE
C NECESSARY EBCDIC CODE. THE CDC CARDS AND THE REMAINDER OF THE
C CARDS WHICH ARE NOT SPECIALLY TAGGED ARE PUNCHED IN BCD WHICH
C IS COMPATIBLE WITH BOTH SYSTEMS. ASA FORTRAN IS USED THRUOUT.
C
C DOUBLE PRECISION AA, BB, CC, DD, EE, FF, AT, A, AM1, AM2,           13MY1IBM
3   1          RI, R2, R3, W, WM1, WM2, WP1, WP2, PW 13MY1IBM
    DIMENSION AA( 27 , 1 ),          BB( 27 , 1 ),          CC( 27 , 5 ), RE-DIMEN
1     DD( 27 , 3 ),          EE( 27 , 1 ),          FF( 27 , 1 ), RE-DIMEN
2     AT( 27 , 1 ),          RE-DIMEN
3     AL( 27 , 1 ),          AM1( 27 , 1 ),          AM2( 27 , 1 ), RE-DIMEN
4     RI( 27 , 27 ),          R2( 27 , 27 ),          R3( 27 , 27 ) RE-DIMEN
    DIMENSION DX( 27 ),          DXM1( 27 ),          DXP1( 27 ), RE-DIMEN
1     DY( 27 ),          DYM1( 27 ),          DYP1( 27 ), RE-DIMEN
2     FX( 27 ),          FYM1( 27 ),          FYP1( 27 ), RE-DIMEN
3     FY( 27 ),          FYM1( 27 ),          FYP1( 27 ), RE-DIMEN
4     Q( 27 ),          SI( 27 ),          RE-DIMEN
5     RX( 27 ),          RYM1( 27 ),          RYP1( 27 ), RE-DIMEN
6     TX( 27 ),          TYM1( 27 ),          TYP1( 27 ), RE-DIMEN
7     CH( 27 ),          CHP1( 27 ),          RE-DIMEN
8     PX( 27 ),          PY( 27 ),          PYP1( 27 ), RE-DIMEN
9     PBX( 27 ),          PBY( 27 ),          PBYP1( 27 ), RE-DIMEN
A     QM( 27 )          RE-DIMEN
    DIMENSION W( 27 ),          WM1( 27 ),          WM2( 27 ), RE-DIMEN
1     WP( 27 ),          WP1( 27 ),          WP2( 27 ), RE-DIMEN
2     BMX( 27 ),          BMX( 27 ),          RE-DIMEN
3     BMYM1( 27 ),          BMY( 27 ),          BMYP1( 27 ), RE-DIMEN
4     BMBYMI( 27 ),          BMBY( 27 ),          BMBYP1( 27 ), RE-DIMEN
5     PW( 27 , 51 ),          PBX( 27 , 51 ),          PBY( 27 , 51 ), RE-DIMEN
6     PSIGO( 27 , 51 )          RE-DIMEN
C----THE FOLLOWING 5 CARDS PERTAIN TO DIRECT ACCESS FILES AND MUST BE
C CHANGED WHEN PROGRAM IS REDIMENSIONED. FOR EXAMPLE, THE DEFINE
C FILE CARD PARAMETERS ARE 2*(L+3), Z1(S+3)*2, 0, ID1 THRU ID4. 14JAZIBM
C COMMON /DA/ ID1, ID2, ID3, ID4 14JAZIBM
 1   1        14JAZIBM
 2   2        14JAZIBM
 3   3        14JAZIBM
 4   4        14JAZIBM
 5   5        14JAZIBM
 6   6        14JAZIBM
 7   7        14JAZIBM
 8   8        14JAZIBM
 9   9        14JAZIBM
C
C CALL SB49S   1   AA, BB, CC, DD, EE, FF, AT,
1   1           A, AM1, AM2, RI, R2, R3, 22AG9
2   2           DX, DXM1, DXP1, DY, DYM1, DYP1, . 22AG9
3   3           FX, FY, FYM1, FYP1, Q, S, 100C9
4   4           RX, RYM1, RYP1, TX, TYM1, TYP1, CH, 100C9
5   5           CHP1, PX, PY, PYP1, PBX, PBY, PBYP1, 100C9
6   6           QM, W, WM1, WM2, WP1, WP2, 100C9
7   7           BMX, BMX, BMYM1, BMY, BMYP1, 100C9
8   8           PW, PBX, PBY, PSIGO, LI, LZ  21JAI
9   9           PW, PBX, PBY, PSIGO, LI, LZ  17FEI
END          05JAB

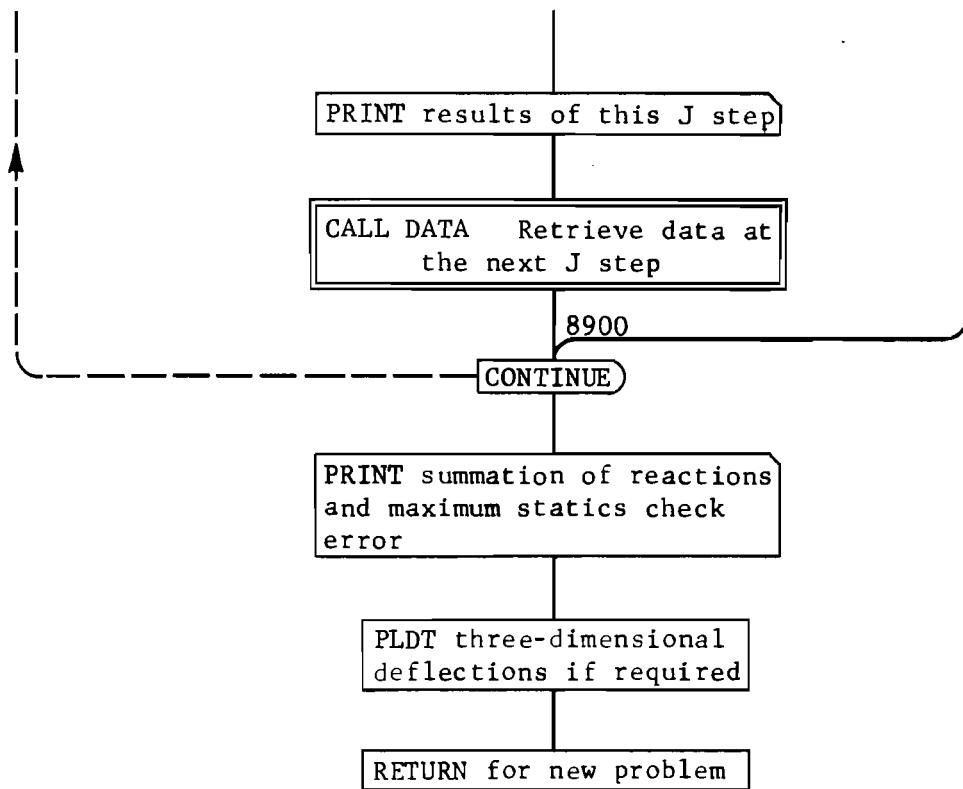
```

SUBROUTINE SB49S

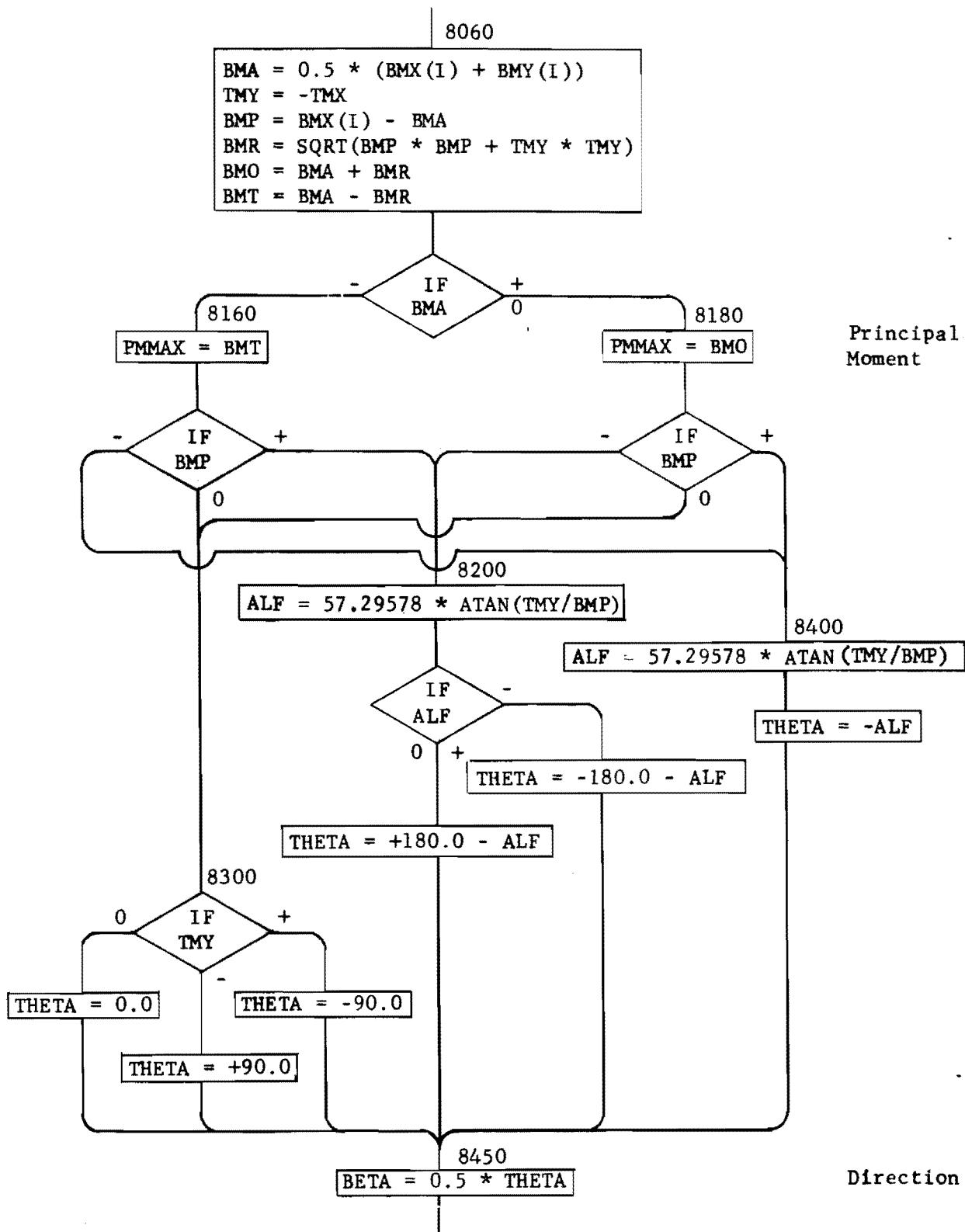
This is the main computational program







Compute Principal Moment and Direction



```

SUBROUTINE SB49S ( AA, BB, CC, DD, EE, FF, AT, 21JAI
1      A, API, AM2, R1, R2, R3, 22A69
2      DX, DXPI, DXPL, DY, DYN1, DYP1, 100C9
3      FX, FY, FYM1, FYP1, Q, S, 100C9
4      RX, RYM1, RYPI, TX, TYM1, TYP1, CH, 100C9
5      CHPI, PX, PY, PYP1, PBX, PBY, PBYP1, 100C9
6      QM, W, WM1, WM2, WP1, WP2, 100C9
7      BMX, BPBX, BMY1, BMY, BMYP1, 100C9
8      BMBY1, BMBY, BMBYP1, 21JAI
9      PW, PBX, PBY, PSIGO, L1, L2 ) 17FE1

C   1 FORMAT ( 52H PROGRAM SLAB 49 -TEX HWY DEPT DECK- MATLOCK, IINOI
1      33HPANAK, ENDRES REV DATE 29 FEB 72 REVISED
B// )
C   DOUBLE PRECISION AA, BB, CC, DD, EE, FF, AT, A, AM1, AM2,
1      R1, R2, R3, W, WM1, WM2, WP1, WP2, PW 13MY1IBM
1      DCUBLE PRECISION PDHXH, HYCHX3, ODHX3, ODHX2, ODHX, 13MY1IBM
1      ODHXHY, HXOHY3, ODHY3, ODHY2, OOMY, CRD 13MY1IBM
C   DOUBLE PRECISION WSUM1, WSUM2, WSUM3, HYDMX, HXDX,
+      QBMX, QBHY, QPXM, QPY, QTMX, QTMY, QTQX, QTQY, 13MY1IBM
+      QBMBX, QBMBY, QPBX, QPY, QRX, QRY, REACT, 13MY1IBM
+      SUMR, TMX, BMA, TMY, BMP, BMR, BMD, BMT, 13MY1IBM
+      ALF, SOT2, PMAX, THK, STACH, SUMR, ROCK 13MY1IBM
DOUBLE PRECISION ROSCI, ROSC2, SOM, SUP, STAT, STRS, RCT, CHK,
1      BMCM, SLBX, SLBY, BLNK, BEMX, BEMY, ITEST, NPROB, KPROB 23FELIBM
DOUBLE PRECISION ID1, ID2, IC3, 104 30N01IBM
DOUBLE PRECISION STEM 01DE1IBM
DIMENSION AA( L1 : 1 ), BB( L1 : 1 ), CC( L1 : 5 ), 26SE8
1      DD( L1 : 3 ), EE( L1 : 1 ), FFI( L1 : 1 ), 26SE8
2      AT( L1 : 1 ), 26SE8
3      A( L1 : 1 ), AM1( L1 : 1 ), AM2( L1 : 1 ), 26SE8
4      R1( L1 : 1 ), R2( L1 : 1 ), R3( L1 : 1 ), 22A69
DIMENSION DX( L1 ), CXM1( L1 ), DXPI( L1 ), 100C9
1      DY( L1 ), OYMI( L1 ), DYP1( L1 ), 100C9
2      FX( L1 ), FYM1( L1 ), FYP1( L1 ), 100C9
3      Q1( L1 ), S1( L1 ), 100C9
5      RX( L1 ), RYM1( L1 ), RYPI( L1 ), 100C9
6      TX( L1 ), TYP1( L1 ), TYP1( L1 ), 100C9
7      CH( L1 ), CHPI( L1 ), 100C9
8      PX( L1 ), PY( L1 ), PYP1( L1 ), 100C9
9      WM( L1 ), WM1( L1 ), WM2( L1 ), 100C9
DIMENSION W( L1 ), WM1( L1 ), WM2( L1 ), 100C9
1      WM1( L1 ), WM2( L1 ), WP1( L1 ), 100C9
2      BMX( L1 ), BPBX( L1 ), 100C9
3      BMY1( L1 ), BMY( L1 ), BMYP1( L1 ), 100C9
4      BMBY1( L1 ), BMBY( L1 ), BMBYP1( L1 ), 17FE1
5      PW( L1 : L2 ), PBX( L1 : L2 ), PBY( L1 : L2 ), 17FE1
6      PSIGO( L1 : L2 ), 17FE1
DIMENSION TEM1( 300 ), TEM2( 300 ), XX( 300 ) 11FE1
DIMENSION IBUF( 500 ) 06DE1IBM

C   COMMON / ZOT / LCP, MC, IROLL, MOP 15JE0
C   COMMON / PLOT / L1, L2, J1, J2 25N09CDC
COMMON / PLT / L1, L2, J1, J2 01DE1IBM
COMMON / CARDS / AN1( 40 ), AN2( 18 ), 21JAI
1      IN13( 300 ), JN13( 300 ), IN23( 300 ), JN23( 300 ), DXN( 300 ), 23SE0
2      DYN1( 300 ), FXN( 300 ), FYN( 300 ), QN1( 300 ), SN( 300 ), 23SE0
3      IN14( 50 ), JN14( 50 ), IN24( 50 ), JN24( 50 ), 23SE0
4      RXN( 50 ), RYM1( 50 ), TXN( 50 ), TYN( 50 ), 23SE0
5      IN15( 100 ), JN15( 100 ), IN25( 100 ), JN25( 100 ), CHN( 100 ), 23SE0
6      IN16( 50 ), JN16( 50 ), IN26( 50 ), JN26( 50 ), 23SE0
7      PXN( 50 ), PYN( 50 ), PBXN( 50 ), PBYN( 50 ), 23SE0
8      IN17( 100 ), JN17( 100 ), IN27( 100 ), JN27( 100 ), QMN( 100 ), 17FE1
9      IN18( 10 ), JN18( 10 ), IN28( 10 ), JN28( 10 ), 17FE1
A      KASEW1( 10 ), KASEX1( 10 ), KASEY1( 10 ), KASEP1( 10 ), 17FE1
B      JN19( 10 ), JN29( 10 ), 17FE1
COMMON / NC13 / NC13, NC14, NC14, NC15, NC15, NC16, NC16, 100C9
1      NC17, NC17, NC18, NC18, NC19, NC19, NC19 17FE1
COMMON / STIFF / PDHXH, HYDHX3, ODHX3, ODHX2, ODHX, 100C9
1      ODHXHY, HXDHY3, ODHY3, ODHY2, ODHY, CRD( 5 ) 24MY1
COMMON / RI / MXP3, MYP3, NF, ITMPP 23DE0

C   DATA SLP / 7HSLPPORT /, STAT / 7HSTATICS /, STRS / 6HSTRESS /, 08FE1
1      RCT / 8HREACTION /, CHK / BH CHECK /, BMOM / 6HMOMENT /, 08FE1
2      SLBX / 6HSLAB X /, SLBY / 6HSLAB Y /, BLNK / 6H /, 08FE1
3      BEMX / 6HBEAM X /, BEMY / 6HBEAM Y /, ITEST / 5H /, 08FE1
C   DATA ID1, ID2, ID3, ID4 / 8HDEFLECTN, BHBIN MOM X, 15JE0CDC
C   BHBIN MOM Y, BH SIGO /, 15JE0CDC
C
C   6 FORMAT ( )
11 FORMAT ( 5H1 , 80X, 1DHI----TRIM ) 04MY3
12 FORMAT ( 20A4 ) 17FE1
13 FORMAT ( 5X, 20A4 ) 18MY0
14 FORMAT ( A5, 5X, 17A4, A2 ) 21JAI
15 FORMAT ( //10H PROB , /5X, A5, 5X, 17A4, A2 ) 07DE0
16 FORMAT ( //17H PROB XCONTOL, /5X, A5, 5X, 17A4, A2 ) 060E1IBM
19 FORMAT ( //50H KEEP RUN TIME RECORDS FOR FUTURE ESTIMATES OF 21JAI
1      31H PARENT AND OFFSPRING RUN TIMES )
20 FORMAT ( 815, 5X, 15, 25X, 15, /, 815, 5X, 315, 12, 13, 3F5.0 ) 06DE1
21 FORMAT ( 215, 10X, 4E10.3 ) 21JAI
33 FORMAT ( 4( 2X, 13 ), 6E10.3 ) 19AG8
43 FORMAT ( 4( 2X, 13 ), 20X, 4E10.3 ) 21JAI
53 FORMAT ( 4( 2X, 13 ), E10.3 ) 21JAI
63 FORMAT ( 4( 2X, 13 ), 20X, 4E10.3 ) 21JAI
73 FORMAT ( 4( 2X, 13 ), 40X, E10.3 ) 21JAI
83 FORMAT ( 4( 2X, 13 ), 4( 4X, 11 ) ) 03FE1
93 FORMAT ( 2110 ) 06DC0
100 FORMAT ( //30H TABLE 1. CCNTROL DATA 21JAI1CDC
100 FORMAT ( //30H TABLE 1. CONTROL CATA 06DE1IBM
1      1 / 5X, 20H TABLE NUMBER 21JAI
2      2 / 40X, 45H 21JAI
C   3 // 5X, 40H KEEP FROM PRECEDING PROBLEM $1YESL , 815, 21JAI1CDC
3 // 5X, 40H KEEP FROM PRECEDING PROBLEM (1=YES) , 815, 06DE1IBM
4 // 5X, 40H NUM CARDS INPUT THIS PROBLEM , 815, 21JAI
5 // 5X, 25H MULTIPLE LCAD OPTION , 15X, 15, 21JAI
6 // 5X, 25H STATICS CHECK OPTION , 15X, 15, 21JAI
7 // 5X, 25H PRIN STRESS OPTION , 15X, 15, 21JAI
8 // 5X, 25H PROFILE PLCT OPTION , 15X, 15, 21JAI
C   9 // 5X, 25H 3-D PLOT OPTCN , 15X, 15, L 21JAI1CDC
5 // 5X, 25H 3-D PLOT OPTCN , 15X, 15 ) 060E1IBM
200 FORMAT ( //24H TABLE 2. CONSTANTS ) 16AG8
201 FORMAT ( // 45H NUMBER OF INCREMENTS IN X DIRECTION 21JAI
1      35X, 15, 21JAI
2      / 10X, 35HNUMBER OF INCREMENTS IN Y DIRECTION , 35X, 15, 21JAI
3      / 10X, 35HINCREMENT LENGTH IN X DIRECTION , 30X, 1PE10.3, 21JAI1IBM
C   3 / 10X, 35HINCREMENT LENGTH IN X DIRECTION , 30X, E10.3, 21JAI1CDC
4 / 10X, 35HINCREMENT LENGTH IN Y DIRECTION , 30X, 1PE10.3, 21JAI1IBM
5 / 10X, 35HINCREMENT LENGTH IN Y DIRECTION , 30X, E10.3, 21JAI1CDC
C   5 / 10X, 35HPCISSONS RATIO , 30X, 1PE10.3, 21JAI1IBM
5 / ICX, 35HPCISSONS RATIO , 30X, E10.3, 21JAI1CDC
6 / 10X, 35HSLAB THICKNESS , 30X, 1PE10.3 L 21JAI1IBM
C   6 / 10X, 35HSLAB THICKNESS , 30X, E10.3 L 21JAI1CDC
300 FORMAT ( //49H TABLE 3. JOINT BENDING STIFFNESSES, LOADS, 100E1
1      15HAND SUPPORTS 100E1
1      1 // 50H FROM THRU DX DY FX , 30AG8
2      2 / 35H FY Q S * 30AG8
3      3 / 20H JOINT JCINT, / 1 30AG8
C   311 FORMAT ( 5X, 21 IX, 12, IX, 13 ), 1PE11.3 ) 21JAI1IBM
C   311 FORMAT ( 5X, 2X, 2X IX, 12, IX, 13 L, 6E11.3 L 21JAI1CDC
400 FORMAT ( //51H TABLE 4. JOINT RESTRAINTS AND APPLIED MOMENTS 100E1
1      1 // 50H FROM THRU RX , 30AG8
2      2 / 35H RY TX TY , 30AG8
3      3 / 20H JOINT JOINT, / 1 30AG8
411 FORMAT ( 5X, 21 IX, 12, IX, 13 ), 22X, 1P4E11.3 ) 21JAI1BM
C   411 FORMAT ( 5X, 2X, 2X IX, 12, IX, 13 L, 22X, 4E11.3 L 21JAI1CDC
500 FORMAT ( //50H TABLE 5. PESH TWISTING STIFFNESSES 100E1
1      1 // 30H FROM THRU C * 30AG8
2      2 / 20H MESH MESH, / 1 30AG8
511 FORMAT ( 5X, 21 IX, 12, IX, 13 ), 1PE11.3 ) 21JAI1IBM
C   511 FORMAT ( 5X, 2X, 2X IX, 12, IX, 13 L, E11.3 L 21JAI1CDC
600 FORMAT ( //40H TABLE 6. BAR AXIAL THRUSTS 100E1
1      1 // 50H FROM THRU PX , 30AG8
2      2 / 35H PY PBX PBY * 30AG8
3      3 / 20H BAR BAR, / ) 30AG8

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IF (NDE1) 9980, 1200, 1182
1182 PRINT 991, NDE1
C
C-----INPUT TABLE 2 -- CONSTANTS
C
1200 PRINT 200
    IF ( KEEP2 ) 9980, 1201, 1240
1201        NDE2 = 0
    IF ( NCD2 - 1 ) 1203, 1205, 1203
1203        NDE2 = NDE2 + 1
    PRINT 985
1205 READ 21, MX, MY, HX, HY, PR, THK
    PRINT 201, MX, MY, HX, HY, PR, THK
        IF ( MX - MY ) 1211, 1211, 1210
1210        NDE2 = NDE2 + 1
    PRINT 984
1211        IF ( HX * HY ) 1212, 1212, 1213
1212        NDE2 = NDE2 + 1
    PRINT 985
1213        IF ( PR * THK ) 1214, 1225, 1225
1214        NDE2 = NDE2 + 1
    PRINT 985
1225        IF ( LI - 3 - MX ) 1226, 1227, 1227
1226        NDE2 = NDE2 + 1
    PRINT 986
1227        IF ( L2 - 3 - MY ) 1228, 1250, 1250
1228        NDE2 = NDE2 + 1
    PRINT 986
        GO TO 1250
1240 PRINT 905
    PRINT 201, MX, MY, HX, HY, PR, THK
1250    IF ( NDE2 ) 5980, 1300, 1270
1270 PRINT 991, NDE2
C
C-----INPUT TABLE 3 -- JOINT STIFFNESS AND LOAD D
C
1300 PRINT 300
    IF ( KEEP3 ) 9980, 1301, 1310
1301        NC13 = 1
        NCT3 = NCD3
        NDE3 = 0
        SWS = 0.0
        SWB = 0.0
        GO TO 1335
1310 PRINT 905
    DO 1325 N = 1, NCT3
    PRINT 311, INI3(N), JN13(N), IN23(N), JN23
    1         FXN(N), FYN(N), CN(N), SN(N)
1325    CONTINUE
    PRINT 910
        NC13 = NC13 + 1
        NCT3 = NCT3 + NCD3
1335    IF ( NCD3 ) 9980, 1337, 1340
1337 PRINT 903
    GO TO 1372
1340    DO 1370 N = NC13, NCT3
    READ 33, INI3(N), JN13(N), IN23(N), JN23
    1         FXN(N), FYN(N), CN(N), SN(N)
    PRINT 311, INI3(N), JN13(N), IN23(N), JN23
    1         FXN(N), FYN(N), CN(N), SN(N)
        IF ( IN13(N) - IN23(N) ) 1342, 1342, 13
1341        NDE3 = NDE3 + 1
    PRINT 985
1342    IF ( JN13(N) - JN23(N) ) 1344, 1344, 13
1343        NDE3 = NDE3 + 1
    PRINT 985
1344    IF ( IN23(N) - MX ) 1346, 1346, 1345
1345        NDE3 = NDE3 + 1
    PRINT 985
1346    IF ( JN23(N) - MY ) 1350, 1350, 1347
1347        NDE3 = NDE3 + 1
    PRINT 985
1350        SWS = SWS + ABS ( CXN(N) + OYN(N)
        SWB = SWB + ABS ( FXN(N) + FYN(N)

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23FE1      1370      CONTINUE
05FE1      1372      IF ( NDE3 ) 9980, 1400, 1375
05FE1      1375      PRINT 991, NDE3
02FE1      C
02FE1      C----- INPUT TABLE 4 -- JCINT STIFFNESS AND LOAD DATA CUNTO
02FE1      C
02FE1      1400      PRINT 400
02FE1      1401      IF ( KEEPS ) 9980, 1401, 1410
02FE1      1401      NC14 = 1
02FE1      1401      NCT4 = NCD4
02FE1      1401      NDE4 = 0
02FE1      1401      GO TO 1435
02FE1      1410      PRINT 905
02FE1      1410      DO 1425 N = 1, NCT4
02FE1      1410      PRINT 411, IN14(N), JN14(N), IN24(N), JN24(N), RXN(N), RYN(N),
02FE1      1410      TXN(N), TYN(N)
02FE1      1425      CONTINUE
02FE1      1425      PRINT 910
02FE1      1425      NC14 = NCT4 + 1
02FE1      1425      NCT4 = NCT4 + NCD4
02FE1      1435      IF ( NCD4 ) 9980, 1437, 1440
02FE1      1437      PRINT 903
02FE1      1437      GO TO 1472
02FE1      1440      DO 1470 N = NC14, NCT4
02FE1      1440      READ 43, IN14(N), JN14(N), IN24(N), JN24(N), RXN(N), RYN(N),
02FE1      1440      TXN(N), TYN(N)
02FE1      1440      PRINT 411, IN14(N), JN14(N), IN24(N), JN24(N), RXN(N), RYN(N),
02FE1      1440      TXN(N), TYN(N)
02FE1      1441      IF ( IN14(N) - IN24(N) ) 1442, 1442, 1441
02FE1      1441      NDE4 = NDE4 + 1
02FE1      1441      PRNTNT 985
02FE1      1442      IF ( JN14(N) - JN24(N) ) 1444, 1444, 1443
02FE1      1443      NDE4 = NDE4 + 1
02FE1      1443      PRINT 985
02FE1      1444      IF ( IN24(N) - MX ) 1446, 1446, 1445
02FE1      1445      NDE4 = NDE4 + 1
02FE1      1445      PRINT 985
02FE1      1446      IF ( JN24(N) - MY ) 1470, 1470, 1447
02FE1      1447      NDE4 = NDE4 + 1
02FE1      1447      PRINT 985
02FE1      1450      CONTINUE
02FE1      1450      IF ( NDE4 ) 9980, 1500, 1475
02FE1      1475      PRINT 991, NDE4
02FE1      C
02FE1      C----- INPUT TABLE 5 -- MESH STIFFNESS DATA
02FE1      C
02FE1      1500      PRINT 500
02FE1      1500      IF ( KEEPS ) 9980, 1501, 1510
02FE1      1501      NC15 = 1
02FE1      1501      NCT5 = NCD5
02FE1      1501      NDE5 = 0
02FE1      1501      GO TO 1535
02FE1      1510      PRINT 905
02FE1      1510      DO 1525 N = 1, NCT5
02FE1      1510      PRINT 511, IN15(N), JN15(N), IN25(N), JN25(N), CHN(N)
02FE1      1525      CONTINUE
02FE1      1525      PRINT 91C
02FE1      1525      NC15 = NCT5 + 1
02FE1      1525      NCT5 = NCT5 + NCD5
02FE1      1535      IF ( NCD5 ) 9980, 1537, 1540
02FE1      1537      PRINT 903
02FE1      1537      GO TO 1572
02FE1      1540      DO 1570 N = NC15, NCT5
02FE1      1540      READ 53, IN15(N), JN15(N), IN25(N), JN25(N), CHN(N)
02FE1      1540      PRINT 511, IN15(N), JN15(N), IN25(N), JN25(N), CHN(N)
02FE1      1540      IF ( IN15(N) - IN25(N) ) 1542, 1542, 1541
02FE1      1541      NDE5 = NDE5 + 1
02FE1      1541      PRINT 985
02FE1      1542      IF ( JN15(N) - JN25(N) ) 1544, 1544, 1543
02FE1      1543      NDE5 = NDE5 + 1
02FE1      1543      PRINT 985
02FE1      1544      IF ( IN25(N) - MX ) 1546, 1546, 1545
02FE1      1545      NDE5 = NDE5 + 1
02FE1      1545      PRINT 985

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1546 IF ( JA25(IN) - MY ) 1548, 1548, 1547
1547 PRINT 985
1548 IF ( IN15(IN) + IN25(IN) + JN15(IN) + JN25(IN) ) 9980, 1549, 1570
1549 NOE5 = NOE5 + 1
1550 PRINT 985
1551 CONTINUE
1552 IF ( NOE5 ) 9980, 1600, 1575
1553 PRINT 991, NOE5
C----INPUT TABLE 6 -- BAR STIFFNESS DATA
C
1600 PRINT 600
1601 IF ( KEEP6 ) 5980, 1601, 1610
1602 NC16 = 1
1603 NC16 = NC06
1604 NOE6 = 0
1605 GO TO 1635
1610 PRINT 905
1611 DO 1625 N = 1, NC16
1612 PRINT 611, IN16(IN), JN16(IN), IN26(IN), JN26(IN), PXN(IN), PYN(IN),
1613 1 PBXN(IN), PBYN(IN)
1614 CONTINUE
1615 PRINT 910
1616 NC16 = NC16 + 1
1617 NC16 = NC16 + NC06
1618 IF ( NC06 ) 5980, 1637, 1640
1619 GO TO 1672
1620 DD 1670 N = NC16, NC16
1621 READ 63, IN16(IN), JN16(IN), IN26(IN), JN26(IN), PXN(IN), PYN(IN),
1622 1 PBXN(IN), PBYN(IN)
1623 PRINT 611, IN16(IN), JN16(IN), IN26(IN), JN26(IN), PXN(IN), PYN(IN),
1624 1 PBXN(IN), PBYN(IN)
1625 IF ( IN16(IN) - JN26(IN) ) 1642, 1642, 1641
1626 NOE6 = NOE6 + 1
1627 PRINT 925
1628 IF ( JN16(IN) - JN26(IN) ) 1644, 1644, 1643
1629 NOE6 = NOE6 + 1
1630 PRINT 985
1631 IF ( IN26(IN) - MX ) 1646, 1646, 1645
1632 NOE6 = NOE6 + 1
1633 PRINT 985
1634 IF ( JN26(IN) - MY ) 1648, 1648, 1647
1635 NOE6 = NOE6 + 1
1636 PRINT 985
1637 IF ( PXN(IN) + PBXN(IN) ) 1665, 1667, 1665
1638 IF ( IN16(IN) + IN26(IN) ) 9980, 1666, 1667
1639 NOE6 = NOE6 + 1
1640 PRINT 985
1641 IF ( PYN(IN) + PBYN(IN) ) 1668, 1670, 1668
1642 IF ( JN16(IN) * JN26(IN) ) 9980, 1669, 1670
1643 NOE6 = NOE6 + 1
1644 PRINT 985
1645 IF ( IN16(IN) + IN26(IN) ) 1672, 1672, 1672
1646 NOE6 = NOE6 + 1
1647 PRINT 985
1648 IF ( IN26(IN) - MX ) 1674, 1674, 1674
1649 NOE6 = NOE6 + 1
1650 PRINT 985
1651 IF ( JN26(IN) - MY ) 1676, 1676, 1676
1652 NOE6 = NOE6 + 1
1653 PRINT 985
1654 IF ( IN16(IN) + IN26(IN) ) 1678, 1680, 1678
1655 NOE6 = NOE6 + 1
1656 PRINT 985
1657 IF ( JN16(IN) * JN26(IN) ) 9980, 1681, 1680
1658 NOE6 = NOE6 + 1
1659 PRINT 985
1660 IF ( IN16(IN) + IN26(IN) ) 1682, 1682, 1682
1661 NOE6 = NOE6 + 1
1662 PRINT 985
1663 IF ( JN16(IN) - JN26(IN) ) 1684, 1684, 1684
1664 NOE6 = NOE6 + 1
1665 PRINT 985
1666 IF ( IN26(IN) - MX ) 1686, 1686, 1686
1667 NOE6 = NOE6 + 1
1668 PRINT 985
1669 IF ( JN26(IN) - MY ) 1688, 1688, 1688
1670 NOE6 = NOE6 + 1
1671 PRINT 985
1672 IF ( NOE6 ) 5980, 1700, 1675
1673 PRINT 991, NOE6
C----INPUT TABLE 7 -- MULTIPLE LLOAD JOINT DATA
C
1700 PRINT 700
1701 IF ( KEEP7 ) 9980, 1701, 1710
1702 NC17 = 1
1703 NC17 = NC07
1704 NOE7 = 0
1705 GO TO 1735
1710 PRINT 905
1711 DO 1725 N = 1, NC17
1712 PRINT 711, IN17(IN), JN17(IN), IN27(IN), JN27(IN), QMN(IN)
1713 CONTINUE
1714 PRINT 910
1715 NC17 = NC17 + 1
1716 NC17 = NC17 + NC07
1717 IF ( NC07 ) 5980, 1737, 1740
C----INPUT TABLE 8 -- PROFILE OUTPUT AREAS
C
1740 PRINT 905
1741 GO TO 1772
1742 DD 1770 N = NC17, NC17
1743 READ 73, IN17(IN), JN17(IN), IN27(IN), JN27(IN), QMN(IN)
1744 PRINT 711, IN17(IN), JN17(IN), IN27(IN), JN27(IN), QMN(IN)
1745 IF ( IN17(IN) - IN27(IN) ) 1742, 1742, 1741
1746 NOE7 = NOE7 + 1
1747 PRINT 985
1748 IF ( JN17(IN) - JN27(IN) ) 1744, 1744, 1743
1749 NOE7 = NOE7 + 1
1750 PRINT 985
1751 IF ( IN27(IN) - MX ) 1746, 1746, 1745
1752 NOE7 = NOE7 + 1
1753 PRINT 985
1754 IF ( JN27(IN) - MY ) 1770, 1770, 1747
1755 NOE7 = NOE7 + 1
1756 PRINT 985
1757 CONTINUE
1758 IF ( NOE7 ) 5980, 1800, 1775
1759 PRINT 991, NOE7
C----INPUT TABLE 8 -- PROFILE OUTPUT AREAS
C
1800 PRINT 800
1801 IF ( KEEP8 ) 5980, 1801, 1810
1802 NC18 = 1
1803 NC18 = NC08
1804 NOE8 = 0
1805 IBCS = 0
1806 IBOST = 0
1807 GO TO 1835
1810 PRINT 905
1811 DO 1825 N = 1, NC18
1812 PRINT 811, IN18(IN), IN28(IN), JN28(IN), KASEW(IN), KASEX(IN),
1813 1 KASEY(IN), KASEP(IN)
1814 CONTINUE
1815 PRINT 910
1816 NC18 = NC18 + 1
1817 NC18 = NC18 + NC08
1818 IF ( NC08 ) 5980, 1837, 1840
1819 PRINT 903
1820 GO TO 1872
1821 DD 1870 N = NC18, NC18
1822 READ 83, IN18(IN), JN18(IN), IN28(IN), JN28(IN), KASEW(IN), KASEX(IN),
1823 1 KASEY(IN), KASEP(IN)
1824 PRINT 811, IN18(IN), JN18(IN), IN28(IN), JN28(IN), KASEW(IN), KASEX(IN),
1825 1 KASEY(IN), KASEP(IN)
1826 NEND = ((IA28(N)-IK18(N)+I)*(JN28(N)-JN18(N)+I))
1827 IF ( NEND - 300 ) 1842, 1842, 1841
1828 NOE8 = NOE8 + 1
1829 PRINT 990, NEND
1830 IF ( IN28(IN) .GE. IN18(IN) ) GO TO 1843
1831 NOE8 = NOE8 + 1
1832 PRINT 985
1833 IF ( JN18(N) - JN28(N) ) 1845, 1845, 1844
1834 NOE8 = NOE8 + 1
1835 PRINT 985
1836 IF ( IA28(N) - MX ) 1847, 1847, 1846
1837 NOE8 = NOE8 + 1
1838 PRINT 985
1839 IF ( JN28(N) - MY ) 1849, 1849, 1848
1840 NOE8 = NOE8 + 1
1841 PRINT 985
1842 CONTINUE
1843 IF ( KASEX(N) + KASEY(N) ) 9980, 1870, 1850
1844 NOE8 = NOE8 + 1
1845 PRINT 985
1846 IF ( KASEX(N) + KASEY(N) - 3 ) 1852, 1851, 1852
1847 NOE8 = NOE8 + 1
1848 PRINT 985
1849 CONTINUE
1850 IF ( KASEX(N) + KASEY(N) ) 9980, 1870, 1850
1851 NOE8 = NOE8 + 1
1852 PRINT 985
1853 GO TO 1870
1854 IF ( KASEX(N).EQ.2 ) GO TO 1856
1855 IF ( KASEY(N).EQ.2 ) GO TO 1856
1856 IBCS = 1
1857 IF ( IBOST .EQ. 2 ) GO TO 1851
1858 IBOST = 1

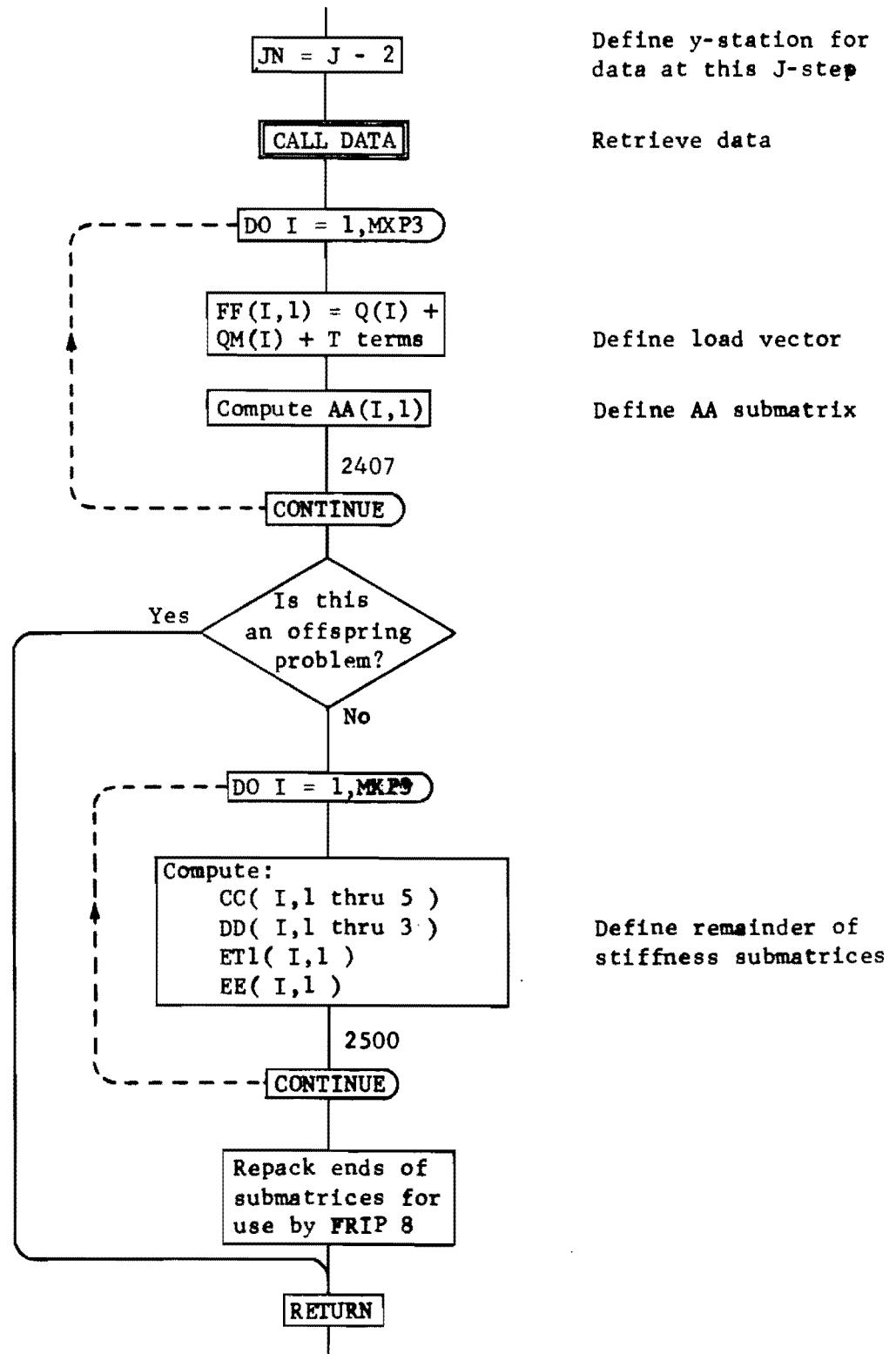
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PRINT 815, SGM
PRINT 6
PRINT 813, BLNK, BLNK, BLNK, BLNK, SLBX,
        & SLBX, SLBY, RDSCI, BMOM, BMOM, BMOM, SDM, SDM, ROSC2
        GD TO 6145
C----PRINT HEADINGS FOR COMBINED SLAB AND BEAM OUTPUT
6140 PRINT 6
        PRINT 815, SDM
        PRINT 812
        PRINT 6
        PRINT 813, SLBX, SLBY, BMOM, BMOM, SLBX,
        & BMEX, BEMY, RDSCI, BMOM, BMOM, SDM, SDM, ROSC2
        6145 IF (IOPPS) 5980, 6180, 6175
        6175 IF ( THK ) 5980, 6180, 6176
        6176 SOT2 = 6.0 / ( THK * THK )
        GO TO 6200
        6180 SOT2 = 1.0
C----COMPUTE BENDING MOMENTS, REACTIONS AND TWISTING MOMENTS
C
6200 DO 8650 LL = 1, MXP3
        J = MYP4 - LL
        DO 6250 I = 1, MXP3
            WP2(I) = WP1(I)
            WP1(I) = W(I)
            W(I) = WM1(I)
            WM1(I) = WM2(I)
        6250 CONTINUE
        DO 6300 I = 1, MXP3
            WM2(E) = PW(I,J)
        6300 CONTINUE
        IF ( MYP1 - J ) 8650, 6400, 6500
6400 DO 6450 I = 2, MXP2
        CRD(2) = AMINI ( DX(I), DY(I) )
        WSUMM1 = ODHX2 * ( WM1(I-1) - 2.0 * WM1(I)
        & + WM1(I+1) )
        WSUMM2 = ODHY2 * ( WM2(I) - 2.0 * WM1(I) + W(I) )
        BMYH1(I) = DY(I) * WSUMM2 + CRD(2) * PR * WSUMM1
        BMYH1(I) = FY(I) * WSUMM2
6450 CONTINUE
        GO TO 8650
6500 DO 7200 I = 2, MXP2
        CRD(2) = AMINI ( DX(I), DY(I) )
        CRD(4) = AMINI ( DXM1(I), DYM1(I) )
        BMYP1(I) = BMY(I)
        BMY1(I) = BMYH1(I)
        BMBYPL(I) = BMBY(I)
        BMBY(I) = BMBYH1(I)
        WSUML = ODHX2 * ( W(I-1) - 2.0 * W(I) + W(I+1) )
        WSUM2 = ODHY2 * ( WM1(I) - 2.0 * W(I) + WP1(I) )
        WSUMM1 = ODHX2 * ( WM1(I-1) - 2.0 * WM1(I)
        & + WM1(I+1) )
        WSUMM2 = ODHY2 * ( WM2(I) - 2.0 * WM1(I) + W(I) )
        BMX(I) = DX(I) * WSUML + CRD(2) * PR * WSUM2
        BMYM1(I) = DY(I) * WSUMM2 + CRD(4) * PR * WSUMM1
        BMBX(I) = FX(I) * WSUML
        BMBYH1(I) = FY(I) * WSUMM2
C----PBMX AND PBMY ARE STORED HERE FROM J = MYP2 TO 3
        IF ( 1805 .LE. 2 ) J GO TO 8525
        PBMX(I,J+2) = BMX(I)
        PBMY(I,J+2) = BMY(I)
        GO TO 7200
6525 PBMX(I,J+2) = BMBX(I)
        PBMY(I,J+2) = BMBY(I)
7200 CONTINUE
        JSTA = J
7300 IF ( IPRINT ) 5980, 7310, 7305
7305 PRINT 6
7310 DO 8550 I = 2, MXP2
        JSTA = 1 - 2
        QBMX = HYDXH * ( BMX(I-1) - 2.0 * BMX(I) + 0.000
        QBMX = HYDXH * % BMX2(I-1L - 2.0 * BMX2(IL
        & - BMX(I) )
        QBMY = HDXHY * ( BMYH1(I) - 2.0 * BMY(I) + 0.000
16API C I QBMX = DDXH * ( BMX2(I-1) - 2.0 * BMX2(IL + 0.000
        QBMY = DDXH * % BMX2(I-1L - 2.0 * BMX2(IL
        & + BMY(I) )
        QBMY = DDXH * ( BMYH1(I) - 2.0 * BMY(I) + 0.000
        QBMY = DDXH * % BMY2(I-1L - 2.0 * BMY2(IL
        & + BMY(I) )
        QBMY = DDXH * ( WM1(I-1) * CH(I)
        & - W(I-1) * ( CH(I) + CHP1(I) )
        & + WP1(I-1) * CHP1(I)
        & - WM1(I) * ( CH(I) + CH(I+1) )
        & + W(I) * ( CH(I) + CHP1(I) + CH(I+1)
        & + CHP1(I+1) )
        & - WP1(I) * ( CHP1(I) + CHP1(I+1) )
        & + WM1(I+1) * CH(I+1)
        & - W(I+1) * ( CH(I+1) + CHP1(I+1) )
        & + WP1(I+1) * CHP1(I+1) )
        CTMY = QTMX
        QPX = ODHX * ( W(I-1) * PX(I)
        & - W(I) * ( PX(I) + PX(I+1) )
        & + W(I+1) * PX(I+1) )
        QPY = CDHY * ( WM1(I) * PY(I)
        & - W(I) * ( PY(I) + PYP1(I) )
        & + WP1(I) * PYP1(I) )
        CPBX = ODHX * ( W(I-1) * PBX(I)
        & - W(I) * ( PBX(I) + PBX(I+1) )
        & + W(I+1) * PBX(I+1) )
        QPBX = CDHY * ( WM1(I) * PBY(I)
        & - W(I) * ( PBY(I) + PBYP1(I) )
        & + WP1(I) * PBYP1(I) )
        IF ( I = 2 ) 9580, 7450, 7500
        7450 QRX = 0.25 * ODHX2 * ( - RX(I-1) * W(I)
        & + RX(I+1) * ( - W(I) + W(I+2) ) )
        GO TO 7650
        7500 IF ( MXP2 - I ) 9980, 7600, 7550
        7550 QRX = 0.25 * ODHX2 * ( - RX(I-1) * (
        & - W(I-2) * W(I) )
        & + RX(I+1) * ( - W(I) + W(I+2) ) )
        GO TO 7650
        7600 QRX = 0.25 * ODHX2 * ( - RX(I-1) * (
        & - W(I-2) * W(I) )
        & - RX(I+1) * W(I) )
        7650 QRY = 0.25 * ODHY2 * ( - RYH1(I) *
        & ( - WM2(I) * W(I) )
        & + RYPL(I) * ( - W(I) * WP2(I) ) )
        QTX = 0.5 * CDHX * ( - TX(I-1) * TX(I+1) )
        QTY = 0.5 * CDHY * ( - TY(I-1) * TY(I+1) )
C----COMPUTE TWISTING MOMENTS
        WSUM3 = 0.0625 * ODHXHY * ( WM1(I-1) - WP1(I-1)
        & - WM1(I+1) * WP1(I+1) )
        TMX = WSUM3 * ( CH(I) + CHP1(I)
        & + CH(I+1) * CHP1(I+1) )
C----COMPUTE SUPPORT REACTIONS
        REACT = -S(I) * W(I)
C----COMPUTE STATICS CHECK ERROR
        STACH = QBMX + QBYM + QBMX + QBMX + QTMY + QTMY
        1 11FE1 2 11FE1 - QPX - CPY - QPBX - QPBX - QRY - QRY
        - QTX - QTY - Q(I) - QM(I) - REACT
        IF ( S(I) .LT. 1.E+19 ) GC TO 7700
        REACT = STACH
        STACH = C.
C----SUMMATION OF REACTIONS
        T700 SUMR = SUMR + REACT
        ROCK = REACT
        IF ( KRCPT .EQ. 1 ) ROCK = STACH
        IF ( DABS(STACH) .LE. DABS(STEMP) ) GO TO 8040
        C IF % ABS(STACH) .LE. ABS(STEMP) L GO TO 8040
        STEM = STACH
        ITEMP = ISEA
        JTEMP = JSTA
C----PRINT BEAM ONLY OUTPUT
        8040 IF ( SWS ) 5980, 8050, 8060
        8050 IF ( IPRINT ) 5980, 8058, 8054
C----PRINT SLAB ONLY OUTPUT
        8060 IF ( SWS ) 5980, 8050, 8060
        8064 IF ( SWS ) 5980, 8058, 8064
        8068 IF ( SWS ) 5980, 8050, 8060
        8072 IF ( SWS ) 5980, 8058, 8064
        8076 IF ( SWS ) 5980, 8050, 8060
        8080 IF ( SWS ) 5980, 8058, 8064
        8084 IF ( SWS ) 5980, 8050, 8060
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SUBROUTINE STIFF



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SUBROUTINE STIF  ( LI , AA , ETI , CC , DD , 23N01
1   EE, FF, ML, JI, NL, N2, N3, 26SE8
2   DX, DYM1, DY, DPY1, FX, FYM1, 26SE8
3   FY, FYP1, Q, S, 26SE8
4   RX, RYM1, RYP1, TX, TYM1, TYP1, CH, CHP1, 26SE8
5   PX, PY, PYP1, PBX, PBY, PBYP1, QM, 29JL9
6   DXP1, DXM1 ) 05AG9
    DOUBLE PRECISION PDHXHY, HYDHX3, ODHX2, ODHX, 24MY1B
1   ODHXHY, HXDHY3, ODHY3, ODHY2, ODHY, CRD 24MY1B
    DOUBLE PRECISION AA, ET1, CC, DD, EE, FF 23ND1B
    DIMENSION AA( LI ), 1, ET1( LI ), CC( LI, 5 ), 26SE8
1   DD( LI, 3 ), EEI( LI, 1 ), FF( LI, 1 ) 26SE8
    DIMENSION DX( LI ), 03MA1
1   DYM1( LI ), ODXP1( LI ), 03MA1
2   DY( LI ), DYM1( LI ), ODPY1( LI ), 03MA1
3   FX( LI ), FYM1( LI ), FYP1( LI ), 03MA1
4   QI( LI ), SI( LI ), 03MA1
5   RX( LI ), RYM1( LI ), RYP1( LI ), 03MA1
6   TX( LI ), TYM1( LI ), TYP1( LI ), 03MA1
7   CH( LI ), CHP1( LI ), 03MA1
8   PX( LI ), PY( LI ), PYP1( LI ), 03MA1
9   PBX( LI ), PBY( LI ), PBYP1( LI ), 03MA1
A   QM( LI ) 100C9
C
COMMON / RI / MXP3, MYP3, NF, ITMPP 02MA1
COMMON / STIFF / PDHXHY, HYDHX3, ODHX2, ODHX, 02MA1
1   ODHXHY, HXDHY3, ODHY3, ODHY2, ODHY, CRD(5) 24MY1
C
C— A SPRING IS PLACED AT PTS BEYOND BOUNDARIES OF THE REAL SLAB
C TO MAKE SOLUTION OF NON-RECTANGULAR SLABS OR SLABS WITH
C HOLES POSSIBLE. THIS IS DONE BY TESTING ON THE CC(1,3L)
C TERMS, AND IF ZERO, SET EQUAL TO 1.0 12FE1
C
58 FORMAT I //40H *** UNDESIGNATED ERROR STOP ***
      JN = JI - 2 25SE8
      CALL DATA ( LI, JN, ML, MXP3,
1   DX, DYM1, DY, DPY1, FX, FYM1, 12FE1
2   FY, FYP1, Q, S, 24JE0
3   RX, RYM1, RYP1, TX, TYM1, TYP1, 24JE0
4   CH, CHP1, 05AG9
5   PX, PY, PYP1, PBX, PBY, PBYP1, 24JE0
6   QM, IPRINT 15M81
      DO 2407 I = 1, MXP3 18JL8
      IF ( I - 1 ) 9980, 2402, 2403 18JL8
2402     FF(I,1) = 0.5 * ODHX * TX(I+1) 18JL8
      GD TO 2406 18JL8
2403     IF ( MXP3 - 1 ) 9980, 2405, 2404 18JL8
2404     FF(I,1) = Q(I) + QM(I) 24JE0
1   + 0.5 * ODHX * (- TX(I-1) + TX(I+1)) 24JE0
2   + 0.5 * ODHY * (- TYM1(I) + TYP1(I)) 24JE8
      GD TO 2406 18JL8
2405     FF(I,1) = - 0.5 * ODHX * TX(I-1) 18JL8
2406     AA(I,1) = HXDHY3 * DYM1(I) + ODHY3 * FYM1(I) 18JL8
1   - 0.25 * ODHY2 * RYM1(I) 24JE8
      CONTINUE 18JL8
2407     IF ( ML ) 2700, 2408, 2408 13SE8
2408     DO 2500 I = 1, MXP3 18JL8
      IF ( I - 1 ) 9980, 2421, 2422 13SE8
2421     CC(1,3) = HYDHX3 * DX(I+1) + ODHX3 * FX(I+1) 25SE8
1   + 0.25 * ODHX2 * RX(I+1) 24JE8
      CRD(3) = AMIN1( DX(I+1), DY(I+1) ) 29JL9
      IF ( CO(1,3) ) 2441, 2440, 2441 24JE8
2440     CC(1,3) = 1.0 29JL9
2441     CC(1,4) = - 2.0 * ( HYDHX3 * DX(I+1) + ODHX3 * FX(I+1) ) 21JE8
1   + PDHXHY * CRD(3) ) 29JL9
2   - ODHX * ( PX(I+1) + PBX(I+1) ) 24JE8
1   CC(1,5) = HYDHX3 * DX(I+1) + ODHX3 * FX(I+1) 24JE8
2   - 0.25 * ODHX2 * RX(I+1) 24JE8
      OD(I,2) = 0.D 08DE7
      DD(I,1) = PDHXHY * CRD(3) 29JL9
      ETI(I,1) = EEI(I,1) 25SE8
      EE(I,1) = D.D 03JA8
      GO TO 2500 24JE8
2428     IF ( MXP3 - 1 ) 9980, 2429, 2429 24JE8
    2429     IF ( CC(1,3) ) 2444, 2443, 2444 24JE8
      CC(1,3) = 1.0 29JL9
      DD(I,3) = PDHXHY * CRD(1) 29JL9
      DD(I,2) = 0.0 08DE7
      ETI(I,1) = EEI(I,1) 25SE8
      EE(I,1) = 0.0 03JA8
    2423     CRD(1) = AMIN1( DX(I-1), DY(I-1) ) 29JL9
      CRD(2) = AMIN1( DX(I) , DY(I) ) 29JL9
      CRD(3) = AMIN1( DX(I+1) , DY(I+1) ) 29JL9
      CRD(5) = AMIN1( DXP1(I) , DPY1(I) ) 29JL9
      CC(1,2) = - 2.0 * ( HYDHX3 * ( DX(I-1) + DX(I) ) 29JL9
1   + ODHX3 * ( FX(I-1) + FX(I) ) 19JE8
2   + PDHXHY * ( CRD(1) + CRD(2) ) 29JL9
3   + ODHXHY * ( CH(I) + CHP1(I) ) ) 19JE8
      CC(1,3) = HYDHX3 * ( DX(I-1) + 4.00* DX(I) + DX(I+1) ) 19JE8IBM
1   + ODHX3 * ( FX(I-1) + 4.00* FX(I) + 19JE8IBM
2   FX(I+1) ) + HXDHY3 * I DYM1(I) + 19JE8IBM
3   4.00* DY(I) + DPY1(I) + ODHY3 * 21JE8IBM
4   ( FYM1(I) + 4.00* FY(I) + FYP1(I) ) 19JE8IBM
5   + PDHXHY * 4.0 * ( CRD(2) + CRD(2) ) 29JL9IBM
6   + ODHXHY * 2.0 * ( CH(I) + CH(I+1) + 0.0019JE8IBM
7   CHP1(I) + CHP1(I+1) ) 19JE8IBM
8   + CDHX * ( PX(I) + PX(I+1) + PBX(I) + 0.00 21JE8IBM
9   + PBX(I+1) ) + ODHY * ( PY(I) + PYP1(I) + 23ND1IBM
A   0.DD + PBV(I) + PBV(I+1) ) + SI(I) 19JE8IBM
B   - 0.25 * ODHX2 * ( - RX(I-1) - RX(I+1) ) 19JE8IBM
C   - 0.25 * ODHY2 * ( - RYM1(I) - RYP1(I) ) 19JE8IBM
CC(1,3L) = HYDHX3 * ( DX(I-1) + 4.0 * DX(I) + DX(I+1) ) 19JE8CDC
1   + ODHX3 * ( FX(I-1) + 4.0 * FX(I) + 19JE8CDC
2   FX(I+1) ) + HXDHY3 * ( DYM1(I) + 19JE8CDC
3   4.0 * DY(I) + DPY1(I) + ODHY3 * 21JE8CDC
4   ( FYM1(I) + 4.0 * FY(I) + FYP1(I) ) 19JE8CDC
5   + PDHXHY * 4.0 * ( CRD(2) + CRD(2) ) 29JL9CDC
6   + ODHXHY * 2.0 * ( CH(I) + CH(I+1) + 0.0019JE8CDC
7   CHP1(I) + CHP1(I+1) ) 19JE8CDC
8   + CDHX * ( PX(I) + PX(I+1) + PBX(I) + 21JE8CDC
9   + PBX(I+1) ) + ODHY * ( PY(I) + PYP1(I) + 23ND1IBM
A   0.DD + PBV(I) + PBV(I+1) ) + SI(I) 19JE8CDC
B   - 0.25 * ODHX2 * ( - RX(I-1) - RX(I+1) ) 19JE8CDC
C   - 0.25 * ODHY2 * ( - RYM1(I) - RYP1(I) ) 19JE8CDC
      IF ( CC(1,3) ) 2425, 2424, 2425 21JE8
      CC(1,3) = 1.0 29JL9
      CC(1,4) = - 2.0 * ( HYDHX3 * ( DX(I) + DX(I+1) ) 05AG8
1   + ODHX3 * ( FX(I) + FX(I+1) ) 05AG8
2   + PDHXHY * ( CRD(3) + CRD(2) ) 29JL9
3   + ODHXHY * ( CH(I+1) + CHP1(I+1) ) ) 19JE8
4   - ODHX * ( PX(I+1) + PBX(I+1) ) 19JE8
      DD(1,3) = PDHXHY * ( CRD(1) + CRD(5) ) 29JL9
1   + PDHXHY * 2.0 * CHP1(I) 19JE8
      DD(1,2) = - 2.0 * ( HXDHY3 * ( DY(I) + DPY1(I) ) 05AG8
2   + ODHY3 * ( FY(I) + FYP1(I) ) 05AG8
3   + PDHXHY * ( CRD(2) + CRD(5) ) 29JL9
4   + ODHXHY * ( CHP1(I) + CHP1(I+1) ) ) 21JE8
5   - ODHY * ( PY(I+1) + PBV(I+1) ) 19JE8
      DD(1,1) = PDHXHY * ( CRD(3) + CRD(5) ) 29JL9
1   + ODHXHY * 2.0 * CHP1(I+1) 19JE8
      ETI(I,1) = EEI(I,1) 25SE8
      EE(I,1) = HXDHY3 * DPY1(I) + ODHY3 * FYP1(I) 19JE8
1   - 0.25 * ODHY2 * RYP1(I) 19JE8
      IF ( I - 2 ) 9980, 2427, 2426 24JE8
      CC(1,1) = HYDHX3 * DX(I-1) + ODHX3 * FX(I-1) 24JE8
1   - 0.25 * ODHX2 * RX(I-1) 19JE8
      IF ( MXP3 - 1 ) 9980, 2500, 2428 02JU1
      CC(1,5) = HYDHX3 * DX(I+1) + ODHX3 * FX(I+1) 19JE8
1   - 0.25 * ODHX2 * RX(I+1) 19JE8
      GO TO 2500 19JE8
      CRD(1) = AMIN1( DX(I-1), DY(I-1) ) 29JL9
      CC(1,1) = HYDHX3 * DX(I-1) + ODHX3 * FX(I-1) 25SE8
1   - 0.25 * ODHX2 * RX(I-1) 21JE8
      CC(1,2) = - 2.0 * ( HYDHX3 * DX(I-1) + ODHX3 * FX(I-1) 19JE8
2   + PDHXHY * CRD(1) ) 29JL9
      CC(1,3) = HYDHX3 * DX(I-1) + ODHX3 * FX(I-1) 21JE8
1   + 0.25 * ODHX2 * RX(I-1) 19JE8
      IF ( CC(1,3) ) 2444, 2443, 2444 19JE8
      CC(1,3) = 1.0 29JL9
      DD(I,3) = PDHXHY * CRD(1) 29JL9
      DD(I,2) = 0.0 08DE7
      ETI(I,1) = EEI(I,1) 25SE8
      EE(I,1) = 0.0 03JA8
    
```

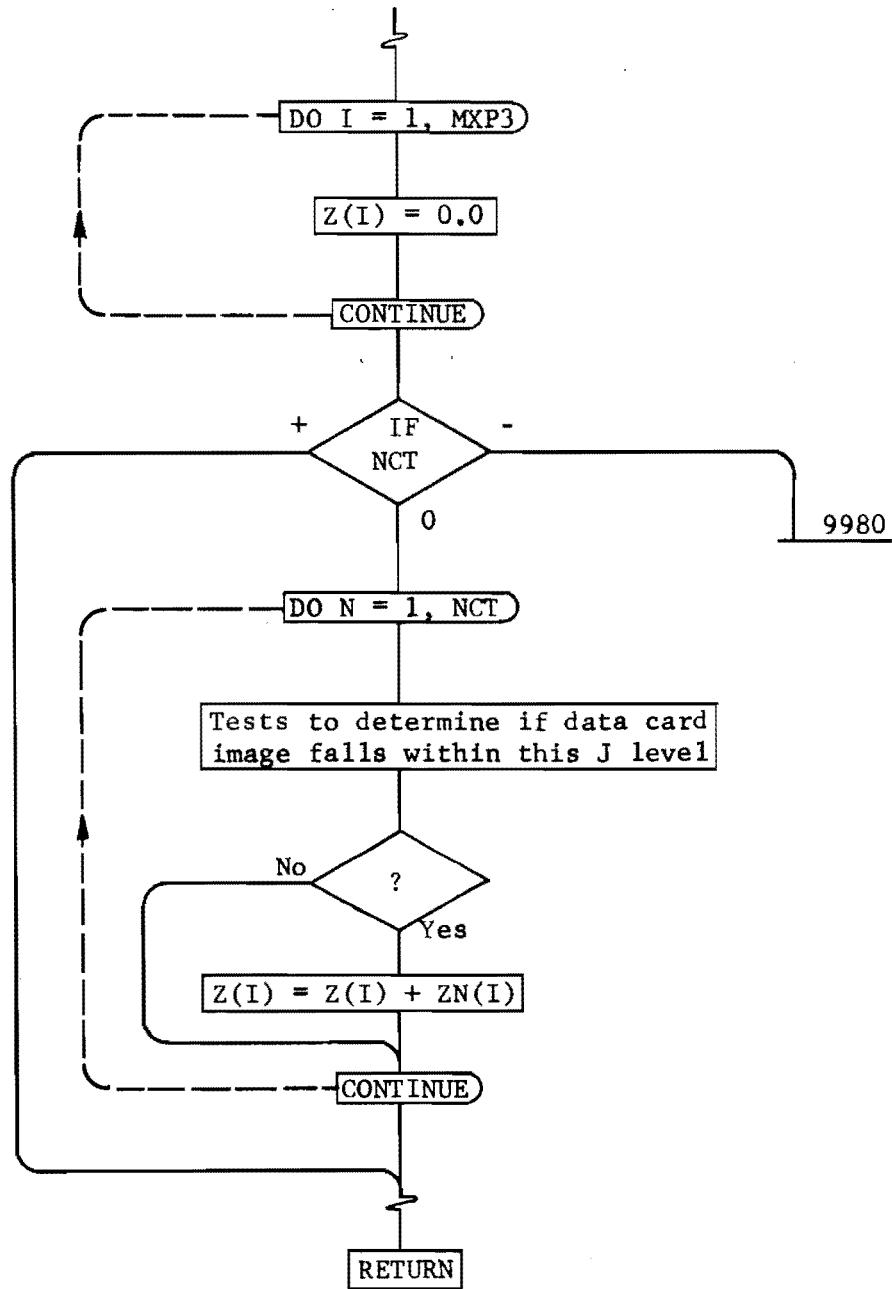
```

2500    CONTINUE
        DD 2610 L = 2, MXP3
              J = MXP3 - L + 2
              DD(J,1) = DD(J-1,1)
2610    CONTINUE
              DD(1,1) * 0.0
        DD 2620 J = 1, MXP3
              DD(1,3) = DD(1+1,3)
2620    CONTINUE
              DD(MXP3,3) = 0.0
              DD(1,1) = DD(1,2)
              DD(1,2) = DD(1,3)
              DD(1,3) = 0.0
              CC(1,1) = CC(1,3)
              CC(1,2) = CC(1,4)
              CC(1,3) = CC(1,5)
              CC(1,4) = CC(2,2)
              CC(1,5) = CC(2,3)
              CC(2,1) = CC(2,3)
              CC(2,2) = CC(2,4)
              CC(2,3) = CC(2,5)
              CC(2,4) = CC(2,6)
              CC(2,5) = 0.0
              CC(1,4) = 0.0
              CC(1,5) = 0.0
              I = MXP3
              DD(I,3) = DD(I,2)
              DD(I,2) = DD(I,1)
              DD(I,1) = 0.0
              CC(I,5) = CC(I,3)
              CC(I,4) = CC(I,2)
              CC(I,3) = CC(I,1)
              CC(I-1,5) = CC(I-1,4)
              CC(I-1,4) = CC(I-1,3)
              CC(I-1,3) = CC(I-1,2)
              CC(I-1,2) = CC(I-1,1)
              CC(I-1,1) * 0.0
              CC(I,2) = 0.0
              CC(I,1) = 0.0
2700    CONTINUE
        RETURN
9980 PRINT 98
END

```

SUBROUTINE DATA

This routine is called at each J (y-direction) level to distribute data from tables 3, 4, 5, 6, 7, and 9. Z(I) refers to the distributed value in each table. ZN(I) is the data input from the data cards.



```

SUBROUTINE DATA I   LI, JN, ML, MXP3,          110E7
1      LX, DYM1, DY, DP1, FX, FY1,           24JEP
2      FY, FPI1, Q, S,                      24JEC
3      RX, RYM1, RYPI, TX, TY1, TYP1,        24JER
4      CH, CHP1, DP1, DX1,                  05AG9
5      PX, PY, PYP1, PBX, PBY, PBYP1,       24JFP
6      QM, IPRTN                         15MR1

C----THIS SUBROUTINE IS CALLED AT EACH J STEP IN THE STIFFNESS MATRIX 12JER
C GENERATION AND AGAIN AT EACH J STEP WHEN COMPUTING RESULTS. 12JEB
C

C DIMENSION DX(LI), DXM1(LI), DXPI(LI), 100C9
1  OY(LI), OYM1(LI), OYPI(LI), 100C9
2  FX(LI), FY(LI), FYPI(LI), 100C9
3  Q(LI), S(LI), 100C9
4  RX(LI), RYM1(LI), RYPI(LI), 100C9
5  TX(LI), TY1(LI), TYP1(LI), 100C9
6  CH(LI), CHP1(LI), 100C9
7  PX(LI), PY(LI), PYP1(LI), 100C9
8  PBX(LI), PBY(LI), PBYP1(LI), 100C9
9  QM(LI) 100C9
A  COMMON /CARDS/ AN1(40), AN2(18),
1  IN1(300), JN1(300), IN23(300), JN23(300), DXN(300), 235E0
2  OYN(300), FYN(300), FYN(300), QN1(300), SN1(300), 235E0
3  IN14(50), JN14(50), IN24(50), JN24(50), 235E0
4  RXN(50), RYN(50), TXN(50), TYN(50), 235E0
5  IN15(100), JN15(100), IN25(100), JN25(100), CHN(100), 235E0
6  IN16(50), JN16(50), IN26(50), JN26(50), 235E0
7  PKN(50), PYN(50), PBXN(50), PBYN(50), 235E0
8  IN17(100), JN17(100), IN27(100), JN27(100), QMN(100), 03MA1
9  IN18(10), JN18(10), IN28(10), JN28(10), 17FPI
A  KASEW1(10), KASEX1(10), KASEY1(10), KASEP1(10), 17FEI
B  JN19(10), JN29(10), 17FEI
COMMON /TABLE/NC13, NC13, NC14, NC15, NC15, NC16, NC16, NCT6,
1  NC17, NC17, NC18, NC18, NC19, NC19, 15MR1

C 98 FORMAT //30H UNDESIGNATED ERROR STOP 1 21A38
C----DISTRIBUTE DATA FROM TABLE 3 12JEB
C

300  DO 305 I = 1, MXP3
      DX(I) = 0.0
      OXPI(I) = 0.0
      OXM1(I) = 0.0
      OYMI(I) = 0.0
      OYPI(I) = 0.0
      FXPI(I) = 0.0
      FYMI(I) = 0.0
      FYPI(I) = 0.0
      Q(I) = 0.0
      S(I) = 0.0
305  CONTINUE
      IF ( NCT3 > 980, 400, 310
310  DO 360 N = 1, NCT3
         11 = IN13(N) + 2
         12 = IN23(N) + 2
         IF ( JN - JN13(N) > 345, 315, 315
315  IF ( JN23(N) - JN > 330, 320, 320
320  DO 325 I = 11, 12
            DX11 = DX(I) + DXN(N)
            DY11 = DY(I) + DYN(N)
            FX11 = FX(I) + FXN(N)
            FY11 = FY(I) + FYN(N)
            Q11 = Q(I) + QMN(N)
            S11 = S(I) + SN1(N)
325  CONTINUE
330  IF I (JN-11 - JN13(N)) > 345, 333, 333
333  IF I (JN23(N) - (JN-11)) > 345, 335, 335
335  DO 340 I = 11, 12
            OYMI(I) = OYMI(I) + OYN(N)
            OXM1(I) = OXM1(I) + DXN(N)
340  CONTINUE
345  IF I (JN+11 - JN13(N)) > 360, 347, 347
347  IF I (JN23(N) - (JN+11)) > 360, 350, 357
350  DO 355 I = 11, 12
            DXPI(I) = DXPI(I) + DXN(N)
            OYPI(I) = OYPI(I) + DYN(N)
            FYPI(I) = FYPI(I) + FYN(N)
355  CONTINUE
360  CONTINUE

C----DISTRIBUTE DATA FROM TABLE 4 12JF6
C

400  DO 405 I = 1, MXP3
      RX(I) = 0.0
      RYM1(I) = 0.0
      RYPI(I) = 0.0
      TX(I) = 0.0
      TY1(I) = 0.0
      TYP1(I) = 0.0
405  CONTINUE
      IF ( NCT4 > 980, 500, 410
410  DO 460 N = 1, NCT4
         11 = IN14(N) + 2
         12 = IN24(N) + 2
         IF I (JN - JN14(N)) > 445, 415, 415
415  IF I (JN24(N) - JN) > 430, 420, 420
420  DO 425 I = 11, 12
            RX(I) = RX(I) + RXN(N)
            TX(I) = TX(I) + TXN(N)
425  CONTINUE
      IF I (JN-11 - JN14(N)) > 445, 433, 433
430  IF I (JN24(N) - (JN-11)) > 445, 435, 435
435  DO 440 I = 11, 12
            RYM1(I) = RYM1(I) + RYN(N)
            TY1(I) = TY1(I) + TYN(N)
440  CONTINUE
      IF I (JN+11 - JN14(N)) > 460, 447, 447
445  IF I (JN24(N) - (JN+11)) > 460, 450, 450
450  DO 455 I = 11, 12
            RYPI(I) = RYPI(I) + RYN(N)
            TYP1(I) = TYP1(I) + TYN(N)
455  CONTINUE
C----DISTRIBUTE DATA FROM TABLE 5 12JF6
C

500  DO 505 I = 1, MXP3
      CH(I) = 0.0
      CHP1(I) = 0.0
505  CONTINUE
      IF ( NCT5 > 980, 600, 510
510  DO 560 N = 1, NCT5
         11 = IN15(N) + 2
         12 = IN25(N) + 2
         IF I (JN - JN15(N)) > 545, 515, 515
515  IF I (JN25(N) - JN) > 545, 520, 520
520  DO 525 I = 11, 12
            CH(I) = CH(I) + CHN(N)
525  CONTINUE
      IF I (JN+11 - JN15(N)) > 560, 547, 547
545  IF I (JN25(N) - (JN+11)) > 560, 550, 550
550  DO 555 I = 11, 12
            CHP1(I) = CHP1(I) + CHN(N)
555  CONTINUE
560  CONTINUE

C----DISTRIBUTE DATA FROM TABLE 6 12JF6
C

600  DO 605 I = 1, MXP3
      PX(I) = 0.0
      PY(I) = 0.0
      PYP1(I) = 0.0
      PBX(I) = 0.0

```

```

PBY(I) = C-0
PBYP(I) = 0.0
605    CONTINUE
IF I NCT6 ) 960, 700, 610
610    DO 660 N = 1, NCT6
      11 = IN16(IN) + 2
      12 = IN26(IN) + 2
      IF ( JN - JN16(IN) ) 645, 615, 615
615    IF ( JN26(IN) - JN ) 645, 620, 620
620    DO 625 I = 11, 12
      PX(I) = PX(I) + PXM(N)
      PY(I) = PY(I) + PYN(N)
      PBX(I) = PBX(I) + PBXN(N)
      PBY(I) = PBY(I) + PBYN(N)
625    CONTINUE
635    IF ( (JN+1) - JN16(IN) ) 660, 647, 647
647    IF ( (JN26(IN) - (JN+1) ) 660, 650, 650
650    DO 655 I = 11, 12
      PYPL(I) = PYPL(I) + PYN(N)
      PBYP(I) = PBYP(I) + PBYN(N)
655    CONTINUE
660    CONTINUE
C-----DISTRIBUTE DATA FROM TABLE 7
C
-700    DO 705 I = 1, NXP3
      QM(I) = 0.0
705    CONTINUE
IF ( NCT7 ) 980, 800, 710
710    DO 760 N = 1, NCT7
      11 = IN17(IN) + 2
      12 = IN27(IN) + 2
      IF ( JN - JN17(IN) ) 760, 715, 715
715    IF ( JN27(IN) - JN ) 760, 720, 720
720    DO 725 I = 11, 12
      QM(I) = QM(I) + QMN(N)
725    CONTINUE
760    CONTINUE
800    CONTINUE
C-----DISTRIBUTE DATA FROM TABLE 9
C
900        IPRINT = 0
IF ( NCT9 ) 980, 965, 910
910    DO 960 N = 1, NCT9
      IF ( JN - JN19(IN) ) 920, 965, 915
915    IF ( JN29(IN) - JN ) 920, 965; 965
920        IPRINT = 0
960    CONTINUE
GO TO 975
965        IPRINT = 1
975    CONTINUE
RETURN
980 PRINT 98
END

```

PRINTER PLOT ROUTINES

These subroutines generate plots of the selected profile output areas specified by Table 8. Subroutines SPL0T3 and SPL0T4 control the printer plots. SPL0T 3 is used for single column plots and SPL0T 4 for double column plots. The columns of values are tabulated vertically at the left of the printer plot display. Subroutine ZOT 1 utilizes the same profile values and creates only line plots on either microfilm or paper depending on the plot option control of Table 1. The IBM version of the program does not have subroutine ZOT 1.

```

SUBROUTINE SPLOT 3 ( X, NEND, WIDTH )           18N09
C * * * * THE LATEST REVISION DATE FOR THIS ROUTINE IS - - 12 DEC 69 REVISED
DIMENSION X(NEND), SPACE(15), SYMB(4)          100C9
COMMON /PLT / II, 12, J1, J2                  01DE1B8M
DATA SPACE / 15*H      /, SYMB / 4H* .4H * .4H * .4H * / 25N09CDC
C***** THIS ROUTINE PRINTS AND PLOTS THE NEND VALUES OF X           100C9
C BEGINNING WITH THE INITIAL VALUE TRANSFERRED,                   100C9
C THE PAPER SHOULD BE POSITIONED PROPERLY AND ALL               100C9
C HEADING PRINTED BEFORE CALLING. F. L. E.                      100C9
C **** INPUT - X, THE FUNCTION TO BE PLOTEO                     100C9
C           NEND, THE NUMBER OF X TO BE PLOTTED                   100C9
C           WIDTH, WIDTH OF PLOT% LESS THAN 61 L                 100C9
C **** DPUTPUT- NO ACTUAL VALUES ARE RETURNED TO THE            100C9
C           CALLING ROUTINE - THE VALUES ARE PRINTED             100C9
C           AND PLOTTED VERTICALLY.                                100C9
10 FORMAT ( 5X, I2, IX, I3, IX, 1PE10.3,16A4 )   25N09IB8
C 10 FORMAT 5X, I2, IX, I3, IX, E10.3,16A4 L       25N09CDC
15 FORMAT ( 5X,I2,IX,I3,IX, 1PE10.3 )            25N09IB8
C 15 FORMAT % 5X,I2,IX,I3,IX, E10.3 L            25N09CDC
IF ( NEND .LE. 0 )                               GO TO 990 11N09
IF ( WIDTH.GT.60. .OR. WIDTH.LT.1. )           WIDTH = 60. 150C9
ISKP = WIDTH / 8 + 1                           140C9
SYMD = SYMB(1)                                 100C9
OMEGA = X(1)                                   100C9
THETA = X(1)                                   100C9
IF ( NEND .EQ. 1 )                               GO TO 120 25N09
DO 50 I = 2, NEND                            100C9
IF ( OMEGA.LT.X(I) )                         OMEGA = X(I) 100C9
IF ( THETA.GT.X(I) )                          THETA = X(I) 100C9
50 CONTINUE                                     100C9
IF ( OMEGA.EQ.THETA )                         GO TO 60   25N09
SIGMA = 1 WIDTH - 1 - 1 / ( OMEGA - THETA ) 100C9
25N09
60 CONTINUE                                     25N09
IF ( (J2-J1-12+II) .GT. 0 )                   GO TO 150 120E9
I = 0                                         25N39
DO 110 JS = J1, J2                           25N09
JSTA = JS - 2                                25N09
DO 100 IS = II, 12                           25N09
ISTA = IS - 2                                25N09
I = I + 1                                    25N09
IF ( OMEGA.EQ.THETA )                         GO TO 80 100C9
BETA = SIGMA * ( X(II) - THETA ) + 1.        100C9
IOTA = BETA                                   100C9
IF ( (BETA - IOTA).GE.0.5 )                  IOTA = IOTA + 1 100C9
ISKP = ( IOTA - 1 ) / 4                      100C9
IR = IOTA - 4 * ISKP                         100C9
ISKP = ISKP + 1                             100C9
SYMD = SYMB(IR)                            100C9
180 PRINT 10, ISTA, JSTA, X(I), ( SPACE(I), L=1, ISKP ), SYMD 100C9
100 CONTINUE                                     100C9
110 CONTINUE                                     100C9
GO TO 990                                     100C9
150 CONTINUE                                     100C9
I = 0                                         120E9
DO 210 IS = II, 12                           120E9
ISTA = IS - 2                                120E9
DO 200 JS = J1, J2                           120E9
JSTA = JS - 2                                120E9
I = I + 1                                    25N09
IF ( OMEGA.EQ.THETA )                         GO TO 180 120E9
BETA = SIGMA * ( X(II) - THETA ) + 1.        100C9
IOTA = BETA                                   100C9
IF ( (BETA - IOTA).GE.0.5 )                  IOTA = IOTA + 1 100C9
180 PRINT 10, ISTA, JSTA, X(I), ( SPACE(I), L=1, ISKP ), SYMD 100C9
200 CONTINUE                                     100C9
210 CONTINUE                                     100C9
GO TO 990                                     100C9
120 PRINT 15, II, J1, X(I)                    120E9
990 RETURN                                     25N39
C END SPLOT 3                                11N09
END                                           18N09
100C9
SUBROUTINE SPLOT 4 ( X, Y, NEND )           18N39
C * * * * THE LATEST REVISION DATE FOR THIS ROUTINE IS - - 25 NOV 69 REVISED
DIMENSION X(NEND), Y(NEND), SPACE( 8 ), SYMB(4) 150C9
COMMON /PLT / II, 12, J1, J2                  01DE1B8M
DATA SPACE / 8*H      /, SYMB / 4H* .4H * .4H * .4H * / 25N09CDC
C***** THIS ROUTINE PRINTS AND PLOTS THE NEND VALUES OF X AND Y 100C9
C BEGINNING WITH THE INITIAL VALUE TRANSFERRED,                   100C9
C THE PAPER SHOULD BE POSITIONED PROPERLY AND ALL               100C9
C HEADING PRINTED BEFORE CALLING. F. L. E.                      100C9
C **** INPUT - X, THE FUNCTION TO BE PLOTEO                     100C9
C           Y, THE FUNCTION TO BE PLOTEO                         140C9
C           NEND, THE NUMBER OF X OR Y TO BE PLOTTED             140C9
C **** DPUTPUT- NO ACTUAL VALUES ARE RETURNED TO THE            100C9

```

```

C          CALLING ROUTINE - THE VALUES ARE PRINTED
C          AND PLOTTED VERTICALLY.
10 FORMAT ( 5X, '12,1X,I3,1X, 1PE10.3,7A4, 1PE10.3,7A4 )      100C9
C          10 FORMAT 5X, 12,1X,I3,1X, E10.3,7A4, E10.3,7A4 L      100C9
15 FORMAT ( 5X,12,1X,I3,1X, 1PE10.3,28X, 1PE10.3 )      25ND91AM
C          15 FORMAT 5X,12,1X,I3,1X, E10.3,28X, E10.3 L      25ND9CDC
C          IF ( NEND .LE. 0 )      GO TO 990      25ND918M
C
C          IW = 20      25ND9CDC
C          IX = IW / 4      11N09
C          ISKP = IW / 8      1BN39
C          ISKP2 = ISKP      140C9
C          ISKPT = 4      140C9
C          ISKPF = ISKP      140C9
C          SYMD = SYMB(1)      140C9
C          SYMD2 = SYMB(1)      140C9
C          WIDTH = IW      140C9
C          OMEGA = X(1)      140C9
C          THETA = X(1)      140C9
C          O2 = Y(1)      140C9
C          T2 = Y(1)      140C9
C          IF ( NEND .EQ. 1 )      GO TO 120      25ND9
C          00 50 I = 2, NEND      100C9
C          IF ( OMEGA.LT.X(1) )      OMEGA = X(1)      100C9
C          IF ( THETA.GT.X(1) )      THETA = X(1)      100C9
C          IF ( O2.LT.Y(1) )      O2 = Y(1)      140C9
C          IF ( T2.GT.Y(1) )      T2 = Y(1)      140C9
C          CONTINUE      100C9
C          IF ( OMEGA.EQ.THETA )      GO TO 60      25ND9
C          SIGMA = ( WIDTH - 1. ) / ( OMEGA - THETA )      100C9
C          60  IF ( O2.EQ.T2 )      GO TO 70      140C9
C          S2 = ( WIDTH - 1 ) / ( O2 - T2 )      140C9
C          CONTINUE      25ND9
C          I = 0      25ND9
C          DO 110 JS = J1, J2      25ND9
C          JSTA = JS - 2      25ND9
C          DO 100 IS = 11, 12      25ND9
C          ISTA = IS - 2      25ND9
C          I = I + 1      25ND9
C          IF ( OMEGA.EQ.THETA )      GO TO 80      100C9
C          BETA = SIGMA * ( X(1) - THETA ) + 1.      100C9
C          IOTA = BETA      100C9
C          IF ( (BETA - IOTA).GE.0.5 )      IOTA = IOTA + 1      100C9
C          ISKP = ( IOTA - 1 ) / 4      100C9
C          IR = IOTA - 4 * ISKP      100C9
C          ISKP = ISKP + 1      100C9
C          SYMD = SYMB(IR)      100C9
C          80  IF ( O2.EQ.T2 )      GO TO 90      140C9
C          BETA = S2 * ( Y(1) - T2 ) + 1.      150C9
C          IOTA = BETA      140C9
C          IF ( (BETA - IOTA).GE.0.5 )      IOTA = IOTA + 1      140C9
C          ISKP2 = ( IOTA - 1 ) / 4      140C9
C          IR = IOTA - 4 * ISKP2      140C9
C          SYMD2 = SYMB(IR)      140C9
C          ISKP2 = ISKP2 + 1      140C9
C          ISKP = IX - ISKP + 1      150C9
C          90 PRINT 10, ISTA, JSTA, X(1), (SPACE(K),K=1,ISKPI), SYMD
C          1 , (SPACE(L),L=1,ISKPT), Y(I), (SPACE(K),K=1,ISKP2), SYMD2      25ND9
C          100  CONTINUE      25ND9
C          110  CONTINUE      25ND9
C          GO TO 990      25ND9
C          120 PRINT 15, I1, J1, X(1), Y(1)      25ND9
C          990 RETURN      11N09
C          END SPLIT 4      18MY0
C          END      100C9

```

```

C          SUBROUTINE ZUT 1 ( XF, YF, NP, ID )
C          **** THE LATEST REVISION DATA FOR THIS ROUTINE IS - - 13 DEC 71 REVISED
C          COMMON / ZCT / LOP, MC, IROLL, MOP      11JE0
C          DIMENSION XF(1), YF(1), ID(1)      11JE0
C          DATA INC, IQ, X, Y, XL, YL, XU, YU / 1,12,0,.0,.9,.4,.0,.90. /      09JE0
C          DATA IT1, IT2 / -1, 0 /      09JE0
C          XF - ARRAY CONTAINING THE X - COORDINATES      09JE0
C          YF - ARRAY CONTAINING THE Y - COORDINATES      09JE0
C          NP - NUMBER OF POINTS TO BE PLOTTED      09JE0
C          LOP - LINE OR POINT PLOT OPTION      09JE0
C          # 0 , LINE PLOT      09JE0
C          # -J , POINT PLOT AT EVERY J-TH POINT      09JE0
C          # C , LINE PLOT WITH A POINT PLOT AT EVERY J-TH PT.      09JE0
C          ID - VARIABLE OR ARRAY CONTAINING TITLE % 0 IF NO TITLE L      09JE0
C          MC - NUMBER OF CHARACTERS IN TITLE % 0 IF NO TITLE L      09JE0
C          IROLL - OPTION TO MOVE TO A NEW FRAME - AFTER THIS PLOT      09JE0
C          # 0 , SAME FRAME      09JE0
C          GREATER THAN 0 , NEW FRAME      09JE0
C          LESS THAN 0 , TERMINATE      09JE0
C          MOP - MICROFILM OR PAPER PLOT OPTION      09JE0
C          # 1 , PAPER PLOTS      09JE0
C          # 2 , MICROFILM      09JE0
C          **** FRANK L ENDRS - PAX 1892 ****      09JE0
C          NC = MC      09JE0
C          IT1 = IT1 + 1      09JE0
C          IF ( IT1.NE.0 )      GO TO 20      09JE0
C          IF % MOP.EQ.1 L      CALL BGNPLT      09JE0CDC
C          20  IF % MOP.NE.1 L      CALL BGNPLT % 6LFILMPL L      09JE0CDC
C          IF ( NC.NE.0 )      GO TO 50      09JE0
C          NC = 10      09JE0
C          ID # 10H      09JE0CDC
C          50  CONTINUE      09JE0
C          XF(NP+1) = XF1      13DE1
C          XF(NP+2) = XF2      13DE1
C          YF(NP+1) = YF1      13DE1
C          YF(NP+2) = YF2      13DE1
C          ---- POSITION ORIGIN      09JE0
C          IF % IT2.EQ.0L      CALL PLT $1.0, 1.0, -3L      09JE0CDC
C          IF ( IT2.EQ.1 )      GO TO 100      09JE0
C          ---- SCALE X - AXIS      09JE0
C          CALL SCALE ( XF, XL, NP, INC )      09JE0
C          XF1 = XF(NP+1)      13DE1
C          XF2 = XF(NP+2)      13DE1
C          ---- SCALE Y - AXIS      09JE0
C          CALL SCALE ( YF, YL, NP, INC )      09JE0
C          YF1 = YF(NP+1)      13DE1
C          YF2 = YF(NP+2)      13DE1
C          ---- SET UP X-AXIS      09JE0
C          YM = - YF(NP+1) / YF(NP+2)      09JE0
C          CALL AXIS ( X, YM, IH, -1, XL, X0, XF(NP+1), XF(NP+2) )      09JE0
C          ---- SET UP Y-AXIS      09JE0
C          XM = - XF(NP+1) / XF(NP+2)      09JE0
C          CALL AXIS ( XM, Y, IH, 1, YL, Y0, YF(NP+1), YF(NP+2) )      09JE0
C          ---- PRINT TITLE      09JE0
C          CALL SYMBOL ( 1., -.75,.14, ID, X0, NC )      29ND01
C          ---- PLOT THE FUNCTION      09JE0
C          100 CALL LINE ( XF, YF, NP, 1, LCP, IQ )      09JE0
C          IF ( IT2.EQ.1 )      IT2 = 0      09JE0
C          IF % IROLL.GT.0 L      CALL PLT % .0, .0, 999 L      09JE0CDC
C          IF % IROLL.LT.0 L      CALL ENOPLT      09JE0CDC
C          IF ( IROLL.EQ.0 )      IT2 = 1      09JE0
C          IF ( IROLL.LT.0 )      IT1 = -1      11JE0
C          RETURN      09JE0
C          END      09JE0

```

3-D PLOT SUBROUTINES

These nine routines are used to generate the paper plot display of the exaggerated deflected shape of the slab or grid. The routines default to values that have been found to give a reasonable optimum implied position of the viewer's eye. The plot may be controlled by input of different options in the second card of Table 1. The IGSW reference grid switch may be input in column 62, the value of the plotted maximum deflection in columns 66-70, the reduction factor of Y lengths relative to X lengths in columns 71-75, and the tangent of the receding plot angle in columns 76-80.

- DIM3 - Defines the plot size and orientation and controls subsequent routines. PLT and SYMBOL are CALCOMP routines.
- TD2 - Plots the relative deflection value at the appropriate position, connecting all grid points. Hidden points are masked out.
- GRD - Controls the reference grid to be plotted as a function of the reference grid switch IGSW:
 - 0 = Tic marks at edge points only (default)
 - 1 = No reference marks
 - 2 = Tic marks at the 4 corner points
 - 3 = Tic marks at all grid points
 - 4 = Dashed lines along edges only
 - 5 = Solid lines along edges only
 - 6 = Dashed grid lines throughout
 - 7 = Solid grid lines throughout
 - 8 = Dashed grid shifted 12 inches to right
 - 9 = Solid grid shifted 12 inches to right
- EDGE 1 - Plots tics at edge points
- CRNR 1 - Plots tics at the 4 corner points
- MESH 1 - Plots tics at all grid points
- GRID 1 - Plots dashed or solid lines at edges or throughout
- MARK 2 - Plots four arm tic oriented in the proper direction
- DASH 2 - Plots a dashed or solid line

```

SUBROUTINE DIM3 ( Z, L1, L2, MX, MY, HX, HY, VEF, RDF, SLOPE, 09FE1
 1          IPOP, NPROB, IGSW )
C-----BY TEMPORARILY NILLING OUT THIS ROUTINE, THE 3 - D PLOT IS
C   ELIMINATED, AND THESE ROUTINES MAY BE REMOVED FROM THE DECK --
C   DIM3, TD2, GRID1, GRO, CRNRI, EDGE1, MESH1, MARK2, AND DASH2.  THUS
C   SAVING 7150 OCTAL STOR AND 4500 OCTAL STOR FOR SYSTEM PLOT CALLS.
C
C-----THE CARDS TAGGED WITH PLT ARE FOR CDC SYSTEM PLDT Routines AND ARE
C   IN GENERAL COMPATIBLE WITH ALL SYSTEMS USING STANDARD CALCOMP PLOT
C   ROUTINES. MODIFICATIONS FOR PARTICULAR PLOTTER INSTALLATIONS MAY
C   BE NECESSARY HOWEVER.
C
C   DOUBLE PRECISION Z, NPROB
C   DIMENSION Z(L1,L2), Z(2500)
C   IF % IPOP.LE.0 .A. START.NE.1.314 L    CALL BGNPLT      10DE11BM
C   CALL PLOT ( 1., 1., -3 )                  260C0
C   CALL PLT  $ 1., 1., -3 L                 190C0CDC
C   CALL SYMBOL (-1., 3., .14, NPROB, 9D., 5 )      260C0CDC
C   IF ( ABS ( VEF ) .LT. 0.01 ) VEF = 1.0        12FE1
C   IF ( ABS ( SLOPE ) .LT. 0.01 ) SLOPE = 0.9     11FE1
C   IF (      RDF .LT. 0.01 ) RDF  = 0.75       11FE1
C   H = 6.5
C   IOFY = H * 100 / MY
C   IOFX = IOFY / SLOPE
C   XX = IOFX * IOFX + IOFY * IOFY
C   IDEL = SQRT( XX ) * HX / ( HY * RDF )
C   W = .01 * IDEL * MX * IABS( IOFX ) * MY
C   IF ( W.LE.10 ) GO TO 5
C   H = 10. * H / W
C   IOFY = H * 100 / MY
C   IOFX = IOFY / SLOPE
C   IDEL = 10. / W * IDEL
C   W = 10.
C   5   VSF = 0.
C   MXP2 = MX + 2
C   MYP2 = MY + 2
C   DO 10 I = 2, MXP2
C   DO 10 J = 2, MYP2
C   IF ( DABSI Z(I,J) .GT. VSF ) VSF = DABSI Z(I,J)      260C0IBN
C   IF % ABSZ Z81,JLL.GT.VSF L VSF # ABSZ Z81, JL L      260C0CDC
C   10  CONTINUE
C   VSF = VSF / VEF
C   IN = MY + 1
C   KN = MX + 1
C   JS = 1
C   PLTMIN = -.5
C   INIT = 1
C   DO 100 I = 1, IN
C   100 CALL TD2 ( Z(2,I+1), IN, KN, IDEL, VSF, IOFX, IOFY, IZ, PLTMIN, JS, 260C0
C   !          INIT )
C   XS = 0
C   IF ( IOFX.LT.0 ) XS = .01 * IABS ( IOFX ) * MY      11FE1
C   CALL GRO ( IGSW, MX, MY, IOFX, IOFY, IDEL, XS, YS )      06MY1
C   CALL PLDT ( 15., 0., -3 )                  300C0IBM
C   CALL PLT  $15., 0., -3 L                 300C0CDC
C   START = 1.314
C   RETURN
C   END

```

```

SUBROUTINE TD2 ( Z, IN, KN, IDEL, VSF, IOFX, IOFY, IZ, PLTMIN, JS, 260C0
 1          INIT )                                         11FE1
C   DOUBLE PRECISION Z
C   DIMENSION Z(L1), IZ(L1)
C   DIMENSION XM(50), YM(50)
C   DO 5 I = 1, KN
C   XM(I) = PLTMIN - 1.
C   IEND = KN * IDEL + IABS(10FX) * (IN - 1) + 1
C   INIT = 0
C   NU = 0
C   L = 0
C   MP = 1
C   KL = KN - 1
C   POV = PLTMIN / VSF
C   IPOV = POV * 100.
C   DEL = IDEL / 100.
C   OFY = IOFY / 100.
C   OFX = IOFX / 100.
C   IF I IOFX .GE. 0 ) GO TO 8
C   L = -IOFX * (IN - 1) + 1
C   8   CONTINUE
C   DO 9 I = 1, IEND
C   IZ(I) = IPOV
C   9   CONTINUE
C   IPEN = 3
C   10  CONTINUE
C   II = NU - 1
C   NU = NU + 1
C   II = NU - 1
C   DELY = OFY * II
C   DELX = OFX * II
C   DO 31 I = 1, KL
C   IF ( MP ) 15, 15, 16
C   K = KN + 1 - I
C   GO TO 17
C   15
C   K = 1
C   GO TO 17
C   16  K = 1
C   17  CONTINUE
C-----COMPUTE INTERPOLATION CONSTANT
C   KMP = K + MP
C   SI = ( Z(KMP) - Z(K) ) / VSF
C   SI = .01 * SI / DEL
C   AT = Z(K) / VSF + DELY - SI
C-----INTERPOLATE POINTS
C   DO 30 LL = 1, IDEL
C   L = LL + MP
C   AT = AT + SI
C-----COMPUTE NEW ATTITUDE
C   IR = AT * 100. * IZ(L)
C   IF ( IR .LE. 0 ) GO TO 19
C   IZ(L) = SIGN ( ( ABS ( AT * 100.) + .5) , AT )
C   19  CONTINUE
C-----GENERATE ABSCISSA
C   C = L * .01
C   GO TO 24, 25 ), JS
C   24  X = C
C   Y = IZ(L) * .01
C   GO TO 26
C   25  Y = -C
C   X = IZ(L) * .01
C   26 CALL PLOT( X, Y, IPEN )
C   C 26 CALL PLT $ X, Y, IPEN L
C   IF ( 1.EQ.1 .OR. LL.EQ.1 ) GO TO 265
C   XM(K) = X
C   YM(K) = Y
C   IPEN = 2
C   30  CONTINUE

```

```

C     IF ( XM(KMP).GE.PLTMIN )      CALL PLOT (XM(KMP),YM(KMP),2)    12FEI1BM
C     IF ( XM$KMP.GE.PLTMIN L    CALL PLT  $XM$KMP,YM$KMP,2L    12FEI1CD
C     CALL PLOT ( X, Y, 3 )          CALL PLT  $XM$KMP,YM$KMP,2L    260CO1BM
C     CALL PLT  Z X, Y, 3 L        260CO1CD
C           XM(KMP) = X
C           YM(KMP) = Y
31     CONTINUE
           L = L + IOFX + MP
           MP = - MP
RETURN
END

SUBROUTINE EDGE1 ( MX, MY, IOFX, IOFY, IDEL, XE, YE )
COMMON /PGRID/ SLOPE, SIZE
              X = XO
              Y = YO
              SIZE = 2.
              OFX = 10FX + .01
              OFY = 10FY + .01
              DEL = IDEL * .01
CALL MARK2 ( X, Y )
DO 100 I = 1, MX
           X = X + DEL
CALL MARK2 ( X, Y )
100   CONTINUE
DO 200 I = 1, MY
           X = X + OFX
           Y = Y + OFY
CALL MARK2 ( X, Y )
200   CONTINUE
DO 300 I = 1, MX
           X = X - DEL
CALL MARK2 ( X, Y )
300   CONTINUE
DO 400 I = 1, MY
           X = X - OFX
           Y = Y - OFY
CALL MARK2 ( X, Y )
400   CONTINUE
RETURN
END

SUBROUTINE GRD ( IGSW, MX, MY, IOFX, IOFY, IDEL, XS, YS )
COMMON /PGRID/ SLOPE, SIZE
              SLOPE = IOFY / IOFX + .01
              IGSW = IGSW + 1
IF ( IGSW ) 999, 999, 105
105  GO TO (110, 300, 130, 140, 150, 160, 170, 180, 190, 200),IGSW 23M01
110  CALL EDGE1 (MX, MY, IOFX, IOFY, IDEL, XS, 0.0)
110   GO TO 300
130  CALL CRNRI (MX, MY, IOFX, IOFY, IDEL, XS, 0.0)
130   GO TO 300
140  CALL MESH1 (MX, MY, IOFX, IOFY, IDEL, XS, 0.0)
140   GO TO 300
150  LOP = 0
155  CALL GRID1 (1, 1, IOFX*MY, IOFY*MY, IDEL*MX, XS, 0.0, LOP )
155   GO TO 300
160  LOP = 1
160   GO TO 155
170  LOP = 0
175  CALL GRID1 (MY, MX, IOFX, IOFY, IDEL, XS, 0.0, LOP )
175   GO TO 300
180  LOP = 1
180   GO TO 175
190  XS = XS + IABS(IOFX) * MX * .01 + 2.
190   GO TO 170
200  XS = XS + IABS(IOFX) * MX * .01 + 2.
200   GO TO 160
300  CONTINUE
999  CONTINUE
RETURN
END

SUBROUTINE CRNRI ( MX, MY, IOFX, IOFY, IDEL, XO, YO )
COMMON /PGRID/ SLOPE, SIZE
              X = XO
              Y = YO
              SIZE = 4.
              CALL MARK2 ( X, Y )
              X = X + MX * IDEL * .01
CALL MARK2 ( X, Y )
              Y = Y + MY * IOFY * .01
              X = X + MY * IOFX * .01
CALL MARK2 ( X, Y )
              X = X - MX * IDEL * .01
CALL MARK2 ( X, Y )
RETURN
END

```

```

SUBROUTINE MESHI ( MX, MY, IDFX, IDFY, IDEL, XO, YI )
COMMON /PGRID/ SLOPE, SIZE
      SIZE = 1.
      OFX = IDFX * .01
      OFY = IDFY * .01
      DEL = IDEL * .01
      MYPI = MY + 1
      MXPI = MX + 1
      XX = XO
      MP = 1
      Y = YO - OFY
DO 2000 J = 1, MYPI
      X = XX - DEL * MP
      Y = Y + OFY
DO 1000 I = 1, MXPI
      X = X + DEL * MP
      CALL MARK2 ( X, Y )
1000  CONTINUE
      XX = XX + OFX + DEL * MX * MP
2000  MP = -MP
      RETURN
END

SUBROUTINE MARK2 ( X, Y )
COMMON /PGRID/ SLOPE, SIZE
      STEP = .02
      ARM = STEP * SIZE
      XI = ARM * SQRT ( 1. / ( 1. + SLOPE*SLOPE ) )
      YI = XI * SLOPE
      IF ( XI.EQ.0. )           YI = ARM
      XX = X - ARM
      CALL PLOT ( XX, Y, 3 )
      CALL PLT  % XX, Y, 3 L
      XX = X + ARM
      CALL PLOT ( XX, Y, 2 )
      CALL PLT  % XX, Y, 2 L
      XX = X + XI
      YY = Y + YI
      CALL PLOT ( XX, YY, 3 )
      CALL PLT  % XX, YY, 3 L
      XX = X - XI
      YY = Y - YI
      CALL PLOT ( XX, YY, 2 )
      CALL PLT  % XX, YY, 2 L
      RETURN
END

SUBROUTINE GR101 ( IN, KN, IDFX, IDFY, IDEL, XO, YO, LOP )
      XX = IDEL * KN * .01
      INI = IN + 1
      MP = -1
DO 100 I = 1, INI
      IMI = I - 1
      XI = XO + IDFX * IMI * .01
      YI = YO + IDFY * IMI * .01
      X2 = XI + XX
      IF ( MP ) 80, 100, 90
80  CALL DASH2 ( XI,YI , X2,YI, LOP )
      GO TO 99
90  CALL DASH2 ( X2,YI , XI,YI, LOP )
99  MP = -MP
100  CONTINUE
      XX = IN + IDFX * .01
      KNI = KN + 1
      YI = YO
      Y2 = YO + IN + IDFY * .01
DO 200 I = 1, KNI
      IMI = I - 1
      XI = XO + IDEL * IMI * .01
      X2 = XI + XX
      IF ( MP ) 180, 200, 190
180 CALL DASH2 ( XI,YI , X2,Y2, LOP )
      GO TO 199
190 CALL DASH2 ( X2,Y2 , XI,YI, LOP )
199  MP = -MP
200  CONTINUE
      RETURN
END

SUBROUTINE DASH2 ( XI,YI , X2,Y2, LOP )
COMMON /PGRID/ SLOPE, SIZE
      STEP = .02
      ARM = STEP * SIZE
      XI = ARM * SQRT ( 1. / ( 1. + SLOPE*SLOPE ) )
      YI = XI * SLOPE
      IF ( XI.EQ.0. )           YI = ARM
      XX = X - ARM
      CALL PLOT ( XX, Y, 3 )
      CALL PLT  % XX, Y, 3 L
      XX = X + ARM
      CALL PLOT ( XX, Y, 2 )
      CALL PLT  % XX, Y, 2 L
      XX = X + XI
      YY = Y + YI
      CALL PLOT ( XX, YY, 3 )
      CALL PLT  % XX, YY, 3 L
      XX = X - XI
      YY = Y - YI
      CALL PLOT ( XX, YY, 2 )
      CALL PLT  % XX, YY, 2 L
      RETURN
END

SUBROUTINE DASH2 ( XI,YI , X2,Y2, LOP )
C --- LOP # 1 FOR SOLID, OTHERWISE DASHED
      CALL PLOT ( XI, YI, 3 )
      CALL PLT  % XI, YI, 3 L
      IP = 2
      IS = -1
      IF ( LOP = 1 ) 6, 5, 6
      5  IS = 0
      6  DIS = SQRT ( (X2-X1)**2 + (Y2-Y1)**2 )
      INC = DIS * 5.
      XD = (X2-X1) / (INC+1) * .5
      YD = (Y2-Y1) / (INC+1) * .5
      X = XI
      Y = YI
      IF ( INC ) 101, 101, 10
      10  INC2 = INC * 2
          XT = XI + XD / 4.
          YT = YI + YD / 4.
          CALL PLOT ( XT, YT, 2 )
          CALL PLT  % XT, YT, 2 L
          DO 100 I = 1, INC2
              X = X + XD
              Y = Y + YD
              IS = -IS
              IP = IP + IS
              CALL PLOT ( X, Y, IP )
              CALL PLT  % X, Y, IP L
100  CONTINUE
      101  X = X + (X2-X) / 2
          Y = Y + (Y2-Y) / 2
          IS = -IS
          IP = IP + IS
          CALL PLOT ( X, Y, IP )
          CALL PLT  % X, Y, IP L
          CALL PLOT ( X2, Y2, 2 )
          CALL PLT  % X2, Y2, 2 L
999  RETURN
END

```

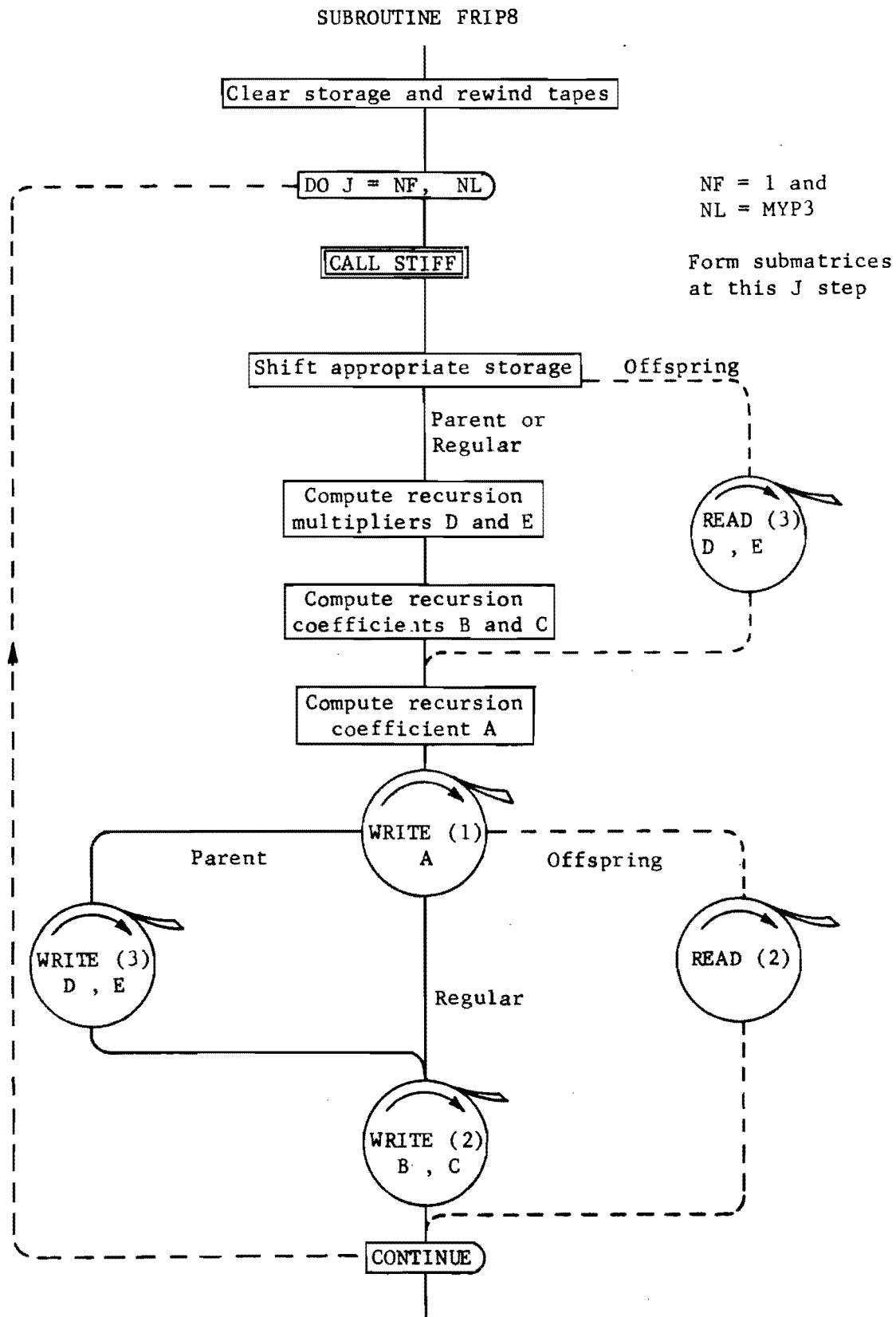
SUBROUTINE FRIP8

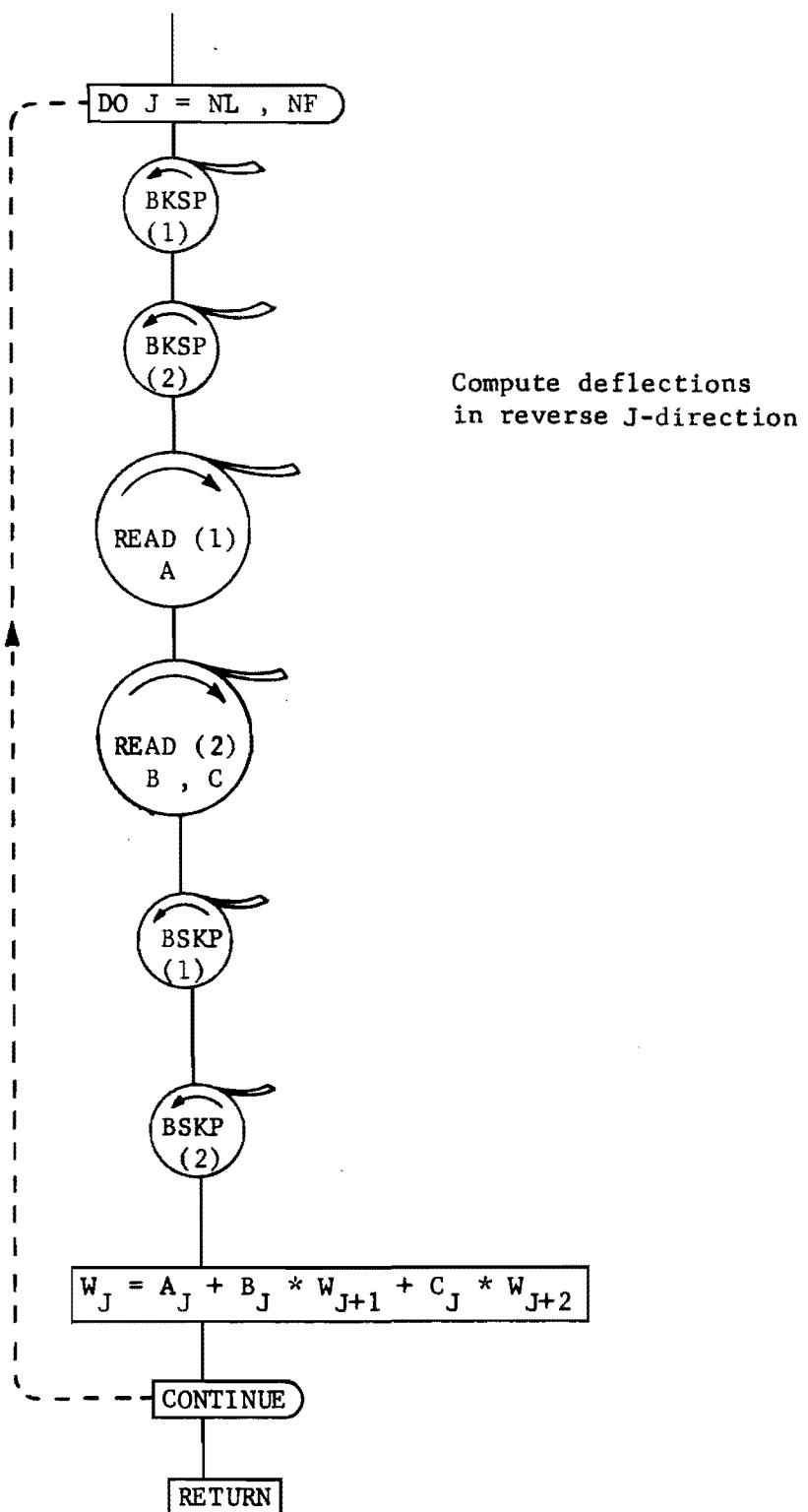
This subroutine is a general version of the FRIP4 Five-Wide Recursion-Inversion Solution Process which is documented in Research Report 56-19, "An Algebraic Equation Solution Process Formulated in Anticipation of Banded Linear Equations," by Frank L. Endres and Hudson Matlock (Ref 3).

This group of 14 subroutines provides an efficient solver for a sparsely banded matrix of equations. It can handle up to 5 groups of bands, each of arbitrary width. As used here, the matrix is assumed to be symmetric and positive definite.

These routines are called by FRIP8 and are completely documented in Ref 3.

RFV	- Replaces a full matrix or a vector by another
MBFV	- Multiplies a banded (packed) matrix times a full matrix or a vector
ABF	- Adds a banded matrix to a full matrix
MFFV	- Multiplies full (square) matrix times a full (square) matrix or a vector
ASFV	- Adds or subtracts two full matrices or two vectors
SMFF	- Symmetric multiplication of a full times a full matrix
INVR6	DCOM1 INVLT1 MLTXL FIX1
CFV	- Multiplies of full matrix or a vector by a constant
MFB	- Multiplies a full matrix times a banded (packed) matrix
MFPT	- Multiplies a full times the transpose of a full matrix






```

C      WRITE(11, 11 R28I,KL , I # 1,NKL , K # 1,NK L          22AG9CDC
C      REWIND 1                                         25HY0CDC
C      CALCULATE RECURSION MULTIPLIER D               04JAB .
C      D # -1 / E T2*CM2 E*BMI ECC L                22AG9CDC
C      CALL SMFF ( R1 , R3 , R2 , L1 , NK )           22AG9 .
C      IF ( ITMPP.GE.2 ) PRINT 93, ((R2(I,J),J=1,NK),I=1,NK ) 22AG9 .
C      READ $3L % R38I,KL , I # 1,NKL , K # 1,NK L          220C9CDC
C      BACKSPACE 3                                     22AG9CDC
C      READ ( 3'JI) (( R3(I,K), I=1,NK), K=1,NK )        18M
C      SAVE RECURSION MULTIPLIER E ON TAPE 3           22AG9 .
C      ID3 = JI
C      WRITE I 3'ID3(( R1(I,K), I=1,NK), K=1,NK )       01DE1IBM
C      READ ( 4'1 ) (( R1(I,K), I=1,NK), K=1,NK )       01DE1IBM
C      WRITE ( 4'1 ) (( R3(I,K), I=1,NK), K=1,NK )       01DE1IBM
C      WRITE$3L % R18I,KL , I # 1,NKL , K # 1,NK L          220C9CDC
C      READ $3L % R18I,KL , I # 1,NKL , K # 1,NK L          220C9CDC
C      REWIND 4                                         22AG9CDC
C      WRITE$3L % R38I,KL , I # 1,NKL , K # 1,NK L          220C9CDC
C      REWIND 4                                         22AG9CDC
C      CALL MFPV ( ET2, R1, R3 , L1 , L1 , NK , N1 )       220C9 .
C      IF ( ITMPP.GE.2 ) PRINT 93, ((R2(I,J),J=1,NK),I=1,NK ) 220C9 .
C      CALL ASFV ( R2 , R3 , R2 , L1 , L1 , NK , +1 )       22AG9 .
C      IF ( ITMPP.GE.2 ) PRINT 93, ((R2(I,J),J=1,NK),I=1,NK ) 22AG9 .
C      CALL ABF ( CC , R2 , R2 , L1 , NK , N3 )           22AG9 .
C      IF ( ITMPP.GE.1 ) PRINT 93, ((R2(I,J),J=1,NK),I=1,NK ) 22AG9 .
C      CALL INVRL ( R2 , L1 , NK )                      22AG9 .
C      CALL CFV ( R2 , L1 , L1 , NK , -1 )              22AG9 .
C      IF ( ITMPP.GE.1 ) PRINT 93, ((R2(I,J),J=1,NK),I=1,NK ) 22AG9 .
C      CALCULATE RECURSION COEFFIECENT C             04JAB .
CCCCC   C # D*EE
C      CALL MFB ( R2 , EE , R1 , L1 , NK , N1 )          22AG9 .
C      SAVE RECURSION COEFFIECENT C ON FILE 2          22AG9 .
C      WRITE ( 2'JI) (( R1(I,K), I=1,NK), K=1,NK )       01DE1IBM
C      WRITE$3L % R18I,KL , I # 1,NKL , K # 1,NK L          22AG9CDC
C      IF ( ITMPP.GE.2 ) PRINT 93, ((R1(I,J),J=1,NK),I=1,NK ) 22AG9 .
C      SAVE RECURSION MULTIPLIER D ON TAPE 3           01DE1IBM
C      WRITE ( 3'ID3)(( R2(I,K), I=1,NK), K=1,NK )       01DE1IBM
C      IF ( J.EQ.NL ) GO TO 280
C      WRITE ( 3'ID3)(( R1(I,K), I=1,NK), K=1,NK )       01DE1IBM
C      READ ( 1'1 ) (( R1(I,K), I=1,NK), K=1,NK )       01DE1IBM
C      WRITE$3L % R28I,KL , I # 1,NKL , K # 1,NK L          22AG9CDC
C      WRITE$3L % R18I,KL , I # 1,NKL , K # 1,NK L          22AG9CDC
C      BACKSPACE 3                                     22AG9CDC
C      READ $3L % R18I,KL , I # 1,NKL , K # 1,NK L          22AG9CDC
C      REWIND 1                                         25HY0CDC
C      CALCULATE RECURSION COEFFIECENT B             04JAB .
CB888   B # D*EP1
C      CALL MFPT ( R2 , R1 , R3 , L1 , NK )          22AG9 .
C      SAVE RECURSION COEFFIECENT B ON FILE 2          22AG9 .
C      WRITE I 2'J2) (( R3(I,K), I=1,NK), K=1,NK )       01DE1IBM
C      WRITE$3L % R38I,KL , I # 1,NKL , K # 1,NK L          22AG9CDC
C      IF ( ITMPP.GE.2 ) PRINT 93, ((R3(I,J),J=1,NK),I=1,NK ) 22AG9 .
C      FINAL CALCULATION RECURSION COEFFIECENT A       22AG9 .
AAAAA   A # D*E*AM1 & ET2*AM2 - FF L               22AG9 .
280 CALL MFPT ( R2 , ATN, A , L1 , 1 , NK )          22AG9 .
IF ( ITMPP.GE.2 ) PRINT 93, ( A(I,1),I=1,NK )        29JL9 .
NKK = NK + NF - 1                                    25HY0 .
DO 300 I = NF, NKK                                  25HY0 .

      I111 = I - NF + 1
      KW(I,J) = A(I111,1)
      300      IF % ML.GE.0 L GO TO 1000
      C      READ $2L
      C      READ $2L
      1000  CONTINUE
      C***** BEGIN BACKWARD PASS -- COMPUTE RECURSION EQUATION 04JAB
      C***** ****
      C      ID2 = 2*NL - 3
      READ ( 2'ID2)(( R1(I,K), I=1,NK), K=1,NK )       01DE1IBM
      READ ( 2'ID2)(( R3(I,K), I=1,NK), K=1,NK )       01DE1IBM
      C      BACKSPACE 2
      C      READ $2L % R18I,KL , I # 1,NKL , K # 1,NK L          20MY8CDC
      C      READ $2L % R38I,KL , I # 1,NKL , K # 1,NK L          22AG9CDC
      C      BACKSPACE 2
      C      BACKSPACE 2
      C      CALL MFFV ( R3, WW(NF,NL), AM1, L1, 1, NK )       20MY8CDC
      C      CALL ASFV ( WW(NF,NL-1), AM1, WW(NF,NL-1), L1, 1, NK, +1 ) 25HY0
      C      NLM2 = NL - 2
      C      DO 2000 L = NF , NLM2
      C      J = NLM2 + NF - L
      C      IF ( J .EQ. 1 ) GO TO 2100
      FIND ( 2*J - 3 )
      2100  CONTINUE
      C      BACKSPACE 2
      C      BACKSPACE 2
      C      READ C AND B COEFFICIENTS FROM FILE 2          17JA8CDC
      C      READ $2L % R18I,KL , I # 1,NKL , K # 1,NK L          22AG9CDC
      C      READ $2L % R38I,KL , I # 1,NKL , K # 1,NK L          22AG9CDC
      C      ID2 = 2*J - 1
      READ ( 2'ID2)(( R1(I,K), I=1,NK), K=1,NK )       01DE1IBM
      READ ( 2'ID2)(( R3(I,K), I=1,NK), K=1,NK )       01DE1IBM
      C      BACKSPACE 2
      C      BACKSPACE 2
      C      CALL MFFV ( R3, WW(NF,J+1), AM1, L1, 1, NK )       17JA8CDC
      C      CALL MFFV ( R1, WW(NF,J+2), AM2, L1, 1, NK )       22AG9CDC
      C      CALL ASFV ( AM1, AM2, AM1, L1, 1, NK, +1 )       25HY0 .
      C      CALL ASFV ( WW(NF,J), AM1, WW(NF,J), L1, 1, NK, +1 ) 22AG9 .
      C      2000  CONTINUE
      C      RETURN
      END
      04JAB .
      04JAB

```

```

SUBROUTINE RFV ( X , Y , L1 , L5 , L2 )
DOUBLE PRECISION X, Y
C***** THIS ROUTINE REPLACES A FULL MATRIX OR A VECTOR
C   % X # Y L
C   DIMENSION X(1:L1,L5) , Y(1:L1,L5)
      M = 1
      IF( L1 .EQ. L5 ) M = L2
      DO 110 J = 1,M
      DO 100 I = 1 , L2
         X(I,J) = Y(I,J)
100   CONTINUE
110   CONTINUE
      RETURN
      END

SUBROUTINE MBFV ( XB , YF , ZF , L1 , L5 , L2 , L4 )
DOUBLE PRECISION XB, YF, ZF, SUM
C***** THIS ROUTINE MULTIPLIES A BANDED MATRIX
C   TIMES A FULL MATRIX OR A VECTOR
C   % XB * YF # ZF L
C   DIMENSION XB( L1:L3 ) , YF( L1:L5 ) , ZF( L1:L5 )
      M1 = 1
      IF( L1 .EQ. L5 ) M1 = L2
      L4 = L8/2
      L6 = L4 + 1
      N1 = L2 - L4
      DO 110 M = 1,M1
      DO 105 I = L6,N1
         J = I - L6
         SUM = 0.0
         DO 100 K = 1,LB
            SUM = XB(I,K) * YF(K+J,M) + SUM
100   CONTINUE
         ZF(I,M) = SUM
105   CONTINUE
110   CONTINUE
      K1 = 0
      I1 = L
      I2 = L4
      I3 = L
      I4 = LB
      IF( L2 ) 150, 900, 150
150   DO 210 M = 1,M1
      DO 205 I = I1,I2
         SUM = 0.0
         N = L
         DO 200 K = I3, I4
            SUM = XB(I,N) * YF(K,M) + SUM
            N = N + 1
200   CONTINUE
         ZF(I,M) = SUM
205   CONTINUE
210   CONTINUE
      IF( K1 ) 900,300,900
300   I1 = L2 - L4 + 1
      I2 = L2
      I3 = L2 - LB + 1
      I4 = L2
      K1 = 1
      GO TO 150
900   RETURN
      END

```

```

SUBROUTINE ABF ( YB , XF , ZF , L1 , L2 , LB )
DOUBLE PRECISION YB, XF, ZF
C***** THIS ROUTINE ADDS A BANDED MATRIX
C   TO A FULL MATRIX
C   % YB & XF # ZF OR XF & YB = ZF L
C   DIMENSION YB( L1:L8 ) , XF( L1:L1 ) , ZF(L1,L1)
      L4 = LB/2
      N1 = L2 - L4
      L6 = L4 + 1
      OO 50 I = 1,L2
      OO 40 J = 1,L2
         ZF(I,J) = XF(I,J)
40    CONTINUE
50    CONTINUE
      OO 110 I = L6,N1
         J = I - L6
      OO 100 K = 1,LB
         ZF(I,K+J) = YB(I,K) + XF(I,K+J)
100   CONTINUE
110   CONTINUE
      K1 = 0
      I1 = 1
      I2 = L4
      I3 = 1
      I4 = LB
      IF( L2 ) 150, 900, 150
150   OO 210 I = I1,I2
         N = 1
      OO 200 K = I3, I4
         ZF(I,K) = YB(I,N) + XF(I,K)
         N = N + 1
200   CONTINUE
210   CONTINUE
      IF(K1) 900, 300, 900
300   I1 = L2 - L4 + 1
      I2 = L2
      I3 = L2 - LB + 1
      I4 = L2
      K1 = 1
      GO TO 150
900   RETURN
      END

SUBROUTINE MFFV ( X , Y , Z , L1 , L5 , L2 )
DOUBLE PRECISION X, SUM, Y, Z
C***** THIS ROUTINE MULTIPLIES A FULL MATRIX
C   TIMES A FULL MATRIX OR A VECTOR
C   % X * Y # Z L
C   DIMENSION X(1:L1,L5) , Y(1:L1,L5) , Z(1:L1,L5)
      M = 1
      IF( L1 .EQ. L5 ) M = L2
      DO 110 J = 1,M
      DO 100 I = 1 , L2
         SUM = 0.0
         DO 100 K = 1,L2
            SUM = SUM + X(I,K) * Y(K,J)
100   CONTINUE
         Z(I,J) = SUM
105   CONTINUE
110   CONTINUE
      RETURN
      END

```

```

SUBROUTINE ASFW ( X , Y , Z , LI , L5 , L2 , SIGN ) 29A.V.
  DOUBLE PRECISION X, Y, Z
C***** THIS ROUTINE ADDS OR SUBTRACTS 2 FULL MATRICES OR 2 VECTORS 20MYC
C      X - Y # Z OR X # Y - Z L 13DE7
C      DIMENSION X(LI,L5) , Y(LI,L5) , Z(LI,L5) 13DE7
C          M = 1 20MYC
C          IF( LI .EQ. L5 ) M = L2 20MYC
C          IF( L SIGN ) 190, 50, 5D 13DE7
50        DO 110 J = 1,M 13DE7
          DO 100 I = 1,L2 13DE7
            Z(I,J) = X(I,J) + Y(I,J) 13DE7
100      CONTINUE 13DE7
110      CONTINUE 13DE7
120      GO TO 300 13DE7
130      DO 210 J = 1,M 13DE7
140      DO 200 I = 1,L2 13DE7
            Z(I,J) = X(I,J) - Y(I,J) 13DE7
150      CONTINUE 13DE7
160      CONTINUE 13DE7
300      RETURN 13DE7
      END 13DE7

SUBROUTINE SMFF ( X , Y , Z , LI , L2 ) 19FEB
  DOUBLE PRECISION X, SUM, Y, Z 18M
C***** THIS ROUTINE MULTIPLIES TWO FULL MATRICES UNDER THE ASSUMPTION 05MRB
C      THAT THEIR PRODUCT WILL BE SYMMETRIC & X,Y, AND Z ARE FULL 05MRB
C      DIMENSIONED BUT ONLY THE LOWER HALF OF EACH IS USED L 05MRB
C      DIMENSION X(LI,LII) , Y(LI,LII) , Z(LI,LII) 19FEB
          DO 110 J = 1 , L2 19FEB
          DO 105 I = 1 , J 19FEB
            SUM = 0.0 19FEB
            DO 100 K = 1 , L2 19FEB
              SUM = SUM + X(J,K) * Y(K,I) 19FEB
100      CONTINUE 19FEB
105      Z(J,I) = SUM 19FEB
110      CONTINUE 19FEB
120      RETURN 19FEB
      END 19FEB

```

```

SUBROUTINE INVRL ( X , L1 , L2 ) 19FEB
DOUBLE PRECISION X, S, SI 31MY11BM

C***** THIS ROUTINE TAKES THE INVERSE OF A SYMMETRIC POSITIVE - DEF
C MATRIX USING A COMPACTED CHOLESKI DECOMPOSITION PROCEDURE .
C A FULL DIMENSIONED MATRIX IS REQUIRED BUT ONLY THE LOWER
C HALF IS USED BY THE 3 Routines DRIVEN BY INVRL
DIMENSION X(L1,L1) 19FEB
      IF ( L2 - 1 ) 600, 10, 20 01OC8
      IF (DABS (X(1,1)).LT.1.0E-10 ) GO TO 600 02DE11BM
C 10     IF % ABSF%X1,ILL.LT.1.0E-10 L GO TO 600 02JUICOC
          X(1,1) = 1. / X(1,1)
          GO TO 500 05MRI
20     IF ( L2 - 2 ) 30, 30, 40 01OC8
30     S1 = X(1,1) * X(2,2) - X(1,2) * X(2,1) 01OC8
      IF (DABS (S1).LT.1.0E-10 ) GO TO 600 05MRI
C 40     IF % ABSF%S1.LT.1.0E-10 L GO TO 600 02DE11BM
          S1 = 1./ S1 02JUICDC
          S = X(1,1) 01OC8
          X(1,1) = S1 * X(2,2) 05MRI
          X(2,2) = S1 * S 01OC8
          X(1,2) = -S1 * X(1,2) 01OC8
          X(2,1) = -S1 * X(2,1) 01OC8
          GO TO 500 01OC8
40 CALL FIX1 ( X , L1 , L2 ) 04MR018M
C 40   CONTINUE 19DC0CDC
    CALL DCOMI ( X , L1 , L2 )
    CALL INVLTI ( X , L1 , L2 )
    CALL MLTXL ( X , L1 , L2 )
    CALL FIX1 ( X , L1 , L2 )
    DO 100 I = 2 , L2 19FEB
        KC = I - 1 19FEB
    DO 50 J = 1 , KC 19FEB
        X(J,I) = X(I,J) 19FEB
50   CONTINUE 19FEB
100  CONTINUE 19FEB
500 RETURN 01OC8
600 PRINT 601,([X(I,J), J=1,L2], I=1,L2) 01OC8
C 601 FORMAT 1 IHI,30H SINGULAR MATRIX ENCOUNTERED ./,21SX,1P2E15.7) 01OC811BM
C 601 FORMAT % IHI,30H SINGULAR MATRIX ENCOUNTERED ./,24SX, 2E15.7L 01OC8C0C
STOP 19FEB
END 19FEB

```


APPENDIX 4

**LISTING OF INPUT DATA FOR PROBLEMS
1001 THROUGH 1004**

RECEIVED
FEDERAL BUREAU OF INVESTIGATION
U.S. DEPARTMENT OF JUSTICE

1-BEAM STRUCTURE SPANS 58-68-58, 33 FT RDWY, HS28 LDG
 ANALYSIS OF THD THREE-SPAN CONTINUOUS RUN 29 FEB 1972 JJP FT-KIP UNITS
 1001A DEAD LOAD OF GIRDERS AND NON-COMPOSITE DECK
 0 0 0 0 0 0 0 0
 1 47 0 0 0 0 3 4
 24 32 1,450E+00 5,000E+00 1
 0 0 24 32 1,000E-03 1,000E+03
 2 0 2 32 6,135E+05-1,772E+00
 2 1 2 31 6,135E+05-1,772E+00
 7 0 7 32 6,135E+05-1,081E+00
 7 1 7 31 6,135E+05-1,081E+00
 12 0 12 32 6,135E+05-1,081E+00
 12 1 12 31 6,135E+05-1,081E+00
 17 0 17 32 6,135E+05-1,081E+00
 17 1 17 31 6,135E+05-1,081E+00
 22 0 22 32 6,135E+05-1,772E+00
 22 1 22 31 6,135E+05-1,772E+00
 2 0 22 0 2,663E+04
 3 0 21 0 2,663E+04
 2 9 22 9 3,256E+04
 3 9 21 9 3,256E+04
 2 9 22 9 3,256E+04
 3 9 21 9 3,256E+04
 2 14 22 14 3,256E+04
 3 14 21 14 3,256E+04
 2 10 22 10 3,256E+04
 3 10 21 10 3,256E+04
 2 23 22 23 3,256E+04
 3 23 21 23 3,256E+04
 2 27 22 27 3,256E+04
 3 27 21 27 3,256E+04
 2 32 22 32 2,663E+04
 3 32 21 32 2,663E+04
 2 0 2 0 1,000E+14
 7 0 7 0 1,000E+14
 12 0 12 0 1,000E+14
 17 0 17 0 1,000E+14
 22 0 22 0 1,000E+14
 2 10 2 10 1,000E+14
 7 10 7 10 1,000E+14
 12 10 12 10 1,000E+14
 17 10 17 10 1,000E+14
 22 10 22 10 1,000E+14
 2 22 2 22 1,000E+14
 7 22 7 22 1,000E+14
 12 22 12 22 1,000E+14
 17 22 17 22 1,000E+14
 22 22 22 22 1,000E+14
 2 32 2 32 1,000E+14
 7 32 7 32 1,000E+14
 12 32 12 32 1,000E+14
 17 32 17 32 1,000E+14
 22 32 22 32 1,000E+14
 12 0 12 32 1,000E+14
 17 0 17 32 1,000E+14
 22 0 22 32 1,000E+14
 0 0 0 0 0 0
 4 0 0 0 0 0
 9 0 0 0 0 0
 14 0 0 0 0 0

2	13	8	13		-1.300E+00	12	8	12	32	6.135E+05
3	13	5	13		-2.000E+00	12	1	12	31	6.135E+05
6	13	6	13		-7.000E-01	12	1	12	7	1.881E+06
7	13	7	13		-3.400E+00	12	12	12	20	1.881E+06
9	13	9	13		-1.700E+00	17	8	17	32	6.135E+05
10	13	10	13		-2.400E+00	17	1	17	31	6.135E+05
13	13	13	13		-1.100E+00	17	1	17	7	1.881E+06
14	13	14	13		-3.000E+00	17	12	17	20	1.881E+06
2	14	2	14		-3.000E-01	22	8	22	32	6.135E+05
2	14	2	14		-7.000E-01	22	1	22	31	6.135E+05
6	14	6	14		-2.000E-01	22	1	22	7	1.829E+06
7	14	7	14		-8.000E-01	22	12	22	20	1.829E+06
9	14	9	14		-4.000E-01	2	8	22	0	2.663E+04
10	14	10	14		-6.000E-01	3	8	21	0	2.663E+04
13	14	13	14		-3.000E-01	2	5	22	5	3.256E+04
14	14	14	14		-7.000E-01	3	5	21	5	3.256E+04
2	16	2	16		-6.300E+00	2	9	22	9	3.256E+04
3	16	3	16		-1.400E+01	3	9	21	9	3.256E+04
6	16	6	16		-3.500E+00	2	14	22	14	3.256E+04
7	16	7	16		-1.600E+01	3	14	21	14	3.256E+04
9	16	9	16		-8.400E+00	2	18	22	10	3.256E+04
10	16	10	16		-1.100E+01	3	18	21	10	3.256E+04
13	16	13	16		-5.600E+00	2	23	22	23	3.256E+04
16	16	16	16		-1.470E+01	3	23	21	23	3.256E+04
2	18	2	18		-1.300E+00	2	27	22	27	3.256E+04
3	18	3	18		-2.000E+00	3	27	21	27	3.256E+04
6	18	6	18		-6.000E-01	2	32	22	32	2.663E+04
7	18	7	18		-3.400E+00	3	32	21	32	2.663E+04
9	18	9	18		-1.700E+00	2	8	2	0	1.000E+14
10	18	10	18		-2.400E+00	7	8	7	0	1.000E+14
13	18	13	18		-1.100E+00	12	8	12	0	1.000E+14
14	18	14	18		-2.900E+00	17	8	17	0	1.000E+14
2	19	2	19		-5.000E+00	22	8	22	0	1.000E+14
3	19	3	19		-1.120E+01	2	18	2	18	1.000E+14
6	19	6	19		-2.800E+00	7	18	7	18	1.000E+14
7	19	7	19		-1.340E+01	12	18	12	18	1.000E+14
9	19	9	19		-6.700E+00	17	18	17	10	1.000E+14
10	19	10	19		-9.900E+00	22	18	22	10	1.000E+14
13	19	13	19		-4.500E+00	2	22	2	22	1.000E+14
14	19	14	19		-1.180E+01	7	22	7	22	1.000E+14
2	8	2	32	1	2	12	22	12	22	1.000E+14
7	8	7	32	1	2	17	22	17	22	1.000E+14
12	8	12	32	1	2	22	22	22	22	1.000E+14
17	8	17	32	1	2	2	32	2	32	1.000E+14
22	8	22	32	1	2	7	32	7	32	1.000E+14
9	9	0				12	32	12	32	1.000E+14
13	9	11				17	32	17	32	1.000E+14
21	9	23				22	32	22	32	1.000E+14
32	32					1	1	24	32	6.210E+03
1803A LANE LOADING FOR MAX NEG MOMENT COMPOSITE DECK WHERE MOM IS POS										
0	0	0	0	0	0	1	0	1	22	-5.600E-02
1	60	0	1	0	13	5	6	15	0	-5.600E-02
24	32					1	15	0	15	-1.790E-01
0	24	32	1.630E+03	1.630E+03	1.630E+03	1.500E-01	15	1	15	-1.790E-01
0	1	24	31	1.630E+03	1.630E+03		2	0	14	-2.900E+01
1	0	32	32	1.630E+03	1.630E+03		2	22	14	-2.900E+01
1	1	23	31	1.630E+03	1.630E+03		1	9	1	-5.930E-01
2	0	2	32			15	5	15	5	-6.330E-01
2	1	2	31			2	5	14	5	-2.813E+00
2	1	2	7			1	16	1	16	-3.335E+00
2	12	2	28			15	16	15	16	-6.330E+00
7	0	7	32			2	16	14	16	-2.813E+00
7	1	7	31							
7	1	7	7							
7	12	7	28							

1	2	32	1	8	2	8		2	22	2	22		1.000E+14
7	8	7	32	1	8	2	8	7	22	7	22		1.000E+14
12	8	12	32	1	8	2	8	12	22	12	22		1.000E+14
17	8	17	32	1	8	2	8	17	22	17	22		1.000E+14
22	8	22	32	1	8	2	8	22	22	22	22		1.000E+14
								2	32	2	32		1.000E+14
								7	32	7	32		1.000E+14
								12	32	12	32		1.000E+14
								17	32	17	32		1.000E+14
								22	32	22	32		1.000E+14
								1	1	24	32	6,210E+03	
18844	2	H828	TRUCKS STAGGERED IN ADJACENT SPANS FOR MAX NEG SUPPORT MOMENT					9	2	9	2		
	8	8	8	8	8	8	8	10	2	10	2		-1.700E+00
1	59	8	1	8	48	6	6	13	2	13	2		-2.400E+00
24	32							14	2	14	2		+1.100E+00
	8	24	32	1,030E+03	1,030E+03	1,030E+03	1,030E+03	9	3	9	3		-3.000E+00
	8	1	24	31	1,030E+03	1,030E+03	1,030E+03	18	3	18	3		-4.000E-01
	8	1	23	32	1,030E+03	1,030E+03	1,030E+03	13	3	13	3		+4.000E-01
	1	1	23	31	1,030E+03	1,030E+03	1,030E+03	14	3	14	3		-3.000E-01
	2	8	2	32				9	5	9	5		+7.000E-01
	2	1	2	31				10	5	10	5		-8.500E+00
	2	12	2	28				13	5	13	5		+1.200E+01
	7	0	7	32				14	5	14	5		-5.600E+00
	7	1	7	31				9	7	9	7		+1.400E+01
	7	1	7	7				10	7	10	7		-1.700E+00
	7	12	7	28				13	7	13	7		-2.400E+00
	12	0	12	32				14	7	14	7		+1.100E+00
	12	1	12	31				9	8	9	8		-3.000E+00
	12	1	12	8				10	8	10	8		-6.500E+00
	12	13	12	21				13	8	13	8		+9.000E+00
	17	0	17	32				14	8	14	8		-4.500E+00
	17	1	17	31				2	13	2	13		+1.100E+01
	17	1	17	9				3	13	3	13		-5.100E+00
	17	16	17	28				6	13	6	13		+1.130E+01
	22	0	22	32				7	13	7	13		-2.500E+00
	22	1	22	31				2	14	2	14		+1.300E+00
	22	1	22	11				3	14	3	14		-1.300E+00
	22	20	22	31				6	14	6	14		-7.000E+01
	2	8	22	8				7	14	7	14		-3.400E+00
	3	6	21	8				2	16	2	16		-6.400E+00
	2	9	22	5				3	16	3	16		+1.410E+01
	3	5	21	5				6	16	6	16		-3.500E+00
	2	9	22	9				7	16	7	16		-1.600E+01
	3	9	21	9				2	16	2	16		+3.000E+01
	2	14	22	14				3	16	3	16		-7.000E+01
	3	10	21	14				6	16	6	16		-2.000E+01
	2	10	22	18				7	16	7	16		-8.000E+01
	3	10	21	18				2	19	2	19		+1.300E+00
	2	23	22	23				3	19	3	19		-2.000E+00
	3	23	21	23				6	19	6	19		-7.000E+01
	2	27	22	27				7	19	7	19		-3.400E+00
	3	27	21	27				2	20	2	20		
	2	32	22	32				7	20	7	20	1	2
	3	32	21	32				12	20	12	20	1	2
	2	8	2	8				17	20	17	20	1	2
	7	8	7	8				22	20	22	20	1	2
	12	8	12	8				0	25	0	25	1	2
	17	8	17	8				0	26	0	26		
	22	8	22	8				0	26	0	26		
	2	10	2	18				0	26	0	26		
	7	10	7	18				0	26	0	26		
	12	10	12	18				14	26	14	26		
	17	10	17	18				21	26	21	26		
	22	10	22	18				32	26	32	26		

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APPENDIX 5

**SELECTED OUTPUT FOR EXAMPLE PROBLEMS INCLUDING
THREE-DIMENSIONAL PLOTS**

С КИЕВА

ОБЩЕСТВО ПОДДЕРЖАНИЯ ВОСПРОИЗВОДСТВА
СОУЧАСТНИКОВ-ДЕРЖАВЫ

PROGRAM SLAB 49 -TEX HWY DEPT DECK- MATLOCK,PANAK

REV DATE 29 FEB 72

I-BEAM STRUCTURE SPANS 50-60-50, 33 FT RDWY, HS20 LDG
ANALYSIS OF TWO THREE-SPAN CONTINUOUS RUN 29 FEB 1972 JJP FT-KIP UNITSPROB (CONTD)
1001A DEAD LOAD OF GIRDERS AND NON-COMPOSITE DECK

RESULTS

BEA MOMENTS ARE TOTAL PER BEAM

X , Y	DEFL	BEAM X MOMENT	BEAM Y MOMENT	SUPPORT REACTION
0	18	-1.5340-02	-5.776E-21	5.891E-08
1	18	-1.5510-02	6.428E-09	5.844E-08
2	18	-1.5660-02	1.492E-05	7.051E 01
3	18	-1.5820-02	1.464E-01	5.825E-08
4	18	-1.5970-02	2.929E-01	5.931E-08
5	18	-1.6110-02	4.393E-01	5.987E-08
6	18	-1.6240-02	5.857E-01	5.980E-08
7	18	-1.6350-02	7.321E-01	7.324E 01
8	18	-1.6440-02	6.805E-01	6.080E-08
9	18	-1.6500-02	6.288E-01	6.179E-08
10	18	-1.6540-02	5.771E-01	6.199E-08
11	18	-1.6570-02	5.255E-01	6.143E-08
12	18	-1.6570-02	4.738E-01	7.458E 01
13	18	-1.6570-02	5.255E-01	6.143E-08
14	18	-1.6540-02	5.771E-01	6.199E-08
15	18	-1.6500-02	6.288E-01	6.179E-08
16	18	-1.6440-02	6.805E-01	6.080E-08
17	18	-1.6350-02	7.321E-01	7.324E 01
18	18	-1.6240-02	5.857E-01	5.980E-08
19	18	-1.6110-02	4.393E-01	5.987E-08
20	18	-1.5970-02	2.929E-01	5.931E-08
21	18	-1.5820-02	1.464E-01	5.825E-08
22	18	-1.5660-02	1.492E-05	7.051E 01
23	18	-1.5510-02	6.428E-09	5.844E-08
24	18	-1.5340-02	8.251E-22	5.891E-08
0	17	-1.8300-02	-5.363E-21	7.684E-08
1	17	-1.8510-02	3.236E-09	7.782E-08
2	17	-1.8720-02	1.000E-08	9.709E 01
3	17	-1.8900-02	4.316E-09	7.977E-08
4	17	-1.9080-02	2.679E-09	8.031E-08
5	17	-1.9250-02	4.858E-09	8.102E-08
6	17	-1.9410-02	1.087E-08	8.189E-08
7	17	-1.9550-02	2.104E-08	8.189E-08
8	17	-1.9610-02	4.393E-01	5.931E-08
9	17	-1.9610-02	4.393E-01	5.987E-08
10	17	-1.9610-02	5.857E-01	5.980E-08
11	17	-1.9610-02	7.321E-01	7.324E 01
12	17	-1.9610-02	6.805E-01	6.080E-08
13	17	-1.9610-02	6.288E-01	6.179E-08
14	17	-1.9610-02	5.771E-01	6.199E-08
15	17	-1.9610-02	5.255E-01	6.143E-08
16	17	-1.9610-02	4.738E-01	7.458E 01
17	17	-1.9610-02	5.255E-01	6.143E-08
18	17	-1.9610-02	5.771E-01	6.199E-08
19	17	-1.9610-02	6.288E-01	6.179E-08
20	17	-1.9610-02	6.805E-01	6.080E-08
21	17	-1.9610-02	7.321E-01	7.324E 01
22	17	-1.9610-02	8.189E-08	0.0

9	14	-8.0410-02	6.393E-01	5.987E-08	0.0
20	14	-1.5970-02	2.929E-01	5.931E-08	0.0
21	14	-1.5820-02	1.464E-01	5.825E-08	0.0
22	14	-1.5660-02	1.492E-05	7.051E 01	0.0
23	14	-1.5510-02	6.428E-09	5.844E-08	0.0
24	14	-1.5340-02	-1.650E-20	5.891E-08	0.0
0	10	-3.4280-04	-5.366E-22	-1.479E-07	0.0
1	10	-1.4210-04	-2.790E-08	-1.634E-07	0.0
2	10	-4.3470-13	-9.469E-08	-2.150E 02	4.3470 01
3	10	-5.7000-05	-9.307E-09	-1.720E-07	0.0
4	10	-1.3360-04	3.286E-08	-1.676E-07	0.0
5	10	-1.4100-04	3.645E-08	-1.689E-07	0.0
6	10	-7.1860-05	1.277E-09	-1.761E-07	0.0
7	10	-4.5630-13	-7.793E-08	-2.250E 02	4.5630 01
8	10	-9.1990-05	3.558E-09	-1.772E-07	0.0
9	10	-1.7650-04	4.119E-08	-1.712E-07	0.0
10	10	-1.7440-04	4.023E-08	-1.718E-07	0.0
11	10	-8.7760-05	5.233E-10	-1.787E-07	0.0
12	10	-4.6080-13	-8.348E-08	-2.278E 02	4.6080 01
13	10	-8.7760-05	5.233E-10	-1.787E-07	0.0
14	10	-1.7440-04	4.023E-08	-1.718E-07	0.0
15	10	-1.7650-04	4.119E-08	-1.712E-07	0.0
16	10	-9.1990-05	3.558E-09	-1.772E-07	0.0
17	10	-4.5630-13	-7.793E-08	-2.250E 02	4.5630 01
18	10	-7.1860-05	1.277E-09	-1.761E-07	0.0
19	10	-1.4100-04	3.645E-08	-1.689E-07	0.0
20	10	-1.3360-04	3.286E-08	-1.676E-07	0.0
21	10	-5.7000-05	-9.307E-09	-1.720E-07	0.0
22	10	-4.3470-13	-9.469E-08	-2.150E 02	4.3470 01
23	10	-1.4210-04	-2.790E-08	-1.634E-07	0.0
24	10	-3.4280-04	-1.031E-22	-1.479E-07	0.0
0	9	-2.4600-03	1.882E-21	-1.002E-07	0.0
1	9	-2.4710-03	-7.309E-09	-9.593E-08	0.0
2	9	-2.4990-03	2.361E-06	-1.134E 02	0.0
3	9	-2.5260-03	2.769E-02	-9.556E-08	0.0
4	9	-2.5520-03	5.537E-02	-9.919E-08	0.0
5	9	-2.5770-03	8.305E-02	-	

3	0	-2.4570-13	-3.937E-08	-2.129E-14	0.0
4	0	-6.9210-13	8.417E-09	-1.388E-22	0.0
5	0	-8.0630-13	1.130E-08	-1.388E-22	0.0
6	0	-4.7430-13	4.335E-12	-1.388E-22	0.0
7	0	-1.4210-13	-2.569E-08	-4.257E-14	1.4210 01
8	0	-8.2400-13	2.365E-09	-1.388E-22	0.0
9	0	-1.4130-12	1.588E-08	-1.388E-22	0.0
10	0	-1.3740-12	1.491E-08	-1.388E-22	0.0
11	0	-7.4750-13	-5.630E-10	-1.388E-22	0.0
12	0	-1.4310-13	-3.063E-08	-4.257E-14	1.4310 01
13	0	-7.4750-13	-5.630E-10	-1.388E-22	0.0
14	0	-1.3740-12	1.491E-08	-1.388E-22	0.0
15	0	-1.4130-12	1.588E-08	-1.388E-22	0.0
16	0	-8.2400-13	2.365E-09	-1.388E-22	0.0
17	0	-1.4210-13	-2.569E-08	-4.257E-14	1.4210 01
18	0	-4.7430-13	4.335E-12	-1.388E-22	0.0
19	0	-8.0630-13	1.130E-08	-1.388E-22	0.0
20	0	-6.9210-13	8.417E-09	-1.388E-22	0.0
21	0	-2.4570-13	-8.503E-09	-1.388E-22	0.0
22	0	-1.3490-13	-3.937E-08	-2.129E-14	1.3490 01
23	0	-3.1320-12	-1.291E-08	-1.388E-22	0.0
24	0	-2.7140-05	8.057E-24	-1.388E-22	0.0

STATICS CHECK. SUMMATION OF REACTIONS = 5.8800 02
MAXIMUM STATICS CHECK ERROR AT STA 17 10 = -6.956D-05

PROGRAM SLAB 49 -TEX HWY DEPT DECK- MATLOCK, PANAK

REV DATE 29 FEB 72

I-BEAM STRUCTURE SPANS 50-60-50, 33 FT RDWY, HS20 LDG
ANALYSIS OF THD THREE-SPAN CONTINUOUS RUN 29 FEB 1972 JJP FT-KIP UNITS

PROB (CONT'D)
1001A DEAD LOAD OF GIRDERS AND NON-COMPOSITE DECK

PROFILE OUTPUT AREAS
X MOMENTS ACT IN X DIRECTION (ABOUT Y AXIS)
THE PLOTTED RESULTS INDICATE THE RELATIVE VALUE EACH HAS WITHIN THAT LIST

DEFLECTIONS BETWEEN (12 , 0) AND (12 , 32)

X , Y DEFLECTION

12 0 -1.431E-13
12 1 -8.219E-03
12 2 -1.517E-02
12 3 -1.990E-02
12 4 -2.213E-02
12 5 -2.153E-02
12 6 -1.842E-02
12 7 -1.340E-02
12 8 -7.749E-03
12 9 -2.651E-03
12 10 -6.608E-13
12 11 -1.990E-03
12 12 -6.507E-03
12 13 -1.182E-02
12 14 -1.657E-02
12 15 -1.981E-02
12 16 -2.095E-02
12 17 -1.981E-02
12 18 -1.657E-02
12 19 -1.182E-02
12 20 -6.507E-03
12 21 -1.990E-03
12 22 -6.608E-13
12 23 -2.651E-03
12 24 -7.749E-03
12 25 -1.340E-02
12 26 -1.842E-02
12 27 -2.153E-02
12 28 -2.213E-02
12 29 -1.990E-02
12 30 -1.517E-02
12 31 -8.219E-03

BEAM Y MOMENT BETWEEN (17 , 0) AND (17 , 32)

X , Y BEAM Y MOM

17 0 -4.257E-14
17 1 6.166E 01
17 2 1.045E 02
17 3 1.286E 02
17 4 1.330E 02
17 5 1.202E 02
17 6 8.877E 01
17 7 3.851E 01
17 8 -3.056E 01
17 9 -1.186E 02
17 10 -2.250E 02

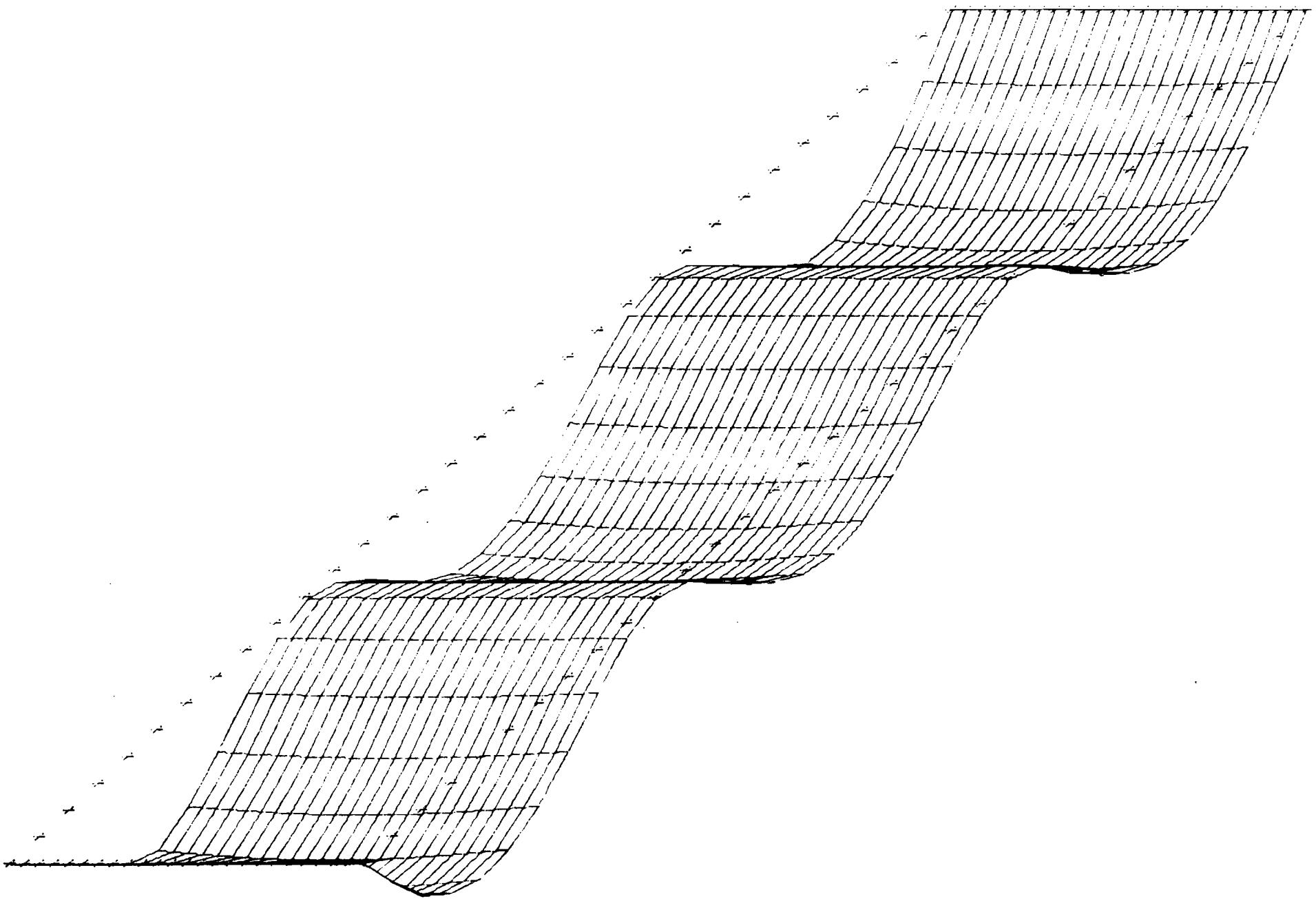
17 11 -1.222E 02
17 12 -3.825E 01
17 13 2.690E 01
17 14 7.324E 01
17 15 1.015E 02
17 16 1.109E 02
17 17 1.015E 02
17 18 7.324E 01
17 19 2.690E 01
17 20 -3.825E 01
17 21 -1.222E 02
17 22 -2.250E 02
17 23 -1.184E 02
17 24 -3.056E 01
17 25 3.851E 01
17 26 8.877E 01
17 27 1.202E 02
17 28 1.338E 02
17 29 1.286E 02
17 30 1.045E 02
17 31 6.166E 01
17 32 -2.129E-14

BEAM Y MOMENT BETWEEN (22 , 0) AND (22 , 32)

X , Y BEAM Y MOM

22 0 -2.129E-14
22 1 5.859E 01
22 2 9.945E 01
22 3 1.226E 02
22 4 1.280E 02
22 5 1.157E 02
22 6 8.504E 01
22 7 3.663E 01
22 8 -2.951E 01
22 9 -1.134E 02
22 10 -2.150E 02
22 11 -1.171E 02
22 12 -3.682E 01
22 13 2.570E 01
22 14 7.051E 01
22 15 9.709E 01
22 16 1.059E 02
22 17 9.709E 01
22 18 7.051E 01
22 19 2.570E 01
22 20 -3.682E 01
22 21 -1.171E 02
22 22 -2.150E 02
22 23 -1.134E 02
22 24 -2.951E 01
22 25 3.663E 01
22 26 8.504E 01
22 27 1.157E 02
22 28 1.280E 02
22 29 1.226E 02
22 30 9.945E 01
22 31 5.859E 01
22 32 0.0

***** TIME SINCE BEGINNING OF EXECUTION IS 233.82 SECONDS ,
TIME SINCE LAST CALL OF PRTIME IS 233.82 SECONDS *****



PROGRAM SLAB 49 -TEX HWY DEPT DECK- MATLOCK, PANAK

REV DATE 29 FEB 72

I-BEAM STRUCTURE SPANS 50-60-50, 33 FT RUNY, HS20 LDG
ANALYSIS OF THD THREE-SPAN CONTINUOUS RUN 29 FEB 1972 JJP FT-KIP UNITSPRDB
1002A 2 HS20 TRUCKS — MAX POS MOMENT CNTR SPAN, GIRDERS NO 2

TABLE 1. CONTROL DATA

	TABLE NUMBER							
	2	3	4	5	6	7	8	9
KEEP FROM PRECEDING PROBLEM (1=YES)	0	0	0	0	0	0	0	0
NUM CARDS INPUT THIS PROBLEM	1	55	0	1	0	40	5	5
MULTIPLE LOAD OPTION	0							
STATICS CHECK OPTION	0							
PRIN STRESS OPTION	0							
PROFILE PLOT OPTION	0							
3-D PLOT OPTION	1							

TABLE 2. CONSTANTS

NUMBER OF INCREMENTS IN X DIRECTION	24
NUMBER OF INCREMENTS IN Y DIRECTION	32
INCREMENT LENGTH IN X DIRECTION	1.450E 00
INCREMENT LENGTH IN Y DIRECTION	5.000E 00
PLISSONS RATIO	1.500E-01
SLAB THICKNESS	0.0

TABLE 3. JOINT BENDING STIFFNESSES, LOADS, AND SUPPORTS

FROM JOINT	THRU JOINT	CX	DY	FX	FY	Q	S
Slab bending stiffness							
0 0 24	32	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0
0 1 24	31	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0
1 0 23	32	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0
1 1 23	31	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0
2 0 2	32 0.0	0.0	0.0	0.0	6.135E 05	0.0	0.0
2 1 2	31 0.0	0.0	0.0	0.0	6.135E 05	0.0	0.0
2 12 2	20 0.0	0.0	0.0	0.0	1.829E 06	0.0	0.0
7 0 7	32 0.0	0.0	0.0	0.0	6.135E 05	0.0	0.0
7 1 7	31 0.0	0.0	0.0	0.0	6.135E 05	0.0	0.0
7 12 7	20 0.0	0.0	0.0	0.0	1.881E 06	0.0	0.0
12 0 12	32 0.0	0.0	0.0	0.0	6.135E 05	0.0	0.0
12 1 12	31 0.0	0.0	0.0	0.0	6.135E 05	0.0	0.0
12 12 12	20 0.0	0.0	0.0	0.0	6.135E 05	0.0	0.0
12 12 12	20 0.0	0.0	0.0	0.0	1.881E 06	0.0	0.0
17 0 17	32 0.0	0.0	0.0	0.0	6.135E 05	0.0	0.0
17 1 17	31 0.0	0.0	0.0	0.0	6.135E 05	0.0	0.0
17 12 17	20 0.0	0.0	0.0	0.0	1.881E 06	0.0	0.0
22 0 22	32 0.0	0.0	0.0	0.0	6.135E 05	0.0	0.0
22 1 22	31 0.0	0.0	0.0	0.0	6.135E 05	0.0	0.0
22 12 22	20 0.0	0.0	0.0	0.0	1.829E 06	0.0	0.0

2 0 22	0	0.0	0.0	0.0	2.663E 04	0.0	0.0	0.0
2 5 22	5	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0
3 5 21	5	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0
2 9 22	9	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0
3 9 21	9	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0
2 14 22	14	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 14 21	14	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 18 22	18	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 18 21	18	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 23 22	23	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 23 21	23	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 27 22	27	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 27 21	27	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 32 22	32	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 32 21	32	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 0 2	0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7 0 7	0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
12 0 12	0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
17 0 17	0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
22 0 22	0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 10 2	10	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7 10 7	10	0.0	0.0	0.0	0.0	0.0	0.0	0.0
12 10 12	10	0.0	0.0	0.0	0.0	0.0	0.0	0.0
17 10 17	10	0.0	0.0	0.0	0.0	0.0	0.0	0.0
22 10 22	10	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 22 2	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7 22 7	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0
12 22 12	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0
17 22 17	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0
22 22 22	22	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 32 2	32	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7 32 7	32	0.0	0.0	0.0	0.0	0.0	0.0	0.0
12 32 12	32	0.0	0.0	0.0	0.0	0.0	0.0	0.0
17 32 17	32	0.0	0.0	0.0	0.0	0.0	0.0	0.0
22 32 22	32	0.0	0.0	0.0	0.0	0.0	0.0	0.0

TABLE 4. JOINT RESTRAINTS AND APPLIED MOMENTS

FROM JOINT	THRU JOINT	RX	RY	TX	TY
NONE					
1 1 24	32	0.210E 03	0.0	0.0	0.0
12 12 12	20 0.0	0.0	0.0	0.0	0.0

TABLE 5. MESH TWISTING STIFFNESSES

FROM MESH	THRU MESH	C	Slab twisting stiffness
1 1 24	32	0.210E 03	0.0

TABLE 6. BAR AXIAL THRUSTS

FROM BAR	THRU BAR	PX	PY	PBX	PBY
NONE					

TABLE 7. MULTIPLE LIADS

FROM	THRU	JOINT	JCINT	QM
2	13	2	13	-1.300E 00
3	13	3	13	-2.800E 00
6	13	6	13	-7.000E-01
7	13	7	13	-3.400E 00
9	13	9	13	-1.700E 00
10	13	10	13	-2.400E 00
13	13	13	13	-1.100E 00
14	13	14	13	-3.000E 00
2	14	2	14	-3.000E-01
2	14	2	14	-7.000E-01
6	14	6	14	-2.000E-01
7	14	7	14	-8.000E-01
9	14	9	14	-4.000E-01
10	14	10	14	-6.000E-01
13	14	13	14	-3.000E 00
14	14	14	14	-7.000E-01
2	16	2	16	-6.300E 00
3	16	3	16	-1.400E 01
6	16	6	16	-3.500E 00
7	16	7	16	-1.680E 01
9	16	9	16	-8.480E 00
10	16	10	16	-1.190E 01
13	16	13	16	-5.600E 00
14	16	14	16	-1.470E 01
2	18	2	18	-1.300E 00
3	18	3	18	-2.800E 00
6	18	6	18	-6.000E-01
7	18	7	18	-3.400E 00
9	18	9	18	-1.700E 00
10	18	10	18	-2.400E 00
13	18	13	18	-1.100E 00
14	18	14	18	-2.900E 00
2	19	2	19	-5.000E 00
3	19	3	19	-1.120E 01
6	19	6	19	-2.800E 00
7	19	7	19	-1.340E 01
9	19	9	19	-6.700E 00
10	19	10	19	-9.500E 00
13	19	13	19	-6.500E 00
14	19	14	19	-1.180E 01

TABLE 8. PROFILE OUTPUT AREAS

FROM	THRU	CFLY	X MOMENT	Y MOMENT	PRIN NOM OR STRESS
JOINT	JOINT	(1=YES)	(1=SLAB,2=BEAM)		(1=YES)
2	0	2	32	1	0
7	0	7	32	1	0
12	0	12	32	1	0
17	0	17	32	1	0
22	0	22	32	1	0

TABLE 9. PRINTED OUTPUT LIMITS

FROM Y STA	THRU Y STA
0	0
9	11
13	19
21	23
22	22

PROGRAM SLAB 44 - TEX HWY DEPF DECK - MATLOCK, PANAK

KEY DATE 29 FEB 72

I-BEAM STRUCTURE SPANS 50-6G-50, 33 FT RDWY, HS20 LDG
ANALYSIS OF THE THREE-SPAN CONTINUOUS RNF 29 FEB 1972 JJP FT-KIP UNITS

PRCB (CONT'D)
1002A 2 HS20 TRUCKS -- MAX POS. MOMENT CNTR. SPAN, GIRDER NO. 2

RESULTS

SLAB X MOMENT AND X TWISTING MOMENT ACT IN THE X DIRECTION (ABOUT Y AXIS)
Y TWISTING MOMENT = -X TWISTING MOMENT; COUNTERCLOCKWISE BETA ANGLES ARE
POSITIVE FROM THE X AXIS TO THE DIRECTION OF THE LARGEST PRINCIPAL MOMENT
SLAB MOMENTS ARE PER UNIT WIDTH

BEAM MOMENTS ARE TOTAL PER BEAM

X + Y	UEFL	SLAB X MOMENT	SLAB Y MOMENT	SLAB X BEAM Y MOMENT	SLAB Y TWISTING MOMENT	LARGEST PRINCIPAL SLAB MOMENT	X TO LARGEST MOMENT	BETA X TO SUPPORT REACTION	
0	32	8.951D-05	1.593E-15	3.010E-16	1.400D-02	1.400E-02	45.0	0.0	
1	32	3.286D-05	0.0	0.0	0.0	0.0	30.1	0.0	
2	32	3.562D-14	4.050E-C2	-1.546E-09	3.538D-02	6.102E-02	34.8	-3.562D 00	
3	32	-1.008D-05	2.964E-02	5.897E-03	4.550D-02	7.130E-02	40.6	0.0	
4	32	-1.162D-05	2.885E-01	-8.550E-03	4.661D-02	5.443E-02	43.0	0.0	
5	32	-8.910D-06	1.452E-02	-4.746E-10	5.141D-02	5.516E-02	44.3	0.0	
6	32	-4.672D-06	2.616E-01	0.0	5.382D-02	5.514E-02	44.8	0.0	
7	32	2.657D-14	1.078E-01	0.0	5.732D-02	5.769E-02	44.6	0.0	
8	32	5.827D-06	2.903E-03	-3.812E-10	6.308D-02	6.424E-02	44.6	-2.657D 00	
9	32	9.360D-06	2.810E-02	0.0	6.929D-02	-7.127E-02	-44.2	0.0	
10	32	9.063D-06	-6.020E-03	4.798E-10	7.886D-02	-8.193E-02	-43.9	0.0	
11	32	5.228D-06	-2.371E-03	7.419E-11	8.472D-02	-8.592E-02	-44.6	0.0	
12	32	-2.146D-14	-3.529E-02	0.0	9.246D-02	9.520E-02	44.4	-2.146D 00	
13	32	-2.505D-06	4.739E-03	7.049E-04	-1.022E-03	1.007E-01	44.8	0.0	
14	32	-4.358D-06	1.110E-03	-4.200E-11	1.059D-01	1.053E-01	44.9	0.0	
15	32	-5.764D-06	7.006E-04	-3.552E-11	1.104D-01	1.120E-01	44.6	0.0	
16	32	-5.290D-06	1.132E-J2	0.0	1.220E-01	1.207E-01	44.0	0.0	
17	32	1.781D-14	2.323E-01	-3.443E-03	1.224D-01	1.318E-01	43.4	-1.781D 00	
18	32	1.446D-05	7.617E-04	-3.275E-11	1.271D-01	1.275E-01	44.9	0.0	
19	32	2.937D-05	1.134E-02	0.0	1.297D-01	-1.362E-01	-43.6	0.0	
20	32	3.690D-05	-1.257E-02	7.856E-10	1.280D-01	-1.412E-01	-42.2	0.0	
21	32	2.961D-05	-3.752E-01	0.0	1.313E-09	1.181D-01	-1.386E-01	-40.4	0.0
22	32	1.211D-15	-3.798E-02	-1.248E-02	-6.105E-01	1.159D-01	-1.694E-01	-36.4	-1.211D-01

STATICS CHECK. SUMMATION OF REACTIONS = 1.8280 02
MAXIMUM STATICS CHECK ERROR AT STA 2 14 = 8.8970-05

PROGRAM SLAD 49 -TEX HWY DEPT DECK- MATLOCK, PANAK

REV DATE 29 FEB 72

I-BEAM STRUCTURE SPANS 50-60-50, 33 FT RDWY, HS20 LDG
ANALYSIS OF THD THREE-SPAN CONTINUOUS RUN 29 FEB 1972 JJP FT-KIP UNITSPROB (CLNTD)
1002A 2 HS20 TRUCKS -- MAX POS MOMENT CNTR SPAN, GIRDER NO 2PROFILE OUTPUT AREAS
X MOMENTS ACT IN X DIRECTION (ABOUT Y AXIS)
THE PLOTTED RESULTS INDICATE THE RELATIVE VALUE EACH HAS WITHIN THAT LIST

DEFLECTIONS BETWEEN (2 , 0) AND (2 , 32)

X , Y DEFLECTION

2	0	3.095E-14
2	1	4.459E-03
2	2	8.630E-03
2	3	1.226E-02
2	4	1.506E-02
2	5	1.675E-02
2	6	1.706E-02
2	7	1.577E-02
2	8	1.263E-02
2	9	7.639E-03
2	10	-2.307E-13
2	11	-9.825E-03
2	12	-1.988E-02
2	13	-2.915E-02
2	14	-3.675E-02
2	15	-4.200E-02
2	16	-4.431E-02
2	17	-4.319E-02
2	18	-3.886E-02
2	19	-3.161E-02
2	20	-2.197E-02
2	21	-1.109E-02
2	22	-2.950E-13
2	23	8.462E-03
2	24	1.441E-02
2	25	1.803E-02
2	26	1.954E-02
2	27	1.921E-02
2	28	1.729E-02
2	29	1.409E-02
2	30	9.930E-03
2	31	5.127E-03
2	32	3.562E-14

DEFLECTIONS BETWEEN (7 , 0) AND (7 , 32)

X , Y DEFLECTION

7	0	2.287E-14
7	1	3.890E-03
7	2	7.558E-03
7	3	1.078E-02
7	4	1.332E-02
7	5	1.496E-02
7	6	1.543E-02
7	7	1.448E-02
7	8	1.181E-02
7	9	7.099E-03
7	10	-2.781E-13
7	11	-9.912E-03
7	12	-2.039E-02
7	13	-3.019E-02

7	14	-3.639E-02
7	15	-4.430E-02
7	16	-4.714E-02
7	17	-4.612E-02
7	18	-4.160E-02
7	19	-3.392E-02
7	20	-2.350E-02
7	21	-1.173E-02
7	22	-4.072E-13
7	23	8.338E-03
7	24	1.383E-02
7	25	1.692E-02
7	26	1.800E-02
7	27	1.742E-02
7	28	1.550E-02
7	29	1.253E-02
7	30	8.782E-03
7	31	4.519E-03
7	32	2.657E-14

DEFLECTIONS BETWEEN (12 , 0) AND (12 , 32)

X , Y DEFLECTION

12	0	1.848E-14
12	1	3.103E-03
12	2	6.034E-03
12	3	8.618E-03
12	4	1.068E-02
12	5	1.202E-02
12	6	1.246E-02
12	7	1.175E-02
12	8	9.631E-03
12	9	5.820E-03
12	10	-2.304E-13
12	11	-8.193E-03
12	12	-1.689E-02
12	13	-2.504E-02
12	14	-3.186E-02
12	15	-3.677E-02
12	16	-3.912E-02
12	17	-3.828E-02
12	18	-3.452E-02
12	19	-2.813E-02
12	20	-1.948E-02
12	21	-9.709E-03
12	22	-3.354E-13
12	23	6.858E-03
12	24	1.133E-02
12	25	1.133E-02

22	0	1.015E-03
22	1	-2.675E-03
22	2	-2.569E-03
22	3	-2.304E-03
22	4	-1.903E-03
22	5	-1.390E-03
22	6	-1.078E-03
22	7	-7.841E-04
22	8	2.216E-14
22	9	9.234E-04
22	10	1.794E-03
22	11	2.486E-03
22	12	2.917E-03
22	13	3.047E-03
22	14	2.671E-03
22	15	2.418E-03
22	16	1.741E-03
22	17	9.094E-04
22	18	1.211E-15

BEAM Y MOMENT BETWEEN (2 , 0) AND (2 , 32)

X , Y BEAM Y MOM

```

2 0 -8.031E-03   *
2 1 -1.370E 01   *
2 2 -2.726E 01   *
2 3 -4.075E 01   *
2 4 -5.414E 01   *
2 5 -6.736E 01   *
2 6 -7.917E 01   *
2 7 -9.044E 01   *
2 8 -1.009E 02   *
2 9 -1.102E 02   *
2 10 -1.171E 02  *
2 11 -1.134E 01   *
2 12 9.659E 01   *
2 13 2.030E 02   *
2 14 2.889E 02   *
2 15 3.575E 02   *
2 16 4.204E 02   *
2 17 3.922E 02   *
2 18 3.576E 02   *
2 19 2.910E 02   *
2 20 1.522E 02   *
2 21 1.019E 01   *
2 22 -1.289E 02  *
2 23 -1.235E 02  *
2 24 -1.142E 02  *
2 25 -1.032E 02  *
2 26 -9.093E 01  *
2 27 -7.779E 01  *
2 28 -6.264E 01  *
2 29 -4.720E 01  *
2 30 -3.159E 01  *
2 31 -1.588E 01  *
2 32 -8.550E-03  *

```

BEAM Y MOMENT BETWEEN (7 , 0) AND (7 , 32)

X , Y BEAM Y MOM

```

7 0 -1.974E-04   *
7 1 -1.090E 01   *
7 2 -2.195E 01   *
7 3 -3.321E 01   *
7 4 -4.475E 01   *
7 5 -5.671E 01   *
7 6 -7.011E 01   *
7 7 -8.442E 01   *
7 8 -1.000E 02   *
7 9 -1.173E 02   *
7 10 -1.380E 02  *
7 11 -2.778E 01  *
7 12 8.417E 01   *
7 13 1.985E 02   *
7 14 2.2859E 02  *
7 15 3.808E 02   *
7 16 4.802E 02   *
7 17 4.343E 02   *
7 18 3.944E 02   *
7 19 3.393E 02   *
7 20 1.679E 02   *
7 21 -1.687E 00   *
7 22 -1.666E 02   *
7 23 -1.396E 02   *
7 24 -1.180E 02   *
7 25 -9.877E 01   *
7 26 -8.148E 01   *
7 27 -6.551E 01   *
7 28 -5.160E 01   *
7 29 -3.825E 01   *
7 30 -2.526E 01  *

```

```

7 31 -1.259E 01   *
7 32 -4.334E-04  *

```

BEAM Y MOMENT BETWEEN (12 , 0) AND (12 , 32)

X , Y BEAM Y MOM

```

12 0 -8.389E-04   *
12 1 -8.438E 00   *
12 2 -1.700E 01   *
12 3 -2.579E 01   *
12 4 -3.496E 01   *
12 5 -4.466E 01   *
12 6 -5.634E 01   *
12 7 -6.906E 01   *
12 8 -8.310E 01   *
12 9 -9.858E 01   *
12 10 -1.165E 02  *
12 11 -2.483E 01  *
12 12 6.884E 01   *
12 13 1.662E 02   *
12 14 2.383E 02   *
12 15 3.174E 02   *
12 16 3.977E 02   *
12 17 3.618E 02   *
12 18 3.282E 02   *
12 19 2.802E 02   *
12 20 1.200E 02   *

```

```

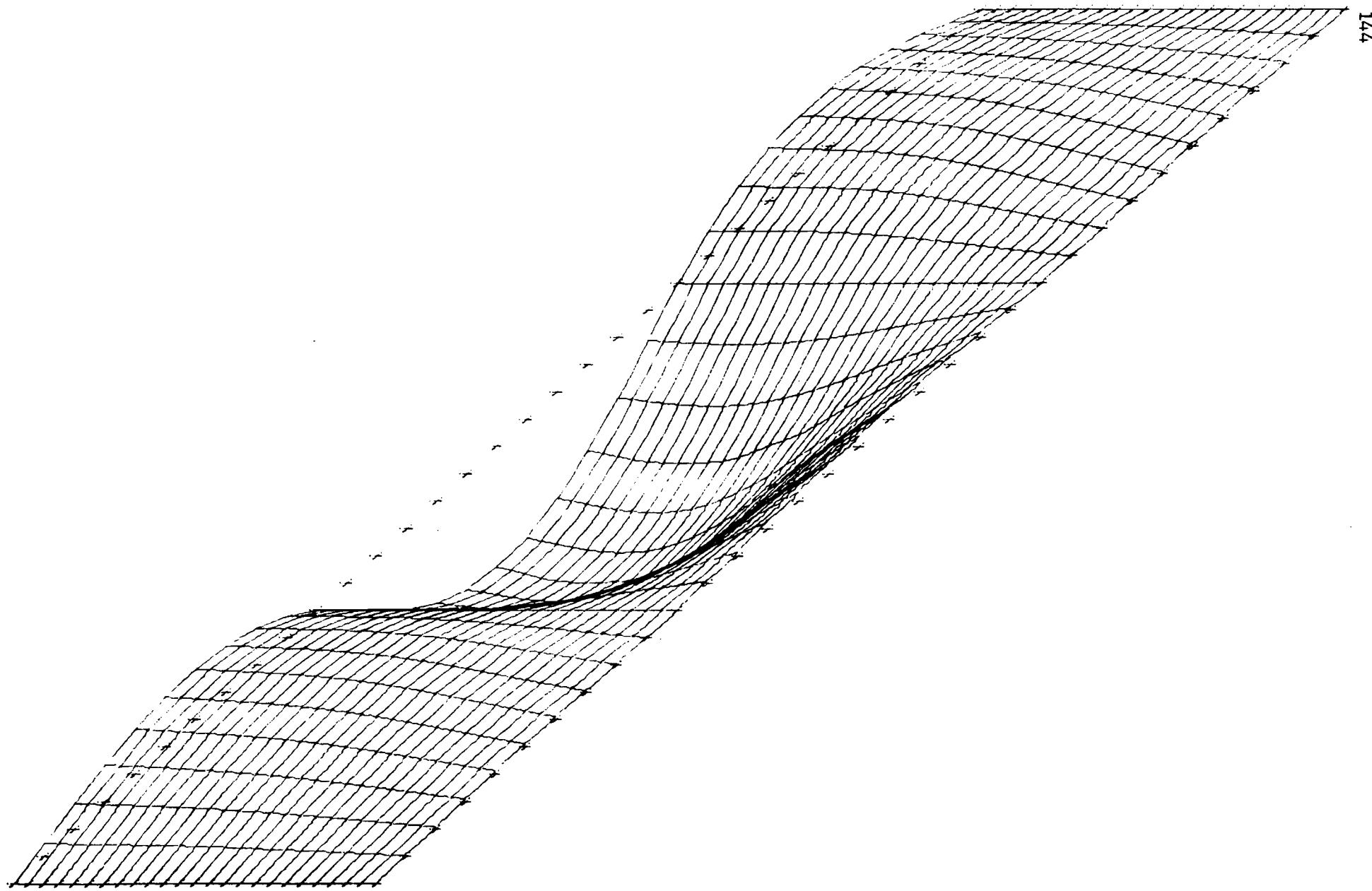
0 1.500E-02
22 1 -3.371E 00
22 2 -6.778E 00
22 3 -9.792E 00
22 4 -1.218E 01
22 5 -1.374E 01
22 6 -1.371E 01
22 7 -1.229E 01
22 8 -6.142E 00
22 9 -3.918E 00
22 10 4.360E 00
22 11 8.168E 00
22 12 1.300E 01
22 13 1.703E 01
22 14 2.005E 01
22 15 2.116E 01
22 16 2.021E 01
22 17 1.945E 01
22 18 1.661E 01
22 19 1.364E 01
22 20 1.143E 01
22 21 8.700E 00
22 22 6.835E 00
22 23 -2.570E 00
22 24 -8.822E 00
22 25 -1.277E 01
22 26 -1.474E 01
22 27 -1.509E 01
22 28 -1.354E 01
22 29 -1.097E 01
22 30 -7.632E 00
22 31 -3.799E 00
22 32 1.809E-02

```

**** TIME SINCE BEGINNING OF EXECUTION IS 209.87 SECONDS .

TIME SINCE LAST CALL OF PRTIME IS 209.87 SECONDS ****

1002A



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PROGRAM SLAB 49 -TEX HWY DEPT DECK- MATLOCK,PANAK

REV DATE 29 FEB 72

I-BEAM STRUCTURE SPANS 50-60-50, 33 FT RDWY, HS20 LDG
 ANALYSIS OF THD THREE-SPAN CONTINUOUS RUN 29 FEB 1972 JJP FT-KIP UNITS

PROB 1003A LANE LOADING FOR MAX NEG MOMENT COMPOSITE DECK WHERE MOM IS POS

TABLE 1. CONTROL DATA

	TABLE NUMBER								
	2	3	4	5	6	7	8	9	

KEEP FROM PRECEDING PROBLEM (1=YES)	0	0	0	0	0	0	0	0
NUM CARDS INPUT THIS PROBLEM	1	60	0	1	0	13	5	6

MULTIPLE LOAD OPTION	0
STATICS CHECK OPTION	0
PRIN STRESS OPTION	0
PROFILE PLOT OPTION	0
3-D PLOT OPTION	1

TABLE 2. CONSTANTS

NUMBER OF INCREMENTS IN X DIRECTION	24
NUMBER OF INCREMENTS IN Y DIRECTION	32
INCREMENT LENGTH IN X DIRECTION	1.450E 00
INCREMENT LENGTH IN Y DIRECTION	5.000E 00
POISONS RATIO	1.500E-01
SLAB THICKNESS	0.0

TABLE 3. JOINT BENDING STIFFNESSES, LOADS, AND SUPPORTS

FROM JOINT	THRU JOINT	CX	DY	FX	FY	Q	S
Slab bending stiffness							
0 0 24	32	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0
0 1 24	31	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0
1 0 23	32	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0
1 1 23	31	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0
2 0 2	32	0.0	0.0	6.135E 05	0.0	0.0	0.0
2 1 2	31	0.0	0.0	6.135E 05	0.0	0.0	0.0
2 1 2	7	0.0	0.0	1.829E 06	0.0	0.0	0.0
2 12 2	20	0.0	0.0	1.829E 06	0.0	0.0	0.0
7 0 7	32	0.0	0.0	6.135E 05	0.0	0.0	0.0
7 1 7	31	0.0	0.0	6.135E 05	0.0	0.0	0.0
7 1 7	7	0.0	0.0	1.881E 06	0.0	0.0	0.0
7 12 2	20	0.0	0.0	0.0	1.881E 06	0.0	0.0
12 0 12	32	0.0	0.0	0.0	1.881E 06	0.0	0.0
12 1 12	31	0.0	0.0	0.0	1.881E 06	0.0	0.0
12 1 12	7	0.0	0.0	0.0	1.881E 06	0.0	0.0
12 12 2	20	0.0	0.0	0.0	1.881E 06	0.0	0.0
17 0 17	32	0.0	0.0	6.135E 05	0.0	0.0	0.0
17 1 17	31	0.0	0.0	6.135E 05	0.0	0.0	0.0
17 1 17	7	0.0	0.0	1.881E 06	0.0	0.0	0.0
17 12 17	20	0.0	0.0	1.881E 06	0.0	0.0	0.0
22 0 22	32	0.0	0.0	6.135E 05	0.0	0.0	0.0
22 1 22	31	0.0	0.0	6.135E 05	0.0	0.0	0.0
22 1 22	7	0.0	0.0	1.829E 06	0.0	0.0	0.0
22 12 22	20	0.0	0.0	1.829E 06	0.0	0.0	0.0

2 0 22	0	0.0	C.0	2.663E 04	0.0	0.0	0.0
3 0 21	0	0.0	C.0	2.663E 04	0.0	0.0	0.0
2 5 22	5	0.0	C.0	3.256E 04	0.0	0.0	0.0
3 5 21	5	0.0	C.0	3.256E 04	0.0	0.0	0.0
2 9 22	9	0.0	C.0	3.256E 04	0.0	0.0	0.0
3 9 21	9	0.0	C.0	3.256E 04	0.0	0.0	0.0
2 14 22	14	0.0	0.0	3.256E 04	0.0	0.0	0.0
3 14 21	14	0.0	0.0	3.256E 04	0.0	0.0	0.0
2 18 22	18	0.0	0.0	3.256E 04	0.0	0.0	0.0
3 18 21	18	0.0	C.0	3.256E 04	0.0	0.0	0.0
2 23 22	23	0.0	C.0	3.256E 04	0.0	0.0	0.0
3 23 21	23	0.0	0.0	3.256E 04	0.0	0.0	0.0
2 27 22	27	0.0	0.0	3.256E 04	0.0	0.0	0.0
3 27 21	27	0.0	0.0	3.256E 04	0.0	0.0	0.0
2 32 22	32	0.0	C.0	2.663E 04	0.0	0.0	0.0
3 32 21	32	0.0	C.0	2.663E 04	0.0	0.0	0.0
2 0 2	0	0.0	C.0	0.0	0.0	0.0	1.000E 14
7 0 7	0	0.0	C.0	0.0	0.0	0.0	1.000E 14
12 0 12	0	0.0	C.0	0.0	0.0	0.0	1.000E 14
17 0 17	0	0.0	0.0	0.0	0.0	0.0	1.000E 14
22 0 22	0	0.0	0.0	0.0	0.0	0.0	0.0
7 22 7	22	0.0	0.0	0.0	0.0	0.0	1.000E 14
12 22 12	22	0.0	C.0	0.0	0.0	0.0	1.000E 14
17 22 17	22	0.0	C.0	0.0	0.0	0.0	1.000E 14
22 22 22	22	0.0	C.0	0.0	0.0	0.0	1.000E 14
2 32 2	32	0.0	C.0	0.0	0.0	0.0	1.000E 14
7 32 7	32	0.0	0.0	0.0	0.0	0.0	1.000E 14
12 32 12	32	0.0	C.0	0.0	0.0	0.0	1.000E 14
17 32 17	32	0.0	0.0	0.0	0.0	0.0	1.000E 14
22 32 22	32	0.0	C.0	0.0	0.0	0.0	1.000E 14

TABLE 4. JOINT RESTRAINTS AND APPLIED MOMENTS

FROM JOINT	THRU JOINT	RX	RY	TX	TY
NONE					

TABLE 5. MESH TWISTING STIFFNESSES

FROM MESH	THRU MESH	C
Slab twisting stiffness		
1	1 24	32 6.210E 03

TABLE 6. BAR AXIAL THRLSTS

FROM BAR	THRU BAR	PX	PY	PBX	PBY
NONE					

TABLE 7. MULTIPLE LOADS

FRGM	THRU	JINT	JOINT
1	0	1	22
1	1	1	21
15	0	15	22
15	1	15	21
2	0	14	0
2	22	14	22
2	1	14	21
1	5	1	5
15	5	15	5
2	5	14	5
1	16	1	16
15	16	15	16
2	16	14	16

Lane loading
apportioned as shown
in Fig 16

QM

-5.600E-02
-5.600E-02
-1.790E-01
-1.790E-01
-2.960E-01
-2.960E-01
-5.930E-01
-6.330E-01
-2.013E 00
-3.335E 00
-6.330E-01
-2.013E 00
-3.335E 00

PROGRAM SLAB 49 -TEX HWY DEPT DECK- MATLOCK, PANAK

REV DATE 29 FEB 72

I-BEAM STRUCTURE SPANS 50-60-50, 33 FT RDWY, HS20 LDG
ANALYSIS OF TWO THREE-SPAN CONTINUOUS RUN 29 FEB 1972 JJP FT-KIP UNITSPROB (CCNTD)
1003A LANE LOADING FOR MAX NEG MOMENT COMPOSITE DECK WHERE MOM IS POS

RESULTS

SLAB X MOMENT AND X TWISTING MOMENT ACT IN THE X DIRECTION (ABOUT Y AXIS)
 Y TWISTING MOMENT = -X TWISTING MOMENT, COUNTERCLOCKWISE BETA ANGLES ARE
 POSITIVE FROM THE X AXIS TO THE DIRECTION OF THE LARGEST PRINCIPAL MOMENT
 SLAB MOMENTS ARE PER UNIT WIDTH

BEAM MOMENTS ARE TOTAL PER BEAM

X , Y	DEFL	SLAB X MOMENT	SLAB Y MOMENT	LARGEST SLAB X PRINCIPAL MOMENT	BETA X TO LARGEST MOMENT	SUPPORT REACTION
0 32	4.3490-05	6.563E-16	1.311E-17	6.7690-03	6.769E-03	45.0 0.0
1 32	1.5940-05	1.975E-02	-7.236E-10	1.7C8D-02	2.960E-02	30.0 0.0
2 32	1.8720-14	1.945E-02	2.893E-03	2.2020-02	3.470E-02	34.7 -1.872D 00
3 32	-4.7680-06	1.415E-01	-4.194E-03	6.568E-03	2.655E-10	40.6 0.0
4 32	-5.4420-06	6.568E-03	-2.655E-10	2.2630-02	2.638E-02	43.1 0.0
5 32	-4.1540-C6	4.969E-02	0.0	1.126E-03	-5.308E-11	2.508D-02 2.681E-02
6 32	-2.2040-06	1.676E-02	0.0	1.676E-02	0.0	2.703E-02
7 32	1.4130-14	4.329E-04	-1.696E-11	6.444E-03	0.0	39.3 0.0
8 11	5.1790-04	6.203E-02	1.425E 00	2.666D-01	3.751E-01	38.0 0.0
9 10	5.3020-04	0.0	0.0	5.997E-03	-1.057E 00	1.204D-02 -1.057E 00 -89.4 0.0
10 10	2.404D-04	0.0	0.0	-3.940D-13	-9.283E-02	-1.110E 00 8.909D-03 -1.110E 00 -89.5 3.940D 01
11 10	-3.940D-13	0.0	0.0	-9.283E-02	-1.110E 00	8.909D-03 -1.110E 00 -89.5 3.940D 01
12 10	-2.187D-04	0.0	0.0	2.261E-01	-1.692E 00	1.813D-02 -1.093E 00 -89.2 0.0
13 10	-3.228D-04	0.0	0.0	3.145E-C1	-1.124E 00	3.943D-02 -1.127E 00 -88.4 0.0
14 10	-2.8490-04	0.0	0.0	1.781E-01	-1.214E 00	5.917D-02 -1.216E 00 -87.6 0.0
15 10	-1.412D-04	0.0	0.0	-2.110E-01	-1.350E 00	6.555D-02 -1.353E 00 -86.7 0.0
16 10	-5.546D-13	0.0	0.0	-5.214E-01	-1.514E 00	4.493D-02 -1.518E 00 -85.7 5.546D 01
17 10	-6.279D-C5	0.0	0.0	-2.710E-01	-1.393E 00	2.309D-02 -1.394E 00 -88.8 0.0
18 10	-1.438D-C4	0.0	0.0	4.685E-02	-1.299E 00	2.542D-02 -1.300E 00 -88.9 0.0

TABLE 8. PROFILE OUTPUT AREAS

FROM	THRU	DEFL	X MOMENT (1=SLAB,2=BEAM)	Y MOMENT (1=SLAB,2=BEAM)	PRIM MOM OR STRESS (1=YES)
JOINT	JOINT	{1=YES}	{1=SLAB,2=BEAM}	{1=SLAB,2=BEAM}	{1=YES}
2	0	2	32	1	0
7	0	7	32	1	0
12	0	12	32	1	0
17	0	17	32	1	0
22	0	22	32	1	0

TABLE 9. PRINTED OUTPUT, LIMITS

FROM	THRU	Y STA	Y STA
0	0	0	0
4		6	
9		11	
14		18	
22		22	
32		32	

STATICS CHECK. SUMMATION OF REACTIONS = 2.719D 02
MAXIMUM STATIC CHECK ERROR AT STA 12 10 = -7.648D-05

PROGRAM SLAB 49 -TEX FAY DEPT DECK- MATTEL K-PANAK REV DATE 29 FEB 74

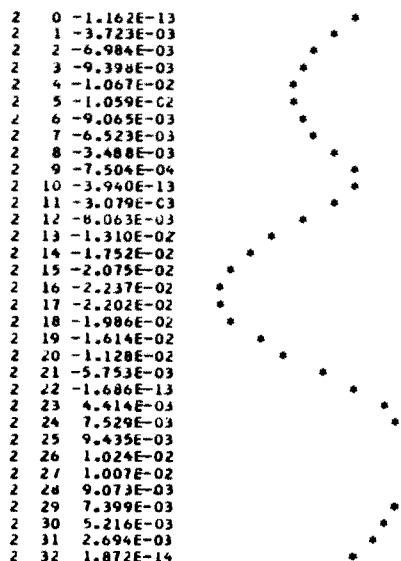
I-BEAM STRUCTURE SPANS 50-60-50, 33 FT RDWY, HS20 LDG
ANALYSIS OF THE THREE-SPAN CONTINUOUS RUN 29 FEB 1972 JJP FT-KIP UNITS

PROB (CCNTO) 1003A LANE LOADING FOR MAX NEG MOMENT COMPOSITE DECK WHERE MOM IS POS

X ELEMENTS ACT IN X DIRECTION (ABOUT Y AXIS)
THE PLOTTED RESULTS INDICATE THE RELATIVE VALUE EACH HAS WITHIN THAT LIST

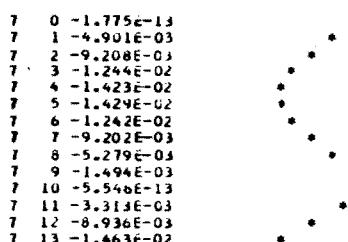
DEFLECTIONS BETWEEN (-2 , 0) AND (-2 , 32)

X , Y DEFLECTION



DEFLECTIONS BETWEEN { -7, 0 } AND { -7, 32 }

X - Y DEFLECTION



```

7 14 -1.966E-02
7 15 -2.339E-02
7 16 -2.523E-02
7 17 -2.472E-02
7 16 -2.215E-02
7 19 -1.790E-02
7 20 -1.243E-02
7 21 -6.261E-03
7 22 -2.482E-13
7 23 4.435E-03
7 24 7.352E-03
7 25 8.988E-03
7 26 9.554E-03
7 27 9.240E-03
7 28 8.219E-03
7 29 6.643E-03
7 30 4.655E-03
7 31 2.399E-03
7 32

```

BEAM Y MOMENT BETWEEN (2 , 0) AND (2 , 32)

X , Y BEAM Y MOM

```

2 0 -1.813E-02
2 1 5.647E 01
2 2 1.036E 02
2 3 1.400E 02
2 4 1.646E 02
2 5 1.767E 02
2 6 1.245E 02
2 7 6.022E 01
2 8 -1.456E 01
2 9 -9.754E 01
2 10 -1.880E 02
2 11 -9.347E 01
2 12 -5.912E 00
2 13 7.463E 01
2 14 1.448E 02
2 15 1.986E 02
2 16 2.400E 02
2 17 2.212E 02
2 18 1.897E 02
2 19 1.409E 02
2 20 8.112E 01
2 21 1.104E 01
2 22 -6.571E 01
2 23 -6.374E 01
2 24 -5.938E 01
2 25 -5.395E 01
2 26 -4.772E 01
2 27 -4.098E 01
2 28 -3.305E 01
2 29 -2.493E 01
2 30 -1.670E 01
2 31 -0.392E 00
2 32 -4.194E-03

```

BEAM Y MOMENT BETWEEN (7 , 0) AND (7 , 32)

X , Y BEAM Y MOM

```

7 0 2.312E-02
7 1 7.382E 01
7 2 1.333E 02
7 3 1.800E 02
7 4 2.151E 02
7 5 2.384E 02
7 6 1.691E 02

```

```

7 7 8.734E 01
7 8 -6.855E 00
7 9 -1.124E 02
7 10 -2.359E 02
7 11 -1.134E 02
7 12 -9.104E 00
7 13 8.290E 01
7 14 1.621E 02
7 15 2.340E 02
7 16 2.924E 02
7 17 2.566E 02
7 18 2.080E 02
7 19 1.529E 02
7 20 8.575E 01
7 21 4.633E 00
7 22 -8.961E 01
7 23 -7.453E 01
7 24 -6.285E 01
7 25 -5.250E 01
7 26 -4.322E 01
7 27 -3.466E 01
7 28 -2.728E 01
7 29 -2.020E 01
7 30 -1.334E 01
7 31 -6.624E 00
7 32 -3.200E-04

```

BEAM Y MOMENT BETWEEN (12 , 0) AND (12 , 32)

X , Y BEAM Y MOM

```

12 0 1.349E-02
12 1 6.316E 01
12 2 1.142E 02
12 3 1.542E 02
12 4 1.844E 02
12 5 2.049E 02
12 6 1.453E 02
12 7 7.551E 01
12 8 -5.059E 00
12 9 -9.556E 01
12 10 -1.996E 02
12 11 -9.691E 01
12 12 -8.929E 00
12 13 6.878E 01
12 14 1.354E 02
12 15 1.975E 02
12 16 2.493E 02
12 17 2.167E 02
12 18 1.739E 02
12 19 1.279E 02

```

```

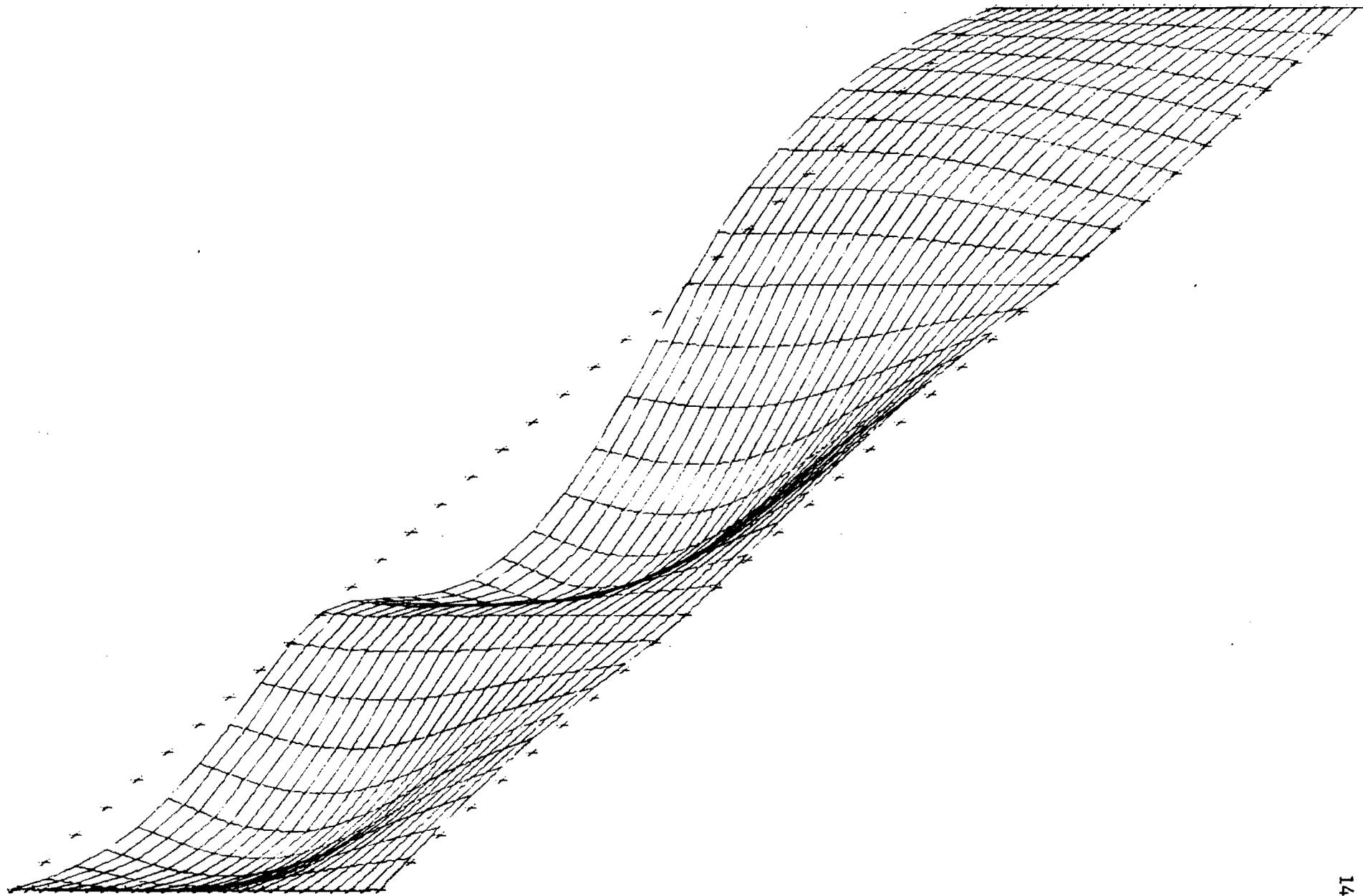
22 0 -5.442E 00
22 1 -5.765E 00
22 2 -7.099E 00
22 3 -7.524E 00
22 4 -6.850E 00
22 5 -5.599E 00
22 6 -3.912E 00
22 7 -1.948E 00
22 8 -9.966E-03

```

***** TIME SINCE BEGINNING OF EXECUTION IS 351.27 SECONDS ,

TIME SINCE LAST CALL OF PRTIME IS 141.40 SECONDS *****

100.3A



PROGRAM SLAB 49 -TEX HWY DEPT DECK- MATLOCK, PANAK

REV DATE 29 FEB 72

I-BEAM STRUCTURE SPANS 50-60-50, 33 FT RDWY, HS20 LOG
ANALYSIS OF TWO THREE-SPAN CONTINUOUS RUN 29 FEB 1972 JJP FT-KIP UNITSPROB
1004A 2 HS20 TRUCKS STAGGERED IN ADJACENT SPANS FOR MAX NEG SUPPORT MOMENT

TABLE 1. CONTROL DATA

	TABLE NUMBER								
	2	3	4	5	6	7	8	9	
KEEP FROM PRECEDING PROBLEM (L=YES)	0	0	0	0	0	0	0	0	
MUM CARDS INPUT THIS PROBLEM	1	59	0	1	0	40	6	6	
MULTIPLE LOAD OPTION	0								
STATICS CHECK OPTION	0								
PRIN STRESS OPTION	0								
PROFILE PLOT OPTION	0								
3-D PLOT OPTION	1								

TABLE 2. CONSTANTS

NUMBER OF INCREMENTS IN X DIRECTION	24
NUMBER OF INCREMENTS IN Y DIRECTION	32
INCREMENT LENGTH IN X DIRECTION	1.450E 00
INCREMENT LENGTH IN Y DIRECTION	5.000E 00
POISSON'S RATIO	1.500E-01
SLAB THICKNESS	0.0

TABLE 3. JOINT BENDING STIFFNESSES, LOADS, AND SUPPORTS

FROM JOINT	THRU MESH	CX	DY	FX	FY	Q	S
Slab bending stiffness							
0 0 24 32	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0	0.0
0 1 24 31	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0	0.0
0 1 23 32	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0	0.0
1 1 23 31	1.830E 03	1.830E 03	0.0	0.0	0.0	0.0	0.0
2 0 2 32	0.0	0.0	0.0	6.135E 05	0.0	0.0	0.0
2 1 2 31	0.0	0.0	0.0	6.135E 05	0.0	0.0	0.0
2 12 2 20	0.0	0.0	0.0	1.029E 06	0.0	0.0	0.0
7 0 7 32	0.0	0.0	0.0	6.135E 05	0.0	0.0	0.0
7 1 7 31	0.0	0.0	0.0	6.135E 05	0.0	0.0	0.0
7 1 7 7	0.0	0.0	0.0	1.081E 06	0.0	0.0	0.0
7 12 7 20	0.0	0.0	0.0	1.081E 06	0.0	0.0	0.0
12 0 12 32	0.0	0.0	0.0	6.135E 05	0.0	0.0	0.0
12 1 12 31	0.0	0.0	0.0	6.135E 05	0.0	0.0	0.0
12 1 12 8	0.0	0.0	0.0	1.081E 06	0.0	0.0	0.0
12 13 12 21	0.0	0.0	0.0	1.081E 06	0.0	0.0	0.0
17 0 17 32	0.0	0.0	0.0	6.135E 05	0.0	0.0	0.0
17 1 17 31	0.0	0.0	0.0	6.135E 05	0.0	0.0	0.0
17 1 17 9	0.0	0.0	0.0	1.081E 06	0.0	0.0	0.0
17 16 17 24	0.0	0.0	0.0	1.081E 06	0.0	0.0	0.0
22 0 22 32	0.0	0.0	0.0	6.135E 05	0.0	0.0	0.0
22 1 22 31	0.0	0.0	0.0	6.135E 05	0.0	0.0	0.0
22 1 22 11	0.0	0.0	0.0	1.029E 06	0.0	0.0	0.0
22 20 22 31	0.0	0.0	0.0	1.029E 06	0.0	0.0	0.0
2 0 22 0	0.0	0.0	0.0	2.663E 04	0.0	0.0	0.0
3 0 21 0	0.0	0.0	0.0	2.663E 04	0.0	0.0	0.0
2 5 22 5	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0
3 5 21 5	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0
2 9 22 9	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0
3 9 21 9	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0
2 14 22 14	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0
3 14 21 14	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0
2 18 22 18	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0
3 18 21 18	0.0	0.0	0.0	3.256E 04	0.0	0.0	0.0

2 23 22	23	0.0	0.0	3.256E 04	0.0	0.0	0.0
3 23 21	23	0.0	0.0	3.256E 04	0.0	0.0	0.0
2 27 22	27	0.0	0.0	3.256E 04	0.0	0.0	0.0
3 27 21	27	0.0	0.0	3.256E 04	0.0	0.0	0.0
2 32 22	32	0.0	0.0	2.663E 04	0.0	0.0	0.0
3 32 21	32	0.0	0.0	2.663E 04	0.0	0.0	0.0
2 0 2	0	0.0	0.0	0.0	0.0	0.0	0.0
7 0 7	0	0.0	0.0	0.0	0.0	0.0	0.0
12 0 12	0	0.0	0.0	0.0	0.0	0.0	0.0
17 0 17	0	0.0	0.0	0.0	0.0	0.0	0.0
22 0 22	0	0.0	0.0	0.0	0.0	0.0	0.0
2 10 2	10	0.0	0.0	0.0	0.0	0.0	0.0
7 10 7	10	0.0	0.0	0.0	0.0	0.0	0.0
12 10 12	10	0.0	0.0	0.0	0.0	0.0	0.0
17 10 17	10	0.0	0.0	0.0	0.0	0.0	0.0
22 10 22	10	0.0	0.0	0.0	0.0	0.0	0.0
7 22 7	22	0.0	0.0	0.0	0.0	0.0	0.0
12 22 12	22	0.0	0.0	0.0	0.0	0.0	0.0
17 22 17	22	0.0	0.0	0.0	0.0	0.0	0.0
22 22 22	22	0.0	0.0	0.0	0.0	0.0	0.0
2 32 2	32	0.0	0.0	0.0	0.0	0.0	0.0
7 32 7	32	0.0	0.0	0.0	0.0	0.0	0.0
12 32 12	32	0.0	0.0	0.0	0.0	0.0	0.0
17 32 17	32	0.0	0.0	0.0	0.0	0.0	0.0
22 32 22	32	0.0	0.0	0.0	0.0	0.0	0.0

TABLE 4. JOINT RESTRAINTS AND APPLIED MOMENTS

FROM JOINT	THRU JOINT	RX	RY	TX	TY

TABLE 5. MESH TWISTING STIFFNESSES

FROM MESH	THRU MESH	C
Slab twisting stiffness		
1	1 24 32	6.210E 03

TABLE 6. BAR AXIAL THRESTS

FROM BAR	THRU BAR	PX	PY	PBX	PBY

NONE

TABLE 7. MULTIPLE LOADS

FROM JOINT	THRU JOINT	QM
9 2	9 2	-1.700E 00
10 2	10 2	-2.400E 00
13 2	13 2	-1.100E 00
14 2	14 2	-3.000E 00
9 3	9 3	-4.000E-01
10 3	10 3	-6.000E-01
13 3	13 3	-3.000E-01
14 3	14 3	-7.000E-01
9 5	9 5	-8.500E 00
10 5	10 5	-1.200E 01
13 5	13 5	-5.600E 00
14 5	14 5	-1.480E 01
9 7	9 7	-1.700E 00
10 7	10 7	-2.400E 00
13 7	13 7	-1.100E 00

14	7	14	7
9	8	9	8
10	8	10	8
13	6	13	8
14	8	14	2
2	13	2	13
3	13	3	13
6	13	6	13
7	13	7	13
2	14	2	14
3	14	3	14
6	14	6	14
7	14	7	14
2	16	2	16
3	16	3	16
6	16	6	16
7	16	7	16
2	18	2	18
3	18	3	18
6	18	6	18
7	18	7	18
2	19	2	19
3	19	3	19
6	19	6	19
7	19	7	19

Two HS-20 trucks apportioned as shown in Fig. 17.

```

-3.000E 00
-6.800E 00
-9.600E 00
-4.500E 00
-1.160E 01
-5.100E 00
-1.130E 01
-2.800E 00
-1.350E 01
-1.300E 00
-2.800E 00
-7.000E-01
-3.400E 00
-6.400E 00
-1.410E 01
-3.500E 00
-1.690E 01
-3.000E-01
-7.000E-01
-2.000E-01
-8.000E-01
-1.300E 00
-2.800E 00
-7.000E-01
-3.400E 00

```

TABLE 8. PROFILE OUTPUT AREAS

FROM JOINT	THRU JOINT	DEFL (1=YES)	X MOMENT (1=SLAB,2=BEAM)	Y MOMENT (1=SLAB,2=BEAM)	PRIN MOM OR STRESS (1=YES)
2 0	2 32	1	0	2	0
7 0	7 32	1	0	2	0
12 0	12 32	1	0	2	0
17 0	17 32	1	0	2	0
22 0	22 32	1	0	2	0
0 5	5 24	1	2	0	0

TABLE 9. PRINTED OUTPUT LIMITS

FROM Y STA	THRU Y STA
0	0
5	5
8	11
14	18
21	23
32	32

PROGRAM SLAB 49 - TEX HWY CEPT DECK- MATLOCK, PANAK REV DATE 29 FEB 72

I-BEAM STRUCTURE SPANS 50-60-50, 33 FT RWWY, HS20 LOG
ANALYSIS OF THD THREE-SPAN CONTINUOUS RUN 29 FEB 1972 JJP FT-KIP UNITS

PROB (CONT'D)
1004A 2 HS20 TRUCKS STAGGERED IN ADJACENT SPANS FOR MAX NEG SUPPORT MOMENT

RESULTS

SLAB X MOMENT AND X TWISTING MOMENT ACT IN THE X DIRECTION (ABOUT Y AXIS)
Y TWISTING MOMENT = -X TWISTING MOMENT, COUNTERCLOCKWISE BETA ANGLES ARE
POSITIVE FROM THE X AXIS TO THE DIRECTION OF THE LARGEST PRINCIPAL MOMENT
SLAB MOMENTS ARE PER UNIT WIDTH

BEAM MOMENTS ARE TOTAL PER BEAM

X	Y	DEFL.	SLAB X MOMENT	SLAB Y MOMENT	SLAB X TWISTING MOMENT	LARGEST PRINCIPAL SLAB MOMENT	BETA X TO LARGEST MOMENT	SUPPORT REACTION
0	32	1.537D-04	1.424E-15	2.679E-16	2.4730-02	2.473E-02	45.0	0.0
1	32	5.686D-05	6.810E-02	5.248E-09	6.363D-02	1.062E-01	30.9	0.0
2	32	2.183D-14	6.345E-C2	9.443E-03	8.094D-02	1.218E-01	35.8	-2.183D 00
3	32	-2.038D-C5	2.761E-02	-1.136E-09	8.192D-02	9.653E-02	40.3	0.0
4	32	-2.490D-05	1.672E-C2	-6.498E-10	8.845D-02	9.721E-02	42.3	0.0
5	32	-1.958D-05	7.768E-C3	-3.674E-10	8.922D-02	9.319E-02	43.8	0.0
6	32	-9.706D-06	-2.913E-04	2.662E-11	8.793D-02	-8.808E-02	-45.0	0.0
7	32	9.635D-15	-7.996E-03	-1.189E-03	8.628D-02	-9.094E-02	-43.9	-9.635D-01
8	32	5.112D-06	-6.087E-03	4.17CE-10	8.409D-02	-8.719E-02	-44.0	0.0
9	32	6.646D-06	-4.296E-03	4.349E-10	8.113D-02	-8.331E-02	-44.2	0.0
10	32	5.656D-C6	-2.588E-03	8.3335E-11	7.894D-02	-8.025E-02	-44.5	
11	32	3.145D-06	-1.079E-03	3.399E-11	7.660D			
12	32	6.442D-15	1.010E-03					

13	11	6.360×10^{-5}	$5.585E-01$	$-6.441E-01$	$6.484D-01$	$-1.251E-00$	-46.9	0.0
		0.0	0.0	0.0	0.0	0.0	0.0	0.0
12	11	$6.169D-04$	$-7.670E-01$	$-6.611E-01$	$5.474D-01$	$-1.264E-00$	-42.2	0.0
		0.0	0.0	0.0	0.0	0.0	0.0	0.0
13	11	$1.134D-03$	$-6.302E-01$	$-6.140E-01$	$4.401D-01$	$-1.062E-00$	-44.5	0.0
		0.0	0.0	0.0	0.0	0.0	0.0	0.0
14	11	$1.493D-03$	$-5.222E-01$	$-5.552E-01$	$3.340D-01$	$-8.731E-01$	-46.4	0.0
		0.0	0.0	0.0	0.0	0.0	0.0	0.0
15	11	$1.723D-03$	$-4.411E-01$	$-4.861E-01$	$2.369D-01$	$-7.016E-01$	-47.7	0.0
		0.0	0.0	0.0	0.0	0.0	0.0	0.0
16	11	$1.864D-03$	$-3.794E-01$	$-4.101E-01$	$1.536D-01$	$-5.491E-01$	-47.9	0.0
		0.0	0.0	0.0	0.0	0.0	0.0	0.0
17	11	$1.873D-03$	$-3.251E-01$	$-3.314E-01$	$8.725D-02$	$-4.156E-01$	-46.0	0.0
		0.0	0.0	0.0	0.0	0.0	0.0	0.0

18 11 1.820D-03 -2.335E-01 -2.523E-01 3.570D-02 -2.800E-01 -52.2 0.0
 0.0 0.0
 19 11 1.710D-03 -1.382E-01 -1.750E-01 -4.144D-03 -1.754E-01 83.6 0.0
 0.0 0.0
 20 11 1.567D-03 -3.875E-02 -9.934E-02 -3.253D-02 -1.135E-01 66.5 0.0
 0.0 0.0
 21 11 1.417D-03 6.563E-02 -2.579E-02 -5.062D-02 8.812E-02 -24.0 0.0
 0.0 0.0
 22 11 1.287D-03 1.770E-01 4.515E-02 -6.029E-02 2.004E-01 -21.2 0.0
 0.0 7.943E-00
 23 11 1.207D-03 8.554E-02 8.162E-02 -6.385D-02 1.475E-01 -44.1 0.0
 0.0 0.0
 24 11 1.149D-03 -2.110E-05 5.852E-02 -3.165D-02 7.236E-02 -66.4 0.0
 0.0 0.0
 0 10 2.912D-04 1.290E-08 -5.384E-01 3.453D-01 -7.071E-01 -64.0 0.0
 0.0 0.0
 1 10 1.246D-04 2.923E-02 -7.577E-01 6.889D-01 -1.158E 00 -59.9 0.0
 0.0 0.0
 2 10 -2.966D-13 2.919E-03 -8.127E-01 7.142D-01 -1.227E 00 -59.9 2.966D 01
 0.0 -1.394E 02
 3 10 -8.791D-05 5.507E-02 -8.488E-01 7.530D-01 -1.275E 00 -60.5 0.0
 0.0 0.0
 4 10 -1.222D-04 4.506E-02 -9.007E-01 7.890D-01 -1.348E 00 -60.5 0.0
 0.0 0.0
 5 10 -1.036D-04 -2.529E-02 -9.711E-01 8.218D-01 -1.446E 00 -60.0 0.0
 0.0 0.0
 6 10 -4.961D-05 -1.735E-01 -1.057E 00 8.486D-01 -1.572E 00 -58.8 0.0
 0.0 0.0
 7 10 -4.259D-13 -4.409E-01 -1.149E 00 8.654D-01 -1.730E 00 -56.1 4.259D 01
 0.0 -1.856E 02
 8 10 -2.931C-05 -1.825E-02 -1.110E 00 8.518D-01 -1.616E 00 -59.3 0.0
 0.0 0.0
 9 10 -6.331D-05 -5.804E-02 -1.070E 00 7.910D-01 -1.503E 00 -61.3 0.0
 0.0 0.0
 10 10 -6.723D-05 -4.010E-02 -1.031E 00 6.899D-01 -1.385E 00 -62.8 0.0
 0.0 0.0
 11 10 -3.751D-05 -1.229E-01 -9.957E-01 5.592D-01 -1.269E 00 -64.0 0.0
 0.0 0.0
 12 10 -3.939D-13 -3.199E-01 -9.644E-01 4.002D-01 -1.156E 00 -64.4 3.939D 01
 0.0 -1.571E 02
 13 10 -1.399D-05 -1.358E-01 -8.417E-01 2.358D-01 -9.132E-01 -73.1 0.0
 0.0 0.0
 14 10 -3.078D-05 -5.246E-02 -7.204E-01 9.145D-02 -7.327E-01 -82.3 0.0
 0.0 0.0
 15 10 -3.123D-05 -3.762E-02 -6.023E-01 -2.927D-02 -6.038E-01 87.0 0.0
 0.0 0.0
 16 10 -1.619C-05 -6.973E-02 -4.910E-01 -1.194D-01 -5.224E-01 75.2 0.0
 0.0 0.0
 17 10 -1.715D-13 -1.362E-01 -3.895E-01 -1.7800-01 -4.813E-01 62.7 1.7150 01
 0.0 -6.329E 01
 18 10 -6.649D-06 -6.991E-02 -2.724E-01 -2.129D-01 -4.070E-01 57.7 0.0
 0.0 0.0
 19 10 -2.183D-05 -1.560E-02 -1.675E-01 -2.330D-01 -3.366E-01 54.0 0.0
 0.0 0.0
 20 10 -3.422D-05 3.890E-02 -7.395E-02 -2.409D-01 -2.649E-01 51.6 0.0
 0.0 0.0
 21 10 -3.191D-05 1.022E-01 1.603E-02 -2.377D-01 2.983E-01 -39.5 0.0
 0.0 0.0
 22 10 3.709D-14 1.829E-01 8.561E-02 -2.234D-01 3.629E-01 -38.9 -3.7090 00
 0.0 2.485E 01
 23 10 8.188D-05 6.426E-02 1.210E-01 -2.096D-01 3.129E-01 -47.5 0.0
 0.0 0.0
 24 10 1.832D-04 -2.960E-05 7.716E-02 -1.028D-01 1.484E-01 -55.3 0.0
 0.0 0.0
 0 9 7.278D-03 9.943E-09 -5.240E-01 3.589D-01 -7.064E-01 -63.1 0.0
 0.0 0.0
 1 9 6.236D-03 -4.083E-02 -6.840E-01 7.2810-01 -1.158E 00 -56.9 0.0
 0.0 0.0
 2 9 5.212D-03 -5.383E-02 -6.659E-01 7.512D-01 -1.186E 00 -55.5 0.0
 3.026E-02 -1.125E 02
 3 9 4.190D-03 -1.007E-01 -6.475E-01 7.839D-01 -1.204E 00 -54.6 0.0
 -3.289E-02 0.0

4 9 3.167D-03 -1.076E-01 -6.190E-01 8.238D-01 -1.226E 00 -53.6 0.0
 -1.143E-01 0.0
 5 9 2.140D-03 -1.142E-01 -5.858E-01 8.664D-01 -1.248E 00 -52.0 0.0
 -2.396E-01 0.0
 6 9 1.104D-03 -1.135E-01 -5.557E-01 9.1010-01 -1.271E 00 -51.8 0.0
 -2.741E-01 0.0
 7 9 6.049D-05 -5.178E-02 -5.494E-01 9.548D-01 -1.302E 00 -51.7 0.0
 -8.526E-02 -9.185E 01
 8 9 -9.863D-04 1.821E-01 -5.792E-01 9.234D-01 -1.197E 00 -56.2 0.0
 2.448E 00 0.0
 9 9 -1.954D-03 4.301E-01 -5.885E-01 7.208D-01 -9.618E-01 4.718E 00 0.0
 10 9 -2.769D-03 5.800E-01 -4.839E-01 5.94CE CC 0.0
 6.346E-01 0.0
 11 9 -3.393D-03 6.346E-01 0.0
 12 9 -3.0

BEAM X MOMENT BETWEEN (0 , 5) AND (24 , 5)

X	Y	BEAM X MOM
0	5	0.0
1	5	0.0
2	5	-6.543E-01
3	5	-4.245E-01
4	5	9.912E-02
5	5	6.023E-01
6	5	9.905E-01
7	5	1.130E 00
8	5	1.057E 01
9	5	2.030E 01
10	5	2.321E 01
11	5	1.611E 01
12	5	5.041E 00
13	5	1.825E 01
14	5	2.260E 01
15	5	1.642E 01
16	5	6.831E 00
17	5	-6.216E-01
18	5	-4.260E-01
19	5	-4.138E-01
20	5	-4.917E-01
21	5	-5.933E-01
22	5	-4.868E-01
23	5	0.0
24	5	0.0

***** TIME SINCE BEGINNING OF EXECUTION IS 491.96 SECONDS ,

TIME SINCE LAST CALL OF PRTIME IS 140.69 SECONDS *****

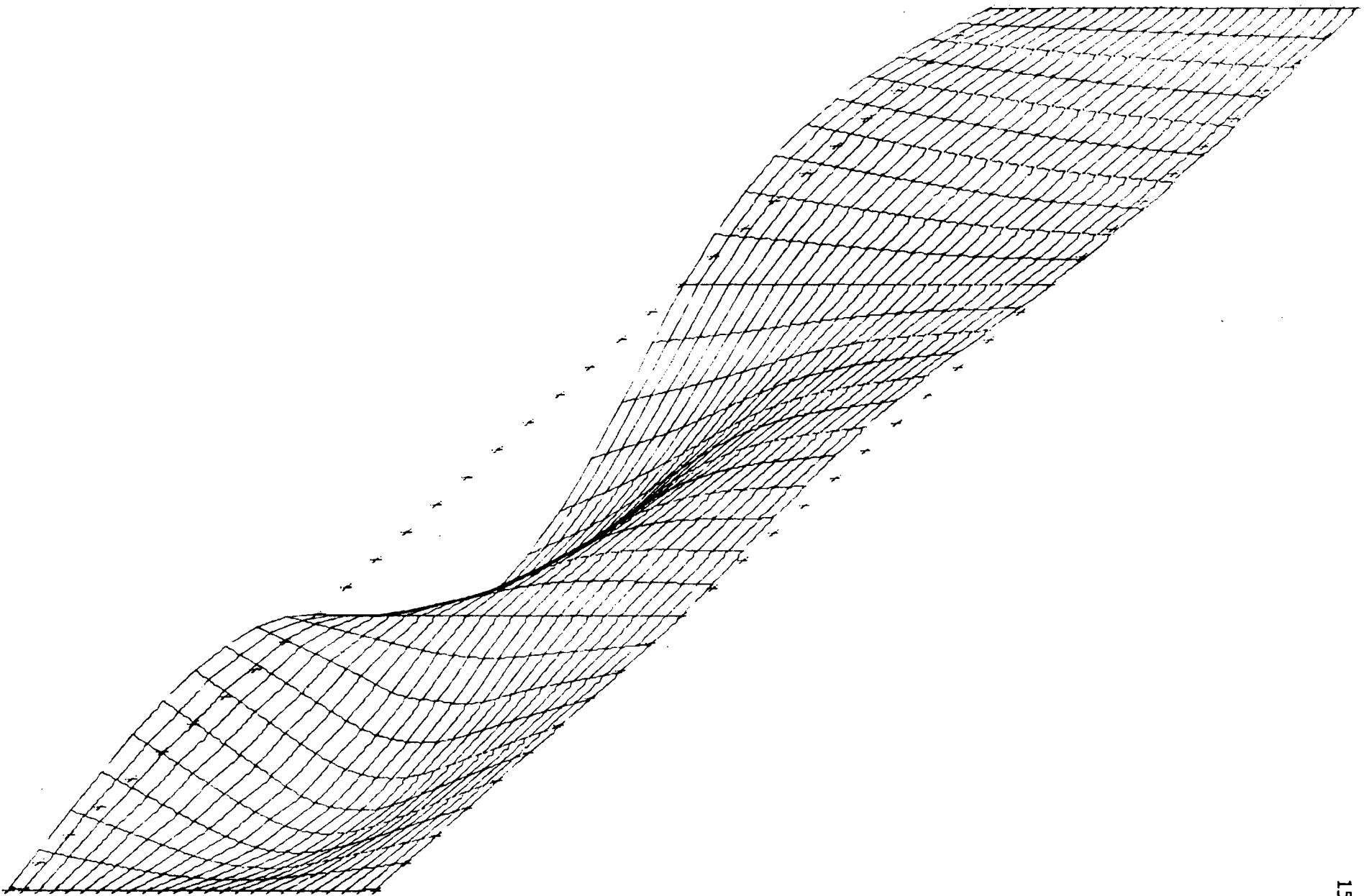
PROGRAM SLAB 49 -TEX HWY DEPT DECK- MATLOCK,PANAK REV DATE 29 FEB 72

I-BEAM STRUCTURE SPAANS 50-60-50, 33 FT RDWY, HS20 LOG ANALYSIS OF THD THREE-SPAN CONTINUOUS RUN 29 FEB 1972 JJP FT-KIP UNITS

***** TIME SINCE BEGINNING OF EXECUTION IS 492.61 SECONDS ,

TIME SINCE LAST CALL OF PRTIME IS 0.65 SECONDS *****

KEEP RUN TIME RECORDS FOR FUTURE ESTIMATES OF PARENT AND OFFSPRING RUN TIMES



10049

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THE AUTHORS

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