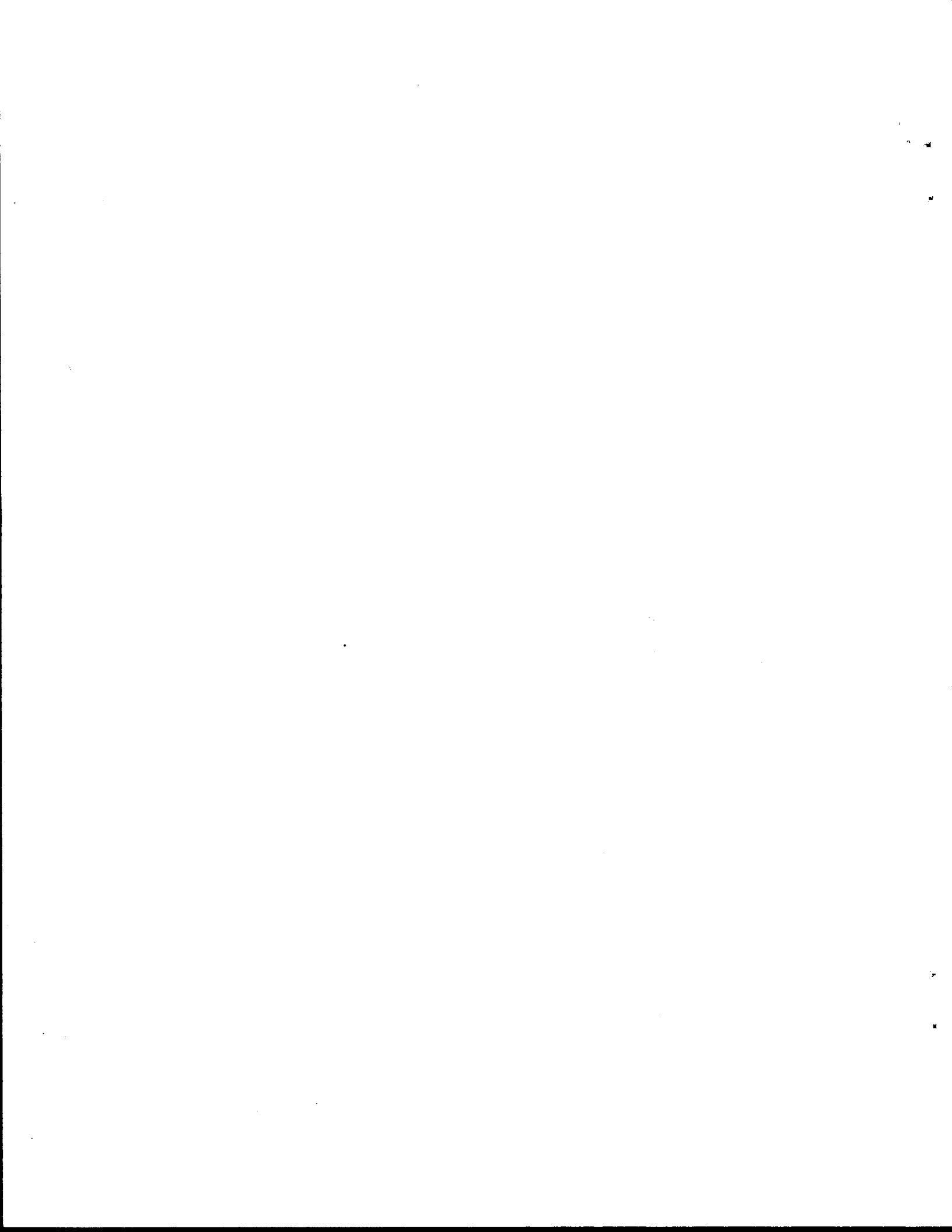


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Estimating Flexible Pavement Maintenance and Rehabilitation Fund
Requirements for a Transportation Network

by

A. Stein

T. Scullion

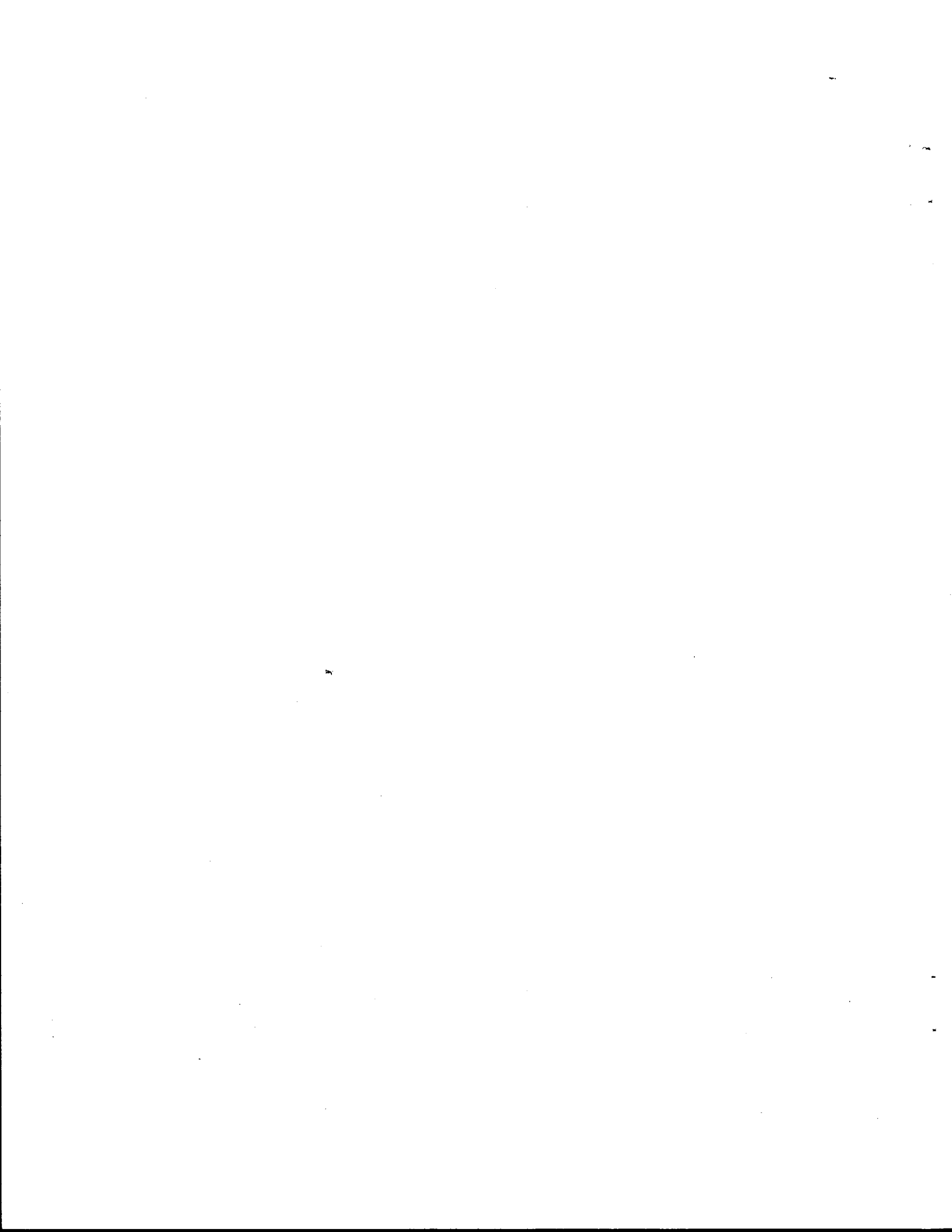
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February 1988

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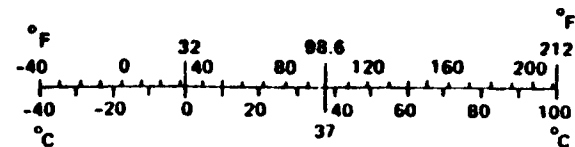
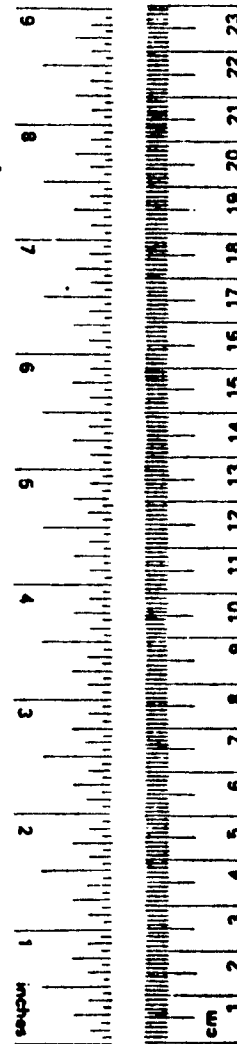
METRIC CONVERSION FACTORS

Approximate Conversions to Metric Measures

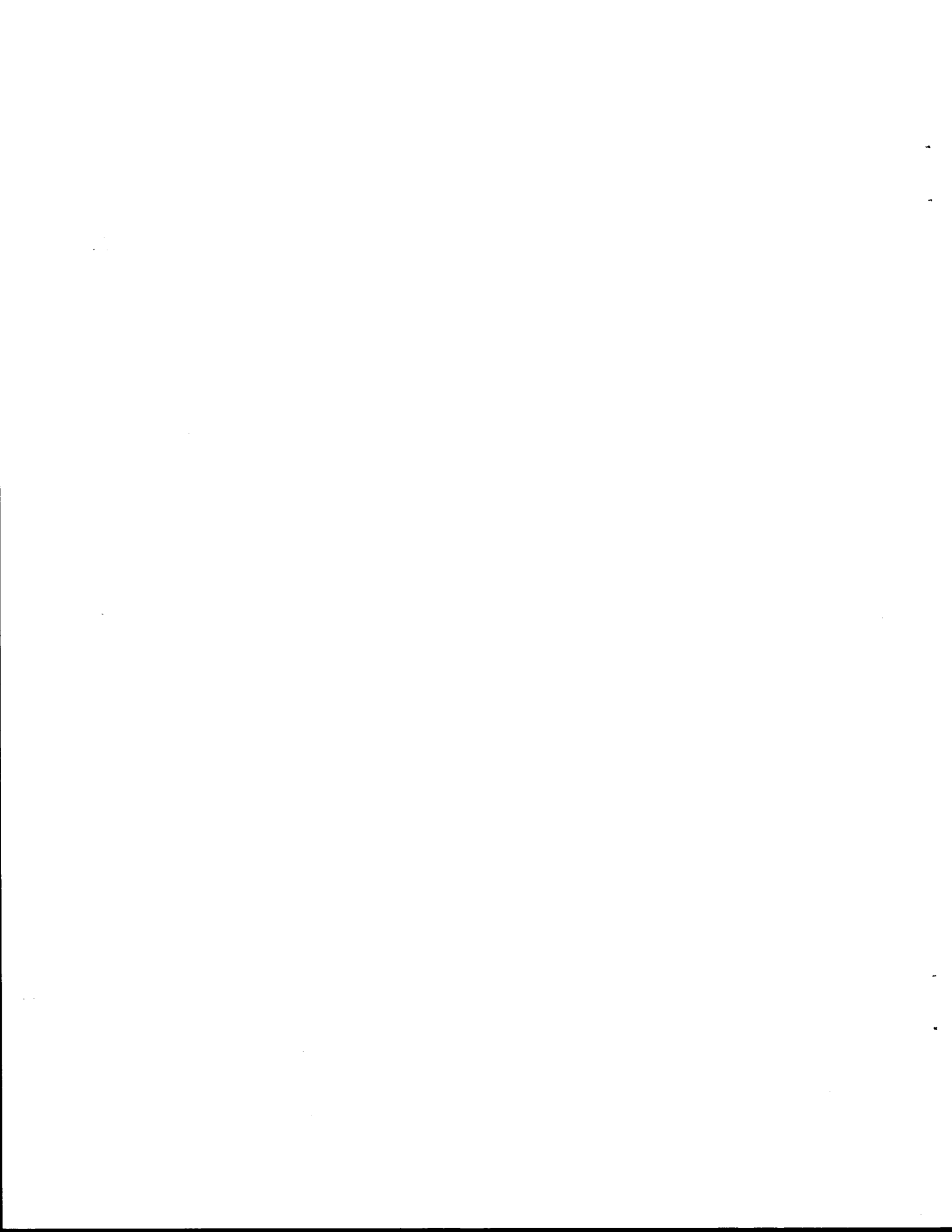
Symbol	When You Know	Multiply by	To Find	Symbol
LENGTH				
in	inches	*2.5	centimeters	cm
ft	feet	30	centimeters	cm
yd	yards	0.9	meters	m
mi	miles	1.6	kilometers	km
AREA				
in ²	square inches	6.5	square centimeters	cm ²
ft ²	square feet	0.09	square meters	m ²
yd ²	square yards	0.8	square meters	m ²
mi ²	square miles	2.6	square kilometers	km ²
	acres	0.4	hectares	ha
MASS (weight)				
oz	ounces	28	grams	g
lb	pounds	0.45	kilograms	kg
	short tons (2000 lb)	0.9	tonnes	t
VOLUME				
tsp	teaspoons	5	milliliters	ml
Tbsp	tablespoons	15	milliliters	ml
fl oz	fluid ounces	30	milliliters	ml
c	cups	0.24	liters	l
pt	pints	0.47	liters	l
qt	quarts	0.95	liters	l
gal	gallons	3.8	liters	l
ft ³	cubic feet	0.03	cubic meters	m ³
yd ³	cubic yards	0.76	cubic meters	m ³
TEMPERATURE (exact)				
°F	Fahrenheit temperature	5/9 (after subtracting 32)	Celsius temperature	°C

Approximate Conversions from Metric Measures

Symbol	When You Know	Multiply by	To Find	Symbol
LENGTH				
mm	millimeters	0.04	inches	in
cm	centimeters	0.4	inches	in
m	meters	3.3	feet	ft
m	meters	1.1	yards	yd
km	kilometers	0.6	miles	mi
AREA				
cm ²	square centimeters	0.16	square inches	in ²
m ²	square meters	1.2	square yards	yd ²
km ²	square kilometers	0.4	square miles	mi ²
ha	hectares (10,000 m ²)	2.5	acres	
MASS (weight)				
g	grams	0.035	ounces	oz
kg	kilograms	2.2	pounds	lb
t	tonnes (1000 kg)	1.1	short tons	
VOLUME				
ml	milliliters	0.03	fluid ounces	fl oz
l	liters	2.1	pints	pt
l	liters	1.06	quarts	qt
l	liters	0.26	gallons	gal
m ³	cubic meters	35	cubic feet	ft ³
m ³	cubic meters	1.3	cubic yards	yd ³
TEMPERATURE (exact)				
°C	Celsius temperature	9/5 (then add 32)	Fahrenheit temperature	°F



* 1 in = 2.54 (exactly). For other exact conversions and more detailed tables, see NBS Misc. Publ. 286, Units of Weights and Measures, Price \$2.25, SD Catalog No. C13.10.286.



ABSTRACT

In the early 1980's the Texas State Department of Highways and Public Transportation implemented its Pavement Evaluation System. This system was designed to (a) document trends in network condition and (b) generate a one year estimate of rehabilitation funding. The information generated by this system has been used for many purposes including funding request, project prioritation and documenting the consequences of changes in funding levels.

However a limitation of this system was its inability to project future conditions and make multi-year needs estimates. This is the subject of this research report. Regression equations were built for each major distress type from a pavement data base containing a 10 year history of condition trends from over 350 random sections in Texas. These equations were used to age individual sections which did not qualify for maintenance or rehabilitation in a particular year. A simple decision tree was developed to estimate the maintenance requirements if rehabilitation is not warranted. This decision tree represents the opinions of experienced maintenance engineers. A case study and sensitivity analysis are presented.

DISCLAIMER

This report is not intended to constitute a standard, specification or regulation, and does not necessarily represent the views or policy of the FHWA or Texas State Department of Highways and Public Transportation.

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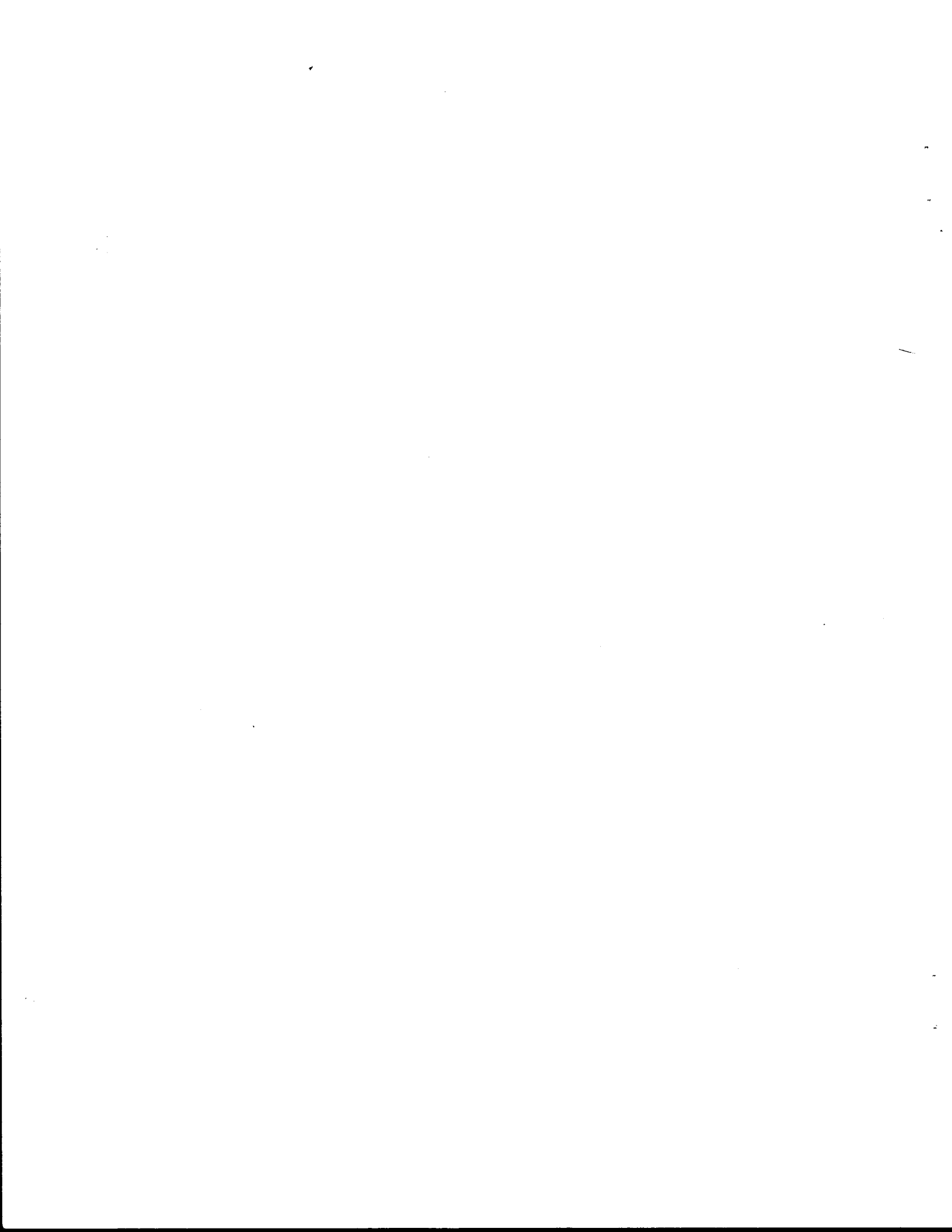
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CHAPTER ONE

INTRODUCTION

To assist with the management of its 70,000 mile pavement network, the Texas State Department of Highways and Public Transportation (SDHPT) has been active in the development of pavement management systems since the early 1970's. The major constraint encountered during the evaluation process was the limited availability of funding for construction and maintenance which created the necessity to develop procedures capable of distributing the available funds in the most optimal way.

The state of Texas is divided into twenty-four districts for the purpose of maintenance and rehabilitation of the highway network (Figure 1). A list of the counties and their districts is given in Table 1. Pavement inspection procedures and systems were developed for individual districts at the operational level(1). Very little analysis or summarization was performed. Although the initial results of this work appeared promising, in the late 1970's the SDHPT focused its attention on the need for information at the district and state network levels.

Subsequently the initial work was incorporated into a set of decision-making tools known as the Rehabilitation and Maintenance System (RAMS). This system was designed by the Texas Transportation Institute (TTI) at Texas A&M University for the purpose of providing the SDHPT's central office and individual districts the allocation models to ensure an efficient distribution of funds(2,3,4,5,6).

Implementation of these models through the various state districts has proceeded since they were completed in 1980. The first program within the RAMS series, the State Cost Estimating Program, was implemented within the Department's Flexible Pavement Evaluation System (PES) in 1981 and was intended to:

- a) calculate current pavement scores,
- b) calculate an appropriate rehabilitation funding strategy for those sections below a minimum score, and
- c) calculate a reinspection date for those sections above a minimum score.

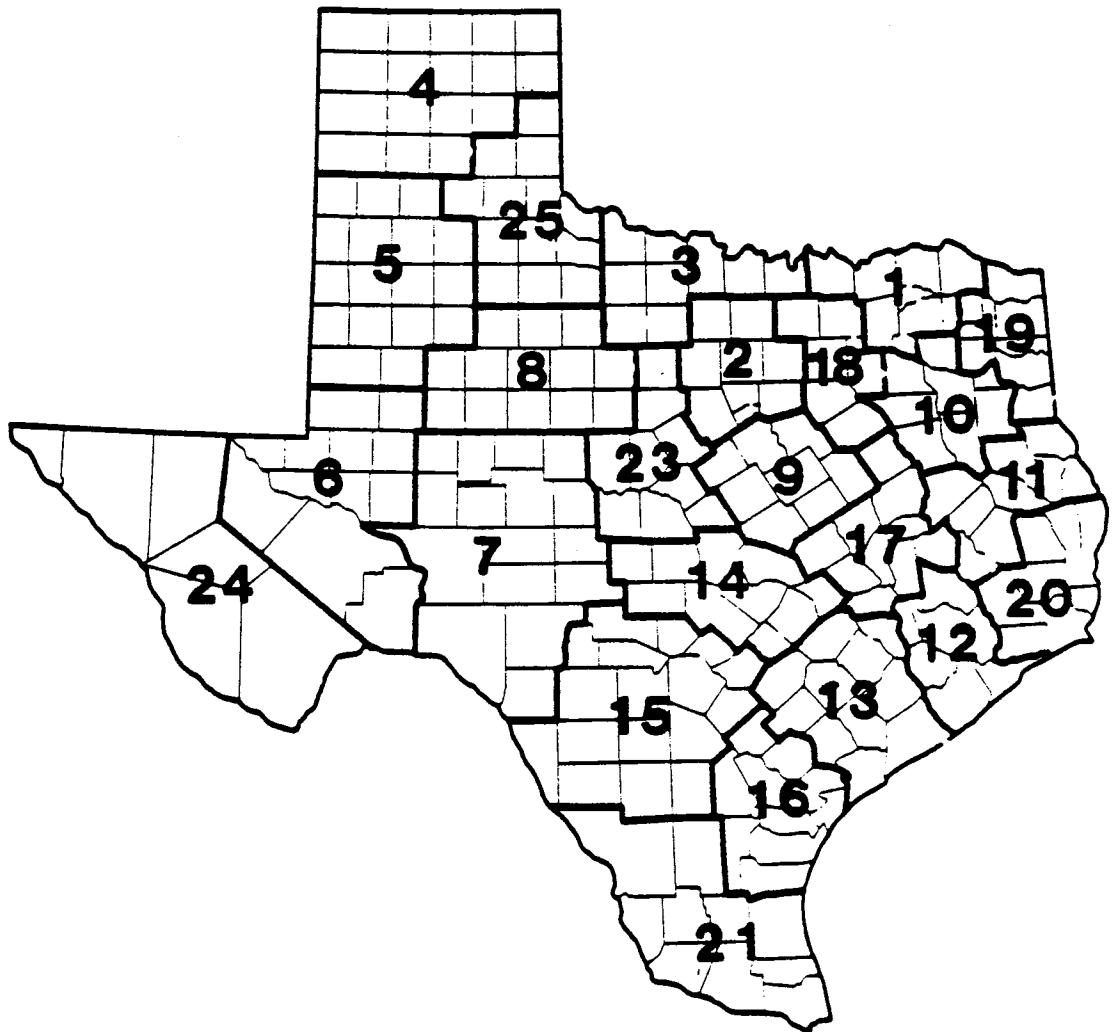


Figure 1. SDHPT District Outline

TABLE 1. County Numbering System

TEXAS COUNTIES
STATE DEPARTMENT OF HIGHWAYS AND PUBLIC TRANSPORTATION

CO. NO.	COUNTY NAME	DIST. NO.	CO. NO.	COUNTY NAME	DIST. NO.	CO. NO.	COUNTY NAME	DIST. NO.	CO. NO.	COUNTY NAME	DIST. NO.
1	ANDERSON	10	65	DONLEY	25	129	KARNES	10	192	REAGAN	7
2	ANDREWS	6	66	KENEDY	21	130	KALFMAN	10	193	REAL	7
3	ANGELINA	11	67	DUVAL	21	131	KENDALL	16	194	RED RIVER	1
4	ARANSAS	10	68	EASTLAND	23	68	KENEDY	21	195	REEVES	6
5	ARCHER	3	69	ECTOR	6	132	KENT	6	196	REFUGIO	16
6	ARMSTRONG	4	70	EDWARDS	7	133	KERR	16	197	ROBERTS	4
7	ATASCOSA	16	71	ELLIS	10	134	KIMBLE	7	198	ROBERTSON	17
8	AUSTIN	13	72	EL PASO	24	135	KING	25	199	ROCKWALL	10
9	BALNEY	6	73	BRATH	2	136	KINNEY	7	200	RUNNELS	7
10	BANDERA	16	74	FALLS	9	137	KLEBERG	10	201	RUSK	10
11	BASTROP	14	75	FANNIN	1	138	KNOX	25	202	SABINE	11
12	BAYLOR	3	76	FAYETTE	13	139	LAMAR	1	203	SAN AUGUSTINE	11
13	BEEL	10	77	FISHER	6	140	LAMB	6	204	SAN JACINTO	11
14	BELL	9	78	FLOYD	6	141	LAMPASAS	23	205	SAN PATRICK	16
15	BELAR	10	79	FOARD	25	142	LA BALLE	15	206	SAN SABA	23
16	BLANCO	14	80	FORT BEND	12	143	LAVACA	13	207	SCHLEICHER	7
17	BORDEN	8	81	FRANKLIN	1	144	LEE	14	208	SCURRY	6
18	BOSSQUE	6	82	FRESSTONE	17	145	LEON	17	209	SHACKELFORD	6
19	BOWNE	10	83	FRO	10	146	LIBERTY	20	210	SHELBY	11
20	BRAZORIA	12	84	GAINES	6	147	LIBERTONE	9	211	SHERMAN	4
21	BRAZOS	17	85	GALVESTON	12	148	LIPSCOMB	4	212	SMITH	10
22	BREWSTER	24	86	GARZA	6	149	LIVE OAK	16	213	SOMERVELL	2
23	BROCK	25	87	GALLIPEE	14	150	LLANO	14	214	STARR	21
24	BROOKS	21	88	GLASSCOCK	7	151	LOVING	6	215	STEPHENS	23
25	BROWN	23	89	GOLIAD	16	152	LUBBOCK	5	216	STERLING	7
26	BURLESON	17	90	GONZALES	13	153	LYNN	6	217	STONEWALL	8
27	BURNET	14	91	GRAY	4	154	MADISON	17	218	BUTTON	7
28	CALDWELL	14	92	GRAYSON	1	155	MARION	10	219	SWISHER	5
29	CALHOUN	13	93	GREGG	10	156	MARTIN	6	220	TARRANT	2
30	CALLAHAN	8	94	GRIMES	17	157	MASON	14	221	TAYLOR	6
31	CAMERON	21	95	GUADALUPE	15	158	MATAGORDA	13	222	TERRELL	6
32	CAMP	10	96	HALL	5	159	MAVERICK	5	223	TERRY	5
33	CARSON	4	97	HALL	25	160	MC CULLOCH	23	224	THROCKMORTON	3
34	CASS	10	98	HAMILTON	9	161	MC LENNAN	9	225	TITUS	10
35	CASTRO	5	99	HANSFORD	4	162	MC MULLEN	5	226	TOM GREEN	7
36	CHAMBERS	20	100	HARDEMAN	25	163	MEDINA	5	227	TRAVIS	14
37	CHEROKEE	10	101	HARDIN	20	164	MENARD	7	228	TRINITY	11
38	CHILDRESS	25	102	HARRIS	12	165	MIDLAND	6	229	TYLER	20
39	CLAY	3	103	HARRISON	10	166	MILAM	17	230	UPSHUR	10
40	COCHRAN	5	104	HARTLEY	4	167	MILLS	23	231	UPTON	6
41	COLE	7	105	HASKELL	6	168	MITCHELL	6	232	UYALDE	15
42	COLEMAN	23	106	HAYS	14	169	MONTAGUE	3	233	VAL VERDE	7
43	COLLIN	10	107	HENPHILL	4	170	MONTGOMERY	12	234	VAN ZANDT	10
44	COLLINGSWORTH	25	108	HENDERSON	10	171	MOORE	4	235	VICTORIA	13
45	COLORADO	13	109	INDALGO	21	172	MORRIS	10	236	WALKER	17
46	COMAL	15	110	HILL	9	173	MOTLEY	25	237	WALLER	12
47	COMANCHE	23	111	HOCKLEY	5	174	NACOGDOCHES	11	238	WARD	6
48	CONCHO	7	112	HOOD	2	175	NAVARRO	10	239	WASHINGTON	17
49	COOKE	3	113	HOPKINS	1	176	NEWTON	20	240	WEBB	21
50	CORYELL	6	114	HOUSTON	11	177	NOLAN	6	241	WHARTON	13
51	COTTE	20	115	HOWARD	9	178	NUCES	16	242	WHEELER	25
52	CRANE	6	116	HUDSPETH	24	179	OCHILTREE	4	243	WICHTA	3
53	CROCKETT	7	117	HUNT	1	180	OLDHAM	4	244	WILBARGER	3
54	CROSBY	6	118	HUTCHINSON	4	181	ORANGE	20	245	WILLACY	21
55	CULBERSON	24	119	IRION	7	182	PALO PINTO	2	246	WILLIAMSON	14
56	DALLAM	4	120	JACK	2	183	PANOLA	10	247	WILSON	15
57	DALLAS	10	121	JACKSON	13	184	PARKER	2	248	WINKLER	6
58	DAWSON	6	122	JASPER	20	185	PARKER	6	249	WISE	2
59	DEAF SMITH	4	123	JEFF DAVIS	24	186	PECOS	6	250	WOOD	10
60	DELTA	1	124	JEFFERSON	20	187	POLK	11	251	YOAKUM	5
61	DENTON	10	125	JIM HOGG	21	188	POTTER	4	252	YOUNG	3
62	DE WITT	13	126	JIM WELLS	10	189	PRESIDIO	24	253	ZAPATA	21
63	DICKENS	25	127	JOHNSON	2	190	RANS	1	254	ZAVALA	15
64	DRYMT	15	128	JONES	6	191	RANDALL	4			

A sound analytical program is needed for the future that will assist in predicting preventive maintenance strategies, using planning models for identifying network maintenance and rehabilitation costs over a planning horizon taking into consideration the current condition of pavement, user safety, and user comfort. It is the purpose of this research effort to design, test and validate a computerized model to provide decision makers with sufficient quantitative information to recommend appropriate courses of action regarding state highway maintenance and rehabilitation strategies.

Specifically, the objectives of this research are:

1. Develop a Fortran-based mainframe computer program to calculate accurate state-wide cost estimates, visual condition schedules, and routine maintenance costs using the annual statewide survey.
2. Use a systematic sampling method whereby sufficient data are collected on a random sampling basis to provide accurate information for funding justification purposes.
3. Develop an input format and examine typical problems using actual data from a state-wide survey from a selected district.
4. Run a case study for a complete district to show whether the results found in the typical sections of road still hold for a full district.
5. Perform limited sensitivity analyses to observe how the minimum acceptable utility, traffic levels, and climatic variations affect the estimated funding requirements.

Chapter Two reviews the Pavement Evaluation System currently used by the State of Texas. It also contains an explanation of the data gathering techniques used by the SDHPT. Chapter Three describes the technique utilized to predict pavement conditions. The fourth chapter explains the proposed system to predict pavement maintenance and rehabilitation needs and describes its different parts. Chapter Five examines typical problems worked with the computer program using actual data from a state-wide survey. Chapter Six contains the results of the case study for a full district (District 11-Lufkin), and the results of the limited sensitivity analysis. The last chapter contains the conclusions and recommendations found in this research.

In Appendix A the regression equations developed for distress types and PSI on each of 4 pavement types are presented. These equations are used to generate deterioration rates for each individual section. In Appendix B the decision rules for applying maintenance treatments are given. In Appendix C the computer code used to make cost estimates is given.

CHAPTER TWO

RELATED BACKGROUND

Condition Rating in Texas

An efficient use of highway funding makes it necessary to develop a complete and efficient method for condition rating. The purpose of a condition rating is to give current information concerning the roughness, structural capacity, safety, and visual distress of a section of road, to be used in a number of activities that can be summarized as follows:

1. Development of a structural rating,
2. Aid in projections of budget requirement,
3. Aid in maintenance and rehabilitation decisions, and
4. Input the relevant pavement performance history.

The roughness of a road can be expressed in terms of what is known as "Serviceability Index" (SI) obtained with the Mays Ride Meter, and it is based on a scale which ranges from 0 to 5. A score of 5 represents a smooth road, and a score of 0 represents a road that is impossible to use. The Mays Ride Meter is a car-mounted device that measures the relative movement between the rear axle and the mass of the car when the car is traveling at 50 miles per hour. This raw value is transformed to the 0 to 5 scale by using a relationship between roughness and Serviceability Index. The structural capacity evaluation is obtained through the use of the Dynaflect (8) or Falling Weight Deflectometer (FWD). The Dynaflect is the most commonly used non-destructive test device in the United States. This machine is mounted on a two-wheel trailer and produces a dynamic force of 1000 lbs at a frequency of 8 cycles per second. The resulting deflections are measured by 5 sensors, each 1 foot apart, with the first one directly between the wheels. The FWD is a new non-destructive testing device capable of applying loads similar to those applied by truck traffic.

Safety on pavements is mainly analyzed in terms of skid resistance. Most skid related accidents occur under wet or icy conditions. For that reason, most skid-resistance tests are conducted on wet pavements. The skid number is the standard factor for measuring skid resistance. Skid data were collected in the initial implementation efforts of the Pavement Evaluation System (PES) of 1978 to 1980. However, it has not been collected for PES in recent years because:

- 1) It was costly to collect at the network level.

- 2) The skid values were having an overriding effect on the pavement score calculation.
- 3) Skid numbers are related to pavement safety, whereas distress and Mays ride are related to pavement's structural condition. A separated system for safety would be more appropriate.
- 4) Skid number itself is not a good predictor of accident potential. Work in Texas is currently underway to improve the Wet Weather Safety Index (9), which has been shown to be a much better indicator of accident potential.

The techniques and instruments described before do not, in general, supply all of the necessary information about the section of road under analysis. Thus, a visual survey of the pavement surface is necessary to determine its level of distress (10). The types of distress rated prior to 1984 were: rutting, raveling, flushing, failures, alligator cracking, longitudinal cracking, and transverse cracking. After 1984 raveling and flushing were dropped and replaced with block cracking and patching. The information is recorded in rating forms (Figure 2) and then transferred into a central data bank where it can be used for different purposes. A brief description of each distress type is given below;

Rutting: a surface depression in the wheel paths. It is caused by consolidation or lateral movement of the materials due to traffic loads.

Raveling: wearing away of the pavement surface caused by the dislodging of aggregate particles and loss of asphalt binder. (Prior to 1984)

Flushing: loss of surface texture due to an excess of asphalt in the pavement surface. (Prior to 1984)

Failures: surface eroded or badly cracked or depressed.

Alligator Cracking: interconnected cracks forming a series of small blocks resembling an alligator's skin or chicken wire.

Longitudinal Cracking: cracks parallel to the pavement centerline.

Transverse Cracking: cracks at right angles to the centerline.

Block Cracking: interconnecting cracks forming blocks ranging in size from 1' x 1' upto 10' x 10'. (after 1984)

Patching: repairs made to pavement distresses. (after 1984)

FLEXIBLE PAVEMENT EVALUATION

6-84

SYSTEM - ID <table border="1" style="display: inline-table; border-collapse: collapse; vertical-align: middle;"> <tr><td>M</td><td>M</td><td>S</td></tr> <tr><td>1</td><td>2</td><td>3</td></tr> </table> <table border="1" style="display: inline-table; border-collapse: collapse; vertical-align: middle;"> <tr><td>E</td><td>I</td></tr> <tr><td>4</td><td>5</td></tr> </table> DISTRICT NO. <table border="1" style="display: inline-table; border-collapse: collapse; vertical-align: middle;"> <tr><td>6</td><td>7</td></tr> </table>															M	M	S	1	2	3	E	I	4	5	6	7	PAVEMENT CONDITIONS							COMMENT CODE	SPEED LIMIT	PAVEMENT TYPE	COMMENTS / NOTES																																										
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Figure 2. Maintenance Rating Form for Flexible Pavements

Pavement scores are calculated by converting the pavement distress data into Utility Values.

Utility values are obtained using the formula

$$U = 1 - a \exp(-b/x) \quad (1)$$

a and b = least square estimates of the regression coefficients
 x = % distress from rating
 U = the visual score given x (range 0 to 1.0)

After U is found, an overall visual utility score (AVUC) is calculated with the formula

$$AVUC = (U_1^{b_1}) (U_2^{b_2}) \dots (U_n^{b_n}) \quad (2)$$

where,

$$b_i = \text{Climatic weighting factors, } i = 1 \text{ to } 7.$$

The original pavement score was defined as a combination of Serviceability Index (riding quality), safety, Maintenance Cost and Visual Utility are combined into a single utility score, between 0-100, that is used as an indicator of the overall condition of the pavement section.

$$\begin{aligned} \text{Pavement Current Score} = & [(\text{Visual Utility})^{a_1} \times \\ & (\text{Riding Quality})^{a_2} \times \\ & [\text{Maintenance Cost}]^{a_3} \\ & [\text{Safety Index}]^{a_4} \end{aligned} \quad (3)$$

where,

$$a_1, a_2, a_3, a_4 = \text{Weighting factors.}$$

On the basis of that overall score and the individual visual distresses, a maintenance strategy or a rehabilitation strategy is selected. Chapter Four gives a more extensive description of the pavement score determination.

Annual Statewide Survey

The necessary information for each section of road analyzed was obtained from the Texas Annual Statewide Survey. In 1982, all roads in every District in Texas were divided into segments of approximately two miles in length. A segment was considered as all pavement areas between two predetermined mileposts. In three of the 25 Districts (8, Abilene; 11, Lufkin; and 15, San Antonio), a 100 percent sample in each roadway system (Interstate, State, U.S., and Farm-to-Market) was taken. In the remaining 21 Districts, five percent of the total number of segments were randomly selected for sampling. Figure 3

shows the location of each District and its percent surveyed. For each section, the visual distresses and Serviceability Index were measured and the visual and riding quality utilities were computed. A value of 1 was given to the safety index and the Maintenance Cost as these items were not available in the initial implementation. Thus, the overall pavement evaluation score could be determined.

In 1983, the SDHPT conducted a more extensive survey of the roads in Texas. One hundred (100) percent of the Interstate roadway system, fifty (50) percent of the U.S. and State roadway systems, and twenty (20) percent of the Farm-to-Market roadway system were surveyed, giving an average of thirty seven (37) percent of the total roadway network. Utility scores, Serviceability Index, and overall pavement evaluation scores were calculated for each section. In recent years the PES has been expanded to include rigid as well as flexible pavements. For the last 3 years the following sampling scheme has been used: evaluate 100% Interstate, 50% US and SH highway and a random 20% sample of all other highways.

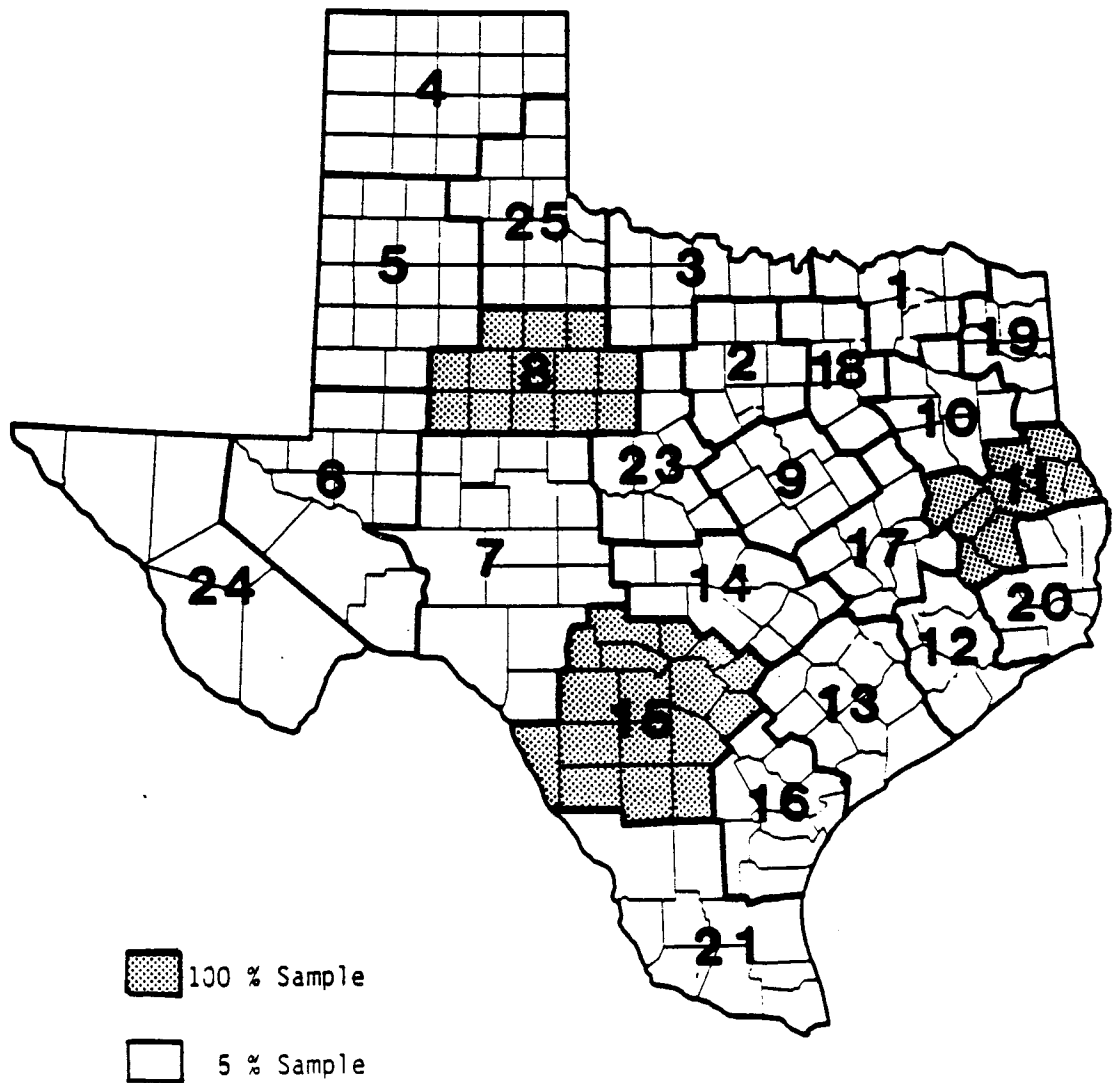


Figure 3. 1983 Statewide Survey

CHAPTER THREE

PERFORMANCE EQUATIONS FOR PREDICTING PAVEMENT CONDITION

In order to be able to predict the pavement performance in terms of Serviceability Index and distress, equations that reflect the functional performance curve of the pavement were selected.

In the American Association of State Highway and Transportation Officials (AASHTO) Road Test, which was conducted in 1958-1960, the performance function was assumed to be of the form

$$g = \left(\frac{W}{\rho} \right)^{\beta}$$

where

- g = the damage function (normalized variable that ranges from 0 to 1)
- W = time, 18 kip-ESAL, or climatic cycles (depending on the type of distress-e.g. alligator cracking-load; transversal cracking-climatic cycles) necessary to reach a level of g. At the AASHTO Road Test 18-kip ESAL were used primarily
- ρ = quantity of normalized 18-kip ESAL, time, or climatic cycles until g reaches a value of 1. It is assumed to be a function of structural variables.
- β = power that dictates the level of curvature of the curve.

The damage function was expressed in terms of Serviceability Index ratio,

$$g = \frac{P_0 - P}{P_0 - P_t} \quad (5)$$

where,

- P_0 = initial Serviceability Index
- P_t = terminal Serviceability Index
- P = actual Serviceability Index

Combining equations (4) and (5) the AASHTO Road Test performance equation can be rewritten as

$$P = P_0 - (P_0 - P_t)(W/\rho)^\beta \quad (6)$$

Figure 4 gives a graphical representation of the AASHTO performance curve.

This form of equation assumes that the serviceability index - versus - traffic curve never reverses its curvature. By way of contrast, Garcia-Diaz, Riggins and Liu, demonstrated that a number of Serviceability Index - versus - traffic relations show a reversal of curvature as illustrated in Figure 5 (11).

The equation for the S-shaped curve is of the form

$$g = e^{-\left(\frac{\rho}{W}\right)^\beta} \quad (7)$$

Combining equations (4) and (6) the S-shaped curve equation can be rewritten as

$$P = P_0 - (P_0 - P_f) e^{-\left(\frac{\rho}{W}\right)^\beta} \quad (8)$$

The same relationship that was used with the Serviceability Index can be applied to the distress area index (A), and the distress severity index (S).

$$A = A_0 - (A_0 - A_f) e^{-\left(\frac{\rho}{W}\right)^\beta} \quad (9)$$

$$S = S_0 - (S_0 - S_f) e^{-\left(\frac{\rho}{W}\right)^\beta} \quad (10)$$

Arithmetic and logarithmic models for asphaltic pavements with granular base, and black base, and overlaid pavements were developed by Garcia-Diaz, Riggins, and Liu using a stepwise regression. These equations were utilized in the development of the deterioration schedules for each pavement section. Appendix A shows Asphalt Concrete(AC) over Black Base, AC over Granular Base, and Overlay regression equations for rutting, alligator cracking, longitudinal cracking, transversal cracking, and Serviceability Index. It also shows Surface Treated pavement regression equations for all seven distresses and PSI.

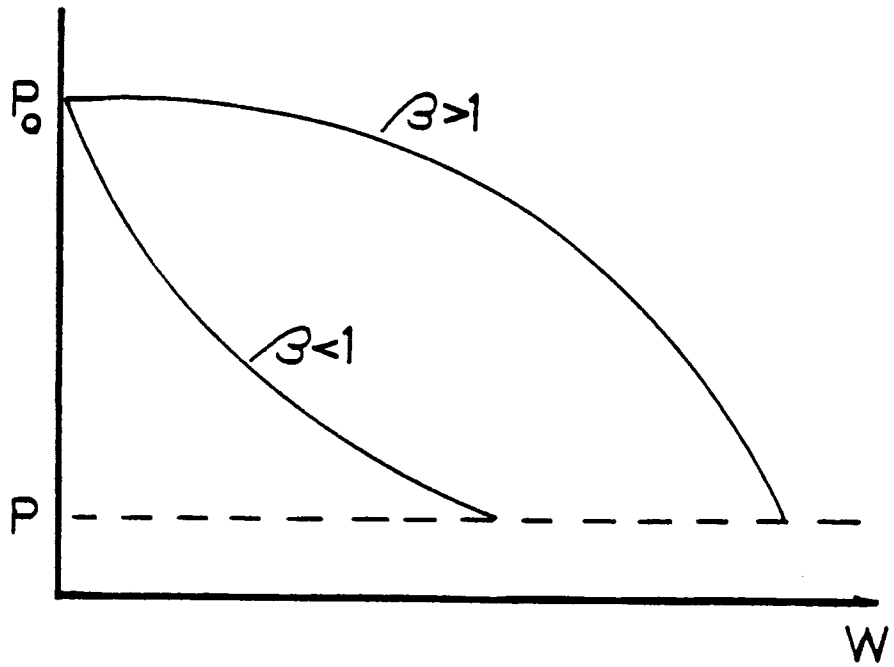


Figure 4. AASHTO Road Test Performance Equation

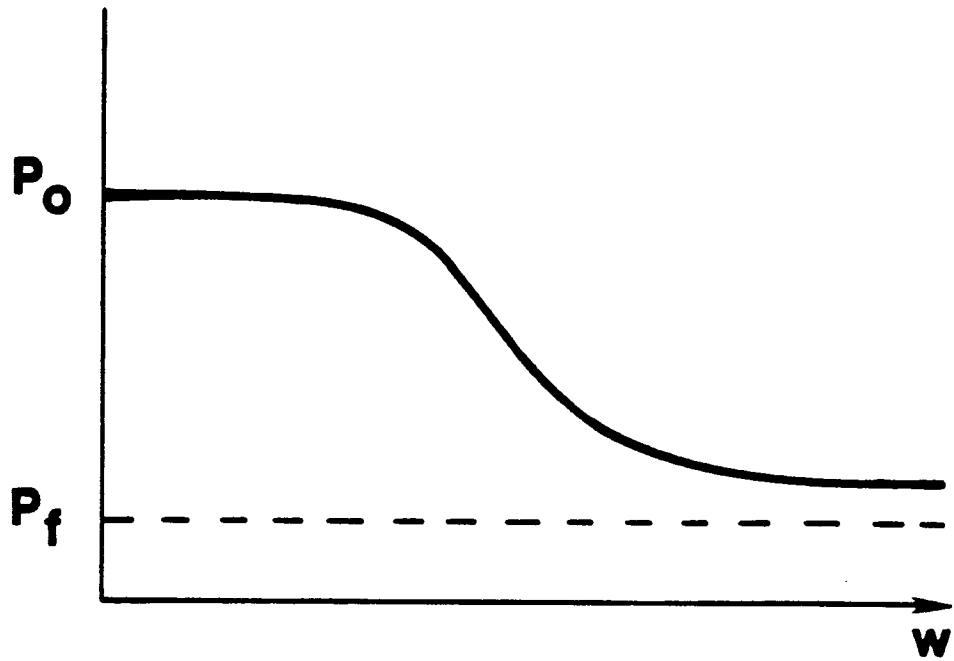


Figure 5. S-Shaped Performance Equation

CHAPTER FOUR

PREDICTIONS OF MAINTENANCE AND REHABILITATION NEEDS FOR THE STATE OF TEXAS

In 1982, the Texas State Department of Highways and Public Transportation implemented its Pavement Evaluation System. This system was designed to a) determine statewide pavement condition and b) estimate one-year statewide rehabilitation needs. Following the successful implementation of the system, the necessity was felt to create a program that could predict the rehabilitation and maintenance needs as well as the budget requirements over any planning horizon. This chapter describes the development of such a system. Appendix C gives the input needs of the program, as well as a listing of the source codes.

The Pavement Evaluation System can be divided into two major areas (Figure 6):

1. Maintenance
2. Rehabilitation and Reconstruction

Within the system the required maintenance is determined by reference to a set of decision trees. Maintenance is only considered when rehabilitation is not warranted. Rehabilitation is defined as any strategy more costly than a 2 1/2 inch overlay.

Overview of the Model Logic

It is important to understand how the overall system works before the individual components are discussed in detail.

The program inputs the percent area for each of 7 distresses (rutting, raveling, flushing, failures, alligator cracking, longitudinal cracking, and transverse cracking), the pavement Serviceability Index, and the current Pavement Score. [NOTE: The procedure described in this report uses the data collected using the rating schemes in existence prior to 1984. It is a simple matter to update this system to the existing rating scheme. Versions of the program are available for both rating scheme]. Then it follows the decision criteria according to Table 2. These decision criteria were developed by the SDHPT. One observation from the initial implementation efforts was the pavements whose score had fallen below the minimum score of 35 were failed pavements, usually in need of structural rehabilitation. However, much of the work proposed by the Districts was on pavements with relatively high scores (i.e. 55-75), these treatments being generally preventive maintenance activities, such as seal coats and thin overlays.

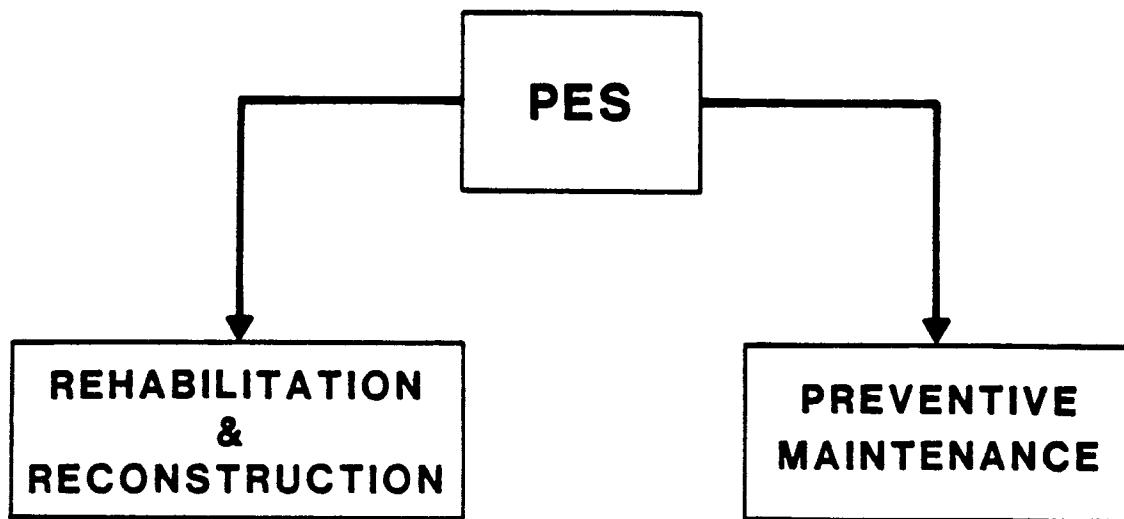


Figure 6. Major Divisions of the Pavement Evaluation System

The set of decision criteria in Table 2 generally describe the kinds of decisions that are made. The model is arranged to follow these criteria. If criterion 1 is selected, the program ages the section for 1 year according to a "deterioration matrix." The program calculates a unique deterioration matrix for each pavement section, based on traffic level, pavement thickness, and climate. The matrices are generated using the performance equations developed with the S-shaped curves discussed in the previous chapter.

Table 2. The Decision Criteria for the Pavement Evaluation System

-
1. If the pavement utility score is greater than the maintenance level (75) - Do nothing
 2. If the pavement utility score is less than the maintenance level but greater than the minimum score - Do maintenance
 3. If the pavement utility score is less than minimum but a seal coat or a thin overlay is recommended - Do maintenance
 4. If maintenance is recommended but the economic analysis of that alternative against a rehabilitation strategy is negative - Do rehabilitation
 5. If pavement score is less than minimum and the minimum strategy is medium overlay - Do rehabilitation
-

After aging the section 1 year, the program then calculates the score for that year. If the score falls into decision criterion 2 or 3, the program selects a preventive maintenance set that can have up to 5 preventive maintenance strategies depending on the pavement type, distress type, percent area, and traffic level. The selection of preventive maintenance strategies is discussed in the next section of this report. The program then resets the existing distress levels for the chosen strategies. It then calculates a new score and starts aging the highway as described above.

If the score is less than minimum, the program selects the best rehabilitation strategy. Each of the rehabilitation strategies are run through deterioration calculations to determine their life expectancy. This life expectancy is compared to a minimum allowable expected life to determine which of the strategies has the smallest positive difference between life expectancy and minimum life, and that one is chosen as the strategy to be implemented. It then calculates the new score and starts aging the highway again. The program has the capability of aging (and rehabilitating) the pavement up to 20 years. Figure 7 gives a flowchart of the major areas of the program.

Description and Input Variables

Pavement types. A listing of the pavement types is shown in Table 3. The list includes ten pavement types and ranges from continuously reinforced concrete (EP-1) to thin surfaced flexible base (EP-10) (6). These descriptions are intended to cover a range of existing pavement types which compose the existing state maintained highway network. These descriptions are based on the current cross section of a pavement structure - not the original construction alone. The Pavement Evaluation System, which calculates score and funding strategy, was initially implemented only for pavement types EP-4 through EP-10. Rigid pavement evaluations were started in 1984.

Maintenance and Rehabilitation Management. Pavement maintenance and rehabilitation can extend the life and improve the performance level of a road.

Maintenance strategies can keep the pavement at an acceptable performance level until rehabilitation is required. Rehabilitation strategies can strengthen the pavement to a level sufficient to extend its life many years. Maintenance and rehabilitation decisions are based on the type of pavement and the type of distresses affecting a section of road.

Once the distresses affecting a section of road have been identified, a decision can be reached on whether to apply maintenance or rehabilitation, and, in either case, what type of strategy will be best to correct the problem.

Preventive Maintenance

Preventive maintenance is any work required to maintain a section of road at a desired level of condition. Maintenance of existing roads is important in pavement management systems because, even though many maintenance strategies do not strengthen the pavement, they help to keep the pavement in usable condition until a rehabilitation can be scheduled.

In Texas, the State Department of Highways and Public Transportation (SDHPT) recommended basically 14 different preventive maintenance strategies for flexible pavements. Table 4 gives typical average cost for each type of maintenance strategy. These numbers are input variables and hence can be changed to meet an agency's requirements. These strategies are applied depending upon variables such as type of distress, area and severity of distress, type of pavement, location, and cost.

Descriptions of the strategies and their presently defined cost estimation functions are given below:

Seal Crack. The process of filling cracks with bituminous materials to prevent further cracking and wetting of the subgrade.

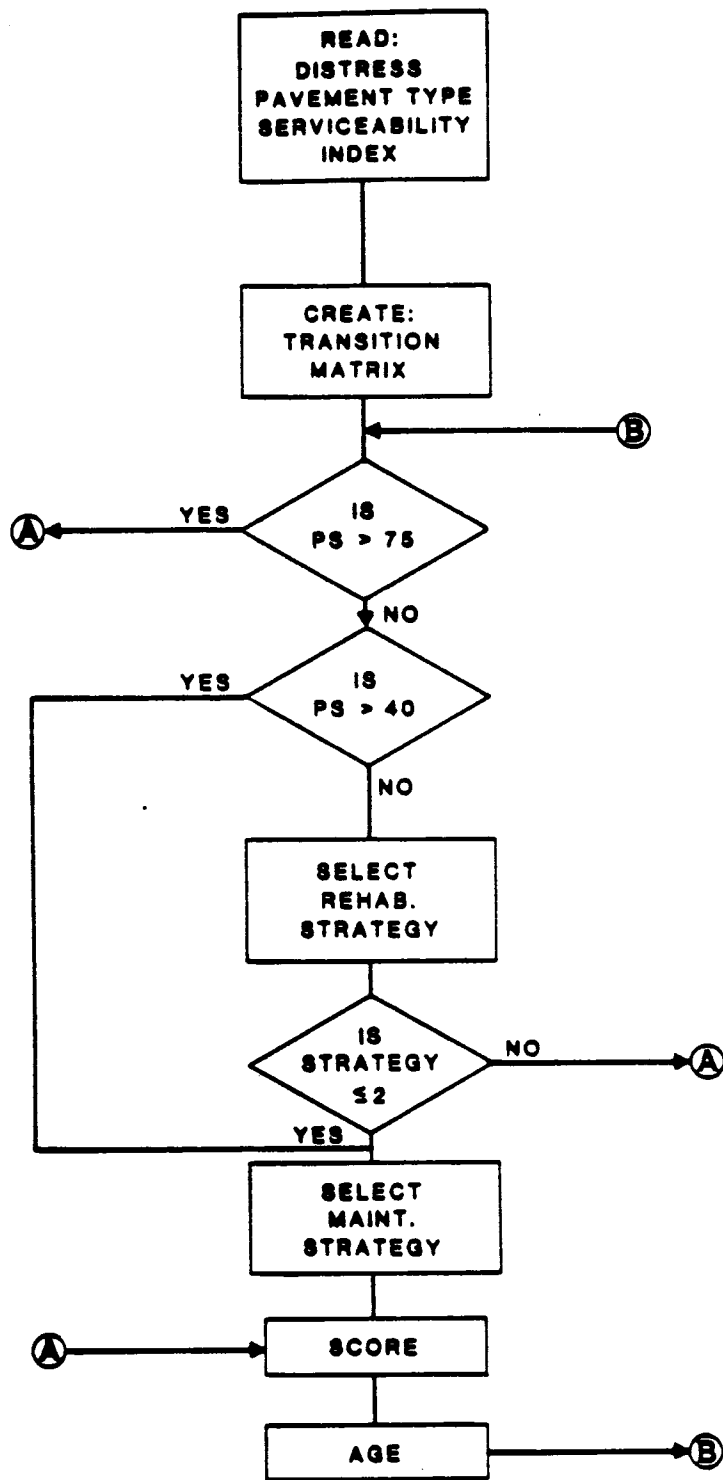


Figure 7. Flowchart of Major Areas of the Program

TABLE 3. Listing of Pavement Types

Pavement Type	Description
EP-1	Continuously reinforced concrete pavement
EP-2	Jointed reinforced concrete pavement
EP-3	Jointed plain concrete pavement
EP-4	Thick asphaltic concrete pavement (greater than 5 1/2" of hot-mixed asphaltic layers)
EP-5	Intermediate thickness asphaltic concrete pavement (2 1/2" to 5 1/2" of hot-mixed asphaltic layers)
EP-6	Thin surfaced flexible base pavement (hot-mixed asphaltic layers less than 2 1/2" thick)
EP-7	Composite pavement (concrete pavement which has received an asphalt overlay)
EP-8	Overlaid and/or widened old concrete pavement
EP-9	Overlaid and/or widened old flexible pavement
EP-10	Thin surfaced flexible base pavement (surface treatment - seal coat combinations)

Cost: length of crack (ft) x unit cost (\$/ft)

Surface Patching. The process of replacing and compacting bituminous material in the pavement surface.

Cost: Area of patch(yd) * depth(in) * unit cost(\$/yd * in)

Full Depth Patching. A full depth asphalt concrete patch that is designed to ensure strength equal to that of the surrounding asphalt. Could involve reworking the base and subgrade.

Cost: Area(yd²) * depth(in) * unit cost(\$/yd² * in)

Fog Seal. Cold mixture of asphaltic emulsion and water that seals the pavement surface against the entrance of air and water, reduces raveling and oxidation (8).

Cost: Width of section(yds) * length(yds)* unit cost(\$/yd²)

Strip Seal. Asphalt concrete layer that is applied to a partial section of road to improve skid resistance and bleeding of pavements. Its cost is based on the percent of the pavement area affected by the existing distresses.

Small area - Cost: 250 yd² * unit cost(\$/yd²)

Medium area - Cost: 500 yd² * unit cost(\$/yd²)

Large area - Cost: 1000 yd² * unit cost(\$/yd²)

Seal Coat. Application of asphalt layer with an aggregate coat to seal the surface against the entrance of air and water, reduce raveling, and improve skid resistance.

Cost: Length of section(yds) * width of section(yds)
* unit cost(\$/yd²)

Asphalt-Rubber Seal Coat. A mixture of asphalt and at least 15 percent recycled ground rubber used to prevent reflection cracks, to seal the surface against the entrance of air and water, and to correct raveling.

Cost: Width(yds) * length(yds) * unit cost(\$/yd)

Slurry Seal. A mix of asphaltic emulsions, water, and fine aggregate that is applied to seal the surface against air and water and to increase durability for the freeze-thaw cycles.

Cost: Width(yds) * length(yds) * unit cost(\$/yd²)

Level-Up. A thin layer of asphaltic concrete cement that will even the pavement surface.

TABLE 4. Mean Costs for Preventive Maintenance

<u>Strategy</u>	<u>Cost</u>
1 - Seal Crack	0.23 ft.
2 - Patching	4.20 yd ² /inch
3 - Full Depth Repair	4.45 yd ² /inch
4 - Fog Seal	0.25 yd ²
5 - Strip Seal	0.70 yd ²
6 - Seal Coat	0.60 yd ²
7 - Asphalt-Rubber Seal Coat	1.25 yd ²
8 - Slurry Seal	0.35 yd ²
9 - Level-Up	1.65 yd ²
10 - Thin Overlay	2.40 yd ²
11 - Rotomill	0.85 yd ²
12 - Spot Seal	0.60 yd ²
13 - Rotomill + Seal Coat	2.50 yd ²
14 - Rotomill + Thin Overlay	3.25 yd ²

Cost: Width(yds) * 1000 yds * unit cost(\$/yd²)

Thin Overlay. A 1 to 1 1/2 inch lift of asphaltic concrete that will not increase the strength of the pavement.

Cost: Length(yds) * width(yds) * unit cost(\$/yd²)

Rotomill. It is a machine designed with the purpose of planning off variable thicknesses of asphalt.

This machine can be used together with an overlay, or a seal coat application creating two new strategies:

Rotomill + Seal coat

Rotomill + Overlay

Spot Seal. Application of asphalt to spots in the surface to prevent cracks, and to seal the surface of the pavement.

The most important factors that affect the selection of a preventive maintenance strategy are: type of pavement, type of distress, extent of distress, traffic level, and the 18-kip equivalent single axle load level. Appendix B gives the tabulation of the different preventive maintenance strategies that can be applied in each of the 672 combinations of pavement type (7), distress type (8), distress extent (3), traffic levels (2), and 18-kip equivalents (2) that can occur. In order to facilitate the selection of preventive maintenance strategies, a decision tree has been created from which the program can select up to five strategies depending on the distresses affecting the section of road. Figure 8 presents an overview of the decision tree related to preventive maintenance feasible strategies. The decision trees used in this project were developed by Highway Department maintenance personnel in the Central Office and Districts. The basic inputs to each tree are:

- extent of each pavement distress
- pavement type
- traffic level

For each individual distress/pavement type/traffic level combination, an appropriate maintenance strategy is defined. The possible strategies are shown in Table 5.

Once individual maintenance strategies have been defined for each distress type (and level of PSI) then a procedure to calculate a dominant strategy is used.

The dominant strategy selection procedure ranks the various selected strategies in order of their ability to repair several

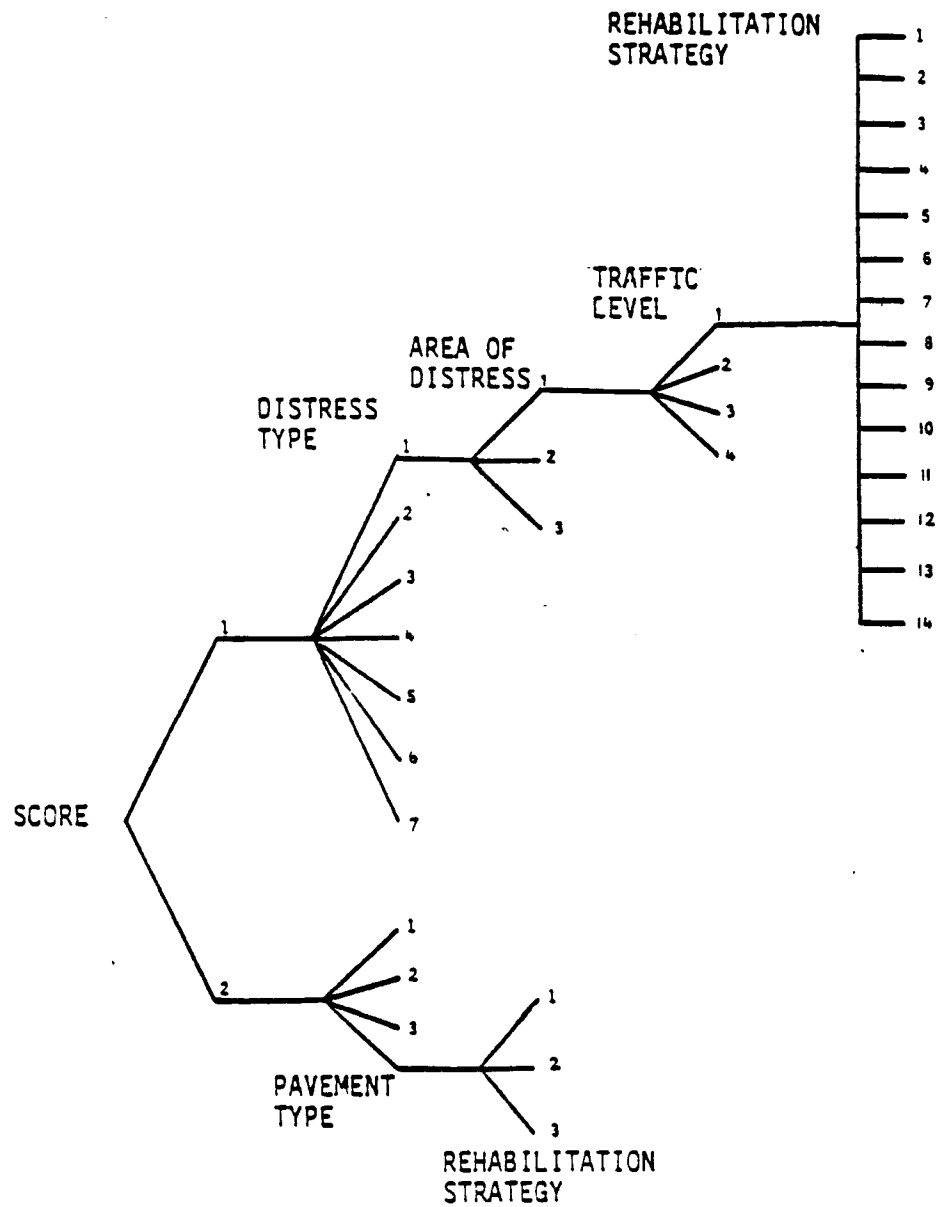


Figure 8. Rehabilitation and Maintenance Decision Tree.

TABLE 5. Listing of Maintenance Strategies.

0	Do Nothing		
1	Seal Cracks	8	Slurry Seal
2	Partial Patch	9	Level-up
3	Full Depth Patch	10	Thin Overlay
4	Fog Seal	11	Rotomill
5	Strip Seal	12	Spot Seal
6	Seal Coat	13	Rotomill + Seal Coat
7	Asphalt-Rubber Seal	14	Rotomill + Thin Overlay

distresses. Rotomill plus thin overlay (Strategy 14) is ranked first followed by Strategies 13, 10, 9, 7, 6,... The selection procedure selects the highest ranked strategy that has been chosen to repair an individual distress and then makes additional checks for routine maintenance requirements (i.e. crack seals). For instance if Strategy 14 has been selected, it only remains to check if any full depth repairs are required. Similarly for thin overlays and level-ups, additional checks are made for full depth repairs, surface patching, and crack seals.

An example of one branch of the decision tree is shown in Figure 9. Similar branches exist for the 7 pavement types and 9 distress types considered in the model

Rehabilitation

The primary purpose of any rehabilitation activity is to improve the structural performance and riding characteristics of a pavement. No pavement is designed to last forever; therefore, it is safe to assume that during the life cycle of a pavement, it will deteriorate to an unacceptable level. It will then require some kind of rehabilitation to an acceptable level in order to continue to serve (12).

The three major rehabilitation activities are: overlays, reconstruction, and recycling.

Overlay. Overlaying is a rehabilitation strategy that consists of placing layers of asphalt concrete (AC) pavement to improve or extend the service life of a section of road. Overlays can be of different thicknesses with a maximum of 7 1/2 inches. An overlay with a thickness of less than 1 1/2 inches does not add structural strength to the pavement. This technique is used to correct rutting, cracking, and raveling and to improve the Serviceability Index.

Reconstruction. Many times just one lane of a section of road has structural damage while the other lane has retained its strength. When such a case occurs, a partial reconstruction of one lane can be more cost effective than an overlay that must be applied to the whole section.

Recycling. Recycling is the technique of removing the existing pavement, processing it, mixing it with new aggregate and a recycling agent, and placing it back onto the roadway.

Table 6 illustrates some specific techniques used in each of the major rehabilitation activities along with the condition they are intended to correct

Five rehabilitation funding strategies are considered within the current PES ranging from the equivalent of seal coat maintenance (R-1) to a 7 1/2 in. thick asphalt concrete overlay.

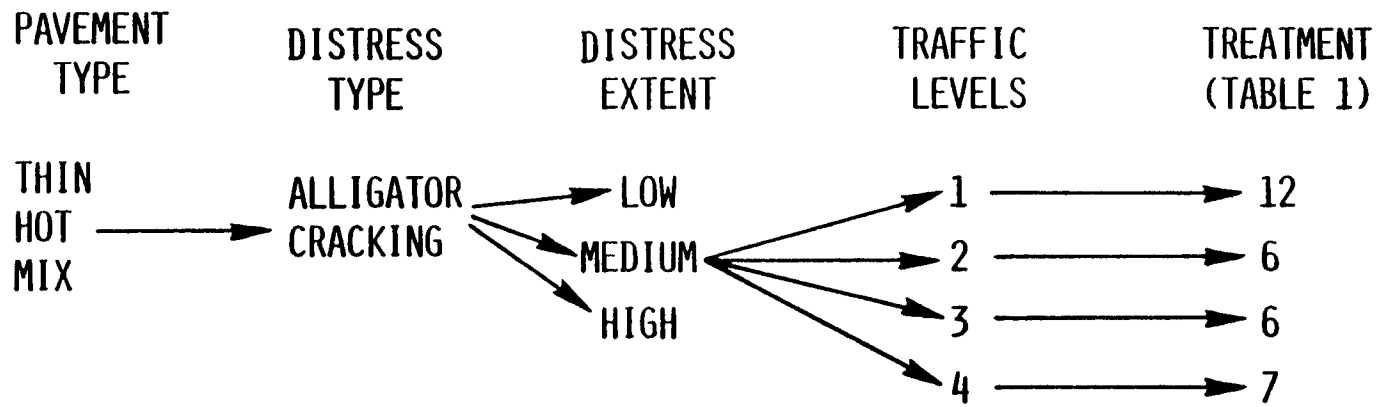


Figure 9. Example Branch of the Decision Tree

These rehabilitation funding strategies were selected from a listing originally prepared by J. L. Brown (13). A description of the rehabilitation strategies is shown in Table 7. Table 8 is a listing of the five separate funding strategies and their associated costs (statewide average) in terms of dollars per lane foot per mile (one foot wide strip a mile long).

Both maintenance and rehabilitation costs should vary somewhat from district to district. Thus, these costs must be developed for each of the twenty-four districts within the state.

The Current Pavement Score (PSC)

The Current Pavement Score was designed to be a combination of Visual Utility, Serviceability Index, Safety, and Maintenance cost, that is used as an indicator of the overall condition of the pavement section at the moment of inspection. However early in the implementation effort it was determined that skid data was too costly to collect on a network and that reliable maintenance cost data was not available. Therefore both of these were dropped from the pavement score calculation procedure. The next sections of this report describe in detail how the pavement score is calculated within the State of Texas Pavement Evaluation System.

Visual Defect Evaluation Form for Flexible Pavements. The form shown in Figure 2 was jointly developed by the SDHPT and TTI for the 1983 data collection effort. The pavement rating procedure is described in detail in the Department's Raters Manual (17). This form is a composite of the original visual condition survey procedure developed by Epps (10) and the new utility concepts. The data collected with this form are used to calculate the visual defect utility which is a component of the current pavement score (PSC). This score will be further discussed in the next subsection.

Additional inputs required for calculating the current PS (PSC)

Table 9 shows the additional inputs necessary to calculate the current PS (PSC) for each highway segment. The inputs which are included in this table fall into the categories used in Tables 10, 11, 12, and 13.

To calculate the PSC for a highway segment these inputs and the appropriate utility curves are required. The proposed overall pavement score equation is as follows:

$$PSC = [(AVU)^{a_1} (SIU)^{a_2} (SKU)^{a_3} (SCU)^{a_4}]^{1/FC} \quad (11)$$

where,

PSC = Pavement Evaluation System score which represents a highway segment's relative priority for rehabilitation

TABLE 6. Rehabilitation Strategy Information

- | | |
|-----------------------------------|--|
| 1. PCC Overlay | - restore structural strength |
| 2. AC Overlay | - restore structural strength
- correct cracking
- correct raveling
- improve ride quality |
| 3. Inverted Overlay | - restore structural strength
- correct cracking
- correct raveling
- improve ride quality |
| 4. Rubber Asphalt
plus Overlay | - restore structural strength
- correct cracking
- correct raveling
- water resistant
- improve ride quality |
| 5. Hot Recycling | - restore structural strength
- conserve material
- correct cracking
- correct raveling |
| 6. Heater Remix | - restore structural strength
- correct cracking
- correct raveling |
| 7. Lane Reconstruction | - restore structural strength |

TABLE 7. Listing of Rehabilitation Funding Strategies

Funding Strategy	Description of Equivalent Maintenance or Rehabilitation	
	Hot Mix Pavement	Surface Treated Pavement
R-1	Seal coat, or fog seal, or extensive patching plus seal	Seal Coat
R-2	1" ACP overlay, or seal plus level-up	Partial reconstruction
R-3	2 1/2" ACP overlay	Full reconstruction, reworking and adding additional base and surfacing
R-4	4" ACP overlay or rotomill plus thin overlay	Not applicable
R-5	7 1/2" ACP overlay or reconstruction	Not applicable

TABLE 8. The Equivalent Statewide Average Cost
for Each PES Funding Strategy

Funding Strategy	Equivalent Cost (\$/foot-mile)
R-1	214
R-2	925
R-3	2000
R-4	3550
R-5	7000

TABLE 9. Additional Inputs Required to Calculate Pavement Score

1. Highway Functional Class
2. ADT/Lane
3. 18-kip Equivalent Single Axles in Design Lane
4. Rainfall (in./year)
5. Freeze-Thaw Factors (cycles/year)

Inputs 4 and 5 are available on a county basis. For each pavement section, a county number is input. These environmental factors are obtained via a table look-up.

TABLE 10. Average Daily Traffic Factors (ADTF)

ADT/Lane	Average Daily Traffic Factors
300 or less	1.00
301 - 750	0.96
751 - 2000	0.92
2001 - 7500	0.88
7501 - 25,000	0.84
greater than 25,000	0.80

TABLE 11. 18-kip Equivalent Axle Load Factors (KEF)

18-kip EAL	18-kip EAL Factors
less than 6×10^6	1.00
6×10^6 - 12×10^6	0.95
greater than 12×10^6	0.90

TABLE 12. Rainfall Factors

Rainfall (in./yr.)	Rainfall Factor (RF)
20 or less	1.00
21 - 40	0.97
greater than 40	0.94

TABLE 13. Freeze-Thaw Factors

Freeze Cycles (cycles/year)	Freeze-Thaw Factors (FF)
10 or less	1.000
11 - 30	0.973
31 - 50	0.967
greater than 50	0.960

AVU = Adjusted visual defect utility.

SIU = Serviceability index utility.

SKU = Skid number utility

SCU = Structural Capacity utility.

a_1, a_2, a_3, a_4 = Weighting factors.

$$a_1 = \frac{1}{(\text{ADTF})(\text{KEF})} \text{ and } a_2 = a_3 = a_4 = 1$$

ADTF = Average daily traffic factor, as given in Table 10.

KEF = 18-kip equivalent axle loading factor (Table 11).

FC = Functional Class weighting factor.

<u>Functional Class</u>	<u>Factor</u>
1	0.80
2	0.80
3	0.80
4	0.90
5	0.95
6	1.00
7	1.00

$$\text{AVU} = (U_{\text{rutting}})^{b_1} (U_{\text{raveling}})^{b_2} (U_{\text{flushing}})^{b_3} (U_{\text{Fail}})^{b_4} (U_{\text{allig.}})^{b_5} (U_{\text{long.}})^{b_6} (U_{\text{trans.}})^{b_7} \quad (12)$$

The utility inputs developed for the original PES, required to compute the AVU can be obtained from utility curves developed by SDHPT personnel. Equations which approximate these curves are as follows:

Rutting.

1/2" - 1" "Slight Rutting"

$$U_{\text{rutting}} = 1 - 0.323 e^{-12.365(1/x)} \quad (13)$$

> 1" "Severe Rutting"

$$U_{\text{rutting}} = 1 - 0.694 e^{-10.132(1/x)} \quad (14)$$

where x = percent of area (wheelpath)

Raveling.

$$U_{\text{raveling}} = 1 - 0.570 e^{-24.911(1/x)} \quad (15)$$

where x = percent of area (total surface)

Flushing.

$$U_{\text{flushing}} = 1 - 0.647 e^{-34.99(1/x)} \quad (16)$$

where x = percent of area (total surface)

Failures.

$$U_{\text{failures}} = 1 - 1.351 e^{-5.778(1/x)} \quad (17)$$

where x = number of failures per mile

Alligator Cracking.

$$U_{\text{alligator cracking}} = 1 - 0.559 e^{-4.962(1/x)} \quad (18)$$

where x = percent of area (wheelpath)

Longitudinal Cracking.

$$U_{\text{longitudinal cracking}} = 1 - 0.774 e^{-161.98(1/x)} \quad (19)$$

where x = lin. ft. per lane per station

Transverse Cracking.

$$U_{\text{transverse cracking}} = 1 - 0.545 e^{-6.798(1/x)} \quad (20)$$

where x = number per station

For all equations listed above, the utility is 1.0 when x is zero. The b coefficients are determined by the following relationships with Rainfall Factor (RF) and Freeze-Thaw Factor (FF):

$$b_1 = 1/\text{RF}, \text{ rutting}$$

$$b_2 = 1/(\text{RF})(\text{FF}), \text{ patching}$$

$$b_3 = 1/(\text{RF})(\text{FF}), \text{ failures}$$

$$b_4 = 1/(\text{RF})(\text{FF}), \text{ block cracking}$$

$$b_5 = 1/(\text{RF})(\text{FF}), \text{ alligator cracking}$$

$$b_6 = 1/(\text{RF})(\text{FF}), \text{ longitudinal cracking}$$

$$b_7 = 1/(RF)(FF), \text{ transverse cracking}$$

The Rainfall Factor and Freeze-Thaw Factor can be obtained from Tables 12 and 13.

Serviceability Index

There are three curves available for use and these curves are a function of a factor defined by multiplying the ADT/Lane by the SPEED for each highway segment. The ADT/Lane is the Average Daily Traffic for the highway segment and SPEED is the posted speed limit for the highway segment.

Curve A: (ADT)(SPEED) < 27,500

$SIU = 1.0$	if $2.5 \leq SI \leq 5.0$
$SIU = 1.0 - 0.10 \left(\frac{2.5 - SI}{0.5} \right)$	if $2.0 \leq SI < 2.5$
$SIU = -0.2666 + 0.58333 (SI)$	if $0.8 \leq SI < 2.0$
$SIU = 0.20 \left(\frac{SI}{0.8} \right)^2$	if $0 \leq SI < 0.8$
$SUY = 0$	if $SI < 0$

where

SIU = Serviceability Index Utility

SI = Serviceability Index (obtained by use of the Mays Ride Meter)

Curve B: 27,500 < (ADT)(SPEED) < 165,000

$SIU = 1.0$	if $3.0 \leq SI \leq 5.0$
$SIU = 1.0 - 0.10 \left(\frac{3.0 - SI}{0.5} \right)$	if $2.5 \leq SI < 3.0$
$SIU = -0.5583 + 0.58333 (SI)$	if $1.3 \leq SI < 2.5$
$SIU = 0.20 \left(\frac{SI}{1.3} \right)^2$	if $0 \leq SI < 1.3$
$SIU = 0$	if $SI < 0$

Curve C: (ADT)(SPEED) > 165,000

$SIU = 1.0$	if $3.5 \leq SI \leq 5.0$
$SIU = 1.0 - 0.10 \left(\frac{3.5 - SI}{0.5} \right)$	if $3.0 \leq SI < 3.5$
$SIU = -0.85 + 0.58333 (SI)$	if $1.8 \leq SI < 3.0$
$SIU = 0.20 \left(\frac{SI}{1.8} \right)^2$	if $0 \leq SI < 1.8$
$SIU = 0$	if $SI < 0$

Determination of Final Attributes as a Function of Current Attributes.

An important component of this system is the ability to estimate what the Final Pavement Score (FPS) will be for a given highway segment after some type of maintenance or rehabilitation is applied. To aid in this task, Tables 14, 15, and 16 were developed.

Table 14 provides a method of determining the final utility value for each distress after the rehabilitation of a highway segment given the initial utility values before rehabilitation. For example, an R-3 strategy (2 1/2" ACP overlay) will have a large effect on deep rutting, and hence the after-treatment utility value will be at its maximum level. The values given in this table indicate how effective a particular strategy is at remedying a particular distress type. Table 14 also provides a method of determining the final serviceability index following each of the maintenance strategies.

Table 15 provides a method of determining the final serviceability index following each of the rehabilitation strategies. The data used to generate this table were obtained from actual condition and performance information available in District 21 and the Texas Flexible Pavement Data Base.

Table 16 provides a method of determining the final utility value of each distress after the maintenance of a highway segment given the initial utility values before maintenance. For example, an M-01 treatment (seal coat) will have no effect on deep rutting, and hence the after-treatment utility value will be the same as the before treatment value.

Selection of strategies R-1 or R-2 (seal coat or thin overlay) indicates that even though the pavement score for a section of road is below the minimum required score, the section of road can be repaired satisfactorily using one or more maintenance treatments. If the PES system recommends either R-1 or R-2 then that section is reprocessed by the maintenance decision tree routine.

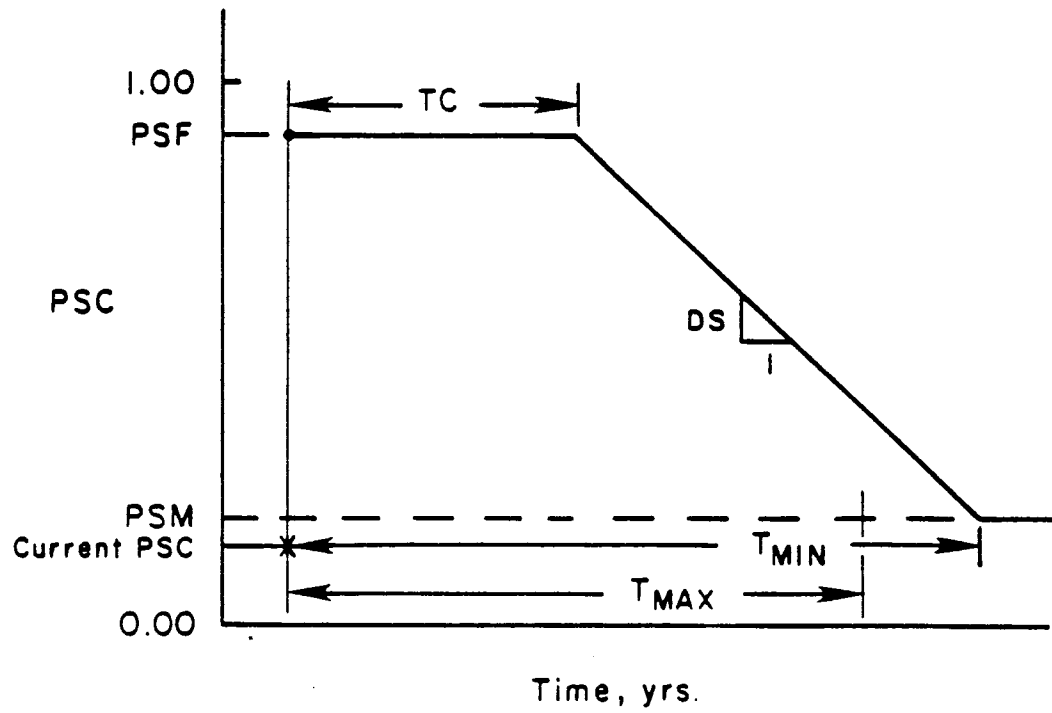


Figure 10. Selection of a Rehabilitation Funding Level

TABLE 14. Gain in PES Components for the Various Rehabilitation Funding Strategies

Distress	Maximum % Recovery of Utility Score Following Various Funding Strategies				
	R-1	R-2	R-3	R-4	R-5
"Slight" Rutting < 1"	33	100	100	100	100
"Severe" Rutting > 1"	0	70	100	100	100
Raveling	100	100	100	100	100
Flushing	100	100	100	100	100
Failures	25	62	75	87	100
Alligator Cracking	60	80	100	100	100
Longitudinal Cracking	60	80	100	100	100
Transverse Cracking	75	100	100	100	100

TABLE 15. Determination of the Final Serviceability Index as a Function of Current Serviceability Index

Attribute	Current Attribute Measure (before Rehabilitation)	Final Values Following Funding Strategy				
		R-1	R-2	R-3	R-4	R-5
Serviceability	0.0 - 1.0	SI Current + 0.2	4.3	4.5	4.5	4.5
	1.0 - 2.0	SI Current + 0.2	4.3	4.5	4.5	4.5
	2.1 - 3.0	SI Current + 0.1	4.3	4.5	4.5	4.5
	3.1 - 4.0	SI Current	4.3	4.5	4.5	4.5
	4.1 - 5.0	SI Current	4.3	4.5	4.5	4.5

TABLE 16. Gain in PES components for the Various Maintenance Strategies

For distress 0 = strategy has no effect on distress

100 = strategy fully repairs distress

For serviceability Index 100 indicate an increase of PSI by 1.00 units.

Strategy	Rutting < 1"	Rutting > 1"	Raveling	Flushing	Failures	Alligator	Longitudinal	Transverse	Serviceability Index
M-01	0	0	0	0	0	0	100	100	0
M-02	0	0	0	0	100	30	0	0	50
M-03	100	100	0	0	100	50	0	0	100
M-04	0	0	100	0	0	10	10	10	0
M-05	0	0	50	50	0	70	0	0	0
M-06	0	0	100	100	0	100	100	100	0
M-07	0	0	100	100	0	100	100	100	0
M-08	0	0	100	100	0	100	100	100	0
M-09	100	100	100	100	100	100	100	100	150
M-10	100	100	100	100	100	100	100	100	200
M-11	100	100	100	100	0	0	0	0	50
M-12	0	0	50	50	0	50	15	15	0
M-13	0	0	100	100	0	100	0	0	50
M-14	100	100	100	100	100	100	100	100	200

Selection of a Rehabilitation Funding Level

The selection of a Rehabilitation Funding level is made by using the concepts illustrated in Figure 10. The graph shows that the current pavement score (PSC) is below the minimum acceptable score (PSM). After a rehabilitation strategy has been applied, the score rises to PSF, remains relatively constant for a period of time, TC, and then begins to deteriorate along a slope, DS. It again reaches a minimum score at a time, TMAX. If TMAX is greater than the minimum acceptable time, TMIN, the rehabilitation strategy is accepted. The rehabilitation strategy that is selected is the one with the least cost which lasts longer than TMIN. Details of how each of these variables is determined are given in the following sections.

Minimum Acceptable PS (PSM). The values for PSM shown in Table 17 are listed for six highway functional classifications. The definitions for these highway functional classification types were as follows (6):

1. Principal Arterial:
 - (a) Interstate System
 - (b) Other principal arterials

These facilities provide continuous and connected routes to all large urban areas and corridor movements with trip length and travel characteristics which are of statewide or interstate interest.

2. Minor Arterial:

This system connects cities and other traffic generators and provides for relatively high speeds over long distances. It is spaced to provide arterials to all developed areas.

3. Major Collector:

Provide service to intercounty travel corridors and connect county traffic generators with cities, towns, or higher classified routes.

4. Minor Collector:

Collect traffic from local roads and provide service to smaller communities.

Minimum Allowable Time to Next Rehabilitation. Table 18 shows how the minimum allowable times to next rehabilitation are organized. These times are a function of highway functional classification and traffic factor. The table considers only the first factor and a simple equation incorporates the traffic factor. The initial allowable time from the table and the traffic factor are related as follows:

$$TMIN = (TMINI)(TF) \quad (21)$$

where

TMIN = the minimum allowable time (years) to the next application of a rehabilitation funding strategy following the application of the rehabilitation strategy currently being considered.

TMINI = same as TMIN except unadjusted for traffic (Table 19).

TF = traffic factor for the highway segment being considered (Table 19), as explained in the next section.

Traffic Factors Required for Calculating TMIN and DS. Table 19 shows the traffic factors which are used to determine the final values of TMIN (Minimum Allowable time between treatments) and DS (Deterioration Slope) for each highway segment. These factors should be a function of highway functional classification, percent trucks, and AADT. Currently, the traffic factors have been developed with available data for only two AADT levels and the four functional classifications because presently available data precluded use of percent trucks at this time. These factors were developed from pavement survival data available from District 21 and the Texas Flexible Pavement Data Base.

Rehabilitation Strategy Deterioration Slopes. Table 20 shows the initial deterioration slopes (PSI) for five funding strategies and seven pavement types. A simple equation is used to determine the final deterioration slope (DS) as a function of traffic, climatic, and subgrade soil factors. This equation is as follows:

$$DS = (DSI)(TF)(CF)(SF) \quad (22)$$

where

DS = deterioration slope of a funding strategy for a given pavement type after adjustment for traffic and climate conditions

DSI = initial deterioration slope obtained from Table 20

TF = traffic factor for the highway segment being considered (Table 19)

CF = climate factor (Table 21)

SF = soil factor (Table 22)

The deterioration slopes and appropriate traffic factors were presented by Lytton and Scullion in the report 239-6F of the Texas Transportation Institute(6).

Climate Factors. The climate factors shown in Table 21 have all been set to unity. As additional research is accomplished in subsequent studies, the climatic effects on pavement deterioration rates will be further examined and developed. Currently, it is expected that these factors can be made a function of freeze-thaw cycles and rainfall.

Soil Factors. The soil factors shown in Table 22 range between 1.00 for non-expansive soil to 1.15 for a highly expansive soil in a climate with moderate rainfall. The soil factor increases the slope of the PES deterioration curve to account for the effect of expansive clays. These clays are known to be most active in the central Texas area where annual wetting and drying cycles are common.

Calculation of final PS (PSF). For a given highway type and funding strategy the PSF is a function of the final (after maintenance) AVU, SI, and SN. The final AVU (AVUF) is calculated from the values given in Table 14, the SI values are selected from Table 15, and the SN is given a value of 1. Then the appropriate utility equation for SI and SN is used to convert these two attributes to utilities. A simple multiplication of the final AVU, SI utility, and SN utility results in the PSF as follows:

$$PSF = [(AVUF)^{a_1} (SIUF)^{a_2} (SKUF)^{a_3} (SCUF)^{a_4}]^{1/FC} \quad (23)$$

where

- AVUF = final AVU after maintenance or rehabilitation
- SIUF = final serviceability index utility after maintenance or rehabilitation
- SKUF = final skid number utility after maintenance or rehabilitation
- SCUF = final structural capacity utility after maintenance or rehabilitation

a_1 , a_2 , a_3 , a_4 , and FC are as defined in Equation

11.

Currently, the routine maintenance cost utility and skid number utility are set at 1.0 and, as such, do not affect the calculated value of PSF.

TABLE 17. Minimum Acceptable PS (PSM)

Highway Functional Classification	F.C. No.	Minimum Acceptable PS
Principal Arterial (IH and Urban Freeway)	1, 2	0.50
Minor Arterial (US and SH)	3, 4	0.45
Major Collector (FM)	5	0.40
Minor Collector (FM)	6	0.30

TABLE 18. Recommended Minimum Allowable Time (TMINI) Until Next Application of Rehabilitation

Functional Class	TMINI (years)
1	9
2	7
3	7
4	5
5	3
6	3

TABLE 19. Traffic Factors Required for Calculating TMIN and DS (TF)

AADT	% Trucks	Highway Functional Classification			
		Principal Arterial, (1, 2)	Minor Arterial, (3, 4)	Major Collector, (5)	Minor Collector, (6)
HIGH Arterial: AADT 10,000 Collector: AADT 2,000	High	1.40	1.40	1.40	1.40
	Low	1.10	1.10	1.00	1.00
LOW Arterial: AADT 10,000 Collector: AADT 2,000	High	1.20	1.20	1.20	1.20
	Low	1.00	1.00	1.00	1.00

TABLE 20. Initial Deterioration Slopes (DSI) for Five Funding Strategies and Seven Pavement Type Combinations (Units: Pes units/year)

Funding Strategies	Pavement Type (refer to Table 3)						
	EP-4	EP-5	EP-6	EP-7	EP-8	EP-9	EP-10
R-1	0.10	0.10	0.10	0.10	0.10	0.10	0.10
R-2	0.083	0.110	0.110	0.110	0.110	0.110	0.110
R-3	0.083	0.10	0.10	0.10	0.10	0.10	0.10
R-4	0.083	0.083	0.083	0.083	0.083	0.083	0.083
R-5	0.059	0.059	0.059	0.059	0.058	0.059	0.059

TABLE 21. Climate Factors (CF)

Freeze-thaw cycles (cycles/yr)	Rainfall (in./yr)		
	> 20	21-40	< 40
> 10	1.0	1.0	1.0
11 - 30	1.0	1.0	1.0
31 - 50	1.0	1.0	1.0
< 20	1.0	1.0	1.0

TABLE 22. Soil Factors

Plasticity Index	Rainfall (in./yr)		
	< 20	21-40	> 40
< 20	1.00	1.00	1.00
20 - 40	1.02	1.07	1.05
> 40	1.05	1.15	1.10

Calculation of TMAX. To calculate the time a given rehabilitation funding strategy will last after it is applied to a highway segment, the PSF, PSM, TC, and DS must be known. They are related by the following equation:

$$TMAX = TC + \frac{PSF - PSM}{DS} \quad (24)$$

where

- TMAX = the time a given maintenance or rehabilitation funding strategy will last to a minimum PS (PSM)
- TC = time of constant service for a given maintenance or rehabilitation funding strategy obtained from Table 20
- PSF = the final PS after a maintenance or rehabilitation funding strategy is applied
- PSM = the minimum PS obtained from Table 17
- DS = deterioration slope obtained from Table 20 and adjusted for traffic, climate, and soil factors (Tables 19, 21, and 22, respectively).

Calculation of Low-High Traffic-Load Factors. A component of the preventive maintenance decision tree is the Low-High factor for the traffic-load combination of the section of road. Table 23 gives the break-over points for average daily traffic per lane and 18-kip respectively depending on the functional class of the road. Above the break-over point, the factor is considered high and below it the factor is considered low. Table 24, gives the possible low-high combinations and the code assigned to each one.

Deterioration Matrix. If a section does not require maintenance in the current year it is aged using deterioration matrices. Equations are available for Asphaltic Concrete (AC) over Black Base, AC over Granular Base, Overlays, and Surface Treated Pavements. The program selects appropriate performance equations for a given highway based on the input pavement type as shown in Table 25.

The deterioration matrix is developed in an iterative process in which the basic S-shape performance equation is used to find W given g. Using this approach the current value of damage g is known, using S-shape curve performance equations, an estimate is made as to the value of damage one year from current. The regression equations used are shown in Appendix A, note using these curves the deterioration rate will be a function of the variable shown in Table A-1.

TABLE 23. Break-Over Points for Average Daily Traffic and 18-kip by Functional Class.

Functional Class	Average Daily Traffic	18-kip x 10 ⁶
1	12,000	8.0
2	12,000	8.0
3	8,000	6.0
4	8,000	6.0
5	2,000	2.5
6	2,000	2.5
7	2,000	2.5

TABLE 24. Low-High Codes for Maintenance Decision Tree

Low-High	Code
LL	1
LH	2
HL	3
HH	4

The applicable relationship, equation (7)

$$g = e^{-\left(\frac{\rho}{W}\right)^{\beta}}$$

where

- g = percent area of distress normalized to a scale 0 to 1
- W = accumulated 18-kip loads, time, or weather cycles
- ρ and β = regression equations for each specific distress and pavement type as shown in Appendix A.

Solving for W yields

$$W = \frac{\rho}{(-\ln g)^{1/\beta}} \quad (25)$$

With this equation, the levels of load, time, or cycles can be determined at which the specified percentage of distress (g) is reached.

The steps of the construction of the deterioration matrix are as follows:

1. Given the current damage level g from pavement inspection data, calculate W, (the term W represents either the theoretical number of 18 kip ESAL or Months (depending on distress type) to reach the level of damage g.)

$$W = \frac{\rho}{(-\ln g)^{1/\beta}}$$

2. Increment the value of W by 1 year. This involves adding either 1 years worth of 18 kip Equivalents to W or adding 12 month to W. The number of 18 kips ESAL per month is known for each section within the Texas PES system.
3. Find g (the damage) for the next year by using the incremented W value in the sigmoidal equation.

An illustration of the matrices is shown on Table 26. This illustrates the predicted growth rate between year N and N+1 for longitudinal cracking in thin asphalt pavements for three different environmental zones. In this case, the freeze-thaw cycles factor chiefly accounts for the varying growth-rate predictions.

The predictions from these deterioration matrices can be also illustrated in graphical form as shown in Figure 11, where the predicted growth in rutting on a hot mix pavement is given for three different traffic loading conditions.

By using the deterioration matrices, the maintenance and rehabilitation prediction routines, the decision trees, and the

decision criteria, it is possible to make predictions of the timing and type of maintenance and rehabilitation activities for each section in the state's network. Typical cases of such predictions are shown in the following chapter.

TABLE 25. Performance Equation Used by Pavement Type.

<u>Pavement Type</u>	<u>Performance Equations Used</u>
4	Hot Mix on Black Base (BB)
5	Hot Mix (HM) on Granular Base
6	Hot Mix (HM) on Granular Base
7	Overlay (OV)
8	Overlay (OV)
9	Overlay (OV)
10	Surface Treatment (ST)

TABLE 26. Predicted Growth of Longitudinal Cracking
in Differing Climatic Zones

% Extent of Distress in Year N	% Extent of Distress (Year N+1)		
	Zone 1	Zone 2	Zone 3
10	14	22	13
15	19	28	19
20	24	34	24
25	29	42	29
Thorntwaite Index	12	-21	-47
Air Freeze-Thaw Cycles	26	80	30
Avg. Max. Monthly Temp. °F	67	59	62
Average Soil PI	20	20	5

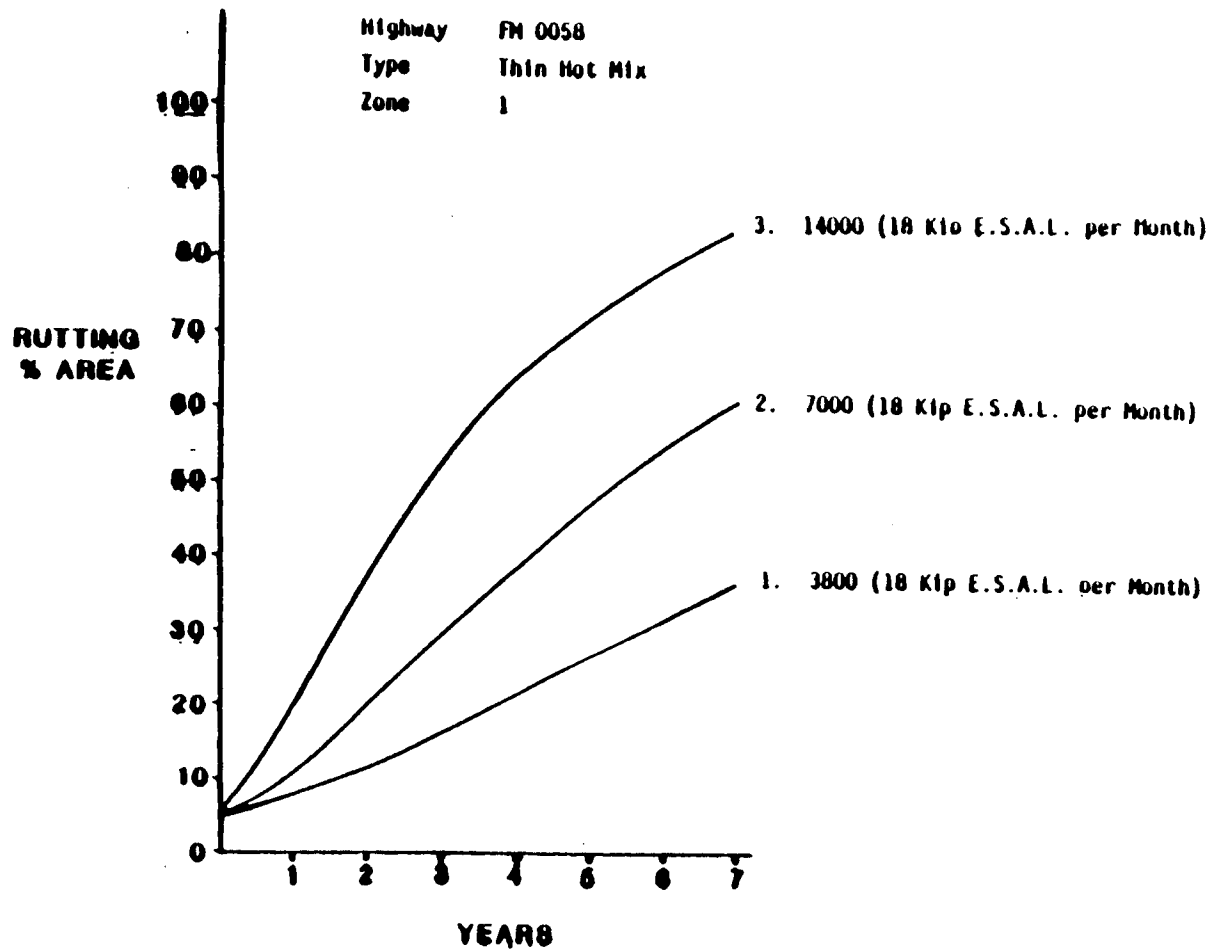
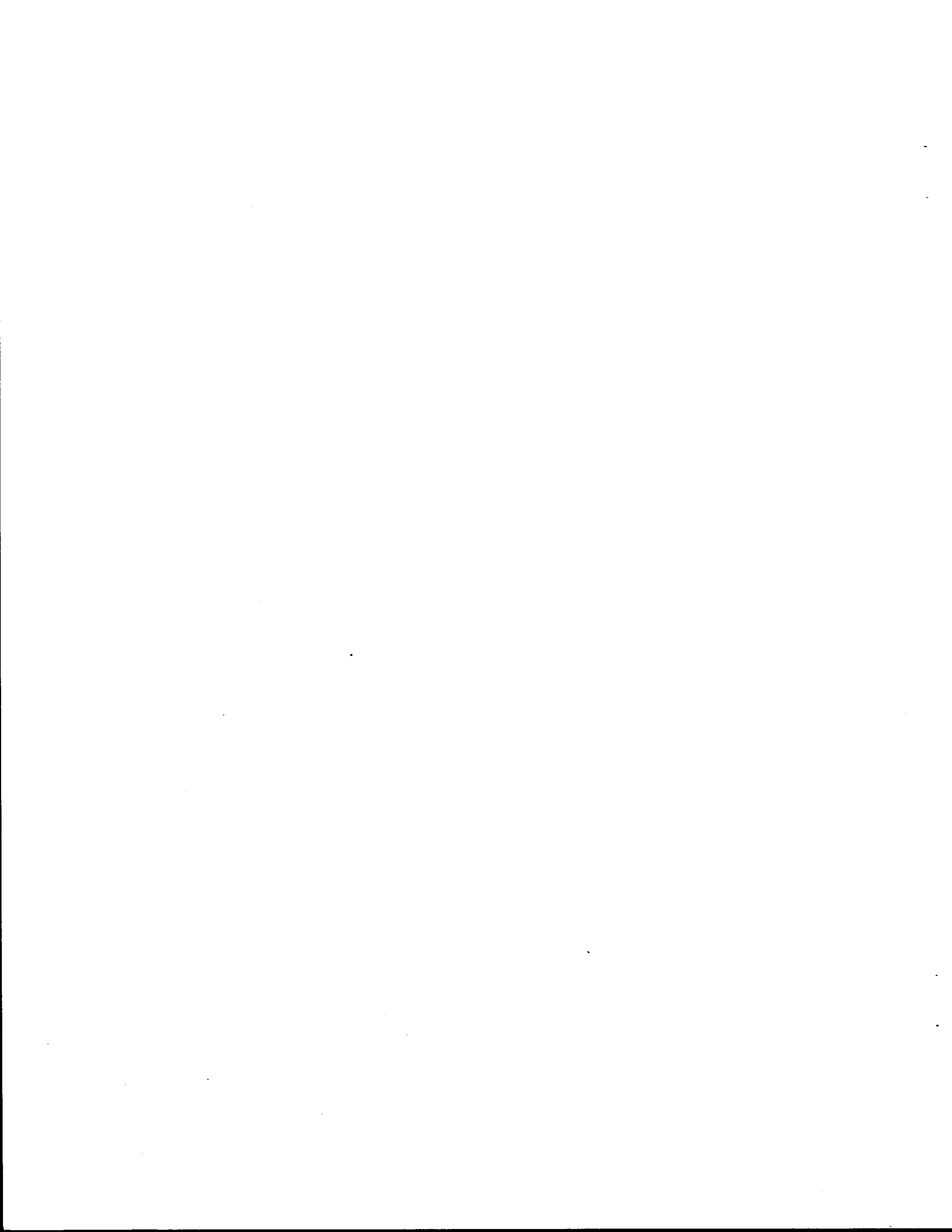


Figure 11. Predicted Growth in Rutting Area for Different Traffic Loadings



CHAPTER FIVE

EXAMPLE PROBLEM

To illustrate the calculation procedure, the data from a single 2-mile highway section will be processed. The information for the example is shown below.

Planning Horizon:	10 years
Maintenance Level:	65 (PES score when maintenance applied)
Rehabilitation Level:	40 (PES score when rehabilitation applied)
Highway:	FM 324
Milepost:	MP 0-2
District:	11
County:	3 (Angelina)
Functional Class:	4 (Collector)
Pavement Type:	8 (Overlaid Concrete Pavement)
ADT/Lane:	1850
18-kip ESAL(20 yrs):	2.65 million

The pavement was evaluated, and the distresses found in the section are shown in Table 27.

The mean Mays Ride value on this section was measured to be 1.6.

Pavement Score Calculation Procedure

Within the Pavement Evaluation System, the following scores are calculated.

1. Unweighted Visual Utility Score (UVU)

where

$$\begin{aligned} \text{UVU} = & (U_{\text{rutting}}) \times (U_{\text{raveling}}) \times (U_{\text{flushing}}) \times (U_{\text{failures}}) \\ & \times (U_{\text{alligator cracking}}) \times (U_{\text{longitudinal cracking}}) \\ & \times (U_{\text{transverse cracking}}) \end{aligned}$$

2. Adjusted Visual Utility Score (AVU)

where

$$\begin{aligned} \text{AVU} = & (U_{\text{rut}})^{b_1} \times (U_{\text{rav}})^{b_2} \times (U_{\text{flu}})^{b_3} \times (U_{\text{fail}})^{b_4} \\ & (U_{\text{allig}})^{b_5} \times (U_{\text{long}})^{b_6} \times (U_{\text{trans}})^{b_7} \end{aligned}$$

TABLE 27. Visually Observed Distresses for FM-342

Distress	Area Covered	As coded on Inspection Form
Slight Rutting	0	000
Severe Rutting	0	000
Raveling	0	000
Flushing	0	000
Failures	0	000
Alligator Cracking	6-25%	010
Longitudinal Cracking	100-199 lin. ft.	010
Transverse Cracking	4 per 100 ft	100

where the b values are environmental weighting factors dependent upon rainfall and freeze-thaw cycles. The values of b are defined in the main body of the report and the environmental factors are obtained from Tables 9 and 10.

3. Weighted Visual Utility Score (WVU)

where

$$WVU = AVU^{a_1}$$

where a_1 is a traffic associated weighting factor, as defined in the main body of the report.

4. Pavement Score (PSC)

where

$$PSC = [(AVU)^{a_1} \times (SIU)^{a_2} \times (SKU)^{a_3} \times (SCU)^{a_4}]^{1/FC}$$

where SKU (Skid Utility) and SCU (Structural Capacity Utility) are both set to 1.0. a_2 , a_3 , and a_4 are set to 1.0 and FC is a factor based on functional class.

For the data presented above for FM324 the following scores are calculated.

$$UVU = (1.00) \times (1.00) \times (1.00) \times (1.00) \times (0.53) \times (0.99) \times (0.71) = 0.40$$

the individual utility values being obtained from formulas (13) to (20). The rainfall and freeze-thaw values for this county are 30 in./yr and 26 cycles/yr, respectively, therefore from Table 10, RF = 0.97 and Table 11, FF = 0.973.

therefore

$$AVU = (1.00)^{1.06} \times (1.00)^{1.06} \times (1.00)^{1.06} \times (1.00)^{1.06} \times (0.53)^{1.06} \times (0.99)^{1.06} \times (0.71)^{1.06} = 0.35$$

From Tables 12 and 13

$$\begin{aligned} a_1 &= \frac{1}{ADTF \times EALT} = \frac{1}{0.92 \times 1.0} \\ &= 1.087 \\ WVU &= (0.35)^{1.087} \\ &= 0.321 \end{aligned}$$

From the SIUC equation for an ADT x Speed = 101,750

$$SIU = -0.5583 + 0.58333 (1.6)$$

$$= 0.375$$

$$PSC = (0.321 \times 0.375 \times 1.00 \times 1.00)^{1/0.95}$$

$$= 0.108$$

When these value are presented in the PES outputs, the scores are rounded and multiplied by 100. For this section of FM 324, the following scores would be reported.

$$UVU = 40$$

$$AVU = 35$$

$$WVU = 32$$

$$PSC = 11$$

Calculating the Appropriate Funding Level

The current pavement score for this section is 0.11. This is below the minimum acceptable of 0.40 (Table 17), therefore a rehabilitation funding level would be calculated.

The first step in calculating the funding level is to determine the final pavement score after each funding strategy (R-1, R-2, or R-3 for surface treated pavements).

Calculating final AVU for Strategy R-1. For each distress utility value the final utility value is determined using the following equation.

$$U_{\text{final}} = U_{\text{initial}} + (1 - U_{\text{initial}}) \times G$$

where G is the % gain factor obtained from Table 14 where U_{final} has a maximum value of 1.0.

The calculation of the final AVU for strategy R-1 on FM 324 is shown below.

Distress	$U_{initial}$	G from Table 14	U_{final} after R-1
Rutting < 1"	1.000	33	1.000
Raveling	1.000	100	1.000
Flushing	1.000	100	1.000
Failures	1.000	25	1.000
Alligator Cracking	0.530	60	0.824
Longitudinal Cracking	0.990	60	0.996
Transverse Cracking	0.710	75	0.928

$$AVU_{final} = (1.000)^{1.06} \times (1.000)^{1.06} \times (1.000)^{1.06} \times (1.000)^{1.06} \times (0.824)^{1.06} \times (0.996)^{1.06} \times (0.928)^{1.06}$$

$$= .745$$

Final PSI = 1.8 from Table 15

$$SIU_{final} = 1.00$$

$$PSF = ((.745)^{1.087} \times 0.783 \times 1.00 \times 1.00)^{1/0.95}$$

$$= 0.338$$

for strategy R-2

$$PSF = ((.899)^{1.087} \times 1.00 \times 1.00 \times 1.00)^{1/0.95}$$

$$= 0.885$$

for strategy R-3

$$PSF = ((1.00)^{1.087} \times 1.00 \times 1.00 \times 1.00)^{1/0.95}$$

$$= 1.000$$

Calculation of T_{max} (Time Until Next Rehabilitation).

$$T_{max} = T_c + \frac{PSF - PSM}{DS}$$

PSM = 0.40 from Table 17

$$DS = (DSI)(TF)(CF)(SF)$$

DSI is obtained from Table 20

TF is obtained from Table 19

CF is obtained from Table 21

SF is obtained from Table 22

$$DS = 0.100 \times 1.00 \times 1.00 \times 1.00 \times 1.00 = 0.100$$

$$R-1 \quad T_{\max} = 0 + \frac{0.338 - 0.40}{0.100} = -0.2 \text{ years}$$

$$R-2 \quad T_{\max} = 0 + \frac{0.885 - 0.40}{0.100} = 4.85 \text{ years}$$

$$R-3 \quad T_{\max} = 0 + \frac{1.000 - 0.40}{0.100} = 6.00 \text{ years}$$

Calculation of T_{min} (Minimum Allowable Time).

$$T_{\min} = T_{\text{mini}} \times TF$$

$$T_{\text{mini}} \text{ (from Table 18)} = 5.0$$

$$TF \text{ (from Table 19)} = 1.0$$

$$T_{\min} = 5.0 \times 1.0 = 5.0 \text{ years}$$

Funding Strategy Selection. Select first strategy such that

$$T_{\max} > T_{\min}$$

$$R-1 \quad T_{\max} = -0.2 \quad T_{\min} = 5.0$$

$$R-2 \quad T_{\max} = 4.52 \quad T_{\min} = 5.0$$

$$R-3 \quad T_{\max} = 6.00 \quad T_{\min} = 5.0$$

Therefore, R-3 would be selected for this highway. This is a 2 1/2-inch thick ACP overlay.

Aging the Pavement.

To age the pavement through time, the iterative process developed in the last part of Chapter Four is used.

1.- Calculate ρ 's and β 's . The constants ρ and β are calculated for each distress that affects the section of road. These distresses will in turn be used to construct the deterioration matrix that will enable the deterioration of the pavement.

Appendix A gives ρ and β formulas for Alligator Cracking on an Overlay pavement as follows:

ρ and β for Alligator Cracking:

$$\rho = - 0.0159 * FTC + 0.0082 * AVT - 0.0121 * PI + 0.0162 * OVTH + 0.145 * HPR2 - 0.0135 * HPR3$$

where,

FTC = Freeze-Thaw Cycles/yr

AVT = Average Temperature

PI = Plasticity Index

HPR2 = Equivalent Thickness X Elastic Modulus of the Subgrade

The Equivalent Thickness is assumed based on pavement type and the Elastic modulus is assigned based on climatic region. Once the FWD is incorporated into PES (planned for 1987) then project specific estimates can be made.

$$HPR3 = 10^{10}/HPR2$$

$$\rho = - 0.0159 * 26 + 0.0082 * 67 - 0.0121 * 20 + 0.0162 * 3 + 0.145 * 30 - 0.0135 * 2$$

$$\rho = 4.2631$$

$$= 0.0185 * XTI + 0.171 * HPR3$$

where,

XTI = Thornthwaite Index + 50.0

$$\beta = 0.0185 * 62 + 0.171 * 2$$

$$\beta = 1.4834$$

ρ and β for Longitudinal Cracking:

$$\rho = 135.08$$

$$\beta = 2.3006$$

ρ and β for Transverse Cracking:

$$\rho = 154.60$$

$$\beta = 0.99673$$

2.- Construct the Deterioration Matrix for the Pavement Section. The deterioration matrix is developed in an iterative process in which the basic S-shape performance equation is used to find W given g .

The applicable relationship, equation (7)

$$g = e^{-\left(\frac{\rho}{W}\right)^\beta}$$

where

g = percent area of distress

W = accumulated 18-kip loads, time, or weather cycles

ρ and β = regression equations for each specific distress and pavement type

a. Given g (% area), find W .

$$W = \frac{\rho}{(-\ln g)^{1/\beta}}$$

$$W = \frac{4.2631}{[-\ln(0.01)]^{1/1.4834}}$$

$$W = 1.5227$$

b. Increase W by 1 year.

W is a load expressed in N18 kips/month therefore compute

$$\begin{aligned} N(\text{months}) &= W * 1000 * 240 / \text{EALT} \\ \text{EALT} &= 20 \text{ year } 18 \text{ kip ESAL (in thousands)} \end{aligned}$$

$$= 1.5227 * 1000 * 240 / 2652$$

$$= 137.8 \text{ months}$$

$$\begin{aligned}
N &= N + 12 \\
&= 137.8 + 12 \\
&= 149.8 \\
W &= \frac{N * EALT}{240 * 1000} \\
&= \frac{149.8 * 2652}{240 * 1000} \\
&= 1.6553
\end{aligned}$$

c. Find g for next year given W.

$$\begin{aligned}
y &= e^{-\left(\frac{3}{W}\right)^3} \\
g &= e^{-\left(\frac{4.2631}{1.6553}\right)^{1.4834}}
\end{aligned}$$

$$g = 0.0171 = 1.71 \text{ (Alligator Cracking at } t+1)$$

For this reason the alligator cracking is calculated to increase from 1.0% to 1.7% in one year.

d. Generate Deterioration Matrix. The previous calculation procedure is followed year-by-year, distress-by-distress until a table, such as Table 28, is complete.

Predicting Long Term Funding Requirements

After the deterioration matrix has been built, the analysis over the planning horizon begins. It is assumed that after a major rehabilitation the time for constant level of service (i.e. time that the section will be in top condition) is 3 years. Thus, during the first three years after rehabilitation nothing happens to the section of road but loss of serviceability due to traffic. This change is minimal and does not affect the overall score of the section. Typically, at year five after rehabilitation, the distresses begin to appear and the score changes in the following way.

$$\begin{aligned}
1989 \quad UVUC &= (1.00)(1.00)(1.00)(1.00)(.972)(1.00)(1.00) \\
&= .972 \\
AVUC &= (1.00) \begin{matrix} 1.06 \\ (1.00) \end{matrix} \begin{matrix} 1.06 \\ (1.00) \end{matrix} \begin{matrix} 1.06 \\ (1.00) \end{matrix} \begin{matrix} 1.06 \\ (1.00) \end{matrix} \begin{matrix} 1.06 \\ (.972) \end{matrix} \\
&\quad (1.00)1.06(1.00)1.06
\end{aligned}$$

TABLE 28. Deterioration Matrix for FM 324, MP 0-2

% Area Now	% Area Next Year		
	Alligator Cracking	Longitudinal Cracking	Transverse Cracking
1.00	1.71	3.90	3.27
2.00	2.94	5.93	4.88
3.00	4.17	7.85	6.07
4.00	5.35	9.44	7.34
5.00	6.52	10.57	8.66
6.00	7.63	12.34	9.67
7.00	8.65	13.57	10.70
8.00	9.71	14.83	12.90
9.00	10.80	16.12	13.14
10.00	11.76	17.43	13.84
.	.	.	.
.	.	.	.
.	.	.	.
100.00	100.00	100.00	100.00

$$= .966$$

$$SIU = 1.00$$

$$PESC = .966$$

1990 The increase of the distresses is shown below (from Table 28).

<u>Distress</u>	<u>t = 1989</u>	<u>t = 1990</u>
Alligator Cracking	1.71 = 2%	2.94%
Longitudinal Cracking	3.90 = 4%	9.44%
Transverse Cracking	3.27 = 4%	7.34%
SI	3.5	3.3

$$UVUC = (1.00)(1.00)(1.00)(1.00)(.896)(.975)(.989)$$

$$= .864$$

$$AVUC = (.890)(.963)(.988)$$

$$= .847$$

$$SIU = 1.00$$

$$PESC = .850$$

1991 PESC = 64

Maintenance Decision Tree In 1991 the score falls into the area where preventive maintenance is needed. Thus, a maintenance schedule has to be recommended. This is done by using the decision tree for composite pavements (Table 29). It can be seen that for the distresses that are affecting the pavement, maintenance strategy 12 (spot seal) is recommended. For the maintenance strategy the final utility value is determined using the following equation.

$$U_{final} = 1 - [U_{initial} - U_{initial}] * [\text{Max gain}]$$

where maximum gain is the % gain factor for the maintenance strategy obtained from Table 16 where U_{final} has a maximum value of 1.00.

Table 29: Selection Maintenance Strategy, Pavement Type 8

Overlay (concrete)

Performance Equation: Overlay
PES RATING

Distress	Traffic	PES Rating		
		100	010	001
Slight Rutting	LL	9	9	9
	LH	9	9	10
	HL	9	9	9
	HH	9	9	10
Severe Rutting	LL	9	9	10
	LH	13	13	14
	HL	9	9	14
	HH	13	14	14
Raveling	LL	4	4	5
	LH	4	4	5
	HL	4	6	6
	HH	4	6	6
Flushing	LL	0	5	5
	LH	0	5	5
	HL	5	6	6
	HH	6	6	6
Failures	LL	3	3	3
	LH	3	3	3
	HL	3	3	3
	HH	3	3	3
Alligator Cracking	LL	12	12	5
	LH	12	12	6
	HL	12	5	5
	HH	12	5	6
Longitudinal Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6
Transverse Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6

<u>Distress</u>	<u>initial</u>	<u>Max Gain from Table 16</u>	<u>U_{final} after M-12</u>
Rutting	0	0	
Raveling	0	.5	
Flushing	0	.5	
Failures	0	0	
Alligator Cracking	0.91	.5	.95
Longitudinal Cracking	0.84	.15	.86
Transverse Cracking	0.88	.15	.90
SI	3.3	0	3.3

$$\begin{aligned}
 AVU &= (1.00)^{1.06} (1.00)^{1.06} (1.00)^{1.06} (1.00)^{1.06} (.95)^{1.06} \\
 &\quad (.88)^{1.06} (.90)^{1.06} \\
 &= .75
 \end{aligned}$$

$$SIUC = 1$$

$$PESC = 75$$

The final schedule for FM324 for the 10 year period is shown in Table 30. Although the computation process is long and involved the results obtained, shown in Table 30, appear to be reasonable. The pavement under analysis was a composite (asphalt over concrete). It was predicted to require an immediate 2 1/2 inch overlay, followed by a crack seal in year 8 and crack seal and seal coat in year 10.

TABLE 30. Rehabilitation and Maintenance Schedule for FM 324

<u>Year</u>	<u>PESM</u>	<u>PES</u>	<u>Strategy</u>	
1984	40	19	R-03	Medium Overlay
1985	40	100		
1986	40	100		
1987	40	100		
1988	40	100		
1989	40	96		
1990	40	84		
1991	40	64	M-12	Seal Coat
1992	40	75		
1993	40	57	M-1, M-12	Seal Cracks, Seal Coats
1994	40	98		

CHAPTER SIX

SENSITIVITY ANALYSIS AND CASE STUDIES

Sensitivity Analysis

A limited sensitivity analysis has been performed using data obtained from the 1983 State survey. The analysis was directed to assess the response of the program to changes in a) maintenance level, b) traffic load, and c) climate.

Seven sections of road from District 11 (Lufkin) were selected for the analysis. These sections of road were selected according to pavement type, functional class, traffic, and load. Specific information about the sections of road are given in Table 31.

Maintenance Levels Sensitivity Analysis. To examine the effect of the minimum allowable pavement score, before maintenance has to be applied, upon the selection of funding requirements, five levels (60, 65, 70, 75, 80) of minimum score were analyzed. Table 32 shows the results of maintenance and rehabilitation costs for all the sections at different minimum allowable score levels for a planning horizon of twenty years.

As can be observed, the maintenance cost appears to be inversely proportional to the minimum score, and the rehabilitation cost directly proportional to the minimum score. This relationship is due to the fact that when the minimum allowable score is high, maintenance would have to be done so frequently that it is more cost effective to do a rehabilitation which will last longer at a high score. On the other hand, when the minimum score is low, the percent of distress of a section increases to a higher level, causing a need for a more extensive and correspondingly more expensive maintenance strategy. However, such maintenance will be required less frequently and thus is more cost effective than a rehabilitation strategy. A level between 70 and 75 minimum allowable score was found to be the most economical for this small data set. This cost was compared to the cost incurred by not having preventive maintenance strategies when the pavement falls below an acceptable score of 45. It was found that the cost of maintaining the road at a level between 70 and 75 will be less expensive than to let the road fall to an unacceptable level of less than 45 and then rehabilitate (Table 32).

Sensitivity to Traffic Loading. To examine the effect of traffic loading, sections of road corresponding to each of the four performance equations (Black Base, Hot Mix, Overlay, Surface Treated) were analyzed with their actual traffic loadings. They were then re-analyzed with one half and double the actual loadings. Figures 11 to 22 show the rehabilitation and maintenance cycles for each of the twelve cases. Also, Table 33 shows the maintenance and rehabilitation

TABLE 31. Information on Selected Pavement Sections

Highway	Pavement Type	Functional Class	Pavement Score	Average Daily Traff	18 kip per day x 10 ⁶
SH 63	4	4	54	4,200	3.8
SH 287	5	5	67	2,100	2.7
FM 58	6	5	44	1,800	1.6
FM 324	7	4	40	2,800	2.5
FM 324	8	4	28	3,100	2.9
SH 103	9	4	65	6,000	5.4
FM 324	10	5	62	2,200	2.1

TABLE 32. Maintenance, Rehabilitation, and Total Cost at Different Minimum Allowable Utility Scores

Minimum PES Score	Maintenance Cost	Rehabilitation Cost	Total Cost
80	321,214	1,180,000	1,501,214
75	326,319	1,066,600	1,392,919
70	334,761	1,032,600	1,367,361
65	375,240	1,032,600	1,407,840
60	395,107	1,032,600	1,427,707

costs for the sections at the different traffic levels. As the traffic loading is increased, the predicted total cost also increases.

Sensitivity to Climatic Conditions. To examine the effect of climatic conditions on pavement life, sections of road corresponding to each of the four performance equations (Black Base, Hot Mix, Overlay, Surface Treated) were analyzed with their actual traffic loadings for three different climatic zones, Districts 21 (Dry, No Freeze), 19 (Wet, Freeze), and 4 (Dry, Freeze-Thaw cycling). Table 34 shows the maintenance and rehabilitation costs for the sections at the different climatic conditions. Higher total costs were observed for the wet and freeze climatic zone (District 19) than for the other two zones. This difference is due to the fact that the regression equations that predict pavement deterioration rates are sensitive to district rainfall. The problem is further increased by the thermal cracking which is a function of freeze-thaw cycles.

Case Studies

Predicting Funding Needs for a Single County. The program has been used to predict the funding requirements for several counties. Typical results for Angelina County in East Texas are shown in Table 35. The rehabilitation costs are for medium and thick overlays and reconstruction. Note that in this county there is a large backlog of roads in very poor condition and hence the high first year rehabilitation costs. The decision criteria used to generate these results are those given in Table 2. However, varying the criteria in Table 2, the consequences of delaying preventive maintenance can be observed. With the existing criteria, preventive maintenance is initiated with a pavement score of 75 (low distress). Table 36 illustrates the consequences of delaying preventive maintenance.

In this table, Criteria A are as shown in Table 2, Criteria B involves delaying preventive maintenance until moderate levels of distress exist (pavement score less than 50), and Criteria C involves delaying maintenance and rehabilitation until extensive distress exists (pavement score less than minimum allowable score for rehabilitation).

As would be anticipated, the predicted rehabilitation costs increase as the preventive maintenance work is delayed. However, the predicted total cost increases from \$1.78 million per year to \$2.35 million per year as the maintenance work is delayed.

A further analysis performed with the Angelina County data was to study the effects of traffic loadings on predicted maintenance and rehabilitation cost estimates. Results of this analysis are shown in Table 37. As the traffic loading on the pavement is increased, the predicted total cost also increases. The true results are even more dramatic since the Rehabilitation cost figure for each traffic level

**TABLE 33. Maintenance and Rehabilitation Costs
at Different Traffic Load Levels**

Traffic Level	Maintenance Cost	Rehabilitation Cost	Total Cost
Half Traffic	345,895	608,000	953,895
Normal Traffic	326,319	1,104,600	1,430,919
Double Traffic	302,110	1,182,600	1,484,710

BLACK BASE Half Traffic

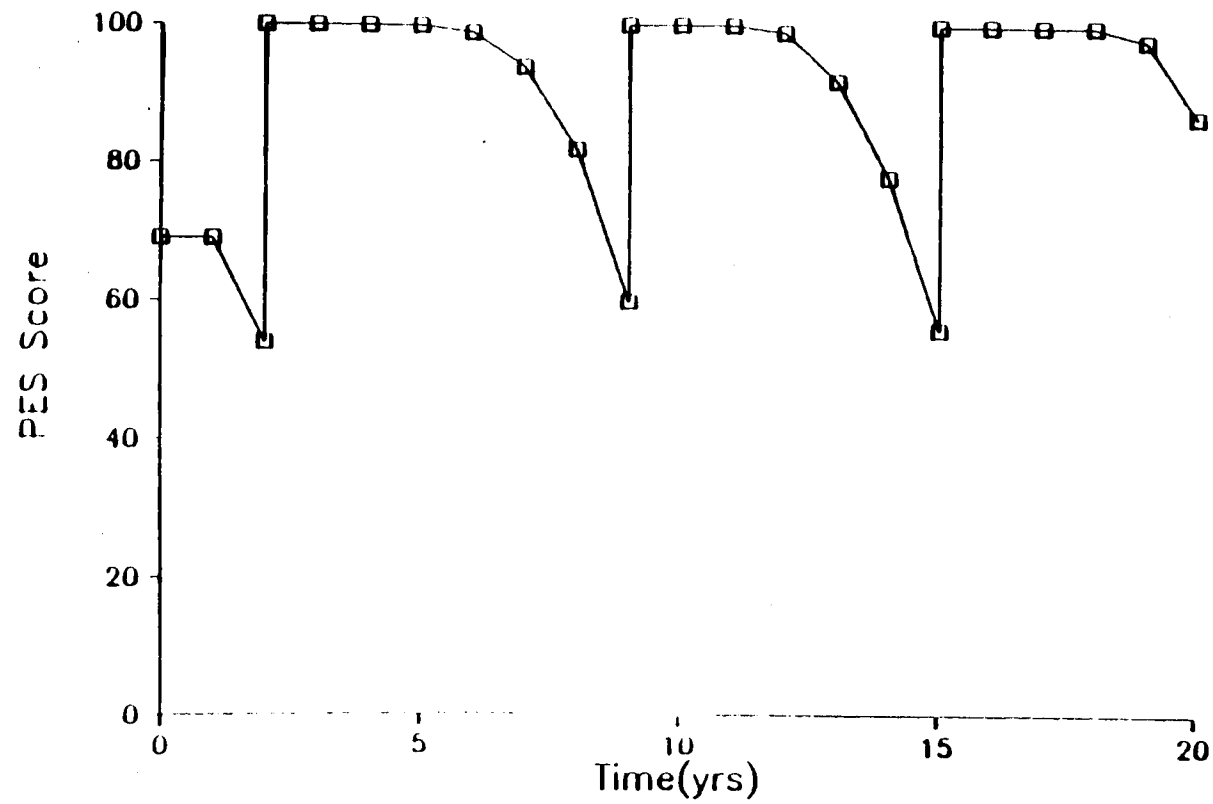


Figure 12. Rehabilitation Cycle for Black Base Pavement, Half Traffic Load.

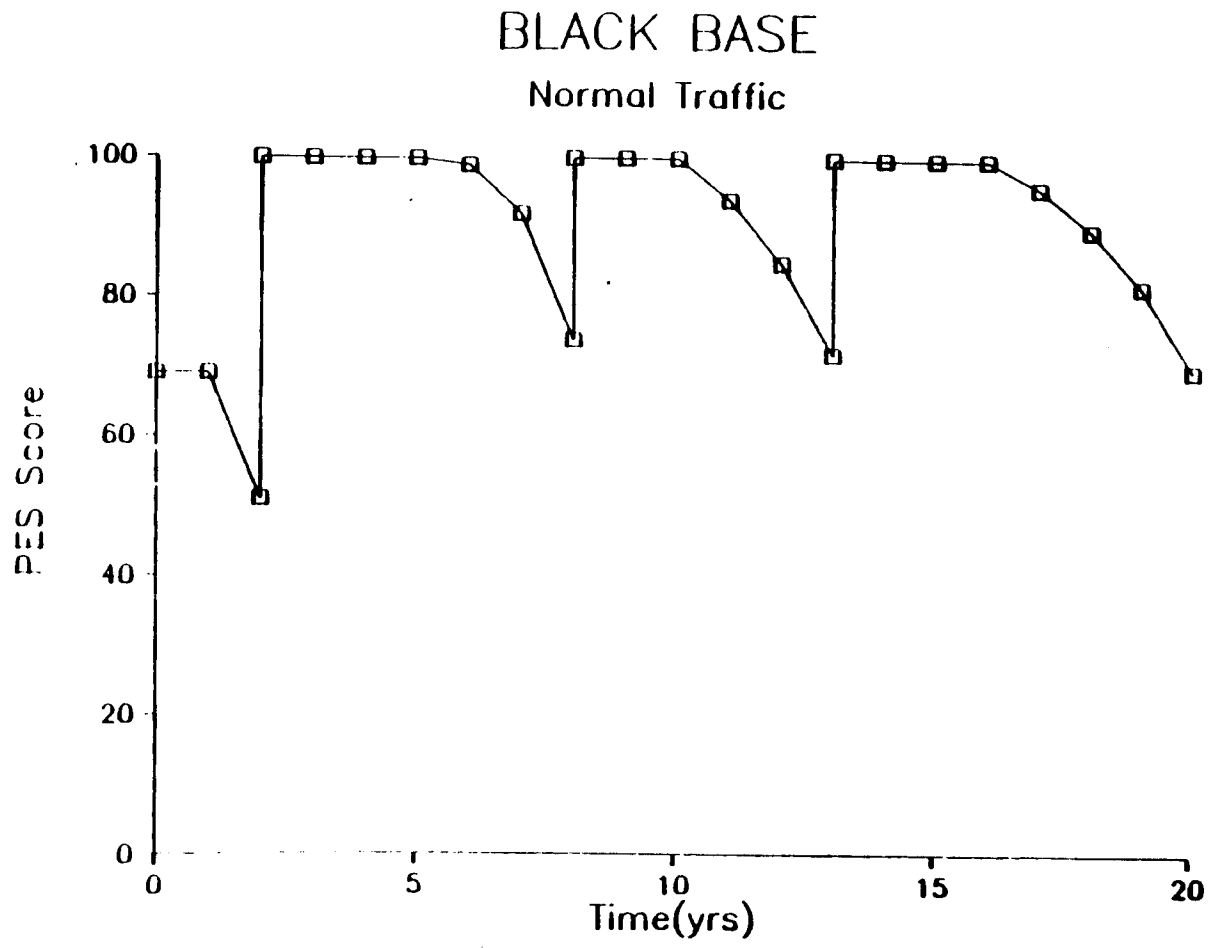


Figure 13. Rehabilitation Cycle for Black Base Pavement, Normal Traffic Load.

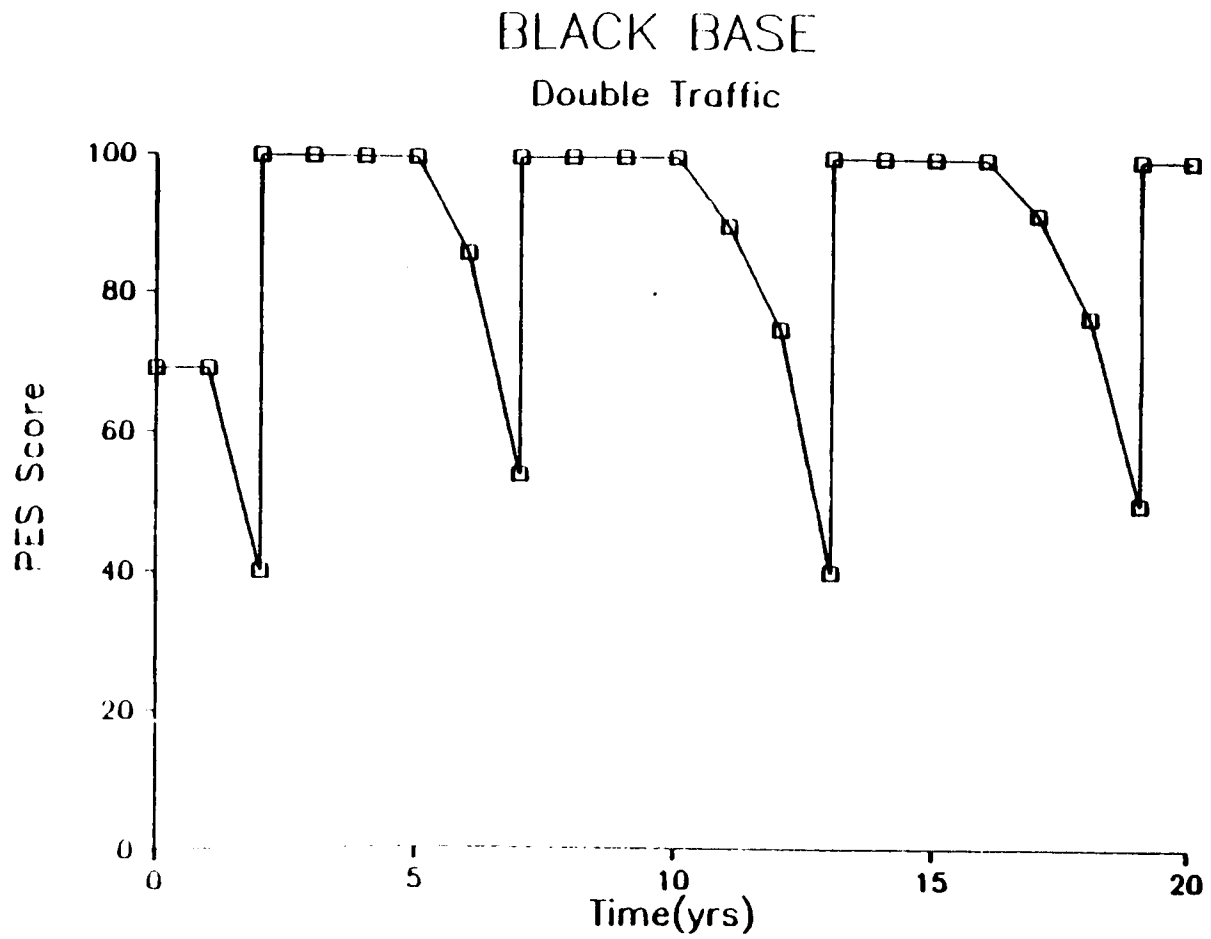


Figure 14. Rehabilitation Cycle for Black Base Pavement, Double Traffic Load.

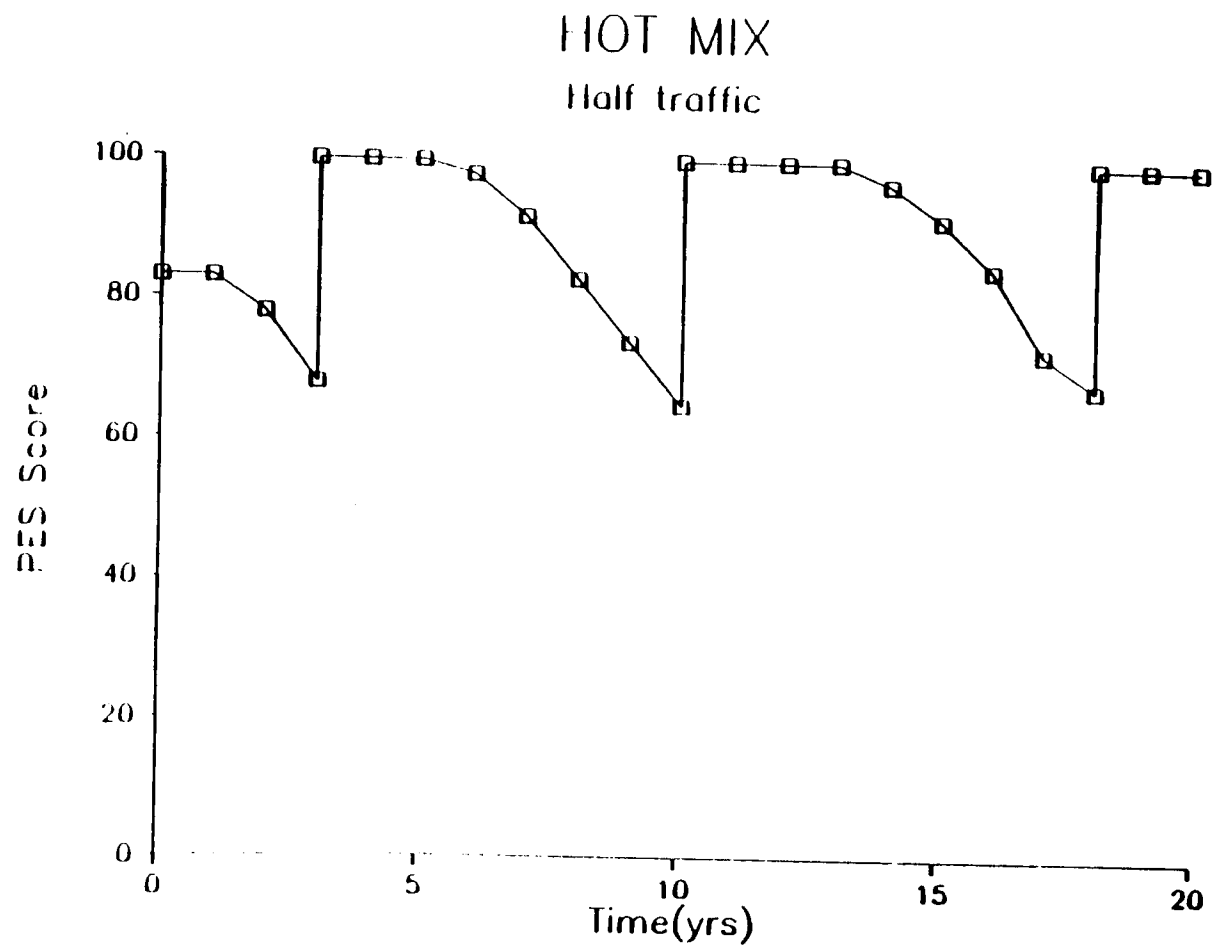


Figure 15. Rehabilitation Cycle for Hot Mix Pavement, Half Traffic Load

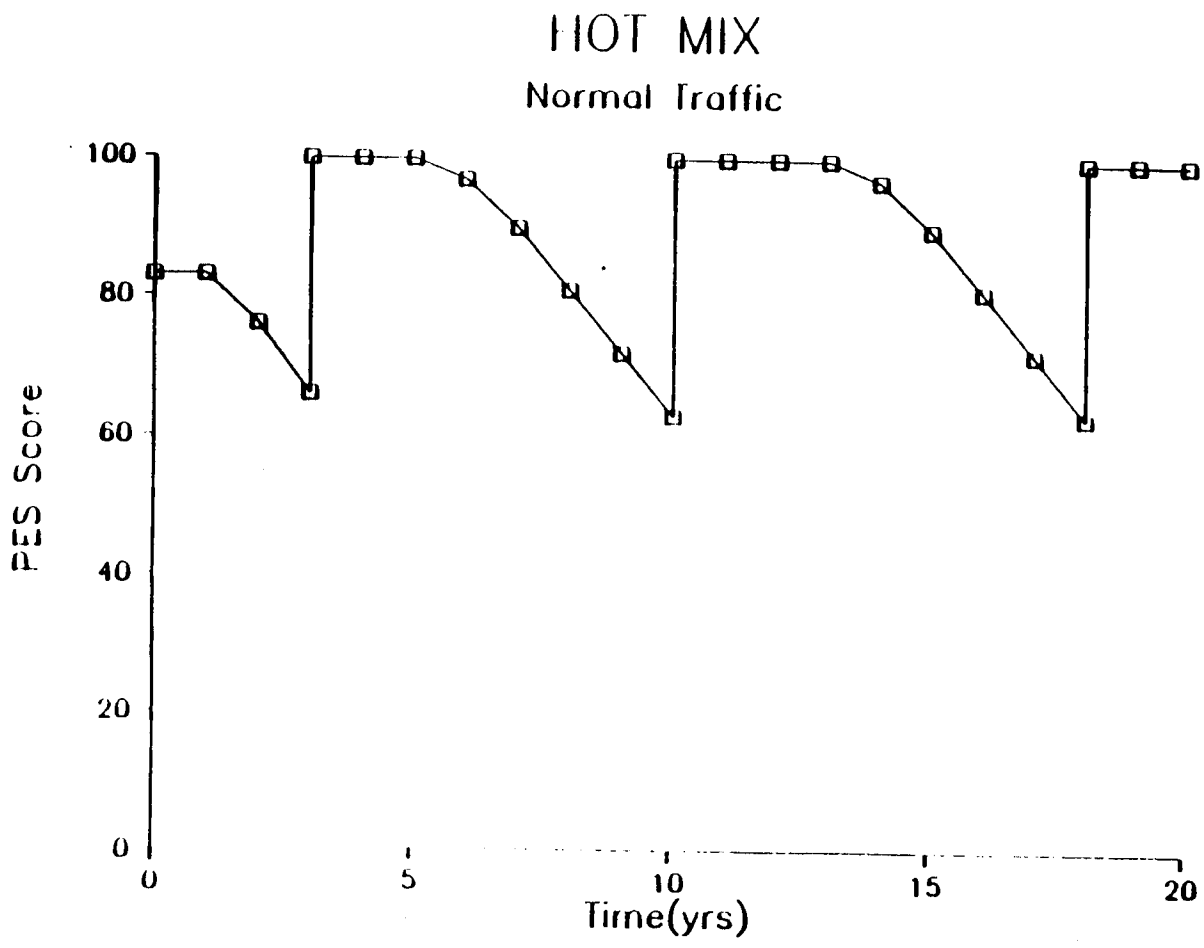


Figure 16. Rehabilitation Cycle for Hot Mix Pavement, Normal Traffic Load

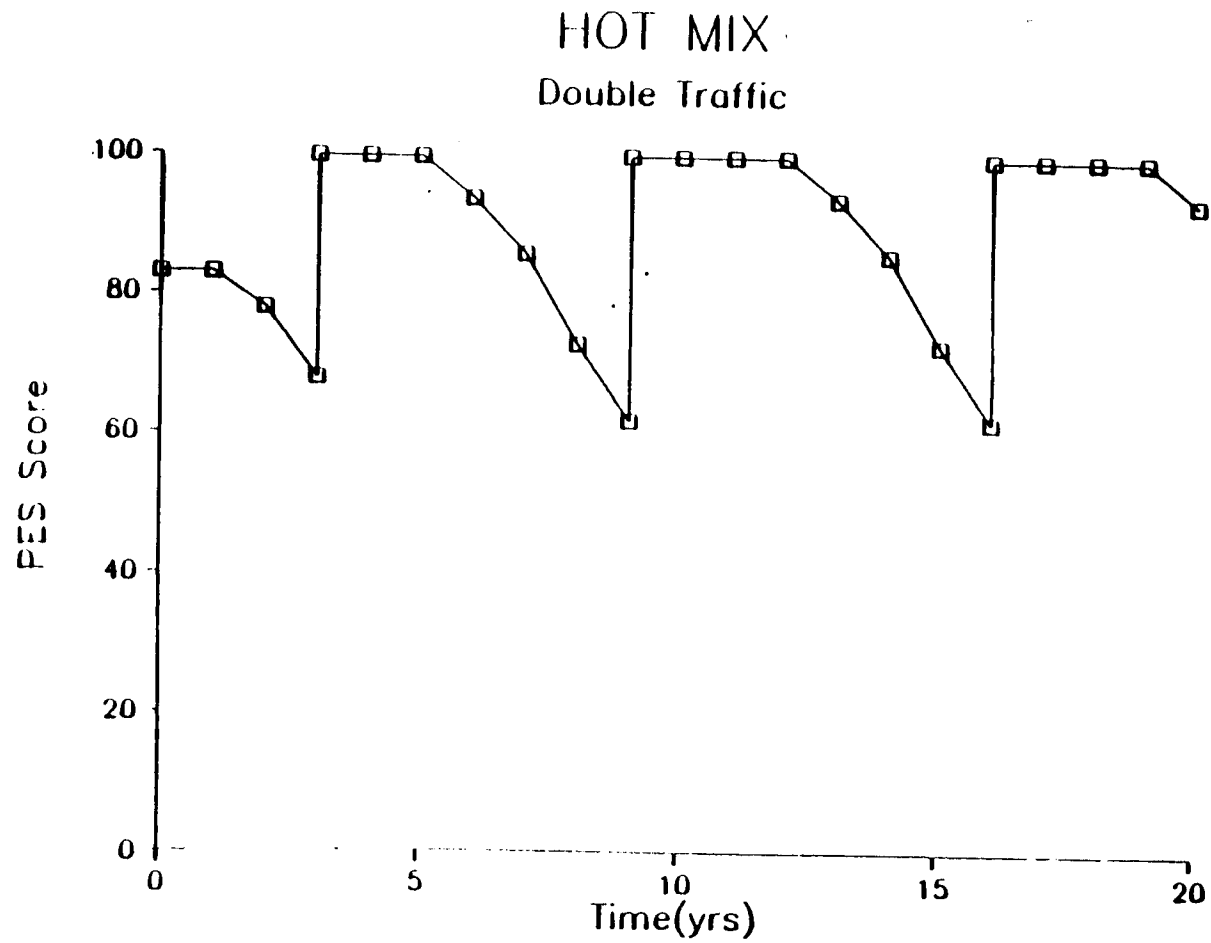


Figure 17. Rehabilitation Cycle for Hot Mix Pavement, Double Traffic Load

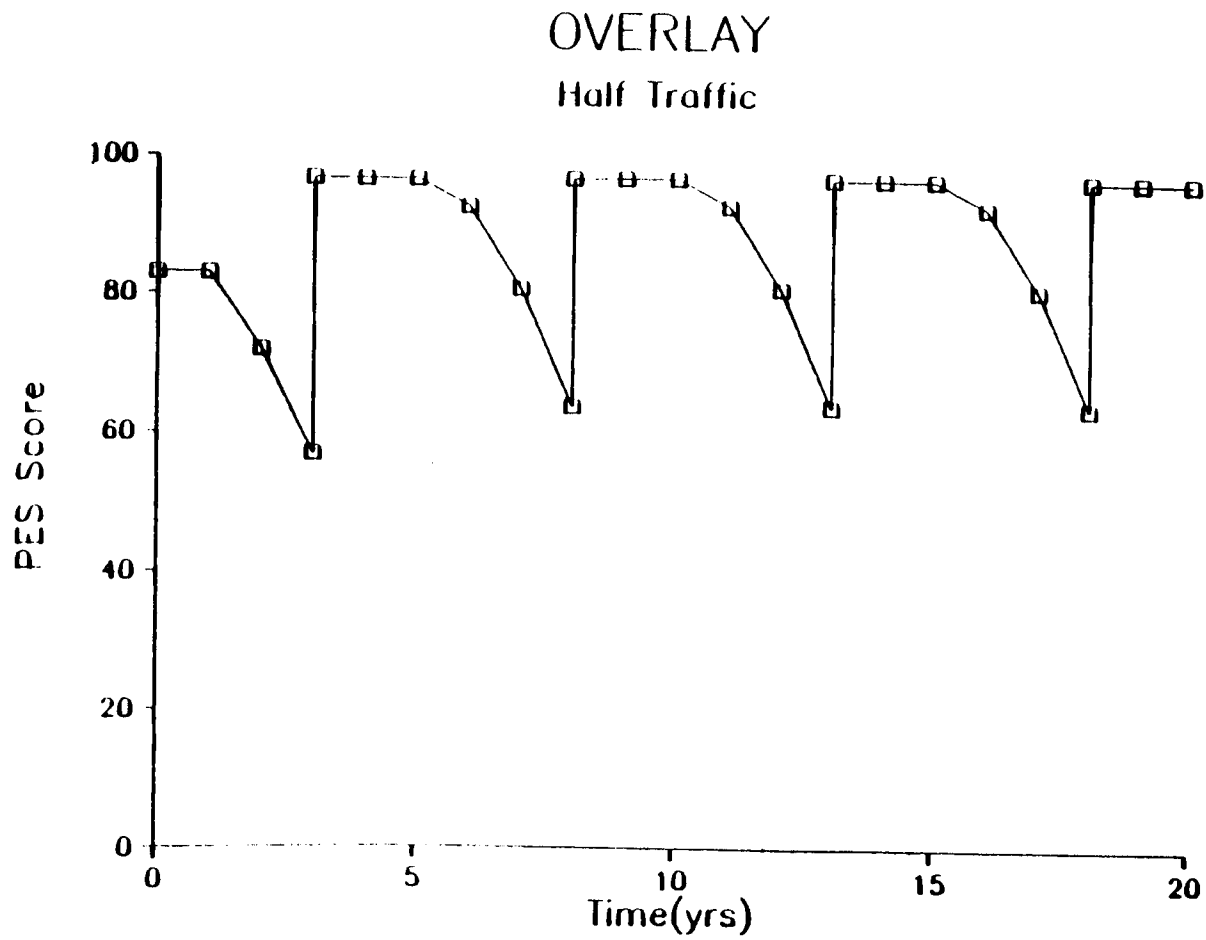


Figure 18. Rehabilitation Cycle for Overlays, Half Traffic Load

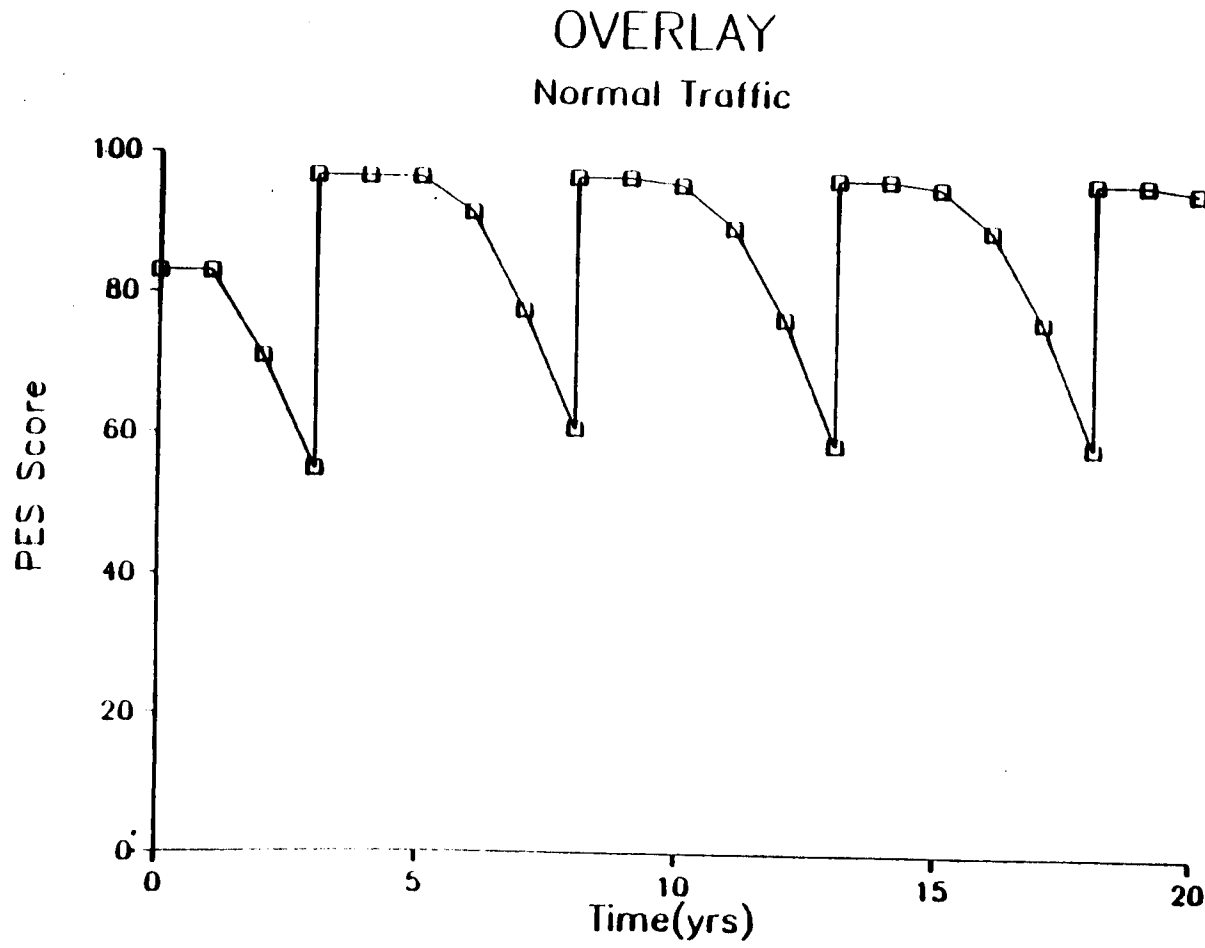


Figure 19. Rehabilitation Cycle for Overlays, Normal Traffic Load

OVERLAY

Double Traffic

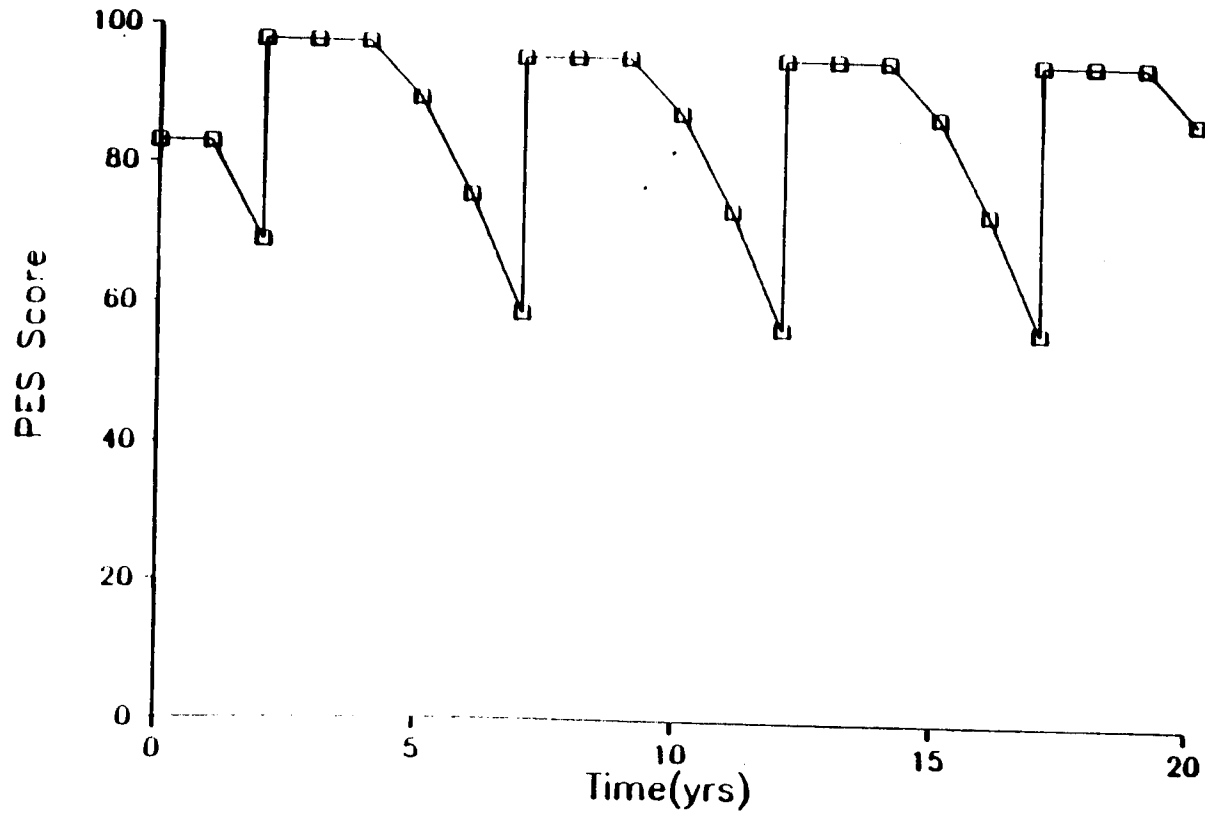


Figure 20. Rehabilitation Cycle for Overlays, Double Traffic Load

SURFACE TREATMENT

Half Traffic

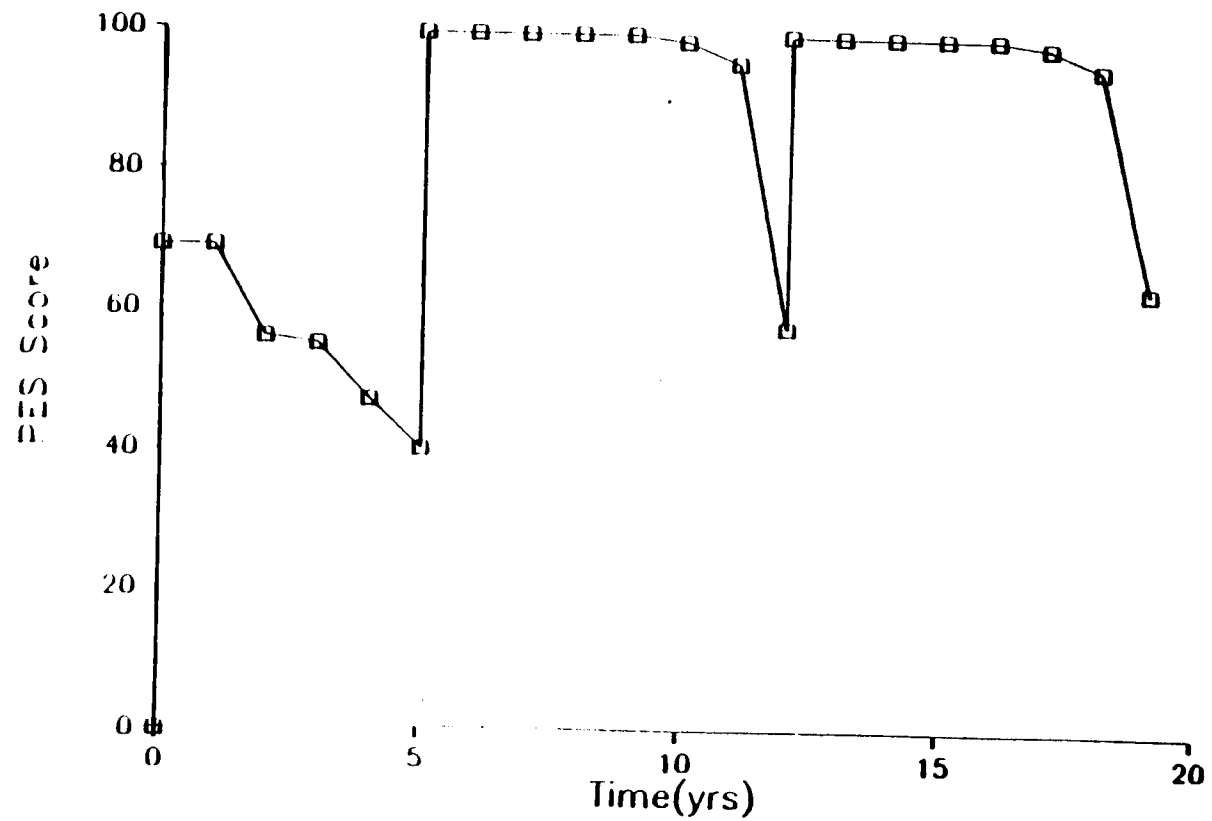


Figure 21. Rehabilitation Cycle for Surface Treated Pavements, Half Traffic Load

SURFACE TREATMENT

Normal Traffic

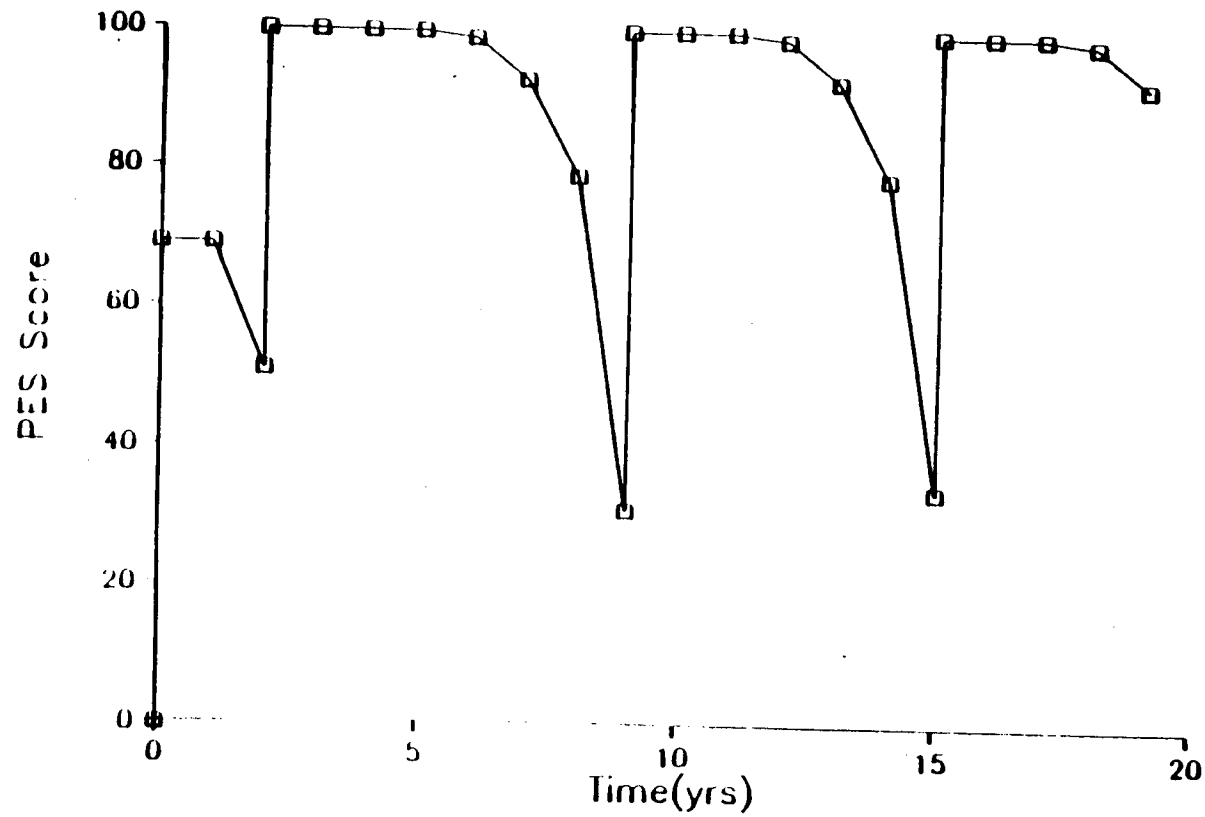


Figure 22. Rehabilitation Cycle for Surface Treated Pavements, Normal Traffic Load

SURFACE TREATMENT

Double Traffic

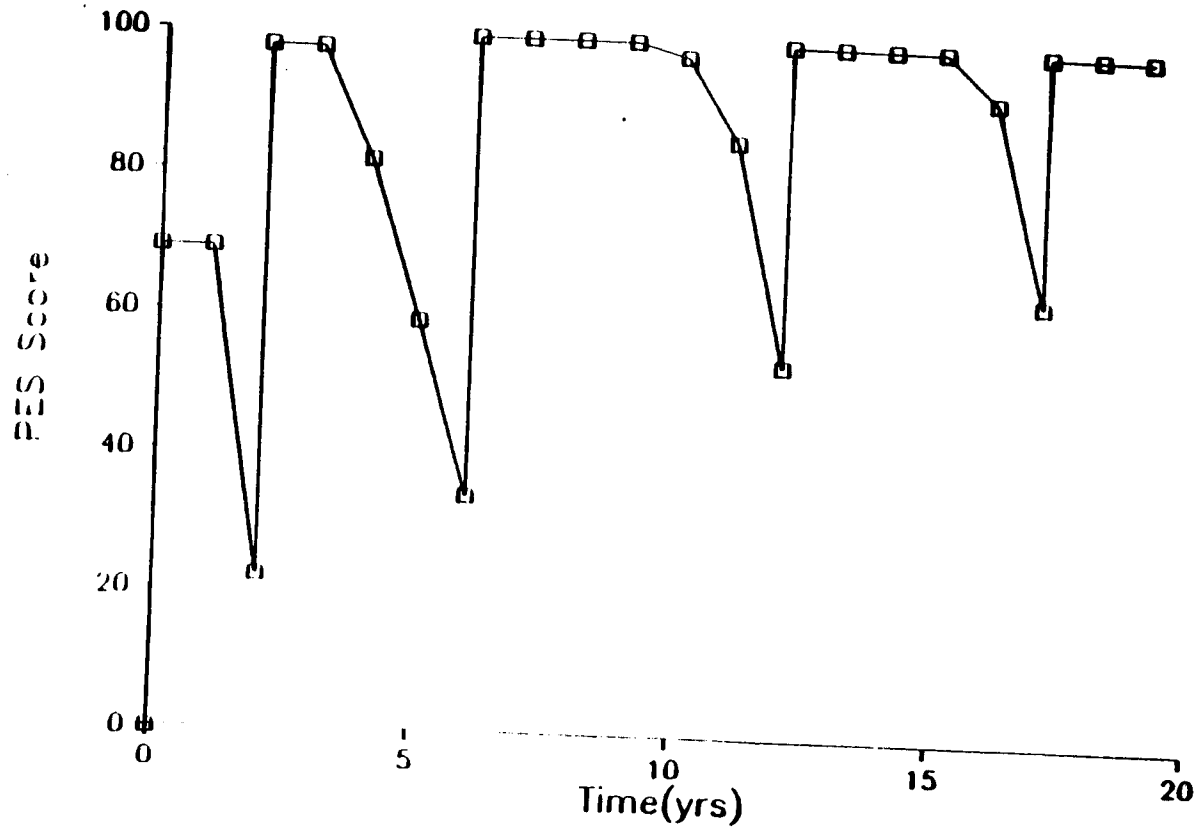


Figure 23. Rehabilitation Cycle for Surface Treated Pavements, Double Traffic Load

includes the large first year figure required to eliminate the backlog of poor pavements.

Predicting Funding Needs for a Single District. The program was used to predict the funding requirements for the low volume Farm to Market roads in District 11. The Farm to Market network in District 11 consists of 875 sections, each approximately two miles long.

Five runs were made with the program using different decision criteria. The runs were made with the following scenarios:

- 1.- No maintenance, and minimum pavement score level of 40.
- 2.- No maintenance, and minimum pavement score level of 60.
- 3.- Maintenance and rehabilitation levels of 40.
- 4.- Maintenance and rehabilitation levels of 60.
- 5.- Maintenance level of 75, and rehabilitation level of 40.

Table 38 gives the results of the runs for a five year analysis period.

As can be anticipated, the total cost is higher for the scenarios where no preventive maintenance is allowed (Runs 1 and 2). Furthermore, the difference in costs is more obvious when different levels of maintenance and rehabilitation are selected (Run 5). The difference in total cost between Runs 2 and 5 is of 55.75 million dollars in five years which can be translated to up to 49% savings in the same period of time using the proposed rehabilitation and maintenance levels of Run 5.

Figures 24 to 27 show the summary tables for Run 5. Figure 24 shows the miles of roadway breakdown by score and functional classification. It can be observed that:

- a.- The mean scores for the FM network in District 11 are between 59 and 75 (average condition with low to moderate levels of distress).
- b.- Fourteen percent of the FM network is below a score of 45 (extensive distress).
- c.- Forty percent of the FM network is between a score of 45 and 75 (low to moderate distress).
- d.- Forty six percent of the FM network is above a score of 75.

Figures 25 to 27 give the maintenance and rehabilitation costs per year, and per functional classification.

**TABLE 34. Maintenance and Rehabilitation Costs
at Different Climatic Zones**

District	Maintenance Cost	Rehabilitation Cost	Total Cost
21 (Dry)	363,138	463,000	826,138
4 (Dry, Cold)	366,370	531,000	897,370
19 (Wet, Cold)	343,049	761,000	1,114,049

TABLE 35. Typical Results for Angelina County

YEAR	1984	1985	1986	1987
Rehabilitation Costs (\$)	3,975,000	396,000	302,000	276,000
Maintenance Costs	1,072,000	687,000	722,000	816,000

TABLE 36. Consequence of Delaying Preventive Maintenance

Criteria	5 year Average Cost per Year (in million \$)		
	Renabilitation	Maintenance	Total
A	1.01	0.77	1.78
B	1.73	0.29	2.02
C	2.30	0.05	2.35

TABLE 37. Effect of Traffic on Predicted M&R Requirements

Traffic (18-kip ESAL)	5 Year Average Cost per Year (in millions)		
	Renabilitation	Maintenance	Total
1/2 Current Level	0.76	0.63	1.39
Current Level	1.01	0.77	1.78
Twice Current Level	1.70	1.07	2.77

TABLE 38. Maintenance and Rehabilitation Costs for Farm to Market Roads in District 11

Run	Rehab. Level	Maint. Level	Rehab. Cost(\$10 ³)	Maint. Cost(\$10 ³)	Total Cost(\$10 ³)
1	40	None	102,468		102,468
2	60	None	112,668		112,668
3	40	40	101,113	228	101,341
4	60	60	105,802	590	106,392
5	40	75	54,322	2,600	56,922

MILES OF ROADWAY BREAKDOWN BY PAVEMENT SCORE AND FUNCTIONAL CLASSIFICATION							
DISTRICT 11							
PAVEMENT SCORE	MILES IN EACH FUNCTIONAL CLASS						
	1 INTERST	2 URBANFW	3 PRINCAR	4 MINORAR	5 MAJORCL	6 MINORCL	7 PARK
0 THRU 9	0 0	0 0	0 0	2 0	4 3	6 0	0 0
10 THRU 19	0 0	0 0	0 0	0 0	17 3	12 3	0 0
20 THRU 29	0 0	0 0	0 0	2 6	40 1	10 5	0 0
30 THRU 39	0 0	0 0	0 0	6 0	59 7	18 2	0 3
40 THRU 49	0 0	0 0	0 0	11 3	62 1	32 3	0 0
50 THRU 59	0 0	0 0	0 0	0 0	124 8	47 5	0 0
60 THRU 69	0 0	0 0	0 0	7 1	114 9	90 3	0 0
70 THRU 79	0 0	0 0	0 0	5 8	145 6	80 2	0 0
80 THRU 89	0 0	0 0	0 0	8 1	152 8	105 7	0 0
90 THRU 99	0 0	0 0	0 0	7 6	171 5	130 2	1 7
100	0 0	0 0	0 0	6 2	83 8	87 0	1 8
MEAN SCORE	0	0	0	59	70	76	74
SAMPLE SIZE	0	0	0	32	493	321	3

Figure 24. Miles of Roadway Breakdown by Pavement Score and Functional Classification

.....REHABILITATION AND MAINTENANCE COST PER YEAR.....

YEAR	REHABILITATION COST	MAINTENANCE COST
1984	38307488 0	789647 06
1985	22410000 0	639332 81
1986	7352500 00	279881 19
1987	3106000 00	258581 56
1988	19503488 0	436364 87
CUMULATIVE COST	90679500 00	2403817.91

.....

Figure 25. Rehabilitation and Maintenance Cost per Year

.....MAINTENANCE COST BREAKDOWN BY YEAR AND FUNCTIONAL CLASSIFICATION.....

DISTRICT 11

YEAR	1 INTERST	2 URBANFW	3 PRINCAR	4 MINORAR	5 MAJORCL	6 MINORCL	7 PARK
1984	0 0	0 0	0 0	126141 06	614095 19	49411 62	0 0
1985	0 0	0 0	0 0	90 00	355498 12	283745 50	0 0
1986	0 0	0 0	0 0	17999 75	129324 00	132467 69	90 00
1987	0 0	0 0	0 0	2503 20	190312 62	65766 37	0 0
1988	0 0	0 0	0 0	106664 06	170398 06	159303 56	0 0

.....

Figure 26. Maintenance Cost Breakdown by Year and Functional Classification

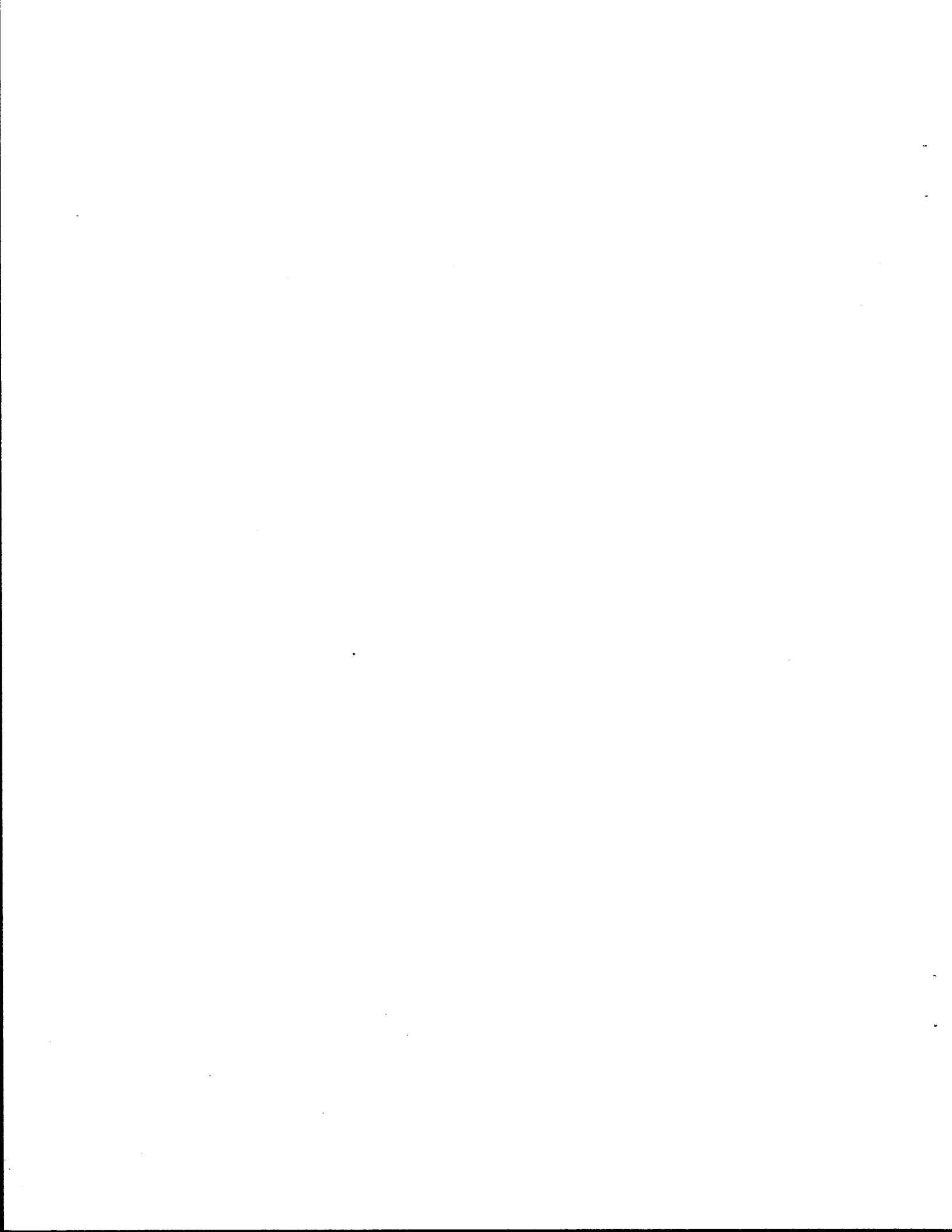
.....REHABILITATION COST BREAKDOWN BY YEAR AND FUNCTIONAL CLASSIFICATION.....

DISTRICT 11

YEAR	1 INTERST	2 URBANFW	3 PRINCAR	4 MINORAR	5 MAJORCL	6 MINORCL	7 PARK
1984	0 0	0 0	0 0	1949000 00	24564496 0	11734000 0	60000 00
1985	0 0	0 0	0 0	200000 00	15434000 0	6776000 00	0 0
1986	0 0	0 0	0 0	412500 00	5240000 00	1700000 00	0 0
1987	0 0	0 0	0 0	0 0	2526000 00	580000 00	0 0
1988	0 0	0 0	0 0	1076000 00	13797500 0	4630000 00	0 0

.....

Figure 27. Rehabilitation Cost Breakdown by Year and Functional Classification



CHAPTER SEVEN

CONCLUSIONS AND RECOMMENDATIONS

This research developed a program that computes the rehabilitation and maintenance funding needs for sections of road taking into consideration visual utility scores, structural conditions, traffic factors, and climatic factors. To evaluate the program, data from the 1983 Texas Annual Statewide Survey was utilized. The methodology used to accomplish the objective of this research was:

- a) Develop a mathematical relationship to predict the increase of distress and decrease of serviceability index.
- b) Generate deterioration matrices that will predict the yearly growth in each distress type using the relationships developed in (a).
- c) Create a decision tree so that the appropriate maintenance procedure is selected.
- d) Run the program with typical sections of road.
- e) Perform a limited sensitivity analysis to see how the minimum acceptable utility score affects the estimated funding requirements. Also, determine how traffic levels and climatic variations will affect needed funds.

Conclusions

Through the application of the limited sensitivity analysis and case studies, the following observations were concluded:

1. The maintenance cost appears to be inversely proportional to the minimum score, and the rehabilitation cost directly proportional to the minimum score. This relationship is due to the fact that when the minimum allowable score is high, maintenance would have to be done so frequently that it is more cost effective to do a rehabilitation which will last longer at a high score. On the other hand, when the minimum score is low, the percent of distress of a section increases to a higher level, causing a need for a more extensive and correspondingly more expensive maintenance strategy. However, such maintenance will be required less frequently and thus is more cost effective than a rehabilitation strategy.
2. A level between 70 and 75 minimum allowable score was found to be the most economical. This cost was compared to the cost incurred by not having preventive maintenance strategies when the pavement

falls below an acceptable score of 45. It was found that the cost of maintaining the road at a level between 70 and 75 will be less expensive than to let the road fall to an unacceptable level of less than 45 and then rehabilitate.

3. As the traffic loading is increased, the life of the pavement decreases, and the predicted total cost increases.
4. Higher total costs were observed for the wet and freeze climatic zone (District 19) than for other climatic zones. This difference is due to the fact that the subgrade soil moisture content increases and it results in pavement breakup. The problem is further increased by the thermal cracking that causes a loss of strength in the pavement.
5. High first year rehabilitation costs were observed for many counties. This condition is due to the large backlog of roads in very poor condition.

Recommendations

The current system has been designed to assist the Texas State Department of Highways and Public Transportation in identifying rehabilitation and maintenance projects and associated costs through time for flexible pavements at a network level.

These goals have been achieved through the use of maintenance decision trees developed by the highway department maintenance personnel and deterioration matrices developed from the Texas performance equations. This current system is viewed as a first-level pavement management system. Efforts are underway to improve and extend this system to meet more of the Department's pavement management requirements.

Below are a list of recommendations as to how the system could be improved and expanded.

1. Evaluation of Preventive Maintenance and Rehabilitation Costs.

The current system contains costs for the 14 maintenance strategies and the 5 rehabilitation strategies. There is a need to evaluate whether the costs are correctly represented within PES. This can be best done by surveying via a fill-in-the-blank questionnaire, the actual maintenance and rehabilitation costs district by district.

2. Evaluation of the Effect of Maintenance Strategies on the Life of a Pavement. Currently the maximum gain table for preventive maintenance strategies has been developed using the field experience of various highway engineers. However, there is a need for a more sound set of decisions in this area. This can be best achieved by monitoring for a period not less than 3 years

typical sections of road that have been treated with one or more of the maintenance strategies. It would be desirable to monitor sections in different areas of the State so the effect of climatic factors on the maintenance strategies can also be measured.

3. Need for Structural Evaluation. Pavements which are structurally very weak but have recently received maintenance such as thin overlay or seal coat could be rated very high within the existing PES, because its true structural condition has been masked. This makes it necessary to develop a methodology to include a structural condition utility in the pavement score calculation.
4. Budget and Time Optimization. An optimization scheme should be developed to deal with the limited availability of funds for the selected projects. A number of methods can be used to obtain a selection of desirable projects. These methods vary from ranking methods to optimization methods. Some of the suggested methods are listed below:
 - a) Benefit/cost ranking
 - b) Linear programming
 - c) Integer programming, and
 - d) Dynamic programming.
5. Link to Project Level Pavement Management System. The department has network level (PES) and a project level (FPS and RPS) pavement management systems. However, there is an urgent need to tie these systems together so that more cost-effective pavement rehabilitation programs can be developed. Specific areas of interest are:
 - a. Interpretation of PES outputs. The Department does a good job in training raters on how to input information into the system. However, more attention should be given to instructing the Districts on how to interpret and use the outputs. This training could take the form of a report or regional schools for the District personnel responsible for using pavement evaluation data in preparing pavement improvement programs.
 - b. Pavement Failure Analysis. PES identifies pavements in poor condition, it does not indicate the cause of the poor condition. Identification of this cause is fundamental to developing a pavement rehabilitation strategy.

Many techniques are available for identifying the causes of pavement deterioration and several ITI reports (15) have given guidelines. It is recommended that schools be developed to train District personnel in pavement failure analysis. The PES data would be used as a starting point;

the need for detailed visual inspection, non-destructive and laboratory testing would be described by analysis of actual sections of highway. The goal of these schools would be to provide a badly needed link between the departments network and project level pavement management activities.

6. Evaluation of Weighting Factors. The current system contains several weighting factors for variables such as area of distress, traffic level, and climatic conditions. There is a need to evaluate whether these weights are correctly represented within PES. This can best be done by comparing the list of candidate rehabilitation projects as prepared by the Districts with their corresponding PES score, traffic level, etc. Statistical techniques such as discriminant analysis can be used to determine if adequate weighting is being given to each variable.
7. Adaptation of Program for Use on Microcomputer. The current system is based on mainframe, efforts are currently underway to transfer it to microcomputer.

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APPENDICES

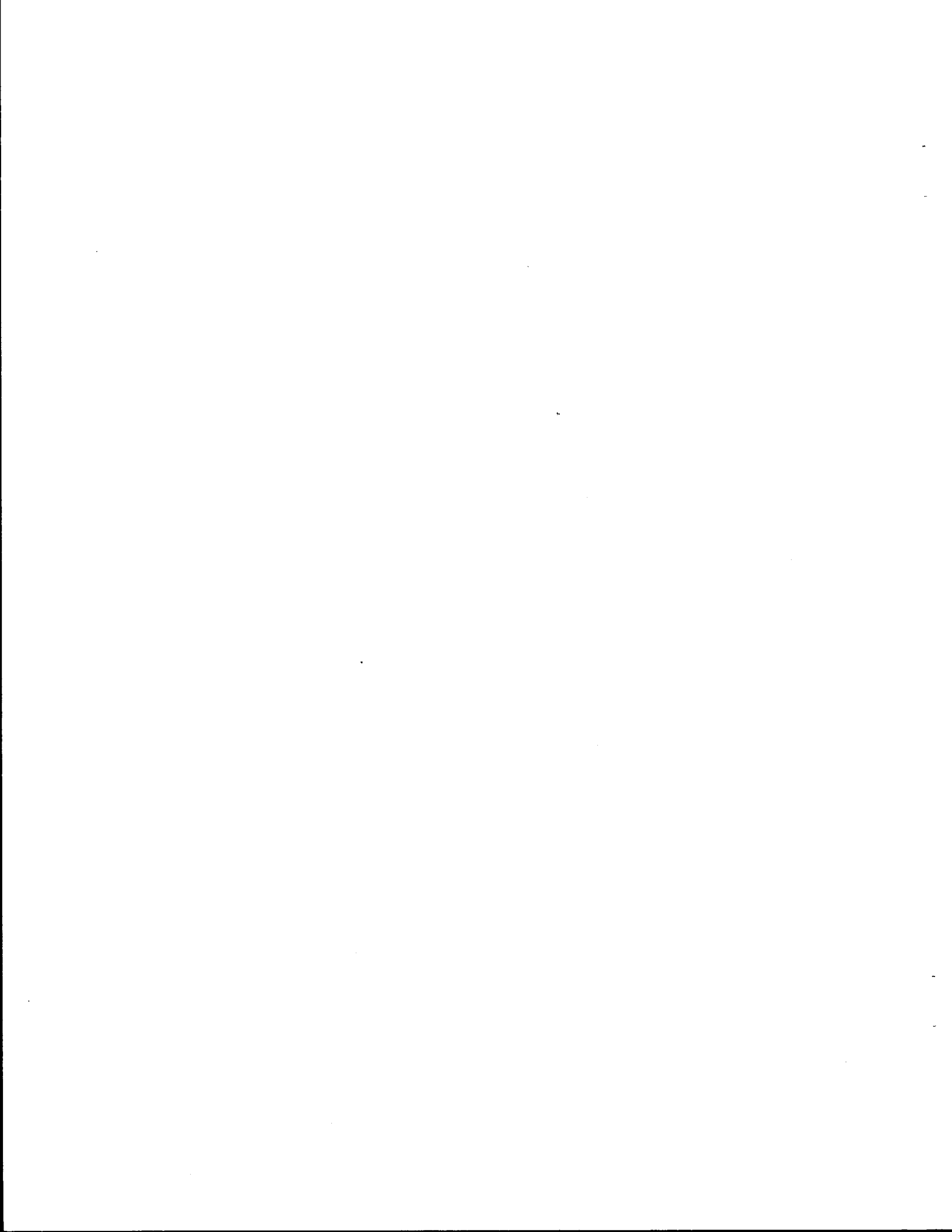


TABLE A-1. Variables Used in the Regression Models

Environmental	Structural	Pavement History
Thorntwaite Index (TI)	Plasticity Index (PI)	
Freeze/Thaw (F/T)	Equivalent Thickness (H') ¹	N-18/month (N-18)
Average Temperature (T _{AVG})	Percent Asphalt Binder (Binder) ²	
Plasticity Index (PI)	Overlay Thickness (OVTH) ³	
Liquid Limit (LL)	Total Asphalt Thickness (ASPH) ⁴	
	Surfacing Thickness (HMAC) ⁵	
	Dynaflect Mean Deflection (DMD)	

1. Equivalent thickness is the transformed pavement thickness based on the following expression:

$$H' = \sum_{i=1}^m (E_i/E_s)^n t_i$$

where

m = number of pavement layers under consideration

E_i = elastic modulus for the i-th layer

t_i = thickness of the i-th layer

E_s = elastic modulus of the subgrade as determined from Dynaflect measurements.

n = Odemark's constant (0.33) or can be obtained from field data.

Appendix A

0 and 3 Equations for Sigmoidal

$$g = \exp\left(-\frac{\rho}{N}\right)^3$$

for longitudinal and transverse cracking N is in terms of Number of Months in service. For all other distress types N is in terms of 18 kip ESAL.

TABLE A-1. Variables Used in the Regression Models (Cont'd)

In the regression models this variable is transformed as follows:

$$\text{HPR2} = H' * E_s / 10^5$$

The HPR3 term appearing in the regression equations is defined as follows:

$$\text{HPR3} = \frac{10^{10}}{E_s * (H')^3}$$

FOOTNOTES TO TABLE A-1

2. This term is for black base and hot mix asphalt concrete pavements.
3. This term is for overlay pavements.
4. This term is for black base pavements. It is the total asphalt thickness of black base + surfacing course.
5. This term is for Hot Mix pavements.
6. The N-18/month value represents the observed value during the first performance period.

TABLE A-2. Arithmetic Regression Models for the Design Parameters (PSI)

Black Base

$$\rho = -0.02182(F/T) - 0.00831(PI) + 0.04499(\text{Binder}) + 0.15019(\text{HPR2})$$

$$\begin{aligned} \beta &= 0.01201(TI) + 0.03166(F/T) + 0.13775(T_{\text{AVG}}) + 0.00114(PI) \\ &\quad - 0.31331(\text{Binder}) - 0.03234(\text{HPR2}) \end{aligned}$$

$$\begin{aligned} P_f &= -0.00637(F/T) - 0.01550(T_{\text{AVG}}) - 0.00658(PI) \\ &\quad + 0.27714(\text{Binder}) + 0.05097(\text{HPR2}) \end{aligned}$$

Hot Mix

$$\begin{aligned} \rho &= -0.02000(TI) - 0.02481(F/T) - 0.03078(PI) + 0.60781(\text{Binder}) \\ &\quad + 0.06424(\text{HPR2}) \end{aligned}$$

$$\beta = 0.04045(F/T) + 0.22931(T_{\text{AVG}}) - 0.53010(\text{Binder})$$

$$\begin{aligned} P_f &= -0.00665(F/T) - 0.07017(T_{\text{AVG}}) - 0.02472(PI) \\ &\quad + 0.57235(\text{Binder}) + 0.00722(\text{HPR2}) \end{aligned}$$

Overlays

$$\rho = 0.26503(\text{OVTH}) + 0.07180(\text{HPR2})$$

$$\begin{aligned} \beta &= 0.00413(TI) + 0.01036(F/T) + 0.04759(T_{\text{AVG}}) + 0.01707(N-13) \\ &\quad - 0.09144(\text{OVTH}) - 0.01066(\text{HPR2}) \end{aligned}$$

$$P_f = 0.33037(\text{OVTH}) + 0.07627(\text{HPR2})$$

TABLE A-3. Logarithmic Regression Models for the Design Parameters (PSI)

Black Base	
ρ	$= (F/T)^{-0.46679} * (T_{AVG})^{-0.86233} * (PI)^{-0.26711} * (HPR2)^{1.65694}$
β	$= (F/T)^{-0.60949} * (T_{AVG})^{0.93499} * (Binder)^{-1.37608} * (HPR2)^{-0.72725}$
P_f	$= (F/T)^{-1.50634} * (T_{AVG})^{-2.69460} * (Binder)^{4.17755} * (HPR2)^{1.60919}$
Hot Mix	
ρ	$= (TI)^{-0.31419} * (F/T)^{-0.69942} * (T_{AVG})^{-0.96204} * (Binder)^{0.44492} * (HPR2)^{1.35110}$
β	$= (F/T)^{0.40391} * (T_{AVG})^{0.44517} * (N-18)^{0.04576} * (Binder)^{-1.50340}$
P_f	$= (F/T)^{-0.89515} * (T_{AVG})^{-3.14575} * (Binder)^{5.31210} * (HPR2)^{0.44486}$
Overlays	
ρ	$= (F/T)^{-0.24351} * (Binder)^{0.71372} * (HPR2)^{0.185059}$
β	$= (F/T)^{0.09767} * (N-18)^{0.17402} * (Binder)^{-0.30623} * (HPR2)^{-0.22623}$
P_f	$= (F/T)^{-0.14525} * (T_{AVG})^{-0.25053} * (N-18)^{-0.24293} * (Binder)^{0.32304} * (HPR2)^{0.62508}$

TABLE A-4. Regression Equations for Black Base Pavements

Rutting

Area $\rho = 0.00175 \text{ F/T} - 0.0141 \text{ T}_{\text{AVG}} + 0.257 \text{ ASPH}$

$\beta = -0.00493 \text{ F/T} + 0.0262 \text{ T}_{\text{AVG}} + 0.0387 \text{ PI}$
 $- 0.0433 \text{ ASPH}$

Severity $\rho = 0.00263 - 0.0137 \text{ T}_{\text{AVG}} + 0.253 \text{ ASPH}$

$\beta = 0.00337 \text{ TI} - 0.00928 \text{ F/T} + 0.0341 \text{ T}_{\text{AVG}}$
 $+ 0.0242 \text{ PI} - 0.071 \text{ ASPH}$

Alligator

Area $\rho = 0.134 \text{ HPR2} - 0.067 \text{ HPR3}$

$\beta = 0.256 \text{ HPR3}$

Severity $\rho = -0.00986 \text{ PI} + 0.0422 \text{ ASPH} + 0.0554 \text{ HPR2}$

$\beta = 1.37 \text{ HPR3}$

Longitudinal

Area $\rho = 5.33 \text{ ASPH} + 29.44 \text{ BINDER} - 6.33 \text{ HPR3}$

$\beta = 0.0181 \text{ T}_{\text{AVG}} + 0.421 \text{ HPR3}$

Severity $\rho = -0.425 \text{ F/T} - 0.0943 \text{ PI} + 2.915 \text{ ASPH} + 22.16 \text{ BINDER}$
 $- 11.59 \text{ HPR3}$

$\beta = 0.118 \text{ TI} + 0.0389 \text{ F/T} - 0.701 \text{ BINDER} + 0.553 \text{ HPR3}$

Transverse

Area $\rho = -1.739 \text{ PI} + 0.423 \text{ ASPH} + 48.38 \text{ BINDER} - 46.7 \text{ HPR3}$

$\beta = 0.0153 \text{ F/T} + 0.625 \text{ HPR3}$

Severity $\rho = -0.502 \text{ PI} + 26.75 \text{ BINDER} - 29.96 \text{ HPR3}$

$\beta = 0.165 \text{ TI} + 0.0362 \text{ F/T} - 1.047 \text{ BINDER} + 1.1488 \text{ HPR3}$

TABLE A-5. Regression Equations for Hot Mix Pavements

Rutting

Area $\rho = 0.2776 \text{ HMAC} + 0.0151 \text{ HPR2}$
 $\beta = 0.0128 \text{ TI} + 0.0326 \text{ T}_{\text{AVG}} - 0.0331 \text{ HMAC}$
 $- 0.00382 \text{ HPR2}$

Severity $\rho = -0.00770 \text{ PI} + 0.386 \text{ HMAC}$
 $\beta = -0.000720 \text{ F/T} + 0.0273 \text{ T}_{\text{AVG}} - 0.00267 \text{ HMAC}$
 $- 0.000418 \text{ HPR2}$

Alligator

Area $\rho = 0.372 \text{ HMAC}$
 $\beta = 2.198 \text{ HPR3}$

Severity $\rho = -0.0000749 \text{ PI} + 0.291 \text{ HMAC}$
 $\beta = 3.145 \text{ HPR3}$

Longitudinal

Area $\rho = -0.988 \text{ F/T} + 4.38 \text{ T}_{\text{AVG}} - 2.99 \text{ PI} + 7.21 \text{ HMAC}$
 $\beta = 0.0422 \text{ F/T} + 0.359 \text{ HPR3}$

Severity $\rho = -0.144 \text{ TI} + 3.018 \text{ T}_{\text{AVG}} - 3.155 \text{ PI} + 3.331 \text{ HMAC}$
 $\beta = 0.0343 \text{ TI} + 0.0502 \text{ F/T}$

Transverse

Area $\rho = -1.97 \text{ TI} - 0.826 \text{ F/T} + 5.193 \text{ T}_{\text{AVG}} - 1.768 \text{ PI}$
 $- 26.3 \text{ HPR3}$
 $\beta = 0.017 \text{ TI} - 0.0433 \text{ F/T} - 0.115 \text{ HMAC} - 0.0159 \text{ HPR2}$
 $+ 0.259 \text{ HPR3}$

Severity $\rho = -0.196 \text{ TI} + 2.90 \text{ T}_{\text{AVG}} - 2.690 \text{ PI} + 5.475 \text{ HMAC}$
 $\beta = 0.0519 \text{ F/T} + 0.537 \text{ HPR3}$

TABLE A-6. Regression Equations for Overlaid Pavements

Rutting

Area $\rho = -0.00119 \text{ PI} + 0.369 \text{ OVTH} + 0.0485 \text{ HPR2}$
 $\beta = 0.0059 \text{ TI} - 0.00217 \text{ F/T} + 0.0206 \text{ T}_{\text{AVG}} - 0.122 \text{ OVTH} + 0.0789 \text{ HPR3}$

Severity $\rho = -0.00507 \text{ PI} + 0.233 \text{ OVTH} + 0.0705 \text{ HPR2} - 0.000779 \text{ HPR3}$
 $\beta = 0.00900 \text{ TI} + 0.0146 \text{ T}_{\text{AVG}} + 0.0024 \text{ PI} - 0.0789 \text{ OVTH} + 0.0840 \text{ HPR3}$

Alligator

Area $\rho = -0.0159 \text{ F/T} + 0.00820 \text{ T}_{\text{AVG}} - 0.0121 \text{ PI} + 0.0162 \text{ OVTH} + 0.145 \text{ HPR2} - 0.0135 \text{ HPR3}$
 $\beta = 0.0185 \text{ TI} + 0.171 \text{ HPR3}$

Severity $\rho = -0.00975 \text{ F/T} + 0.0152 \text{ T}_{\text{AVG}} - 0.0106 \text{ PI} + 0.0568 \text{ HPR2} - 0.0315 \text{ HPR3}$
 $\beta = 0.0301 \text{ TI} + 0.2267 \text{ HPR3}$

Longitudinal

Area $\rho = -0.0168 \text{ TI} - 0.0870 \text{ F/T} + 1.63 \text{ T}_{\text{AVG}} - 0.179 \text{ PI} + 2.68 \text{ OVTH} + 0.340 \text{ HPR2}$
 $\beta = 0.0331 \text{ TI} + 0.00433 \text{ F/T} - 0.00713 \text{ T}_{\text{AVG}} - 0.0589 \text{ OVTH} + 0.399 \text{ HPR3}$

Severity $\rho = -0.214 \text{ F/T} + 1.55 \text{ T}_{\text{AVG}}$
 $\beta = 0.0218 \text{ TI} + 0.0134 \text{ F/T} - 0.0156 \text{ HPR2} + 0.073 \text{ HPR3}$

Transverse

Area $\rho = -0.794 \text{ F/T} + 1.922 \text{ T}_{\text{AVG}} + 22.31 \text{ OVTH}$
 $\beta = -0.0097 \text{ TI} + 0.0149 \text{ F/T} - 0.0229 \text{ T}_{\text{AVG}} - 0.0441 \text{ PI} - 0.129 \text{ OVTH} + 0.480 \text{ HPR3}$

TABLE A-6. Regression Equations for Overlaid Pavements (cont'd)

Transverse (cont'd)

$$\text{Severity } \rho = -0.0627 \text{ F/T} + 1.23 \text{ T}_{\text{AVG}} + 5.273 \text{ OVTH}$$

$$\text{S} = 0.0187 \text{ TI} + 0.0117 \text{ F/T} + 0.0109 \text{ PI} - 0.0305 \text{ HPR2} \\ + 0.108 \text{ HPR3}$$

TABLE A-7. Regression Equations for Surface Treated Pavements

PSI

$$\rho = -0.173 + 0.00687 * T_{AVG} - 0.000632 * TI + 0.0133 * FLEXL + 0.00075 * LL + 0.00153 * F/T - 0.0214 * DMD$$

$$\beta = 1.0$$

$$P_f = 0.83$$

Rutting

$$\text{Area } \rho = -0.1035 + 0.00544 * T_{AVG} + 0.0067 * FLEXL - 0.0015 * LL + 0.00162 * PI + 0.00077 * F/T$$

$$\beta = 1.540 + 0.0169 * TI - 0.072 * FLEXL$$

$$\text{Severity } \rho = -0.0678 + 0.00320 * T_{AVG} + 0.00566 * FLEXL - 0.00031 * LL + 0.00048 * F/T$$

$$\beta = 1.78$$

Ravelling

$$\text{Area } \rho = 1.03 + 0.0146 * TI + 0.0064 * F/T - 0.609 * DMD$$

$$\beta = 1.28$$

$$\text{Severity } \rho = 0.62 + 0.0129 * TI + 0.0066 * F/T - 0.449 * DMD$$

$$\beta = 1.40$$

Flushing

$$\text{Area } \rho = 0.488 + 0.013 * TI + 0.00345 * F/T - 0.213 * DMD$$

$$\beta = 1.27$$

$$\text{Severity } \rho = -0.14 + 0.031 * T_{AVG} + 0.0103 * TI + 0.0064 * F/T - 0.201 * DMD$$

$$\beta = 1.50$$

Alligator

$$\text{Area } \rho = -0.179 + 0.0121 * T_{AVG} + 0.004 * FLEXL - 0.0011 * LL + 0.00153 * F/T$$

TABLE A-7. Regression Equations for Surface Treated Pavements
(Cont'd)

Alligator (cont'd)

Area $\rho = 1.867 - 0.009 * TI + 0.144 * FLEXL - 0.577 * DMD$
(cont'd)

Severity $\rho = -0.22 + 0.012 * T_{AVG} + 0.00033 * TI + 0.0027 * FLEXL - 0.00058 * LL + 0.0017 * F/T$

$B = 2.91 - 0.099 * T_{AVG} + 0.013 * FLEXL - 1.567 * DMD$

Longitudinal

Area $\rho = -63.1 + 4.52 * T_{AVG} + 0.541 * TI + 7.41 * FLEXL + 1.11 * F/T$

$B = 1.15$

Severity $\rho = -120 + 6.77 * T_{AVG} + 1.14 * TI + 4.78 * FLEXL + 1.32 * F/T$

$B = 1.20$

Transverse

Area $\rho = -66.4 + 2.156 * TI + 10.1 * FLEXL + 0.718 * F/T$

$B = 2.06 + 0.0734 * FLEXL - 0.06 * LL + 0.061 * PI - 0.0037 * F/T$

Severity $\rho = 96.3 - 1.04 * T_{AVG} + 1.07 * TI - 0.318 * F/T$

$B = 1.10 + 0.16 * LL - 0.24 * PI - 0.015 * F/T$

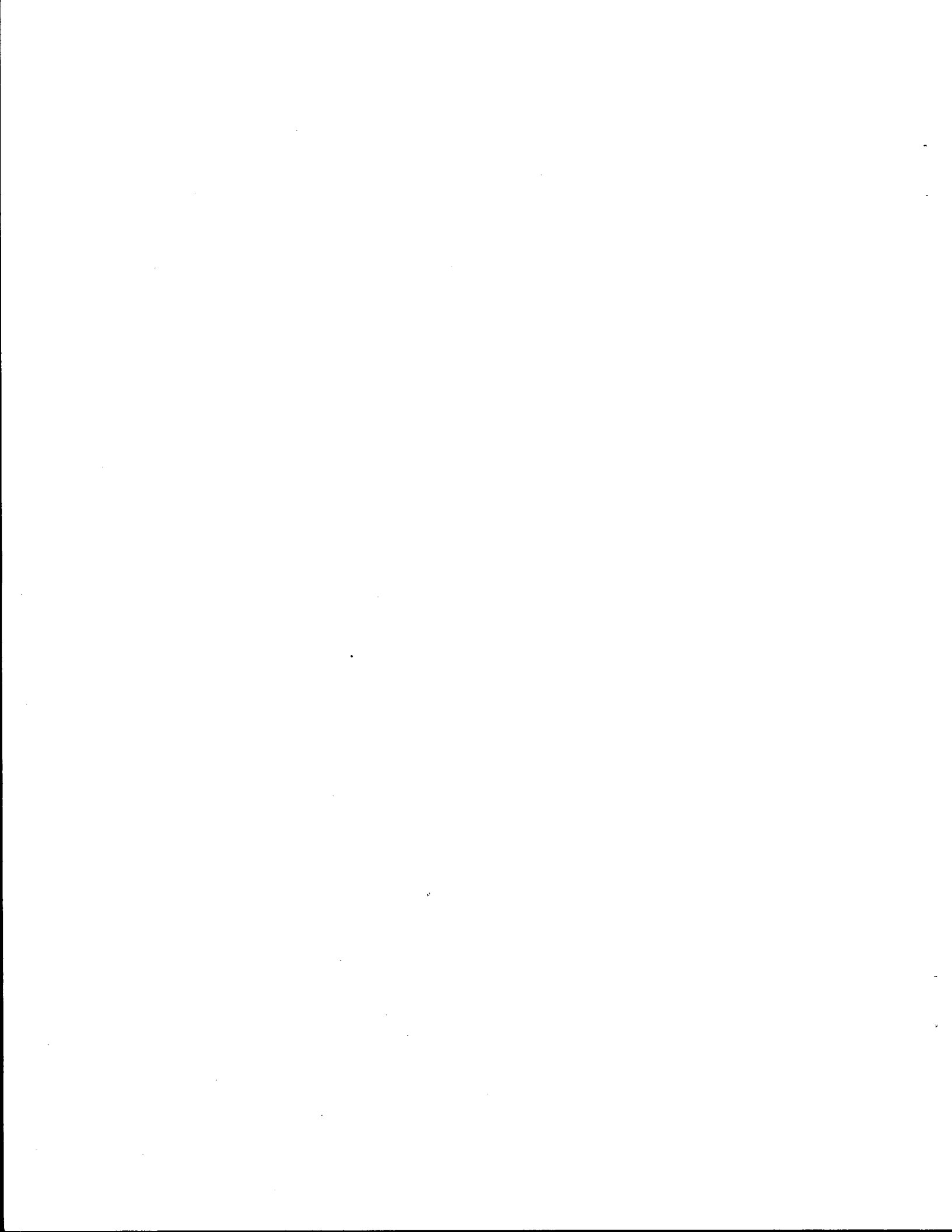
Patching

Area $\rho = 0.008 + 0.0025 * T_{AVG} + 0.00022 * TI + 0.0017 * FLEXL - 0.0012 * PI$

$B = 1.75$

Severity $\rho = -0.04 + 0.0035 * T_{AVG} + 0.003 * FLEXL - 0.0004 * LL + 0.00039 * F/T$

$B = -0.16 + 0.050 * T_{AVG} + 0.090 * FLEXL - 0.069 * LL + 0.082 * PI - 0.027 * F/T$



Appendix B

Decision Rules proposed to generate maintenance alternatives for flexible pavements.

Codes

- 1 Seal Cracks
- 2 Partial (Skin) Patch
- 3 Full Depth Patch
- 4 Fog Seal
- 5 Strip Seal
- 6 Seal Coat
- 7 Asphalt-Rubber Seal
- 8 Slurry Seal
- 9 Level-up
- 10 Thin Overlay
- 11 Rotomill
- 12 Spot Seal
- 13 Rotomill + Seal Coat
- 14 Rotomill + Thin Overlay

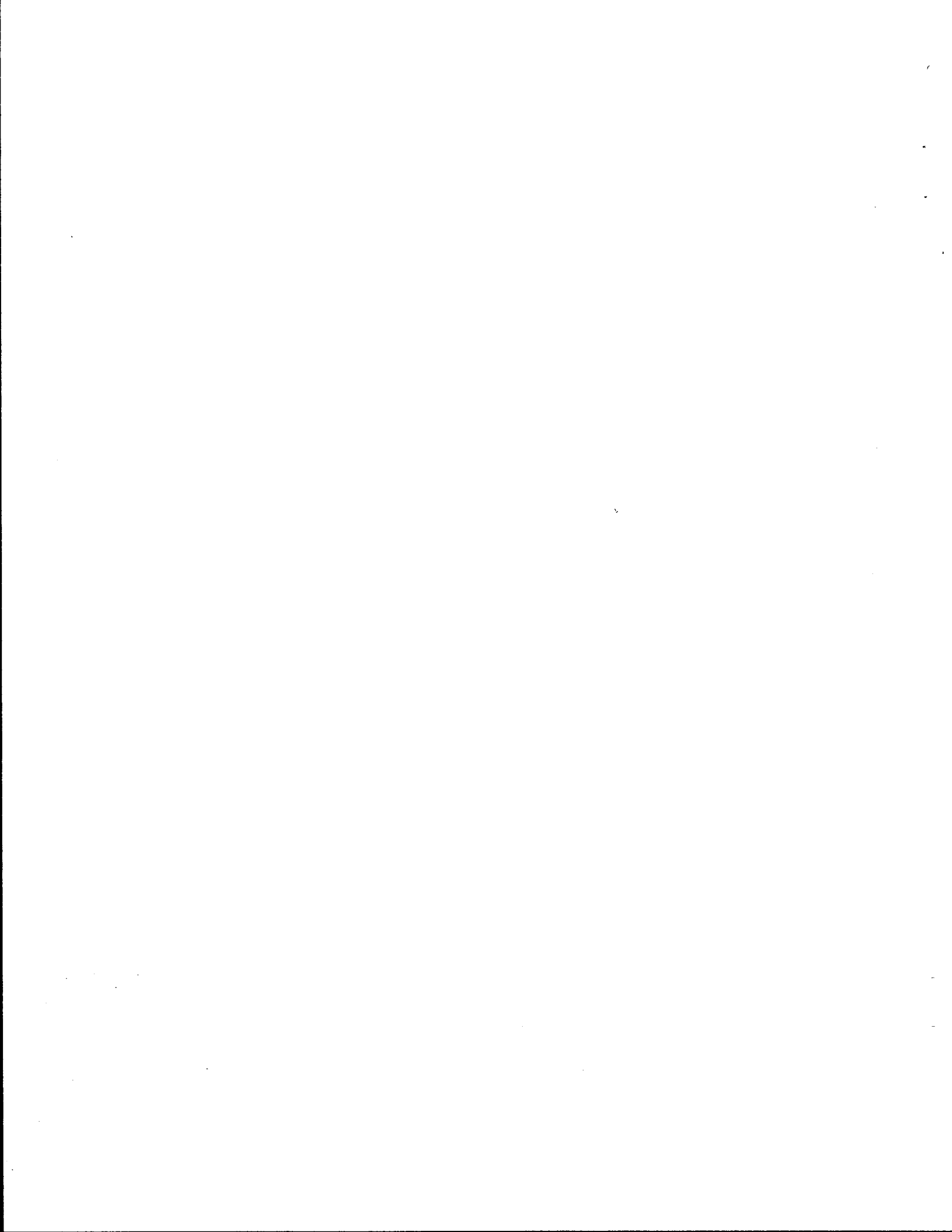


Table B-1. Selection Maintenance Strategy, Serviceability Index

Performance Equation: PSI

Pavement Type	Traffic	PSI			
		3.0-3.5	2.5-3.0	1.5-2.0	1.5
4	LL	0	9	9	10
	LH	0	9	10	10
	HL	0	9	9	10
	HH	0	9	10	10
5	LL	0	9	9	9
	LH	0	9	9	10
	HL	0	9	9	10
	HH	0	9	10	10
6	LL	0	9	9	9
	LH	0	9	9	10
	HL	0	9	9	9
	HH	0	9	10	10
7	LL	0	0	9	9
	LH	0	9	9	9
	HL	0	9	9	10
	HH	0	9	10	10
8	LL	0	0	9	9
	LH	0	9	9	10
	HL	0	9	10	10
	HH	0	9	10	10
9	LL	0	0	9	9
	LH	0	9	9	9
	HL	0	9	10	10
	HH	0	9	10	10
10	LL	0	0	0	0
	LH	0	0	0	9
	HL	0	9	9	9
	HH	0	9	10	10

Table B-2. Selection Maintenance Strategy, Pavement Type 4

Thick ACP - 5 1/2"

Performance Equation: Black Base

Distress	Traffic	Area	Area	Area
		100	010	001
Slight Rutting	LL	11	11	11
	LH	11	11	11
	HL	11	11	11
	HH	11	11	11
Severe Rutting	LL	9	14	14
	LH	13	14	14
	HL	13	14	14
	HH	13	14	11
Raveling	LL	4	6	6
	LH	4	6	6
	HL	4	10	10
	HH	4	10	10
Flushing	LL	11	11	11
	LH	11	11	11
	HL	11	10	10
	HH	11	10	10
Failures	LL	3	3	3
	LH	3	3	3
	HL	3	3	3
	HH	3	3	3
Longitudinal Cracking	LL	1	1	6
	LH	1	1	7
	HL	1	1	7
	HH	1	1	7
Transverse Cracking	LL	1	1	6
	LH	1	1	7
	HL	1	1	7
	HH	1	1	7
Alligator Cracking	LL	12	6	7
	LH	7	7	7
	HL	12	6	10
	HH	7	7	10

Table B-3. Selection Maintenance Strategy, Pavement Type 5

Medium ACP-2 1/2 - 5 1/2"

Performance Equation: Hot Mix

Distress	Traffic	Area	Area	Area
		100	010	001
Slight Rutting	LL	9	9	9
	LH	9	9	11
	HL	9	9	9
	HH	9	9	11
Severe Rutting	LL	9	11	14
	LH	9	14	14
	HL	11	11	14
	HH	11	14	14
Raveling	LL	4	6	5
	LH	4	6	5
	HL	4	10	10
	HH	4	10	10
Flushing	LL	5	6	6
	LH	5	6	6
	HL	5	6	6
	HH	5	6	6
Failures	LL	3	3	3
	LH	3	3	3
	HL	3	3	3
	HH	3	3	3
Alligator Cracking	LL	12	12	6
	LH	12	6	7
	HL	12	6	7
	HH	12	7	7
Longitudinal Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6
Transverse Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6

Table 8-4. Selection Maintenance Strategy, Pavement Type 6

Thin ACP - 2 1/2"

Performance Equation: Hot Mix

Distress	Traffic	Area	Area	Area
		100	010	001
Slight Rutting	LL	9	9	9
	LH	9	9	9
	HL	9	9	9
	HH	9	9	10
Severe Rutting	LL	9	9	9
	LH	9	10	10
	HL	9	9	9
	HH	9	10	10
Raveling	LL	4	4	5
	LH	4	4	5
	HL	4	6	6
	HH	4	6	6
Flushing	LL	0	5	6
	LH	0	5	6
	HL	5	6	6
	HH	6	6	6
Failures	LL	3	3	3
	LH	3	3	3
	HL	3	3	3
	HH	3	3	3
Alligator Cracking	LL	12	12	5
	LH	12	12	6
	HL	12	5	5
	HH	12	5	6
Longitudinal Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6
Transverse Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6

Table B-5. Selection Maintenance Strategy, Pavement Type 7

Composite		Performance Equation: Overlay		
Distress	Traffic	Area	Area	Area
		100	010	001
Slight Rutting	LL	11	9	9
	LH	11	9	9
	HL	11	9	9
	HH	11	9	10
Severe Rutting	LL	9	9	10
	LH	13	13	14
	HL	9	9	14
	HH	9	13	14
Raveling	LL	4	4	5
	LH	4	4	5
	HL	4	6	6
	HH	4	6	6
Flushing	LL	0	5	6
	LH	0	5	6
	HL	5	6	6
	HH	6	6	6
Failures	LL	3	3	3
	LH	3	3	3
	HL	3	3	3
	HH	3	3	3
Alligator Cracking	LL	12	12	5
	LH	12	12	6
	HL	12	5	5
	HH	12	5	6
Longitudinal Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6
Transversal Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6

Table B-6. Selection Maintenance Strategy, Pavement Type 8

Overlay (concrete)

Performance Equation: Overlay

Distress	Traffic	Area	Area	Area
		100	010	001
Slight Rutting	LL	9	9	9
	LH	9	9	10
	HL	9	9	9
	HH	9	9	10
Severe Rutting	LL	9	9	10
	LH	13	13	14
	HL	9	9	14
	HH	13	14	14
Raveling	LL	4	4	5
	LH	4	4	5
	HL	4	6	6
	HH	4	6	6
Flushing	LL	0	5	5
	LH	0	5	5
	HL	5	6	6
	HH	6	6	6
Failures	LL	3	3	3
	LH	3	3	3
	HL	3	3	3
	HH	3	3	3
Alligator Cracking	LL	12	12	5
	LH	12	12	6
	HL	12	5	5
	HH	12	5	6
Longitudinal Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6
Transverse Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6

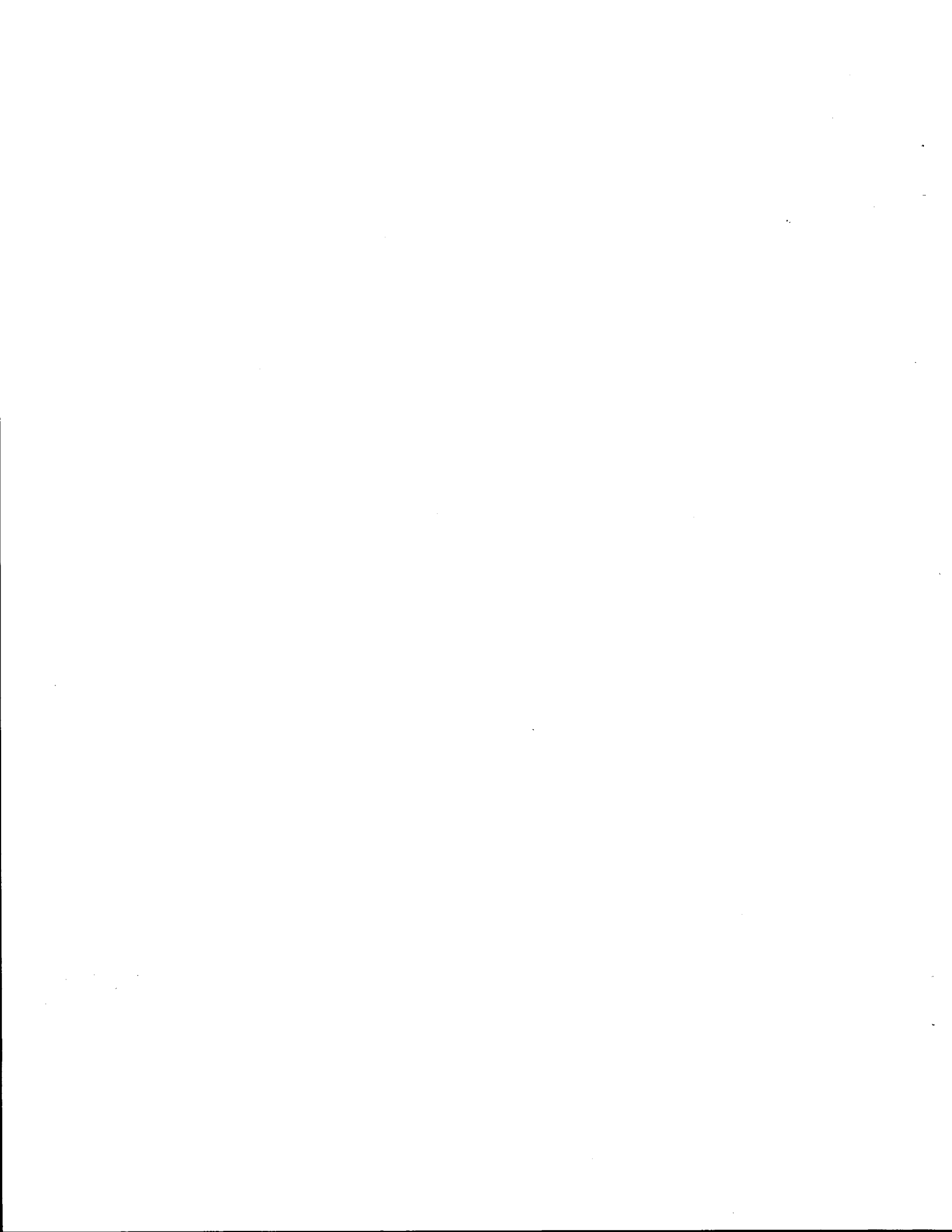
Table B-7. Selection Maintenance Strategy, Pavement Type 9
 Overlay (flexible) Performance Equation: Overlay

Distress	Traffic	Area	Area	Area
		100	010	001
Slight Rutting	LL	9	9	9
	LH	9	9	9
	HL	9	9	10
	HH	9	9	10
Severe Rutting	LL	9	9	10
	LH	9	10	10
	HL	9	9	10
	HH	9	10	10
Raveling	LL	4	4	5
	LH	4	4	5
	HL	4	6	6
	HH	4	6	6
Flushing	LL	0	5	6
	LH	0	5	6
	HL	5	6	6
	HH	6	6	6
Failures	LL	3	3	3
	LH	3	3	3
	HL	3	3	3
	HH	3	3	3
Alligator Cracking	LL	12	12	5
	LH	12	12	6
	HL	12	5	5
	HH	12	5	6
Longitudinal Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6
Transverse Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6

Table B-8. Selection Maintenance Strategy, Pavement Type 10

Surface Treatment		Performance Equation: Surf. Treat.		
Distress	Traffic	Area	Area	Area
		100	010	001
Slight Rutting	LL	0	0	9
	LH	0	0	9
	HL	0	9	9
	HH	0	9	10
Severe Rutting	LL	9	9	9
	LH	9	9	10
	HL	9	9	9
	HH	9	10	10
Raveling	LL	4	4	5
	LH	4	4	5
	HL	4	6	6
	HH	4	6	6
Flushing	LL	0	5	5
	LH	0	5	5
	HL	0	6	6
	HH	5	6	6
Failures	LL	2	3	3
	LH	3	3	3
	HL	3	3	3
	HH	3	3	3
Alligator Cracking	LL	12	12	5
	LH	12	12	6
	HL	23	5	5
	HH	12	5	6
Longitudinal Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6
Transverse Cracking	LL	0	1	6
	LH	1	1	6
	HL	1	1	6
	HH	1	1	6

APPENDIX C



PAVEMENT EVALUATION SYSTEM DATA

Four different sets of data are used throughout the program. The first two are related to information for every county such as rainfall or average temperature. The third data set is the information for the decision tree, and the last one is the survey information for every pavement section analyzed.

Data Set #1 (PESTAC)

This data set consists of 27 Tac Tables that can be read in any order. The information stored in the tables is:

No.	Name	Description
1.	MMSADTBN	ADT boundaries by highway functional classification
2.	MMSATRAF	Traffic factors by adjusted adt ranges
3.	MMSCFREZ	County average annual freeze-thaw cycles
4.	MMSCOMPE	Composite pavement distress type and severity
5.	MMSCRAIN	County average annual inches of rainfall
6.	MMSFLEXL	Flexible pavement distress type and severity
7.	MMSFREEZ	Average annual freeze-thaw cycle boundaries
8.	MMSFUNCL	Functional classification pavement score factors
9.	MMSKIPBN	18-KIP boundaries by highway functional class.
10.	MMSMAXDS	Pavement remaining life maximum deterioration slope
11.	MMSMCCLC	Roadway maintenance cost calculation boundaries
12.	MMSMINPS	Functional classification minimum allowable pavement score
13.	MMSRAINS	Average annual inches of rainfall boundaries
14.	MMSREAVU	Rehabilitated pavement estimated adjusted visual utility

No.	Name	Description
15.	MMSREHDS	Rehabilitated pavement deterioration slope
16.	MMSREHEF	Rehabilitated pavement enviromental factors
17.	MMSREHFS	Rehabilitated pavement foot-mile cost per strategy
18.	MMSREHSF	Rehabilitated pavement soil factors
19.	MMSREHSI	Rehabilitated pavement serviceability index
20.	MMSREHSN	Rehabilitated pavement skid number
21.	MMSREHTC	Rehabilitated pavement life time constant
22.	MMSSICLC	Serviceability Index utility calculation boundaries
23.	MMSSOILS	County average soil plasticity indices
24.	MMSTMING	Pavement life calculation variables
25.	MMSTMINC	Rehabilitated pavement minimum life increase
26.	MMS18KIP	Traffic factors by 18-KIP ranges
27.	MMSSNCLC	Skid number utility calculation boundaries

A complete description of these tables is given in the Pavement Evaluation System Technical Reference Manual of the SDHPT (16).

Data Set #2 (SUBOVDAT)

This data set consists of a two-dimensional array of 2 by 254. The purpose of the data set is to provide information of the average weather and Thornthwaite Index and the average temperature for every one of the 254 counties in Texas. The input format is:

C1 - C7 County identification
 C8 - C16 Thornthwaite Index
 C27 - C37 Average temperature

Data Set #3 (DT DATA)

DT DATA stands for decision tree data. In this file maintenance strategies are assigned to every combination of factors that might come up in the maintenance analysis. The file consists of a three-

dimensional array of 7 by 28 by 4, which corresponds to seven pavement types, eight distresses with three levels of distress each, plus Serviceability Index with four levels, and four possible combinations of traffic and 18-kips.

Input form:

C1 - C11 Four strategies for possible traffic and 18-kip combinations

Lines	1 - 28	Distresses and PSI for Pavement Type 04
	29 - 56	Distresses and PSI for Pavement Type 05
	57 - 84	Distresses and PSI for Pavement Type 06
	85 - 112	Distresses and PSI for Pavement Type 07
	113 - 140	Distresses and PSI for Pavement Type 08
	141 - 168	Distresses and PSI for Pavement Type 09
	169 - 196	Distresses and PSI for Pavement Type 10

Data Set #4

This data set is the Texas Annual Statewide Survey data that will be analyzed by the program. The input format is as follows.

Card 1

CC	Format	Name	
1 - 4	I4	INYEAR	Year of the Survey
5 - 12	A8	RUNDAT	Date of the Run
13 - 15	I3	IHOR	Planning Horizon
16 - 18	I3	IPMNT	Maintenance Level

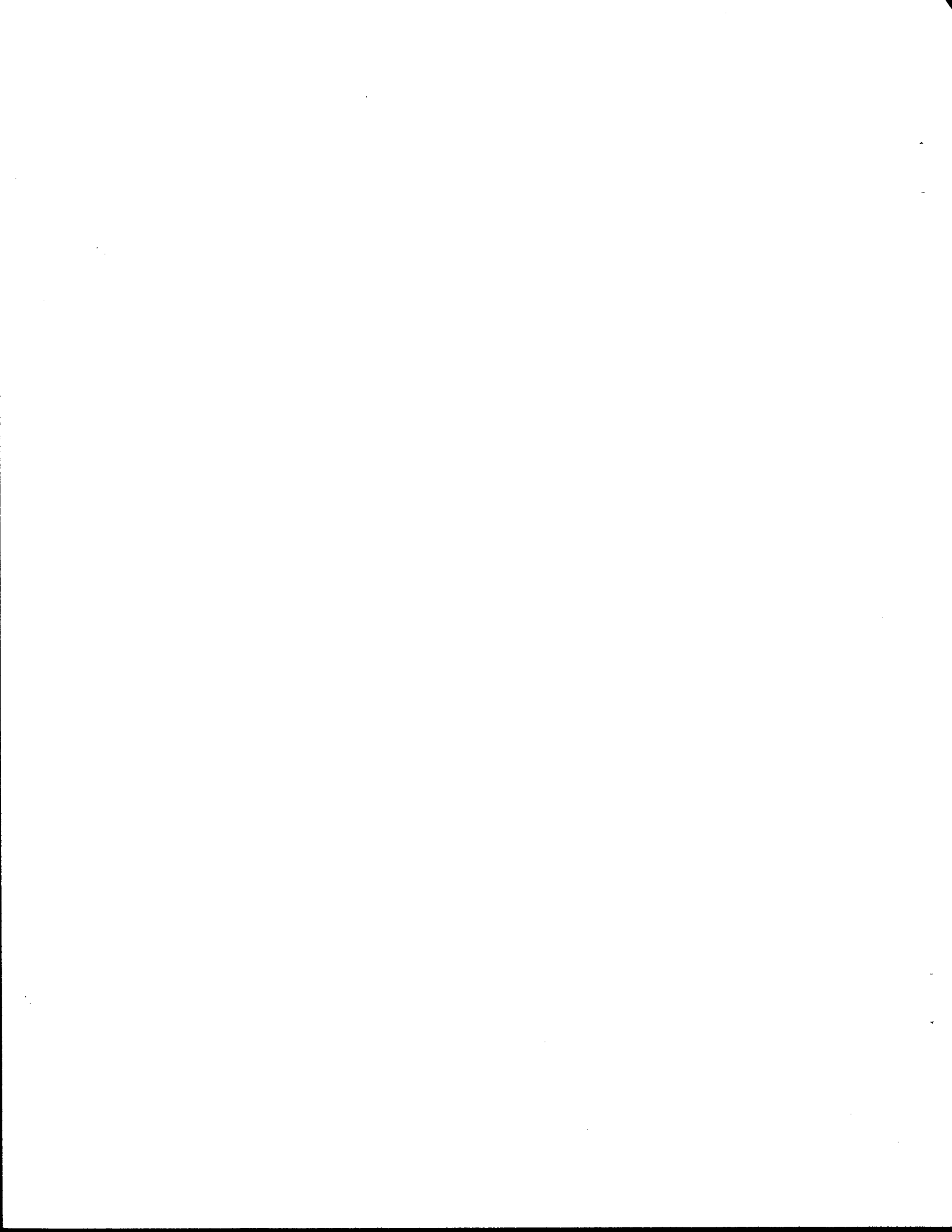
Card 2

CC	Format	Name	
1 - 2	I2	DIST	District Number
3 - 5	I3	CNTY	County Number
6 - 12	A7	HWAY	Highway ID Number

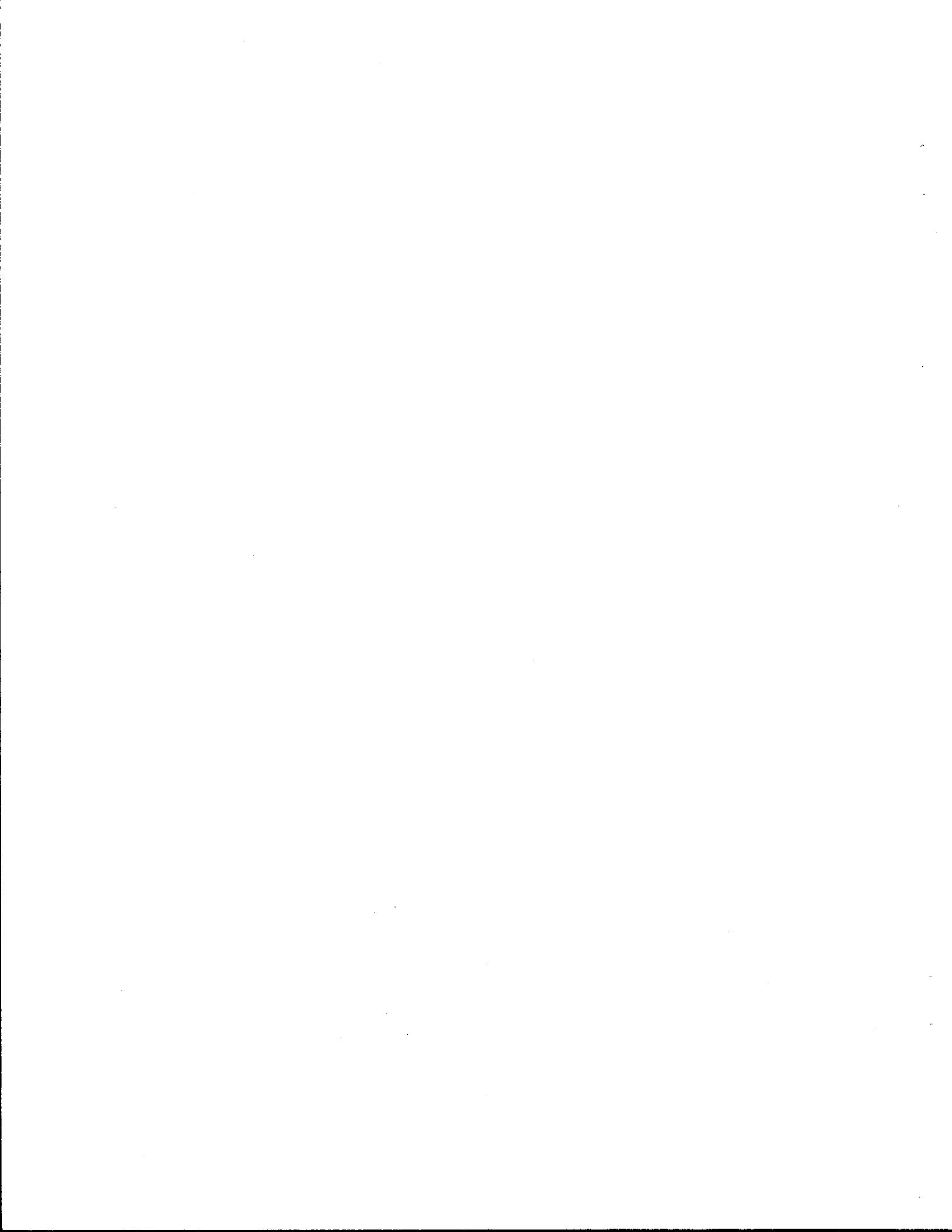
13 - 15	I3	BMIL	Beginning Milepost of section in analysis
16	A1	BSIGN	+ or - miles
17 - 18	F2.1	BDISP	Portion of road over or under milepost sign
19 - 21	I3	EMIL	Ending milepost
22	A1	ESIGN	+ or - miles
23 - 24	F2.1	EDISP	Portion over or under milepost
25	A1	LANE	Right or left lane
26	I1	LCOUNT	0
CC	Format	Name	
27 - 47	7I3	IVIS(I), I=1,7	Distress: rutting, raveling, flushing, failures, trans- versal cracking, alligator cracking, longitudinal cracking
48 - 49	F2.1	SRVC	Serviceability Index
50 - 51	F2.0	SKID	Skid resistance
52 - 53	I2	SLMT	Maximum velocity
54 - 55	I2	TYPE	Pavement type
56	I1	HWFC	Highway functional classifi- cation
57 - 58	I2	NLANES	Number of lanes
59 - 61	F3.0	WDTH	Width of the section
62 - 67	I6	ADTL	Average daily traffic for the next 20 years

68 - 72	I5	EALT	Average equivalent load for the next 20 years
73 - 74	F2.1	LGTH	Length of section
75 - 77	3X		
78 - 80	F3.2	AVUC	Adjusted visual utility score
81 - 83	I3	WVUC	Weighted visual utility score
84 - 86	F3.2	PESC	Pavement evaluation score
87 - 89	3X		
90 - 92	I3	SIUC	Serviceability Index utility score
93 - 95	I3	SKUC	Skid number utility score
CC	Format	Name	
96 - 98	I3	RMUC	Road maintenance utility score
99 -101	A3	RWAY	Roadway
102-112	11X		
113	I1	DESIGN	Design factor

Last Card: 99



COMPUTER CODE



```

***** CURRENT VESION AS OF MARCH 1, 1985 *****
.....................................................................
THIS PROGRAM PREDICTS PAVEMENT PERFORMANCE IN TERMS OF DISTRESS
AND PRESENT SERVICEABILITY INDEX FOR A TWENTY YEAR PERIOD.
THIS IS ACHIEVED THROUGH THE USE OF "S-SHAPED" CURVES OF THE
FORM:
                                BETA
DIST = EXP-(RHO/W)
.....................................................................
THE STRUCTURAL PERFORMANCE OF THE PAVEMENT IS EVALUATED IN
TERMS OF THE FOLLOWING DISTERSS TYPES:
.....................................................................
DISTRESS                                PRIMARY VARIABLE
1) RUTTING                               N-18
2) RAVELLING                             ADT
3) FLUSHING                              ADT
4) FAILURES                              N-18
5) ALLIGATOR CRACKING                    N-18
6) LONGITUDINAL CRACKING                 TIME
7) TRANSVERSAL CRACKING                  TIME
.....................................................................
DIMENSION  FRLANE(6), STGY(5), T(5), FUNC(7), PESM(7)
DIMENSION  PRTY(5), UCOST(5), SCOST(5,5), HMMILES(11,7)
DIMENSION  AUPL(6), ADTF(6), EUPL(3), EALF(3)
DIMENSION  COMPSC(8,3), FLEXSC(8,3), TCLS(5,10)
DIMENSION  FRTH(254), RAIN(254), SOILPI(254)
DIMENSION  RUPL(3), RFFR(3), FUPL(4), PFR(4)
DIMENSION  FAVU(10,5), FSDS(5,10), ECFS(25,5)
DIMENSION  FSIU(5,5), FSU(6,5), IVIS(7), V(8)
DIMENSION  IORDER(8), MMCODE(27), ITOTAL(2,7)
DIMENSION  SIBNRY(3,3), SNBRY(3,3), CMBNRY(3,3)
DIMENSION  TRAF(7,4), TRAF(7), TRAFD(7)
DIMENSION  ATNR(7,10), DSMAX(10), CLIF(3,4), SOLF(3,3)
DIMENSION  A(20), B(20), ENDVIS(8), VI(7), RVIS(8), RVIS(8)
DIMENSION  DISL(8,100), PSIL(50), V1(8), TIN(254), AVTP(254)
DIMENSION  OV2(4,4), OV3(4,4), BB2(4,2), BB3(4,2), RYCOST(20)
DIMENSION  MYCOST(20), RFCOST(20,7), IEXT(6), MFCOST(20,7)
DIMENSION  MSTRAT(14), UNITS(8), UNIT2(9), MTREE(7, 28, 4)
DIMENSION  IST(9), MAREA(9), DST(6), DAREA(6), DCOST(6)
INTEGER    BMIL,EMIL,WVUC,SIUC,SKUC,RMUC
INTEGER    STGY, TYPE, HWFC, DESIGN, LHI
INTEGER    DIST, CNTY, SLMT, ADTL, EALT, MNTH
INTEGER    RUT, RAVEL, FLUSH, FAIL, ALGCRK, TRNCRK
REAL*8     OLDHW, HWAY, RUNDAT, MMCODE, MMSIN, CMANT, CTOT
REAL*8     TMPSIV, TMPSNV, TMPAVU, JCOST, CSUM, SCOST, UCOST
REAL*8     CREHAB, CYMANT
REAL       LANE, LGTH, MYCOST, MFCOST
LOGICAL    FLAG, FRNTAG
DATA LINENO /00/
DATA MSTRAT / 'M-01', 'M-02', 'M-03', 'M-04', 'M-05', 'M-06',
1           'M-07', 'M-08', 'M-09', 'M-10', 'M-11', 'M-12',
2           'M-13', 'M-14' /
DATA IORDER / 6, 5, 4, 7, 3, 1, 2, 8 /
DATA OV2 /10.0,14.0,18.0,22.0,
1         18.0,22.0,26.0,30.0,
2         18.0,22.0,26.0,30.0,
3         14.0,18.0,22.0,26.0/
DATA OV3 /14.0,12.0,10.0,8.0,
1         8.0, 6.0, 4.0,2.0,
2         8.0, 6.0, 4.0,2.0,
3         10.0, 8.0, 6.0,4.0/
DATA BB2 /18.0,22.0,26.0,30.0,
1         14.0,18.0,22.0,26.0/

```

```

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1 2.0, 1.6, 1.2, 0.8/ 760
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1 'MMSFLEXF', 'MMSFREEZ', 'MMSFUNCL', 'MMSRAINS', 780
2 'MMSREAVU', 'MMSREHDS', 'MMSREHFS', 'MMSREHSI', 790
3 'MMSREHSN', 'MMSREHTC', 'MMS18KIP', 'MMSOILS', 800
4 'MMSICLC', 'MMSNCLC', 'MMSMCLC', 'MMSMINC', 810
5 'MMSADTBN', 'MMSKIPBN', 'MMSTMINI', 'MMSMAXDS', 820
6 'MMSREHEF', 'MMSREHSP', 'MMSMINPS' / 830
DATA OLDHW // 840
DATA FRLANE // 'A', 'B', 'C', 'X', 'Y', 'Z' // 850
DATA PRTY // '*' // 860
DATA STGY // 'R-01', 'R-02', 'R-03', 'R-04', 'R-05' / 870
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168 FORMAT ( 22X, F2.1) 970
184 FORMAT ( 22X, F3.1) 980
190 FORMAT ( 22X, F5.0) 990
206 FORMAT ( 22X, F2.2) 1000
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1 F3.0, I6, I5, F2.1, 3X, F3.2, I3, F3.2, 3X, 3I3, A3, 11X, I1) 1030
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2 27X, 'RUN DATE', A8, 27X, 'PAGE', I3, //, 1060
3 'PAVEMENT EVALUATION SYSTEM (PES) - PROGRAM NO. 413551', //, 1070
4 'REHABILITATION STRATEGY AND COST ESTIMATES - REPORT R06', //, 1080
5 'DISTRICT', I3, //) 1090
605 FORMAT(' COUNTY', I4, //) 1100
610 FORMAT ( ' +--- MILEPOST ---+ +--- PV', 1110
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2 '----- STRATEGY -----+', //, 1130
3 ' + & DISP. + F RD/ PVT. + SCO', 1140
4 'RE + +', 1150
5 'YEAR MAINT REHAB. +', //, 1160
6 'HIGHWAY BEGIN END C WAY TYPE MIN.', 1170
7 'CALC RUT1 RUT2 RAVL FLUS FAIL ALIG LONG TRNS PSI', 1180
8 ' COST COST') 1190
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1 15X, 'CODE', 16X, 'STRATEGY', 37X, 'CODE', 16X, 'STRATEGY', //, 1210
2 15X, 'M-01', 14X, 'SEAL CRACKS', 36X, 'R-01', 14X, 'SEAL COAT', //, 1220
3 15X, 'M-02', 14X, 'PATCH', 42X, 'R-02', 14X, 'THIN OVERLAY', //, 1230
4 15X, 'M-03', 14X, 'FULL DEPTH RPAIR', 31X, 'R-03', 14X, 'MEDIUM', 1240
5 'OVERLAY', //, 1250
6 15X, 'M-04', 14X, 'FOG SEAL', 39X, 'R-04', 14X, 'THICK OVERLAY', 1260
7 //, 15X, 'M-05', 14X, 'STRIP SEAL', 37X, 'R-05', 14X, 'RECONSTRUC', 1270
8 'TION', //, 15X, 'M-06', 14X, 'SEAL COAT', 38X, //, 1280
9 15X, 'M-07', 14X, 'ASPHALT-RUBBER SEAL', //, 1290
1 15X, 'M-08', 14X, 'SLURRY SEAL', //, 1300
2 15X, 'M-09', 14X, 'LEVEL UP', //, 1310
3 15X, 'M-10', 14X, 'THIN OVERLAY', //, 1320
4 15X, 'M-11', 14X, 'ROTOMILL', //, 1330
5 15X, 'M-12', 14X, 'SPOT SEAL', //, 1340
6 15X, 'M-13', 14X, 'ROTOMILL + SEAL', //, 1350
7 15X, 'M-14', 14X, 'ROTOMILL + OVERLAY', ///) 1360
621 FORMAT ( ' ') 1370
622 FORMAT (1X, A7) 1380
623 FORMAT (1H+, 7X, 2(1X, I3, A1, F3.1), 2X, I1) 1390
624 FORMAT (1H+, 27X, A3, 2X, I2, 3X, I3, 2X, I3, 3X) 1400
625 FORMAT (1H+, 92X, I4, 2X, A4, 12X, F9.2, //) 1410
626 FORMAT (1H+, 92X, I4, 2X, A4, 1X, A4, 7X, F9.2, //) 1420
627 FORMAT (1H+, 92X, I4, 2X, A4, 1X, A4, 1X, A4, 2X, F9.2, //) 1430
628 FORMAT (1H+, 92X, I4, 2X, 3(A4, 1X), //, 99X, A4, 12X, F9.2, //) 1440
629 FORMAT (1H+, 92X, I4, 2X, 3(A4, 1X), //, 99X, 2(A4, 1X), 6X, F9.2, //) 1450
630 FORMAT (1H+, 92X, I4, 2X, 3(A4, 1X), //, 99X, 3(A4, 1X), 1X, F9.2, //) 1460
632 FORMAT ( 1H+, 47X, 8(1X, F4.1), 1X, F3.1) 1470
634 FORMAT ( 1H+, 81X, //) 1480

```

635	FORMAT (1H+,119X, F8.2,/) 1490
636	FORMAT (41X, 'TOTALS:',5X, 'MAINT.', 1500
1	2X,F11.2,3X, 'REHAB.',2X, F11.2,2X,/) 1510
640	FORMAT (1H+, 92X,I4,2X, A4,21X, F9.2,/) 1520
641	FORMAT (1H+, 91X,I4,2X, '*', A4, '*',20X, F9.2,/) 1530
650	FORMAT (21X, '-----CUMULATIVE COSTS:',5X, 'MAINTENANCE', 1540
1	2X,F12.2,3X, 'REHABILITATION',2X, F12.2,2X, 1550
2	'-----',/) 1560
652	FORMAT(14X, 12(1H*), 1570
1	40H*MILES OF ROADWAY BREAKDOWN BY PAVEMENT , 1580
1	36HSCORE AND FUNCTIONAL CLASSIFICATION", 12(1H*), /, 1590
2	14X, 1H*, 98X, 1H*, /, 1600
3	14X, 1H*, 42X, 10HDISTRICT , I2, 44X, 1H*, /, 1610
4	14X, 1H*, 42X, 12H-----, 44X, 1H*, /, 1620
5	14X, 1H*, 98X, 1H*, /, 1630
6	14X, 1H*, 36X, 30HMILES IN EACH FUNCTIONAL CLASS, 32X, 1640
6	1H*, /, 14X, 1H*, 32X, 1H1, 9X, 1H2, 9X, 1H3, 9X, 1H4, 1650
7	9X, 1H5, 9X, 1H6, 9X, 1H7, 5X, 1H*, /, 1660
8	14X, 1H*, 29X, 7HINTERST, 3X, 7HURBANFW, 3X, 7HPRINCAR, 1670
9	3X, 7HMINORAR, 3X, 7HMAJORCL, 3X, 7HMINORCL, 1680
1	4X, 4HPARK, 4X, 1H*, /, 1690
1	14X, 1H*, 11X, 14HPAVEMENT SCORE, 73X, 1H*, /, 1700
1	14X, 1H*, 98X, 1H*) 1710
654	FORMAT(14X, 1H*, 13X, I2, ' THRU ', I2, 5X, 7(F6.1, 4X), 1720
1	1H*, /, 14X, 1H*, 98X, 1H*) 1730
656	FORMAT(14X, 1H*, 17X, '100', 8X, 7(F6.1, 4X), 1H*, /, 1740
2	14X, 1H*, 98X, 1H*, /, 1750
2	14X, 1H*, 98X, 1H*, /, 1760
2	14X, 100(1H*) 1770
657	FORMAT(14X, 1H*, 98X, 1H*, /, 1780
1	14X, 1H*, 98X, 1H*, /, 1790
1	14X, 1H*, 12X, 'MEAN SCORE', 6X, 7(I6,4X), 1H*, /, 1800
1	14X, 1H*, 98X, 1H*, /, 1810
1	14X, 1H*, 12X, 'SAMPLE SIZE', 5X, 7(I6,4X), 1H*, /, 1820
1	14X, 1H*, 98X, 1H*, /, 1830
1	14X, 1H*, 98X, 1H*, /, 1840
1	14X, 100(1H*) 1850
658	FORMAT(//,14X, 'NOTE - FRONTAGE ROADS (ROADWAYS A-C AND X-Z) ', 1860
1	'ARE CONSIDERED AS FUNCTIONAL CLASS 5 (MAJORCL).', /, 1870
1	14X, 'NOTE - SAMPLE SIZE INDICATES NO. OF ROADWAYS', 1880
1	' INSPECTED * CARE NEEDED WHEN INTERPRETING RESULTS') 1890
660	FORMAT(12X, 47(1H*), 15HCOST BREAKDOWN, 47(1H*),/, 1900
1	12X, 1H*, 107X, 1H*, /, 1910
2	12X, 1H*, 45X, 9HDISTRICT , I3, 50X, 1H*,/, 1920
3	12X, 1H*, 45X, 12H-----,50X, 1H*,/, 1930
4	12X, 1H*, 107X, 1H*, /, 1940
4	12X, 1H*, 107X, 1H*, /, 1950
5	12X, 1H*, 4X, 7HURGENCY,14X, 1960
5	35HCOSTS FOR EACH RECOMMENDED STRATEGY, 1970
6	20X,24HSUM OF % OF TOTAL, 3X, 1H*, /, 1980
7	12X,1H*,4X, 6HRATING,7X,4HR-01,7X,4HR-02,7X,4HR-03, 1990
7	7X,4HR-04,7X,4HR-05,16X,5HCOSTS,10X,5HCOSTS,6X,1H*) 2000
665	FORMAT(14X,27(1H*), 35HREHABILITATION AND MAINTENANCE COST, 2010
1	9H PER YEAR,29(1H*),/, 2020
2	14X,1H*,98X,1H*,/,14X,1H*,98X,1H*,/, 2030
3	14X,1H*, 20X, 4HYEAR, 15X,14HREHABILITATION, 15X, 2040
4	11HMAINTENANCE,19X,1H*,/, 14X, 1H*,44X,4HCOST,23X, 2050
5	4HCOST,23X,1H*,/,14X, 1H*, 98X, 1H*) 2060
666	FORMAT (14X, 1H*, 9X, 15HCUMULATIVE COST, 13X, F12.2, 16X 2070
1	F12.2, 21X, 1H*) 2080
667	FORMAT(14X, 1H*, 20X, I4, 13X, F12.2, 16X, F12.2,21X,1H*,/, 2090
1	14X,1H*,98X,1H*) 2100
668	FORMAT (14X, 100(1H*) 2110
669	FORMAT(14X,1H*, 98X, 1H*,/, 14X, 1H*, 98X, 1H*) 2120
670	FORMAT(12X, 1H*, 107X, 1H*,/, 2130
1	12X, 1H*, 5X, A4, 2X,5(F11.2), 12X, F11.2, 8X, F5.1, 5X, 1H*) 2140
671	FORMAT(12X, 19(1H*), 2150
1	38HREHABILITATION COST BREAKDOWN BY YEAR , 2160
1	29HAND FUNCTIONAL CLASSIFICATION,18(1H*),/, 2170
2	12X, 1H*,102X, 1H*, /, 2180
3	12X, 1H*, 44X, 10HDISTRICT , I2, 46X, 1H*, /, 2190
4	12X, 1H*, 44X, 12H-----, 46X, 1H*, /, 2200
5	12X, 1H*,102X, 1H*, /, 2210
6	12X, 1H*,102X, 1H*,/, 2220

C	4 DO 126 I = 1, 254	2970
	READ (25, 118, END=99) RAIN(I)	2980
	126 CONTINUE	2990
	GOTO 999	3000
C		3010
C	NOTE - ORDER STORED IS NOT ORDER READ IN.	3020
C		3030
	5 DO 128 I = 1, 8	3040
	DO 130 J = 1, 3	3050
	READ (25, 124, END=99) FLEXSC(IORDER(I),J)	3060
	130 CONTINUE	3070
	128 CONTINUE	3080
	GOTO 999	3090
C		3100
	6 DO 132 I = 1, 4	3110
	READ (25, 134, END=99) FUPL(I), FTFR(I)	3120
	132 CONTINUE	3130
	GOTO 999	3140
C		3150
	7 DO 136 I = 1, 7	3160
	READ (25, 138, END=99) FUNC(I)	3170
	136 CONTINUE	3180
	GOTO 999	3190
C		3200
	8 DO 140 I = 1, 3	3210
	READ (25, 142, END=99) RUPL(I), RFFR(I)	3220
	140 CONTINUE	3230
	GOTO 999	3240
C		3250
	9 DO 144 I = 1, 10	3260
	DO 146 J = 1, 5	3270
	READ (25, 138, END=99) FAVU(I,J)	3280
	146 CONTINUE	3290
	144 CONTINUE	3300
	GOTO 999	3310
C		3320
	10 DO 148 I = 1, 5	3330
	DO 150 J = 1, 10	3340
	READ (25, 125, END=99) FSDS(I,J)	3350
	150 CONTINUE	3360
	148 CONTINUE	3370
	GOTO 999	3380
C		3390
	11 DO 152 I = 1, 25	3400
	DO 154 J = 1, 5	3410
	READ (25, 156, END=99) ECFS(I,J)	3420
	154 CONTINUE	3430
	152 CONTINUE	3440
	GOTO 999	3450
C		3460
	12 DO 157 I = 1, 5	3470
	DO 158 J = 1, 5	3480
	READ (25, 168, END=99) FSIU(I,J)	3490
	158 CONTINUE	3500
	157 CONTINUE	3510
	GOTO 999	3520
C		3530
	13 DO 160 I = 1, 6	3540
	DO 162 J = 1, 5	3550
	READ (25, 118, END=99) FSKU(I,J)	3560
	162 CONTINUE	3570
	160 CONTINUE	3580
	GOTO 999	3590
C		3600
	14 DO 164 I = 1, 5	3610
	DO 166 J = 1, 10	3620
	READ (25, 168, END=99) TCLS(I,J)	3630
	166 CONTINUE	3640
	164 CONTINUE	3650
	GOTO 999	3660
C		3670
	15 DO 170 I = 1, 3	3680
	READ (25, 142, END=99) EUPL(I), EALF(I)	3690
		3700

170 CONTINUE		3710
GOTO 999		3720
C		3730
16 DO 174 I = 1, 254		3740
READ (25, 118, END=99) SOILPI(I)		3750
174 CONTINUE		3760
GOTO 999		3770
C		3780
17 DO 176 I = 1, 3		3790
DO 178 J = 1, 3		3800
READ (25, 168, END=99) SIBNRY(I,J)		3810
178 CONTINUE		3820
176 CONTINUE		3830
GOTO 999		3840
C		3850
18 DO 180 I = 1, 3		3860
DO 182 J = 1, 3		3870
READ (25, 184, END=99) SNBNRY(I,J)		3880
182 CONTINUE		3890
180 CONTINUE		3900
GOTO 999		3910
C		3920
19 DO 186 I = 1, 3		3930
DO 188 J = 1, 3		3940
READ (25, 190, END=99) CMBNRY(I,J)		3950
188 CONTINUE		3960
186 CONTINUE		3970
GOTO 999		3980
C		3990
20 DO 192 I = 1, 7		4000
DO 194 J = 1, 4		4010
READ (25, 138, END=99) TRAF(I,J)		4020
194 CONTINUE		4030
192 CONTINUE		4040
GOTO 999		4050
C		4060
21 DO 196 I = 1, 7		4070
READ (25, 190, END=99) TRAF(I)		4080
196 CONTINUE		4090
GOTO 999		4100
C		4110
22 DO 198 I = 1, 7		4120
READ (25, 168, END=99) TRAFD(I)		4130
198 CONTINUE		4140
GOTO 999		4150
C		4160
23 DO 200 I = 1, 7		4170
DO 202 J = 1, 10		4180
READ (25, 168, END=99) ATNR(I,J)		4190
202 CONTINUE		4200
200 CONTINUE		4210
GOTO 999		4220
C		4230
24 DO 204 I = 1, 10		4240
READ (25, 206, END=99) DSMAX(I)		4250
204 CONTINUE		4260
GOTO 999		4270
C		4280
25 DO 208 I = 1, 3		4290
DO 210 J = 1, 4		4300
READ (25, 168, END=99) CLIF(I,J)		4310
210 CONTINUE		4320
208 CONTINUE		4330
GOTO 999		4340
C		4350
26 DO 212 I = 1, 3		4360
DO 214 J = 1, 3		4370
READ (25, 138, END=99) SOLF(I,J)		4380
214 CONTINUE		4390
212 CONTINUE		4400
GOTO 999		4410
C		4420
27 DO 216 I = 1, 7		4430
READ (25, 206, END=99) PESM(I)		4440

216	CONTINUE	4450
	GOTO 999	4460
C		4470
C	INVALID TACS TABLE NAME CAUSES RUN ABORT.	4480
		4490
99	WRITE (6, 98) MMSIN, MMCODE(ICODE)	4500
98	FORMAT (10X, 21HERROR IN READING TACS, //,	4510
1	14HMMSCODE READ ,A8, 12HINSTEAD OF , A8)	4520
	GOTO 6000	4530
C		4540
C	REWIND SEQUENTIAL TACS TABLE FILE FOR SUBROUTINE BIGSUB,	4550
C	PGM. NO. 413550, USE.	4560
240	REWIND 25	4570
		4580
		4590
		4600
		4610
	*****	4620
	READ YEAR OF INSPECTION AND PGM. RUN DATE.	4630
	*****	4640
		4650
		4660
		4670
	READ (2,515) INYEAR, RUNDAT, IHOR, IPMNT	4680
	DO 50 I = 1,254	4690
50	READ(1,55)TIN(I),AVTP(I)	4700
55	FORMAT(7X,G9.3,20X, 20X, G10.3)	4710
	INP98 = 0	4720
	IREAD = 0	4730
	IALV = 0	4740
		4750
		4760
		4770
	*****	4780
	READ FIRST NON-98 AND NON-99 (DISTRICT) RECORD AND BEGIN WORK.	4790
	*****	4800
		4810
		4820
		4830
	WRITE (6,611)	4840
770	READ (2, 560) DIST, CNTY, HWAY, BMIL, BSIGN, BDISP,	4850
1	EMIL, ESIGN, EDISP, LANE, LCOUNT, (IVIS(I),I=1,7),	4860
2	SRVC, SKID, SLMT, TYPE, HWFC, NLANES, WPTH, ADTL,	4870
1	EALT, LGTH, AVUC, WVUC, PESC,	4880
3	SIUC, SKUC, RMUC, ROADWY, DESIGN	4890
		4900
		4910
		4920
	*****	4930
	THIS SUBROUTINE CHANGES THE FIELD RATING, E.G. 100, 010, 001,	4940
	TO A PERCENTAGE OF THE AREA. RANGE FROM 0 TO 100.	4950
	*****	4960
		4970
		4980
		4990
	CALL DECODE(IVIS,RVIS,RVISO)	5000
	IDIST = DIST	5010
		5020
	DISTRICT = 99 -- END OF RUN.	5030
	DISTRICT = 98 -- NO RECORDS SELECTED FOR REPORTING FOR	5040
	THE USER-SUBMITTED REPORT REQUEST.	5050
		5060
	IF(DIST.EQ.99) GO TO 6000	5070
	IF(DIST.NE.98) GO TO 780	5080
	DIST=1000	5090
	IPAGE=1	5100
	LINENO=0	5110
	WRITE (6,600) RUNDAT, IPAGE, INDIST	5120
	WRITE (6,690)	5130
	WRITE (6,681)	5140
	INDIST=0	5150
	IPAGE=0	5160
	GOTO 770	5170
780	CONTINUE	5180

CCCCCCCCCCCC

INITIALISE COST ARRAYS PREPARATORY TO BEGINNING NEW SEGMENT.

THESE ARRAYS ARE USED IN DETERMINING COST FIGURES FOR OUTPUT
AT THE END OF A REPORT REQUEST OR END OF A DISTRICT WHICHEVER
COMES FIRST.

```

840 DO 850      IZ1= 1,5
      UCOST(IZ1) = 0.0
850 CONTINUE
      DO 841      IZ1 = 1,20
      RYCOST(IZ1) = 0.0
      MYCOST(IZ1) = 0.0
841 CONTINUE
      DO 842      IZ1 = 1,20
      DO 843      IZ2 = 1,7
      RFCOST(IZ1,IZ2) = 0.0
843 CONTINUE
842 CONTINUE
      DO 844      IZ1 = 1,20
      DO 845      IZ2 = 1,7
      MFCOST(IZ1,IZ2) = 0.0
845 CONTINUE
844 CONTINUE
      DO 860      IZ1= 1, 5
      DO 870      IZ2=1, 5
      SCOST (IZ1,IZ2) = 0.0
870 CONTINUE
860 CONTINUE
      DO 880      IZ1 = 1 ,11
      DO 890      IZ2 = 1, 7
      HMILES(IZ1,IZ2) = 0.0
890 CONTINUE
880 CONTINUE
      DO 892      IZ1 = 1, 2
      DO 894      IZ2 = 1, 7
      ITOTAL(IZ1,IZ2) = 0
894 CONTINUE
892 CONTINUE
      CSUM      = 0.0
      CHANT     = 0.0
      CTOT     = 0.0
      MNTH     = 0
      IPAGE    = 0
      CREHAB   = 0.0

```

ASSIGN BASIC PROGRAM VARIABLES AND CALCULATE ITEMS SUCH AS
ADT, 18-KIP, AND SURFACE WIDTH FOR THE ROADWAY.

ADT IS INPUT TO THIS ROUTINE AS ALL LANES BOTH DIRECTIONS.
18-KIP IS INPUT AS ALL LANES IN ONE DIRECTION ONLY.
SURFACE WIDTH (FOR COST COMPUTATIONS) IS JUDGED TO BE
ALL LANES FOR HWY. DESIGN 1 AND 2,
0.5 OF TOTAL FOR ALL OTHER HWY. DESIGNS, OR
EXACTLY 24.0 FEET FOR ANY FRONTAGE LANE ROADWAY NO
MATTER WHAT HWY. DESIGN.

```

895 IT      = TYPE
      IFIST = 0
      CYMANT = 0.0
      ISWITH = 0
      FLAG   = .FALSE.

```

5190
5200
5210
5220
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5240
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5270
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5920

CCCCCCCCCCCC

```

IC          = HWFC
INX = 0
JK = 0
IENT = 0
C          5930
C          5940
C          5950
C          5960
C          5970
C          5980
C          5990
C          6000
C          6010
C          6020
C          6030
C          6040
C          6050
C          6060
C          6070
C          6080
C          6090
C          6100
C          6110
C          6120
C          6130
C          6140
C          6150
C          6160
C          6170
C          6180
C          6190
C          6200
C          6210
C          6220
C          6230
C          6240
C          6250
C          6260
C          6270
C          6280
C          6290
C          6300
C          6310
C          6320
C          6330
C          6340
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C          6370
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C          6590
C          6600
C          6610
C          6620
C          6630
C          6640
C          6650
C          6660

          DETERMINE IF FRONTAGE ROAD OR NOT.  IF SO, SET FLAG FOR
          FURTHER CALCULATIONS BELOW.

          FRNTAG = .FALSE.
          DO 902 ILNO = 1, 6
          IF ( LANE.EQ.FRLANE(ILNO)) GOTO 905
902 CONTINUE
          GOTO 910
905 IC = 5
          FRNTAG = .TRUE.
910 CONTINUE
          IF (DESIGN .GT. 2) WPTH = WPTH / 2.0
          IF (FRNTAG) WPTH = 24.0
          RFAL = RAIN(CNTY)
          FTCC = FRTH(CNTY)
          PLSX = SOILPI(CNTY)
          AADT = FLOAT(ADTL)/2.0
          AKIP = FLOAT(EALT)/1000.0
          IF (NLANES .LE. 3) GOTO 930
          IF (NLANES .GT. 4) GOTO 920
          AADT = AADT * .60
          AKIP = AKIP * .70
          GOTO 930
920 AADT = AADT * .40
          AKIP = AKIP * .50
930 CONTINUE
C          4 LANE 60/40 SPLIT          MORE THAN 4 LANES 40/60 SPLIT IN AADT
          IF ( .NOT. FRNTAG ) GOTO 950
          AADT = 0.05 * AADT
          AKIP = 0.05 * AKIP
950 PSMN = PESM(IC)
          ADTS = AADT * SLMT

          *****
          SELECT/GENERATE TRANSITION MATRICES BASED ON PAVEMENT
          TYPE.
          DISL --- DISTRESS TRANSITION MATRIX
                   (100 X 7)  R R F F A L T
          T(I,J) --- FINAL STATE GIVEN CURRENT STATE I
                   AND DISTRESS TYPE J
          PSIL (50 X 1)  T(I) FINAL PSI GIVEN INITIAL PSI
                   VALUE = I/10
          *****

          SELECT FROM 4 SUBROUTINES DEPENDING ON PAVEMENT TYPE:

          PAV. TYPE          SUBROUTINE

          4          BLACK BASE (BB)
          5          HOT MIX (HM)
          6          HOT MIX (HM)
          7          OVERLAY (OV)
          8          OVERLAY (OV)
          9          OVERLAY (OV)
          10         SURF. TREAT. (ST)

          *****
          *****SURFACE TREATMENT*****

          IF (TYPE.NE.10) GO TO 970
          N18MTH = (EALT = 1000.0)/240.0
          IF (N18MTH.LT.3000.0) GO TO 659
          ID = 2
          GO TO 989
C          659 CONTINUE

```

	KPESC = INT((PESC + 0.001) * 100.0)	6670
	IF (KPESC.LT.80) GO TO 960	6680
	DMD = 1.06	6690
	GO TO 964	6700
960	IF (KPESC.LT.60) GO TO 961	6710
	DMD = 1.30	6720
	GO TO 964	6730
961	IF (KPESC.LT.40) GO TO 962	6740
	DMD = 1.55	6750
	GO TO 964	6760
962	IF (KPESC.LT.20) GO TO 963	6770
	DMD = 1.80	6780
	GO TO 964	6790
963	DMD = 2.04	6800
964	CONTINUE	6810
	FLEXL = 6.0	6820
	IF (AADT .GT. 400) FLEXL = 8.0	6830
	IF (AADT .GT. 750) FLEXL = 10.0	6840
	CALL ST(CNTY, IT, PESC, TIN, FRTH, AVTP, DMD, PLSX, FLEXL, DISL, PSIL,	6850
	1 EALT, AADT, AKIP)	6860
	GO TO 995	6870
C		6880
C	*****BLACK BASE*****	6890
C		6900
970	IF (TYPE.NE.4) GO TO 980	6910
	ASPH = 7.50	6920
	KPESC = INT((PESC + 0.001) * 100.0)	6930
	ID = IT - 3.0	6940
	IF (KPESC.LT.25) GO TO 971	6950
	IF (KPESC.LT.51) GO TO 972	6960
	IF (KPESC.LT.76) GO TO 973	6970
	HPR2 = BB2(4, ID)	6980
	HPR3 = BB3(4, ID)	6990
	GO TO 974	7000
971	HPR2 = BB2(1, ID)	7010
	HPR3 = BB3(1, ID)	7020
	GO TO 974	7030
972	HPR2 = BB2(2, ID)	7040
	HPR3 = BB3(2, ID)	7050
	GO TO 974	7060
973	HPR2 = BB2(3, ID)	7070
	HPR3 = BB3(3, ID)	7080
974	CONTINUE	7090
C		7100
	CALL BB (CNTY, IT, PESC, TIN, FRTH, AVTP, ASPH, HPR2, HPR3, PLSX,	7110
	1 DISL, PSIL, EALT)	7120
	GO TO 995	7130
C		7140
C	*****HOT MIX*****	7150
C		7160
980	IF (IT.NE.5. AND . IT.NE.6) GO TO 988	7170
	IF (IT.EQ.6) GO TO 981	7180
	HMAC = 4.0	7190
	GO TO 983	7200
981	HMAC = 2.0	7210
983	CONTINUE	7220
	KPESC = INT((PESC+0.001) * 100)	7230
	ID = IT - 4.0	7240
	IF (KPESC.LT.25) GO TO 984	7250
	IF (KPESC.LT.51) GO TO 985	7260
	IF (KPESC.LT.76) GO TO 986	7270
	HPR2 = OV2(4, ID)	7280
	HPR3 = OV3(4, ID)	7290
	GO TO 987	7300
984	HPR2 = OV2(1, ID)	7310
	HPR3 = OV3(1, ID)	7320
	GO TO 987	7330
985	HPR2 = OV2(2, ID)	7340
	HPR3 = OV3(2, ID)	7350
	GO TO 987	7360
986	HPR2 = OV2(3, ID)	7370
	HPR3 = OV3(3, ID)	7380
987	CONTINUE	7390
	CALL HM(CNTY, IT, PESC, TIN, FRTH, AVTP, HMAC, HPR2, HPR3, PLSX,	7400

```

1          DISL,PSIL,EALT)
GO TO 995
*****OVERLAY*****
988 CONTINUE
  ID = IT - 5.0
989 KPESC = INT((PESC+0.001) * 100.0)
  OVTH = 2.0
  IF (KPESC.LT.25) GO TO 990
  IF (KPESC.LT.51) GO TO 991
  IF (KPESC.LT.76) GO TO 992
  HPR2 = OV2(4, ID)
  HPR3 = OV3(4, ID)
  GO TO 993
990 HPR2 = OV2(1, ID)
  HPR3 = OV3(1, ID)
  GO TO 993
991 HPR2 = OV2(2, ID)
  HPR3 = OV3(2, ID)
  GO TO 993
992 HPR2 = OV2(3, ID)
  HPR3 = OV3(3, ID)
993 CONTINUE
  CALL OV (CNTY, IT, PESC, TIN, FRTH, AVTP, HPR2, HPR3, OVTH, PLSX,
1          DISL, PSIL, EALT)
995 CONTINUE

*****
DETAIL LINE PRINT CONTROL
*****

PAGE EJECT - NEW COUNTY (NEW DIST.), NEW R06 REQUEST,
50 OR MORE DETAIL LINES PRINTED (48 OR MORE IF
STARTING NEW HIGHWAY IN SAME CO. OF SAME REQ.).
BLANK LINE - STARTING NEW HIGHWAY IN SAME CO. OF SAME REQ.
NO CO. OR HWY. - ON SAME HIGHWAY IN SAME CO. OF SAME REQ.
BUT STARTING NEW PES SEGMENT.
NO CO., HWY., SEGMENT POST INFO., OR FUNC. CLASS. - OTHER
THAN 1ST RECORD ASSOCIATED WITH ONE PES SEGMENT ON
SAME HIGHWAY IN SAME CO. OF SAME REQ.

IF (INP98.NE.0) GO TO 1000
IF (CNTY.NE.ICNTY) GO TO 1000
IF (LINENO.GE.40) GO TO 1000
IF (HWAY.NE.OLDHW) GO TO 1100
GO TO 1200
1000 IPAGE = IPAGE + 1
  LINENO = 0
  WRITE(6,600) RUNDAT, IPAGE, DIST
  WRITE (6,605) CNTY
  WRITE(6,610)
1100 IF (LINENO.GE.38) GO TO 1000
  LINENO = LINENO + 2
  WRITE(6,621)
  WRITE(6,622) HWAY
  GO TO 1300
1200 LINENO = LINENO + 1
  WRITE(6,621)
  IF (LCOUNT.GT.1) GO TO 1400
1300 WRITE(6,623) BMIL, BSIGN, BDISP, EMIL, ESIGN, EDISP, HWFC
  MNTH = 0
  ISTEP = 0
1400 CONTINUE
  KPESC = INT(PESC * 10.)
  KPESC = KPESC + 1
  HMMILES(KPESC, IC) = HMMILES(KPESC, IC) + LGTH
  LPESC = INT(PESC * 100.)
  ITOTAL( 1, IC) = ITOTAL( 1, IC) + LPESC
  ITOTAL( 2, IC) = ITOTAL( 2, IC) + 1
  RFAL = RAIN(CNTY)

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8080
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C *****
C      WHEN THE PAVEMENT SCORE IS LESS THAN 75 BUT GREATER THAN
C      THE MINIMUM ALLOWABLE SCORE, THE PROGRAM WILL SELECT A
C      PREVENTIVE MAINTENANCE STRATEGY. THE ONLY EXCEPTION TO
C      THE RULE IS WHEN IT IS MORE COST EFFECTIVE TO HAVE A MAJO
C      BILITATION THAT WILL LAST X NUMBER OF YEARS, THAN TO HAVE
C      MANY MAINTENANCE STRATEGIES THAT WILL LAST Y NUMBER OF YEARS
C      WHERE X = NY
C *****
C
C      SUBROUTINE                PURPOSE
C
C      MAITRE                    SUBROUTINE THAT WILL SET UP
C                                THE INPUT TO SUBROUTINE TRE
C      TRE                        SELECT BEST PREVENTIVE MAINT.
C                                STRATEGY (OR UP TO 5 STRAT.)
C      IMPROV                     RESET DISTRESSES ACCORDING TO
C                                MAINT. STRAT. SELECTED.
C      TEST                       ECONOMIC ANALYSIS.
C      SORT                       ARRANGE IN NUMERICAL ORDER
C                                THE MAINTENANCE STRATEGIES
C                                SELECTED.
C      SCORE                      CALCULATE THE NEW SCORE.
C
C 1600 CONTINUE
C      CALL MAITRE (DIST,CNTY,HWAY,BMIL,BSIGN,BDISP,EMIL,ESIGN,EDISP,
C 1      LANE,IVIS,SRVC,IT,IC,NLANES,WDTH,ADTL,EALT,
C 2      KPESC,LHI,JX,RCOST,RVIS,RVISO,IEXT,ISTE,IALV,
C 3      IST,MAREA,DST,DAREA,DCOST,JS,TOT)
C      IF (IT.EQ.10) GO TO 1601
C      IF (KPESC.GT.KPESM) GO TO 1603
C 1601 IF (KPESC.GE.KPESM + 7) GO TO 1603
C      IENT = 1
C      IMY = 0
C      GO TO 1610
C 1603 IENT = 0
C      IF (IEXT(1).EQ.0) GO TO 4000
C      CALL SORT(IEXT,ISTE)
C      ISWITH = 1
C 1602 CALL IMPROV(IEXT,ISTE,RVIS,SRVC)
C      CALL SCORE(RVIS,SRVC,V,FLEXSC,ADTS,SIBNRY,FUNC,IC,IAVUC,
C 1      ISIUC,PESC,IT)
C      RMPT = IPMNT/100.0
C      IF (PESC.GT.RMPT) GO TO 1604
C      ISTE = 1
C      IEXT(1) = 10
C      TOT = 2.55 * LGTH * WDTH * 586.7
C      GO TO 1602
C 1604 CONTINUE
C      IMY = 0
C      IF (IEXT(1).EQ.10) GO TO 1607
C      IF (ISTE.GT.3) GO TO 1607
C      JX = 1
C      GO TO 1610
C 1607 JX = 2
C 1610 CONTINUE
C
C      INX = 0
C      CALL TEST (DIST,IT,J,AVUC,SRVC,SKID,FAVU,FSIU,FSKU,LGTH,
C 1      AVU,SIV,SNV,RVIS,ENDVIS,V,FLEXSC,ADTS,WDTH,
C 2      SIBNRY,FUNC,IC,JX,RVISO,TCLS,ISWITH,ECPS,TOT,
C 3      DISL,PSIL,IAVUC,ISIUC,PESC,PESM,INX,V1,SIV1,
C 4      CNTY,IT,TIN,FRTH,AVTP,PLSX,
C 1      OV2,OV3,BB2,BB3,OVTH,ASPH,DMD,
C 2      FLEXL,EALT,HPR2,HPR3,AADT,AKIP,HMAC,IENT)
C
C      INX = 0
C      JX = J
C      IF (J.EQ.3) IMY = 0
C      IF (J.EQ.3) GO TO 3200
C 1675 IF (ISTE.GT.5) GO TO 1760

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IF (ISTE.EQ.5) GO TO 1755
IF ( ISTE.GE.4 ) GO TO 1750
IF ( ISTE.EQ.3 ) GO TO 1725
IF ( ISTE.EQ.2 ) GO TO 1700
WRITE(6,625) IYR, MSTRAT(IEXT(1)), TOT
GO TO 1775
1700 WRITE(6,626) IYR, (MSTRAT(IEXT(1)),I=1,2), TOT
GO TO 1775
1725 WRITE(6,627) IYR, (MSTRAT(IEXT(1)), I=1,3), TOT
GO TO 1775
1750 WRITE(6,628) IYR, (MSTRAT(IEXT(1)), I=1,4), TOT
GO TO 1775
1755 WRITE(6,629) IYR, (MSTRAT(IEXT(1)), I=1,5), TOT
GO TO 1775
1760 WRITE(6,630) IYR, (MSTRAT(IEXT(1)), I=1,6), TOT
1775 CONTINUE
IFIST = 1
CYMANT = CYMANT + TOT
CMANT = CMANT + TOT
MFCOST(IY,IC) = MFCOST(IY,IC) + TOT
MYCOST(IY) = MYCOST(IY) + TOT
IF(IMY.EQ.1) GO TO 3030
GO TO 4000

```

```

*****
WHEN PRESENT PAVEMENT SCORE L.T. MINIMUM, CALCULATE
REHABILITATION NEEDS.
*****

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```

WHEN THE ROADWAY'S PAVEMENT SCORE IS UNDER THE MINIMUM
ALLOWED FOR THAT PAVEMENT TYPE USED IN THE GIVEN FUNCTIONAL
CLASS, REHAB. IS REQUIRED AND A SERIES OF COMPUTATIONS ARE
MADE. THE MINIMUM ACCEPTABLE LIFE OF A REHABILITATED
PAVEMENT GIVEN THE SAME PAVEMENT TYPE AS IN PLACE AND THE
SAME FUNCTIONAL CLASS AS PRESENT IS GAINED. FOR EACH OF
5 POSSIBLE REHAB. STRATEGIES (PROGRESSIVELY MORE ALL-
ENCOMPASSING), THE ESTIMATED REHABILITATED PAVEMENT SCORE
IS COMPUTED AND RUN THRU DETERIORATION CALCULATIONS TO
GAIN THE LIFE EXPECTANCY. THIS EXPECTED LIFE IS COMPARED
TO THE MINIMUM ALLOWABLE TO DETERMINE WHICH OF THE 5
STRATEGIES HAS THE NEAREST ABOVE-MINIMUM LIFE AND THAT ONE
IS CHOSEN AS THE STRATEGY TO USE. GIVEN THE CHOSEN STRATEGY,
COST OF REHABILITATION IS COMPUTED AND RUNNING TOTAL FOR THE
DISTRICT OR REPORT-REQUEST DISTRICT PORTION (WHICHEVER IS
LESS) ARE KEPT. AN URGENCY-OF-REHAB-NEED IS THEN CREATED
BY DETERMINING JUST HOW FAR BELOW THE MINIMUM ALLOWABLE THE
PRESENT PAVEMENT SCORE IS. THE 5 STRATEGY LIVES, CHOSEN
STRATEGY AND ITS COST, AND THE URGENCY DETERMINATION ARE
THEN PRINTED.
NEW TRANSITION MATRICES ARE CREATED FOR THE REHABILITATED
SECTION AND THEN THE SECTION IS AGED BASED ON THE NEW
MATRIX. IF STRATEGIES 1 OR 2 ARE SELECTED, THE PROGRAM WILL
TRY TO SELECT A MAINTENANCE STRATEGY INSTEAD.

```

THE SEQUENCE OF SUBROUTINES IS AS FOLLOWS:

SUBROUTINE	PURPOSE	
FINDTI	ASSIGN MINIMUM LIFE FOR MAINTENANCE STRATEGY	10210
FINAVU	CALCULATE ESTIMATED ADJUSTED VISUAL UTILITY(AVU), ESTIMATED SKID NUMBER(SN), AND ESTIMATED SERVICEABILITY INDEX(SI) FOR EACH OF 5 MAINTENANCE STRATEGIES.	10220
SCORE	CALCULATE THE NEW SCORE FOR EACH MAINTENANCE STRATEGY.	10230
FITMAX	CALCULATE EXPECTED LIFE FOR EACH MAINTENANCE STRATEGY.	10240
LIMIT	PLACE ALIMIT UPON STRAT. SELECTION	10250
SURVTA	ASSIGN NEW VALUES FOR GENERATION OF TRANSITION MATRIX AFTER REHAB.	10260
AGING	AGE DISTERSSES	10270
SCORE	CALCULATE PES.	10280
		10290
		10300
		10310
		10320
		10330
		10340
		10350
		10360


```

C
C
C
2001 CALL FINDTI ( IC, IT, ATNR, TMNI )
      ISWITH = 1
      INX = 0
      MNTH = 0
      TMIN = TMNI * TF
C
C
C
      LOOP THRU ONCE FOR EACH OF 5 REHAB. STRATEGIES.
C
C
      DO 3000 J = 1, 5
      IF ((IT.EQ.7) .OR. (IT.EQ.8)) GOTO 3010
      CALL FINAVU ( IT, J, AVUC, SRVC, SKID, FAVU, FSIU, FSKU,
1        AVU, SIV, SNV, RVIS,ENDVIS)
      GOTO 3020
3010 CALL FINAVU ( IT, J, AVUC,SRVC, SKID, FAVU, FSIU, FSKU,
1        AVU, SIV, SNV, RVIS,ENDVIS)
3020 CONTINUE
C
      ISKUC = 1000
      IRMUC = 1000
      CALL SCORE (ENDVIS,SIV,V,FLEXSC,ADTS,SIBNRY,FUNC,IC,IAVUC,ISIUC,
1        PESF,IT)
      CALL FITMAX ( J, IT, RFAL, RUPL, FTCC, FUPL, PLSX, TF,
1        PESF, PSMN, TCLS, FSDS, CLIF, SOLF, TMAX)
      T(J) = TMAX
      IF ( FLAG ) GO TO 3000
      IRMS = STGY(J)
      JX = J
      PESF = PESF
      WVUC = IAVUC
      SIUC = ISIUC
      SKUC = ISKUC / 10
      RMUC = IRMUC / 10
      DO 2555 I=1,8
      V1(I) = ENDVIS(I)
2555 CONTINUE
      SIV1 = SIV
      IF ( TMAX .GE. TMIN ) FLAG = .TRUE.
3000 CONTINUE
      FLAG = .FALSE.
C
C
C
      USE SUBROUTINE LIMIT TO CHECK FOR HIGH VOL ROADS WITH NO LOAD
      ASSOCIATED DISTRESS OR LOW VOLUME ROADS
      FOR EITHER SET JMAX AS MAXIMUM STRATEGY LEVEL
C
C
      JSET = 0
      CALL LIMIT ( IVIS, SRVC, IC, AADT, JMAX)
      IF ( JMAX .EQ. 0) GOTO 3100
      IF ( JX .LE. JMAX) GOTO 3100
      JX = JMAX
      IRMS = STGY(JMAX)
      JSET = 1
3100 AREA = LGTH*WDTH
      IRMS = STGY(JX)
      ISTEP = 0
      IF (JX.GT.2) GO TO 3200
      GO TO 1600
C
C
C
      *****
      CREATE A NEW TRANSITION MATRIX FOR THE REHABILITATED SECTION
      USING THE END DISTRESSES AND END PAVEMENT SCORE.
      INPUTS TO THIS SUBROUTINE ARE ALL THE NECESSARY VARIABLES FOR
      FOR THE SECTION, PLUS, THE REHAB. STRATEGY TO BE USED.
      OUTPUT FOR THIS SUBROUTINE IS THE NEW TRANSITION MATRIX.
      *****
C
3200 CONTINUE

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10690
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11090
11100

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          IFIST = 1
          AREA = LGTH*WDTH
          IRMS = STGY(JX)
C
3030 CALL SURVTA (CNTY,JX,IT,PESC,TIN,FRTH,AVTP,PLSX,
1             OV2,OV3,BB2,BB3,OVTH,ASPH,DMD,DISL,
2             FLEXL,PSIL,EALT,HPR2,HPR3,AADT,AKIP,HMAC)
          JCOST = AREA * ECFS(DIST,JX)
          IF(IMY.EQ.1) GO TO 4000
C
          ROUND JCOST TO NEAREST 100.00.  ENTIRE REPORT WILL THEN SHOW
          COST FIGURES IN HUNDREDS.
C
          JCOST = (JCOST + 50.00) / 100.00
          KOST = JCOST
          KOST = KOST * 100
          IF(KOST.LT.100) KOST=100
          JCOST = KOST
          PDIF = (PESM(IC) - PESC)*10.
          IPDIF = INT(PDIF) + 1
          IF ( IPDIF .GT. 5 ) IPDIF = 5
          URGENCY = PRTY(IPDIF)
C
          COMPUTE SUMS FOR TABLES AT END OF DISTRICT OR REQUEST
          WHICHEVER COMES FIRST.
C
          CREHAB = CREHAB + JCOST
          CSUM = CSUM + JCOST
          SCOST (IPDIF,JX) = SCOST (IPDIF,JX) + JCOST
          UCOST (IPDIF) = UCOST (IPDIF) + JCOST
          IF ( JSET .EQ. 1 ) GOTO 3040
          WRITE (6,640) IYR, IRMS, JCOST
          GOTO 3050
3040 WRITE (6,641) IYR, IRMS, JCOST
3050 CONTINUE
          INDIST = DIST
          INP98 = 0
          DO 95 I = 1,8
295 RVIS(I) = V1(I)
          SRVC = SIV1
          RFCOST(IY,IC) = RFCOST(IY,IC) + JCOST
          RYCOST(IY) = RYCOST(IY) + JCOST
          GO TO 6998
4000 CONTINUE
          INDIST = DIST
          INP98 = 0
C
          *****
          CALCULATE DISTRESSES FOR NEXT YEAR, AND SCORE FOR NEXT YEAR.
          INPUT IS: ACTUAL DISTRESSES, PAVEMENT TYPE, STRATEGY, AND
          TRANSITION MATRIX.
          OUTPUT IS: NEW DISTRESSES, AND PAVEMENT SCORE.
          *****
C
          IF (IFIST.EQ.1) GO TO 6990
          WRITE(6,634)
C
          CALL AGING (JX,IT,RVISO,TCLS,INX,ISWITH, RVIS,SRVC,DISL,PSIL)
6990 CONTINUE
          CALL SCORE (RVIS,SRVC,V,FLEXSC,ADTS,SIBNRY,FUNC,IC,IAVUC,ISIUC,
1             PESC,IT)
          IFIST = 0
          LINENO = LINENO + 1
          IF (LINENO.GE.37) GO TO 6999
          GO TO 6998
6999 IPAGE = IPAGE + 1
          LINENO = 0
          WRITE (6,600) RUNDAT, IPAGE, DIST

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WRITE (6,605) CNTY
WRITE (6,610)
WRITE (6,621)
WRITE(6,622) HWAY
WRITE(6,623) BMIL, BSIGN, BDISP, EMIL, ESIGN, EDISP, HWFC
*****
                                END OF THE 20 YEAR LOOP
*****
6998 CONTINUE
IYR = INYEAR + IY
7000 CONTINUE
IF (IHOR.EQ.1) GO TO 1199
WRITE(6,636) CYMANT, CREHAB
CREHAB = 0.0
CYMANT = 0.0
LINENO = LINENO + 2
C
1199 CONTINUE
*****
ALL READS OTHER THAN 1ST READ OF NON-98 AND NON-99 DISTRICT
INPUT RECORDS ARE DONE BELOW.
*****
4010 READ ( 2, 560) DIST, CNTY, HWAY, BMIL, BSIGN, BDISP,
1      EMIL, ESIGN, EDISP, LANE, LCOUNT, (IVIS(I),I=1,7),
2      SRVC, SKID, SLMT, TYPE, HWFC, NLANES, WIDTH, ADTL,
1      EALT, LGTH, AVUC, WVUC, PESC,
3      SIUC, SKUC, RMUC, ROADWY, DESIGN
IF ( DIST .EQ. 99 ) GOTO 6000
*****
THIS SUBROUTINE CHANGES THE FIELD RATING, E.G. 100, 010, 001,
TO A PERCENTAGE OF THE AREA. RANGE FROM 0 TO 100.
*****
CALL DECODE(IVIS,RVIS,RVISO)
IF ( DIST .NE. 98 ) GOTO 4020
INP98 = INP98 + 1
IF (INP98.GT.1) GO TO 4025
GO TO 4100
4025 INDIST=1000
IPAGE = 1
LINENO = 0
WRITE (6,600) RUNDAT, IPAGE, INDIST
WRITE (6,690)
WRITE (6,681)
INDIST=98
IPAGE=0
GO TO 4010
4020 IF (INDIST.EQ.98) GO TO 840
IF ( SRVC .LT. 0.1 ) GOTO 4010
IF (DIST.EQ.INDIST) GO TO 895
*****
OUTPUT MILEAGE AND COST SUMMARY TABLES (AT END OF DISTRICT OR
END OF REPORT REQUEST WHICHEVER COMES FIRST.
*****
4100 WRITE (6,650) CMANT, CSUM
CTOT = CMANT + CSUM

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C	WRITE (6,651) CTOT	12590
	IPAGE = IPAGE + 1	12600
	LINENO = 0	12610
	WRITE (6,600) RUNDAT, IPAGE, IDIST	12620
	WRITE (6,652) INDIST	12630
	DO 4200 IZ1 = 1, 10	12640
	IFROM = (IZ1 - 10) - 10	12650
	IF (IZ1.EQ.1) IFROM=0	12660
	ITO = (IZ1 - 10) - 1	12670
	WRITE (6,654) IFROM, ITO, (HMILES(IZ1, IZ2), IZ2 = 1, 7)	12680
4200	CONTINUE	12690
	WRITE (6,656) (HMILES(11, IZ2), IZ2 = 1, 7)	12700
	DO 4490 IZ1 = 1, 7	12710
	IF (ITOTAL(2, IZ1) .GE. 1) GOTO 4495	12720
	ITOTAL(1, IZ1) = 0	12730
	GOTO 4490	12740
4495	ITOTAL(1, IZ1) = ITOTAL(1, IZ1) / ITOTAL(2, IZ1)	12750
4490	CONTINUE	12760
	WRITE (6,657) ((ITOTAL(IZ1, IZ2), IZ2 = 1, 7), IZ1 = 1, 2)	12770
	WRITE(6,600) RUNDAT, IPAGE, IDIST	12780
	WRITE(6,665)	12790
	IYERS = INYEAR	12800
	DO 4220 IZ1 = 1, 20	12810
	WRITE(6,667) IYERS, RYCSOST(IZ1), MYCOST(IZ1)	12820
	IYERS = INYEAR + IZ1	12830
4220	CONTINUE	12840
	WRITE(6,668)	12850
	WRITE(6,669)	12860
	WRITE(6,666) CSUM, CMANT	12870
	WRITE(6,669)	12880
	WRITE(6,668)	12890
	WRITE(6,600) RUNDAT, IPAGE, IDIST	12900
	WRITE(6,671) IDIST	12910
	IYRS = INYEAR	12920
	DO 4240 IZ1 = 1, 20	12930
	WRITE(6,672) IYRS, (RFCOST(IZ1, IZ2), IZ2 = 1, 7)	12940
	IYRS = INYEAR + IZ1	12950
4240	CONTINUE	12960
	WRITE(6,673)	12970
	WRITE(6,600) RUNDAT, IPAGE, IDIST	12980
	WRITE(6,675) IDIST	12990
	IYRS = INYEAR	13000
	DO 4241 IZ1 = 1, 20	13010
	WRITE(6,672) IYRS, (MFCOST(IZ1, IZ2), IZ2 = 1, 7)	13020
	IYRS = INYEAR + IZ1	13030
4241	CONTINUE	13040
	WRITE(6,673)	13050
	UCOST(4) = UCOST(4) + UCOST(5)	13060
	DO 4500 IZ1 = 1, 5	13070
	SCOST(4, IZ1) = SCOST(4, IZ1) + SCOST(5, IZ1)	13080
4500	CONTINUE	13090
	WRITE (6,658)	13100
	IPAGE = IPAGE + 1	13110
	LINENO = 0	13120
	WRITE (6,600) RUNDAT, IPAGE, INDIST	13130
	WRITE (6,660) INDIST	13140
	DO 5000 N2 = 1, 4	13150
	IF (CSUM.GT.0.00) GO TO 4998	13160
	PERCEN=0.0	13170
	GO TO 4999	13180
4998	PERCEN = (UCOST(N2)/CSUM) * 100.0	13190
4999	WRITE(6,670) PRY(N2), (SCOST(N2, IZ1), IZ1=1, 5), UCOST(N2), PERCEN	13200
5000	CONTINUE	13210
	WRITE (6,680)	13220
	IF (DIST.NE.98) GO TO 840	13230
	INDIST=98	13240
	WRITE(6,681)	13250
	GO TO 4010	13260
C		13270
C		13280
C		13290
C		13300
C		13310
C		13320

	WHEN DISTRICT = 99, END PROGRAM RUN.	

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C
C
C
6000 CONTINUE
WRITE(6,691)
STOP
END
SUBROUTINE FINDRF ( RFAL, RUPL, RFFR, FTCC, FUPL, FTFR, V )
*****
CALCULATE CLIMATIC WEIGHTING FACTORS
*****
RFAL - ANNUAL RAINFALL FOR COUNTY IN WHICH SEGMENT RESIDES.
RUPL - ARGUMENT VALUES FROM TACS TABLE MMSRAINS. THESE ARE
      INCHES-OF-RAINFALL-PER-YEAR BOUNDARIES.
RFFR - RESULT VALUES FROM TACS TABLE MMSRAINS. THESE ARE FACTORS
      ASSOCIATED WITH EACH BOUNDARY (SEE RUPL).
FTCC -ANNUAL FREEZE-THAW CYCLES FOR COUNTY IN WHICH SEG. RESIDES.
FUPL - ARGUMENT VALUES FROM TACS TABLE MMSFREEZ. THESE ARE
      FREEZE/THAW-CYCLES-PER-YEAR BOUNDARIES.
FTFR - RESULT VALUES FROM TACS TABLE MMSFREEZ. THESE ARE FACTORS
      ASSOCIATED WITH EACH BOUNDARY (SEE FUPL).
V - 8-ELEMENT ARRAY WHICH HOLDS THE FACTORS TO BE APPLIED IN
    COMPUTATION OF ADJUSTED VISUAL UTILITY (AVU) FROM
    UNADJUSTED VISUAL UTILITY (UVU) IN LATER WORK.

    DIMENSION V(8), RUPL(3), FUPL(4), RFFR(3), FTFR(4)
    DATA RUPL /20.0, 40.0, 99.0/
    DATA RFFR /1.00, 0.97, 0.94/
    DATA FUPL /10.0, 30.0, 50.0, 99.0/
    DATA FTFR /1.000, 0.973, 0.967, 0.960/
    RF = RFFR(1)
    IF ( RFAL .LE. RUPL(1) ) GO TO 1200
    RF = RFFR(3)
    IF ( RFAL .GT. RUPL(2) ) GO TO 1200
    RF = RFFR(2)
1200 CONTINUE
    FF = FTFR(1)
    IF ( FTCC .LE. FUPL(1) ) GO TO 1500
    FF = FTFR(4)
    IF ( FTCC .GT. FUPL(3) ) GO TO 1500
    FF = FTFR(3)
    IF ( FTCC .GT. FUPL(2) ) GO TO 1500
    FF = FTFR(2)
1500 CONTINUE
NOTE - V(1) = RUTTING (1/2 IN. - 1 IN.)
      V(2) = RUTTING (OVER 1 IN.)
      V(3) = RAVELING
      V(4) = FLUSHING
      V(5) = FAILURES
      V(6) = ALLIGATOR CRACKING
      V(7) = LONGITUDINAL CRACKING
      V(8) = TRANSVERSE CRACKING

V(1) = 1.00 / RF
V(2) = V(1)
V(3) = 1.0
V(4) = V(1)
V(5) = V(1) / FF
V(6) = V(5)
V(7) = V(5)
V(8) = V(5)
RETURN
END
SUBROUTINE FINDTI ( IC, IT, ATNR, TMNI )
*****
ASSIGN MINIMUM LIFE FOR MAINTENANCE STRATEGY.
*****
IC - FUNCTIONAL CLASSIFICATION OF ROADWAY FOR REHAB.

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          CALCULATE EXPECTED LIFE FOR MAINTENANCE STRATEGY J.
          .....
          J      - 1 OF 5 STRATEGIES NOW UNDER CONSIDERATION.
          IT     - PAVEMENT TYPE OF THE ROADWAY FOR REHAB.
          RFAL   - AVG. ANNUAL INCHES OF RAINFALL FOR COUNTY IN WHICH
                  ROADWAY RESIDES.
          RUPL   - TACS TABLE MMSRAINS (ARGUMENT ONLY).
                  ARG.   - BOUNDARIES (IN INCHES OF RAINFALL) WHICH
                          SEPARATE LOW, MEDIUM, AND HIGH RAINFALL.
                  RESULT - NOT USED.
          FTCC   - AVG. ANNUAL FREEZE-THAW CYCLES FOR COUNTY IN WHICH
                  ROADWAY RESIDES.
          FUPL   - TACS TABLE MMSFREEZ (ARGUMENT ONLY).
                  ARG.   - BOUNDARIES (IN FREEZE-THAW CYCLES) WHICH
                          SEPARATE LOW, MEDIUM, AND HIGH CYCLES.
          PLSX   - SOIL PLASTICITY INDEX FOR COUNTY IN WHICH
                  ROADWAY RESIDES.
          TF     - TRAFFIC FACTOR CALCULATED IN SUBROUTINE FINDTF.
          PESF   - EXPECTED PAVEMENT SCORE OF ROADWAY GIVEN
                  REHABILITATION USING THE STRATEGY NOW UNDER
                  CONSIDERATION (COMPUTED IN BIGSUB, 413550).
          PSMN   - MINIMUM ALLOWABLE PAVEMENT SCORE FOR THE ROADWAY.
          TCLS   - TACS TABLE MMSREHTC.
                  ARG.   - COMBINATION OF PAVEMENT TYPE (PRESENTLY IN
                          PLACE) AND STRATEGY UNDER INVESTIGATION.
                  RESULT - TIME (OR LIFE) CONSTANT USED IN COMPUTATION
                          OF REHAB. STRATEGY PVT. LIFE.
          FSDS   - TACS TABLE MMSREHDS.
                  ARG.   - COMBINATION OF PAVEMENT TYPE (PRESENTLY IN
                          PLACE) AND STRATEGY UNDER INVESTIGATION.
                  RESULT - FACTOR USED IN COMPUTATION OF REHAB.
                          STRATEGY PVT. LIFE.
          CLIF   - TACS TABLE MMSREHEF.
                  ARG.   - COMBINATION OF RAINFALL BOUNDARIES (RUPL)
                          AND FREEZE-THAW CYCLE BOUNDARIES (FUPL).
                  RESULT - CLIMATIC FACTOR FOR EACH RAIN/FREEZE-THAW
                          COMBINATION.
          SOLF   - TACS TABLE MMSREHSF.
                  ARG.   - COMBINATION OF RAINFALL BOUNDARIES (RUPL)
                          AND PLASTICITY INDEX BOUNDARIES.
                  RESULT - SOIL FACTOR FOR EACH RAINFALL/PLASTICITY
                          COMBINATION.
          TMAX   - MAXIMUM PAVEMENT LIFE (IN YEARS) FOR THE STRATEGY
                  UNDER INVESTIGATION RETURNED TO CALLER.

          DIMENSION RUPL(3), FUPL(4)
          DIMENSION TCLS(5,10), FSDS(5,10), CLIF(3,4), SOLF(3,3)
          TC      = TCLS(J,IT)
          DSI     = FSDS(J,IT)
          L = 1
          IF (RFAL.LE.RUPL(1)) GO TO 1100
          L = 3
          IF (RFAL.GT.RUPL(2)) GO TO 1100
          L = 2
1100 CONTINUE
          K = 1
          IF (FTCC.LE.FUPL(1)) GO TO 1200
          K = 4
          IF (FTCC.GT.FUPL(3)) GO TO 1200
          K = 3
          IF (FTCC.GT.FUPL(2)) GO TO 1200
          K = 2
1200 CONTINUE
          CF      = CLIF(L,K)

          NO TACS TABLE FOR PLASTICITY INDEX BOUNDARIES.

          K = 3
          IF ( PLSX .GT. 40.00 ) GO TO 1300
          K = 2
          IF ( PLSX .GT. 20.00 ) GO TO 1300
          K = 1
1300 CONTINUE

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SF          = SOLF(L,K)                                15550
DS          = DSI * TF * CF * SF                      15560
TMAX       = TC + ( PESF - PSMN ) / DS                15570
RETURN                                            15580
END                                              15590
SUBROUTINE LIMIT ( IVIS, SRVC, IC, AADT, JMAX)        15600
----- 15610
THIS SUBROUTINE PLACES A LIMIT UPON STRATEGY SELECTION. 15620
----- 15630
JMAX IS THE MAXIMUM STRATEGY ( 1, 2 ,... ) WHICH CAN BE APPLIED 15640
THE RULES ARE ; 15650
----- 15660
1) AADT LT 50 THEN JMAX = 1 15670
2) NO SEVERE RUTTING IVIS(1), ALLIGATORING IVIS(5), OR 15680
   FAILURES IVIS(4), AND PSI ABOVE MINIMUM THEN JMAX = 2 15690
3) AS 2) WITH PSI BELOW MINIMUM JMAX = 3 15700
JMAX IS RETURNED TO MAIN WHERE IT IS COMPARED WITH THE CHOSEN 15710
STRATEGY JK 15720
THIS ROUTINE HANDLES THE PROBLEMS OF HIGH VOLUME ROADS WHOSE 15730
PAVEMENT SCORES ARE BELOW MINIMUM BUT HAVE NO LOAD ASSOCIATED 15740
DISTRESS AND THOSE VERY LOW VOLUME FM'S FOR WHICH ONLY MINIMUM 15750
STRATEGIES ARE APPROPRIATE 15760
----- 15770
DIMENSION IVIS(7), PSIMIN(7) 15780
DATA PSIMIN / 3.5, 3.5, 3.0, 3.0, 2.5, 2.5, 2.5 / 15790
JMAX = 0 15800
IF ( AADT .GT. 50 ) GOTO 10 15810
JMAX = 1 15820
GOTO 100 15830
10 SEVRUT = 0.0 15840
IF ( IVIS(1) .EQ. 002 .OR. IVIS(1) .EQ. 020 .OR. IVIS(1) .EQ. 200) 15850
1 SEVRUT = 1.0 15860
CRACKS = 0.0 15870
ICRK = IVIS(4) + IVIS(5) 15880
IF ( ICRK .GE. 1 ) CRACKS = 1.0 15890
IF ( SEVRUT .EQ. 1.0 .OR. CRACKS .EQ. 1.0 ) GOTO 100 15900
JMAX = 2.0 15910
IF (SRVC .GT. PSIMIN(IC)) GOTO 100 15920
JMAX = 3.0 15930
100 CONTINUE 15940
RETURN 15950
END 15960
SUBROUTINE DECODE(IVIS,RVIS,RVISO) 15970
----- 16000
THIS SUBROUTINE TRANSFORMS THE CODED VISUAL READINGS 16010
TO PERCENTAGES 16020
----- 16030
IVIS - 8-ELEMENT ARRAY WHICH HOLDS THE CODED VISUAL READINGS 16040
RVISO - 8-ELEMENT ARRAY WHICH HOLDS THE TRANSFORMED VISUAL 16050
        READINGS. RANGE 0 - 1 16060
----- 16070
REAL W(4),Z(4),F(4),L(4),T(4) 16080
INTEGER X(4),Y(4) 16090
DIMENSION RVIS(8),IVIS(7),RVISO(8) 16100
----- 16110
DATA F/0.0,10.71,24.99,50.0/ 16120
DATA L/0.0,10.0,30.0,50.0/ 16130
DATA T/0.0,8.33,29.17,50.0/ 16140
DATA W/0.0,5.0,30.0,45.0/ 16150
DATA X/000,100,010,001/ 16160
----- 16170

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DATA Y/000,200,020,002/
DATA Z/0.0,12.0,30.0,50.0/
DO 10 I = 1,8
10 RVIS(I)= 0.0
DO 100 I =1,4
RUTTING
IF (IVIS(1).EQ.X(I)) GO TO 110
IF (IVIS(1).EQ.Y(I)) RVIS(2) = Z(I)
GO TO 120
110 RVIS(1) = Z(I)
120 CONTINUE
RAVELLING
IF (IVIS(2).EQ.X(I)) RVIS(3) = Z(I)
FLUSHING
IF (IVIS(3).EQ.X(I)) RVIS(4) = Z(I)
FAILURES
IF (IVIS(4).EQ.X(I)) RVIS(5) = F(I)
ALLIGATOR CRACKING
IF (IVIS(5).EQ.X(I)) RVIS(6) = W(I)
LONGITUDINAL CRACKING
IF (IVIS(6).EQ.X(I)) RVIS(7) = L(I)
TRANSVERSAL CRACKING
IF (IVIS(7).EQ.X(I)) RVIS(8) = T(I)
100 CONTINUE
DO 130 I = 1,8
130 RVIS(I) = RVIS(I)
RETURN
END
SUBROUTINE FINAVU ( IT, J, AVUC, SRVC, SKID, FAVU, FSIU, FSKU,
1 AVU, SIV, SNV, RVIS, ENDVIS)
*****
CALCULATE ESTIMATED ADJUSTED VISUAL UTILITY (AVU), ESTIMATED
SKID NUMBER (SN), AND ESTIMATED SERVICABILITY INDEX (SI) FOR
EACH OF 5 MAINTENANCE STRATEGIES.
*****
J - MAINTENANCE STRATEGY NO.
AVUC - PRESENT AVU OF THE PAVEMENT TO BE REHABILITATED.
SRVC - PRESENT AVG. SI VALUE OF THE PVT. TO BE REHABILITATED.
SKID - PRESENT AVG. SN VALUE OF THE PVT. TO BE REHABILITATED.
FAVU - TACS TABLE MMSREAVU.
ARG. - 10 AVU BOUNDARIES FOR EACH OF 5 STRATEGIES.
RESULT - ESTIMATED AVU AFTER REHAB. FOR THAT STRATEGY.
FSIU - TACS TABLE MMSREHSI.
ARG. - 5 SI BOUNDARIES FOR EACH OF 5 STRATEGIES.
RESULT - ESTIMATED SI OR, FOR STRATEGY 1 ONLY,
INCREASE IN SI AFTER REHAB. FOR THAT STRATEGY.
FSKU - TACS TABLE MMSREHSN.
ARG. - 6 SN BOUNDARIES FOR EACH OF 5 STRATEGIES.
RESULT - ESTIMATED SN AFTER REHAB. FOR THAT STRATEGY.
AVU - SELECTED ENTRY FROM FAVU RETURNED TO CALLER.
SIV - SELECTED ENTRY OR, FOR STRATEGY 1 ONLY, COMPUTED ITEM
FROM FSIU RETURNED TO CALLER.
SNV - SELECTED ENTRY FROM FSKU RETURNED TO CALLER.
DIMENSION FAVU(10,5), FSIU(5,5), FSKU(6,5), MXGAIN(8,5)
DIMENSION RVIS(8), ENDVIS(8)

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16990
17000
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17020

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REAL MXGAIN
DATA MXGAIN /0.33, 0.0, 1.0, 1.0, 0.25, 0.6, 0.6, 0.75, 17030
1 1.00, 0.7, 1.0, 1.0, 0.62, 0.8, 0.8, 1.00, 17040
2 1.00, 1.0, 1.0, 1.0, 0.75, 1.0, 1.0, 1.00, 17050
3 1.00, 1.0, 1.0, 1.0, 0.87, 1.0, 1.0, 1.00, 17060
4 1.00, 1.0, 1.0, 1.0, 1.0, 1.0, 1.0, 1.00/ 17070
DATA FAVU /0.79,0.81,0.82,0.84,0.86,0.88,0.89,0.91,0.93,0.94, 17080
1 0.80,0.83,0.85,0.88,0.90,0.93,0.95,0.98,1.00,1.00, 17090
2 40=1.00/ 17100
DATA FSIU /0.2,0.2,0.1,0.0,0.0,5=4.3,20=4.5/ 17110
DATA FSKU /36=45.0/ 17120
DO 1200 K = 1, 9 17130
AL = FLOAT(K-1) / 10.0 17140
AU = FLOAT(K) /10.0 17150
IF ( AVUC .GE. AL .AND. AVUC .LE. AU ) GO TO 1300 17160
1200 CONTINUE 17170
K = 10 17180
1300 AVU = FAVU(K,J) 17190
SIV = SRVC 17200
DO 1400 K = 1, 4 17210
AL = FLOAT(K-1) 17220
AU = FLOAT(K) 17230
IF ( SRVC .GE. AL .AND. SRVC .LE. AU ) GO TO 1500 17240
1400 CONTINUE 17250
K = 5 17260
1500 IF ( J .GT. 1 ) GO TO 1600 17270
SIV = SIV + FSIU(K,J) 17280
GO TO 1700 17290
1600 SIV = FSIU(K,J) 17300
1700 DO 1800 K = 1, 5 17310
AL = FLOAT(K-1) * 10.0 17320
AU = FLOAT(K) * 10.0 17330
IF ( SKID .GE. AL .AND. SKID .LE. AU ) GO TO 1900 17340
1800 CONTINUE 17350
K = 6 17360
1900 SNV = FSKU(K,J) 17370
IJ = J 17380
IF ( IT .NE. 10 ) GOTO 1999 17390
IJ = IJ + 1 17400
IF ( IJ .GT. 5 ) IJ = 5 17410
DO 2000 I = 1, 8 17420
ENDVIS(I) = RVIS(I) - (MXGAIN(I,IJ)*RVIS(I)) 17430
2000 IF (ENDVIS(I) .LE. 0.0) ENDVIS(I) = 0.0 17440
RETURN 17450
END 17460
SUBROUTINE SCORE (RVIS,SRVC,V,FLEXSC,ADTS,SIBNRY,FUNC,IC, 17470
1 IAVUC,ISIUC,PESC,IT) 17480
17490
***** 17500
THIS SUBROUTINE CALCULATES THE PAVEMENT EVALUATION SCORE BASED 17510
ON THE DISTRESSES AND THE PSI. 17520
***** 17530
THIS SUBROUTINE CALLS TWO OTHER SUBROUTINES: 17540
UTLTY1 - CALCULATES VISUAL UTILITY VALUE 17550
UTLTY2 - CALCULATES RIDE UTILITY VALUE 17560
***** 17570
RVIS - 8-ELEMENT ARRAY WHICH HOLDS THE ACTUAL VISUAL DIST. 17580
SRVC - ACTUAL PSI. 17590
ADTS - AVERAGE DAILY TRAFFIC. 17600
SIBNRY- FACTOR ASSOCIATED WITH EACH OF 3 SI BOUNDARIES FOR 17610
EACH OF 3 EQUATIONS USED IN THE DETERMINATION OF 17620
SERVICEABILITY INDEX (SI) UTILITY. 17630
FUNC - FACTOR ASSOCIATED WITH THE FUNCTIONAL CLASSIFICATION 17640
OF THE ROAD USED IN THE DETERMINATION OF THE PAVEMENT 17650
SCORE. 17660
IC - FUNCTIONAL CLASSIFICATION OF ROADWAY FOR REHAB. 17670
IAVUC - CALCULATED ADJUSTED VISUAL UTILITY 17680
ISIUC - CALCULATED SERVICEABILITY INDEX. 17690
PESC - CALCULATED PAVEMENT SCORE. 17700
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C
DIMENSION V(8),RVIS(8),FLEXSC(8,3),SIBNRY(3,3),FUNC(7)
CALL UTLTY1(V,RVIS,UVUC,AVUC,IT)
CALL UTLTY2(ACTS,SRVC,SIBNRY,SIUC)
UPSC = SIUC * AVUC
PNC = FUNC(IC)
PESC = UPSC ** (1/FUNC(IC))
PESC = (PESC + 0.00001)
SIUCS = SIUC * 100.0
ISIUC = INT(SIUCS)
AVUCS = AVUC * 100.0
IAVUC = INT(AVUCS)
RETURN
END
SUBROUTINE UTLTY1 ( V, RVIS, UVUC, AVUC,IT)
RVIS - ACTUAL % OF EACH DISTRESS RANGE 0 TO 100%
RVIS 1 AND RVIS 2 ARE FOR RUTTING.
***NOTE: THIS SUBROUTINE NOW USES EQUATIONS RATHER THAN
A TABLE LOOK-UP TO CALCULATE SCORES.
*****
CALCULATE VISUAL UTILITY VALUE
NOTE - V(1) AND V(2) ARE BOTH RUTTING, THEN THE REMAINING
6 VISUAL ITEMS FOLLOW AS V(3) THRU V(8). THIS IS
DUE TO SPECIAL CODING ABILITIES FOR RUTTING.
*****
IVIS - 7-ELEMENT ARRAY CONTAINING THE VISUAL EVALUATION ITEMS
WHICH ARE RUTTING, RAVELING, FLUSHING, FAILURES,
ALLIGATOR, LONGITUDINAL, AND TRANSVERSE CRACKING.
V - 8-ELEMENT ARRAY CONTAINING THE CLIMATIC WEIGHTING FACTORS
FOR EACH OF THE 7 VISUAL EVALUATION ITEMS.
SCORE - 3 FACTORS FOR EACH OF THE 7 VISUAL EVALUATION ITEMS
(8 TOTAL - REMEMBER, RUTTING TAKES UP 1 AND 2) FOR FLEXIBLE
OR COMPOSITE PAVEMENTS.
UVUC - UNADJUSTED VISUAL UTILITY CALCULATED HEREIN.
AVUC - ADJUSTED VISUAL UTILITY CALCULATED HEREIN.
DIMENSION V(8), RVIS(8),XVIS(8)
DIMENSION A(8),B(8),A1(8)
DATA A/0.3229,0.6940,0.5703,0.6467,1.3507,0.5592,0.7738,0.5446/
DATA A1/1.0,1.0,1.0,1.0,0.28,1.0,5.0,0.24/
DATA B/12.365,10.13,24.91,34.99,5.7778,4.962,161.98,6.7973/
DO 10 I = 1,8
XVIS(I) = RVIS(I) * A1(I)
10 CONTINUE
UVUC = 1.00
AVUC = 1.00
IF(RVIS(1).GT.RVIS(2)) GO TO 20
RVIS(1) = 0.0
GO TO 30
20 RVIS(2) = 0.0
30 CONTINUE
DO 100 I = 1,8
IF(RVIS(I).LT.0.5) GO TO 100
U = 1 - A(I)*(EXP(-B(I)/XVIS(I)))
UVUC = UVUC * U
IF(IT.NE.10) GO TO 40
IF(I.GT.2) GO TO 40
V(I) = V(I) * 0.5
40 CONTINUE
AVUC = AVUC * U ** V(I)
100 CONTINUE
RETURN
END
SUBROUTINE UTLTY2 ( AACTS, AVGSI, SIBNRY, SIUC )
*****
CALCULATE RIDE UTILITY VALUE
*****
C
C
C
C

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18020
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18100
18110
18120
18130
18140
18150
18160
18170
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18450
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18470
18480
18490
18500

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BINDER = 6.0
PI = PLSX
KNT = 0
PO = 4.2

KAVT = AVT - 50.0

N18MTH = (EALT * 1000.0)/240.0

OVERLAY PAVEMENTS PSI & DISTRESS

LINEAR RHO & BETA, PSI
X(9) = 0.26503*OVTH + 0.07182*HPR2
X(10) = 0.00413*XTI+0.01036*FTC+0.04769*KAVT+0.01707*
& (N18MTH/1000.0)-0.009144*OVTH-0.01066*HPR2
X(11) = 0.33037*OVTH + 0.07627*HPR2
IF( X(9).LT. 0.0 .OR.X(10) .LT. 0.0 .OR.X(11).LT. 0.0 ) GO TO 312
GO TO 313

LOG RHO & BETA, PSI
312 X(9) = FTC**(-0.24351)*BINDER**(0.71372)*HPR2**(0.18059)
X(10) = FTC**(0.09767)*(N18MTH/1000.0)**(0.17402)*BINDER**
& (-0.30623)*HPR2**(-0.22623)
X(11) = FTC**(-0.14525)*KAVT**(-0.25053)*(N18MTH/1000.0)**
& (-0.24283)*BINDER**(0.32304)*HPR2**(0.62508)

LINEAR RHO & BETA, RUTTING AREA
313 X(1) = -0.00119*PI+0.369*OVTH+0.0485*HPR2
X(2) = 0.0059*XTI-0.00217*FTC+0.0206*AVT-0.122*OVTH+0.0789*HPR3
IF( X(1).LT. 0.0 .OR. X(2) .LT. 0.0 ) GO TO 322
GO TO 323

LOG RHO & BETA, RUTTING AREA
322 X(1) = PI**(-0.1925)*OVTH**(0.6058)*HPR2**(0.1246)*HPR3**(-0.4419)
X(2) = XTI**(0.1025)*FTC**(0.0163)*OVTH**(-0.2295)*HPR3**(0.1078)

LINEAR RHO & BETA, RUTTING SEVERITY
323 Y(1) = -0.00507*PI+0.233*OVTH+0.0705*HPR2-0.000779*HPR3
Y(2) = 0.009*XTI+0.0146*AVT+0.0024*PI-0.0789*OVTH+0.084*HPR3
IF(Y(1) .LT. 0.0 .OR. Y(2) .LT. 0.0 ) GO TO 332
GO TO 333

LOG RHO & BETA, RUTTING SEVERITY
332 Y(1) = PI**(-0.2342)*OVTH**(0.578)*HPR2**(0.1396)*HPR3**(-0.4808)
Y(2) = XTI**(0.0575)*OVTH**(-0.1112)*HPR3**(0.1201)

LINEAR RHO & BETA, ALLIGATOR CRACK AREA
333 X(3) = -0.0159*FTC+0.0082*AVT-0.0121*PI+0.0162*OVTH+0.145*HPR2
& -0.0135*HPR3
X(4) = 0.0185*XTI+0.171*HPR3
IF(X(3) .LT. 0.0 .OR. X(4) .LT. 0.0 ) GO TO 342
GO TO 343

LOG RHO & BETA, ALLIGATOR CRACK AREA
342 X(3) = FTC**(-0.2777)*AVT**(0.4)*PI**(-0.2165)*OVTH**(0.4861)
& *HPR3**(-0.6399)
X(4) = XTI**(0.2211)*AVT**(-0.1326)*OVTH**(-0.396)*HPR3**(0.1744)

LINEAR RHO & BETA, ALLIGATOR CRACK SEVERITY
343 Y(3) = -0.00975*FTC+0.0152*AVT-0.0106*PI+0.0568*HPR2-0.0315*HPR3
Y(4) = 0.0301*XTI+0.2267*HPR3
IF(Y(3) .LT. 0.0 .OR. Y(4) .LT. 0.0 ) GO TO 352

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19700
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19800
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19980

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GO TO 353
LOG RHO & BETA, ALLIGATOR CRACK SEVERITY
352 Y(3) = FTC**(-0.1235)*AVT**(0.3885)*PI**(-0.4525)*HPR3**(-0.6859)
      Y(4) = XTI**(0.3689)*FTC**(0.178)*AVT**(-0.3259)*OVTH**(-0.4326)
      & *HPR3**(0.1931)
LINEAR RHO & BETA, LONG. CRACK AREA
353 X(5) = -0.0168*XTI-0.087*FTC+1.63*AVT-0.179*PI+2.68*OVTH+0.84*HPR2
      X(6) = 0.0331*XTI+0.00433*FTC-0.00713*AVT-0.0589*OVTH+0.399*HPR3
      IF(X(5) .LT. 0.0 .OR. X(6) .LT. 0.0 ) GO TO 362
      GO TO 363
LOG RHO & BETA, LONG. CRACK AREA
362 X(5) = FTC**(-0.1447)*AVT**(1.2948)*PI**(-0.2014)*OVTH**(0.1864)
      & *HPR2**(0.1004)
      X(6) = XTI**(0.2914)*FTC**(0.1358)*AVT**(-0.2956)*OVTH**(-0.0808)
      & *HPR2**(-0.0324)*HPR3**(0.2972)
LINEAR RHO & BETA, LONG. CRACK SEVERITY
363 Y(5) = -0.214*FTC+1.55*AVT
      Y(6) = 0.0218*XTI+0.0134*FTC-0.0156*HPR2+0.073*HPR3
      IF(Y(5) .LT. 0.0 .OR. Y(6) .LT. 0.0 ) GO TO 372
      GO TO 373
LOG RHO & BETA, LONG. CRACK SEVERITY
372 Y(5) = XTI**(-0.0015)*FTC**(-0.0989)*AVT**(1.2089)*PI**(-0.0841)
      Y(6) = XTI**(0.2159)*FTC**(0.1175)*AVT**(-0.24)*HPR3**(0.1103)
LINEAR RHO & BETA, TRANS. CRACK AREA
373 X(7) = -0.794*FTC+1.922*AVT+22.81*OVTH
      X(8) = -0.0097*XTI+0.0149*FTC-0.0229*AVT+0.0441*PI-0.129*OVTH
      & +0.43*HPR3
      IF(X(7) .LT. 0.0 .OR. X(8) .LT. 0.3 ) GO TO 382
      GO TO 383
LOG RHO & BETA, LONG. CRACK AREA
382 X(7) = FTC**(-0.1509)*AVT**(1.1947)*OVTH**(0.4683)
      X(8) = XTI**(0.188)*FTC**(0.2676)*AVT**(-0.3966)*PI**(0.1576)
      & *OVTH**(-0.3127)*HPR2**(-0.0719)*HPR3**(0.1899)
LINEAR RHO & BETA, TRANS. CRACK SEVERITY
383 Y(7) = -0.0627*FTC+1.23*AVT+5.273*OVTH
      Y(8) = 0.0187*XTI+0.0117*FTC+0.0109*PI-0.0305*HPR2+0.108*HPR3
      IF(Y(7) .LT. 0.0 .OR. Y(8) .LT. 0.0 ) GO TO 392
      GO TO 393
LOG RHO & BETA, TRANS. CRACK SEVERITY
392 Y(7) = FTC**(-0.0805)*AVT**(1.1307)*PI**(-0.0506)*OVTH**(0.173)
      Y(8) = XTI**(0.1606)*FTC**(0.0922)*AVT**(-0.2142)*PI**(0.0618)
      & *HPR3**(0.1114)
393 CONTINUE
      RHORA = X(1)
      BETRA = X(2)
      RHOAA = X(3)
      BETAA = X(4)
      RHOLA = X(5)
      BETLA = X(6)
      RHOTA = X(7)
      BETTA = X(8)
      RHOP = X(9)
      BETAP = X(10)
      PF = X(11)

```

C			20730
C	WRITE (6,300) RHORA, BETRA, RHOAA, BETAA, RHOA, BETTA, RHOLA,		20740
C	1 BETLA, RHOP, BETAP, PF		20750
C	300 FORMAT(// 1X,10G13.5/1X,1G13.5)		20760
C			20770
	DO 15 I = 1, 5		20780
	DO 15 J = 1, 100		20790
	15 DISL(I,J) = 0.0		20800
C			20810
C	CALCULATE DISTRESS		20820
C			20830
	DO 30 J = 1, 100		20840
	IF (J.EQ.100) GO TO 507		20850
C			20860
	W(J) = J		20870
	TO = W(J)/100.0		20880
	GO TO 508		20890
	507 TO = .9910		20900
	508 CONTINUE		20910
C			20920
C	RUTTING AREA NOW		20930
C			20940
	SO = ALOG (TO)		20950
	RO = ABS (SO)		20960
	ANW = RO**(1/BETRA)		20970
	N = (1/ANW)*RHORA		20980
	NX = N*1000000.0/N18MTH		20990
C			21000
C	RUTTING AREA NEXT YEAR		21010
C			21020
	IN = INT(NX)		21030
	IN = IN + 12		21040
	N18 = N18MTH*IN/1000000.0		21050
	PWR = (RHORA/N18)**BETRA		21060
	DISL(1,J) = EXP(-PWR) * 100.0		21070
	DISL(2,J) = EXP(-PWR) * 100.0		21080
C			21090
C	RAVELLING		21100
C			21110
	DISL(3,J) = J + 1.0		21120
C			21130
C	FLUSHING		21140
C			21150
	DISL(4,J) = J + 1.0		21160
C			21170
C	FAILURES		21180
C			21190
	DISL(5,J) = J + 1.0		21200
C			21210
C	ALLIGATOR CRACKING NOW		21220
C			21230
	BA = 1/BETAA		21240
	ANW = RO**BA		21250
	N = (1/ANW)*RHOAA		21260
	NX = N*1000000.0/N18MTH		21270
C			21280
C	ALLIGATOR CRACKING NEXT YEAR		21290
C			21300
	IN = INT(NX)		21310
	IN = IN + 12		21320
	N18 = N18MTH*IN/1000000.0		21330
	PWR = (RHOAA/N18)**BETAA		21340
	DISL(6,J) = EXP(-PWR) * 100.0		21350
C			21360
C	LONGITUDINAL CRACKING NOW		21370
C			21380
	BD = 1/BETLA		21390
	ANW = RO**BD		21400
	N = (1/ANW)*RHOLA		21410
C			21420
C	LONGITUDINAL CRACKING NEXT YEAR		21430
C			21440
			21450
			21460

```

      IN = INT(N)
      IN = IN + 12
      PWR = (RHOLA/IN)**BETLA
      DISL(7,J) = EXP( -PWR) * 100.0
C
C
C
      TRANSVERSAL CRACKING NOW
      BL = 1/BETTA
      ANW = RO**BL
      N = (1/ANW)*RHOTA
C
C
      TRANSVERSAL CRACKING NEXT YEAR
      IN = INT(N)
      IN = IN + 12
      PWR = (RHOTA/IN)**BETTA
      DISL(8,J) = EXP( -PWR) * 100.0
30 CONTINUE
C
C
C
      PSI
      WRITE(6,251) XTI, FTC, AVT, PI, OVTH, BINDER, HPR2, HPR3, N18MTH
C 251 FORMAT(T10, 'DATA INPUTS: '//T10, 'TI+50 FTC AVT PI OVTH
C & , ' BINDER HPR2 HPR3 N18/MTH'/T7,4F7.0,2F7.1,F7.0,F7.2,F10.0)
C
      DO 40 J = 1,50
      PSIL(J) = PSTE
      IF( RHOP .LE. 0.0 ) GO TO 40
      PS(J) = J/10.0
      PIS(J) = PS(J)
      IF(PS(J).GE.PO) PIS(J) = PO - 0.001
      B1 = (PO-PIS(J))/(PO-PF)
      B2 = ALOG(B1)
      B3 = B2 * (-1.0)
      IF (B3.LT.0.0) GO TO 99
      CALL MONTHS (B3, NS, RHOP, TI50, FTC, AVT50, OVTH, HPR2,
& N18MTH)
      N1 = NS
      N2 = N1*1000000.0/N18MTH
      N2 = N2 + 12
      N18 = N2 * N18MTH/1000000.0
      BETAP = 0.00413*XTI+0.01036*FTC+0.04769*AVT+0.01707*
& (N18MTH/1000.0)-0.009144*OVTH-0.01066*HPR2
      IF( BETAP .LT. 0.0 ) GO TO 27
      GO TO 28
27 BETAP = FTC*(0.09767)*(N18MTH/1000.0)**(0.17402)*BINDER**
& (-0.30623)*HPR2**(-0.22623)
28 CONTINUE
      PWR = (RHOP/N18)**BETAP
      IF(PWR.GT.80.0) PWR = 80.0
      PSIL(J) = PO - (PO - PF) * EXP( -PWR)
      IF (PSIL(J).GT.PS(J) ) PSIL(J) = PS(J) - 0.15
C
      PSTE = PSIL(J)
      GO TO 29
99 PSIL(J) = PIS(J)
29 CONTINUE
40 CONTINUE
C
C
C
      WRITE(6,252)
C 252 FORMAT(/T26, 'DISTRESS', T60, 'DISTRESS' /T12, 'N RUTTING',
C & ' RAVL FLUSH FAIL ALIG LONG TRNS ' / T11,
C $'ACT ONE TWO AREA AREA AREA AREA AREA AREA')
C
      DO 41 J = 1, 100
C
C
C
      WRITE(6,255) W(J), (DISL(I,J), I = 1, 8)
C 255 FORMAT( F14.2, 2X, 8F6.2)
C 41 CONTINUE
C
C
C
      DO 44 J = 1, 50
      WRITE(6,257) PS(J), PSIL(J)
C 257 FORMAT(T8,F5.3,10X,F5.3)
C 44 CONTINUE

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XTI = TIN(ICTY) + 50.0
AVT = AVTP(ICTY)
FTC = FRTH(ICTY)
NISMTH = (EALT*1000.0)/240.0
BINDER = 6.0
PI = PLSX
KNT = 0
PO = 4.2
XAVT = AVT - 50.0

LINEAR RHO & BETA, PSI
X(9) = -0.02182*FTC-0.00831*PI+0.04499*BINDER+0.15013*HPR2
X(10) = 0.01201*XTI+0.03166*FTC+0.13775*XAVT+0.00114*PI
      -0.31331*BINDER-0.03234*HPR2
X(11) = -0.00637*FTC-0.0155*XAVT-0.00658*PI+0.27714*BINDER
      +0.05097*HPR2
IF( X(9).LT. 0.0 .OR. X(10).LT. 0.0 .OR.X(11).LT. 0.0 ) GO TO 112
GO TO 113

LOG RHO & BETA, PSI
112 X(9)= FTC*(-0.46679)*XAVT*(-0.86233)*PI*(-0.26711)
      & *HPR2*(1.65694)
      X(10)= FTC*(0.60949)*XAVT*(0.93499)*BINDER*(-1.37608)
      & *HPR2*(-0.72725)
      X(11)= FTC*(-1.50634)*XAVT*(-2.6946)*BINDER*(4.17755)
      & *HPR2*(1.60919)

LINEAR RHO & BETA, RUTTING AREA
113 X(1)= 0.00175*FTC-0.0141*AVT+0.257*ASPH
      X(2) = -0.00493*FTC+0.0262*AVT+0.0387*PI-0.2433*ASPH
      IF( X(1).LT. 0.0 .OR. X(2) .LT. 0.0 ) GO TO 122
      GO TO 123

LOG RHO & BETA, RUTTING AREA
122 X(1)= AVT*(-0.6844)*PI*(-0.1021)*ASPH*(1.4756)*HPR3*(-0.0021)
      X(2) = FTC*(-0.1668)*AVT*(0.1231)*PI*(0.2238)*ASPH*(-0.0266)

LINEAR RHO & BETA, ALLIGATOR CRACK AREA
123 X(3)= 0.134*HPR2 - 0.067*HPR3
      X(4) = 0.856*HPR3
      IF( X(3).LT. 0.0 .OR. X(4) .LT. 0.0 ) GO TO 142
      GO TO 143

LOG RHO & BETA, ALLIGATOR CRACK AREA
142 X(3)= FTC*(-0.1961)*PI*(-0.2231)*HPR2*(0.644)*HPR3*(-0.31)
      X(4) = 1.4686

LINEAR RHO & BETA, LONG. CRACK AREA
143 X(5)= 5.33*ASPH+29.44*BINDER-6.88*HPR3
      X(6) = 0.0181*AVT+0.421*HPR3
      IF( X(5).LT. 0.0 .OR. X(6) .LT. 0.0 ) GO TO 162
      GO TO 163

LOG RHO & BETA, LONG. CRACK AREA

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23680

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C			24430
		DISL(3,J) = J + 1.0	24440
		FLUSHING	24450
			24460
		DISL(4,J) = J + 1.0	24470
			24480
		FAILURES	24490
			24500
		DISL(5,J) = J + 1.0	24510
			24520
		ALLIGATOR CRACKING NOW	24530
			24540
		BA = 1/BETAA	24550
		ANW = RO**BA	24560
		N = (1/ANW)*RHOAA	24570
		NX = N*1000000.0/N18MTH	24580
			24590
		ALLIGATOR CRACKING NEXT YEAR	24600
			24610
			24620
		IN = INT(NX)	24630
		IN = IN + 12	24640
		N18 = N18MTH*IN/1000000.0	24650
		PWR = (RHOAA/N18)**BETAA	24660
		DISL(6,J) = EXP(-PWR) * 100.0	24670
			24680
		LONGITUDINAL CRACKING NOW	24690
			24700
		BD = 1/BETLA	24710
		ANW = RO**BD	24720
		N = (1/ANW)*RHOLA	24730
			24740
		LONGITUDINAL CRACKING NEXT YEAR	24750
			24760
			24770
		IN = INT(N)	24780
		IN = IN + 12	24790
		PWR = (RHOLA/IN)**BETLA	24800
		DISL(7,J) = EXP(-PWR) * 100.0	24810
			24820
		TRANSVERSAL CRACKING NOW	24830
			24840
		BL = 1/BETTA	24850
		ANW = RO**BL	24860
		N = (1/ANW)*RHOTA	24870
			24880
		TRANSVERSAL CRACKING NEXT YEAR	24890
			24900
		IN = INT(N)	24910
		IN = IN + 12	24920
		PWR = (RHOTA/IN)**BETTA	24930
		DISL(8,J) = EXP(-PWR) * 100.0	24940
		30 CONTINUE	24950
			24960
		PSI	24970
			24980
			24990
		DO 40 J= 1,50	25000
		PSIL(J) = PO	25010
		IF(RHOP .LE. 0.0) GO TO 40	25020
		IF(PF.GE.PO) GO TO 39	25030
		PS(J) = J/10.0	25040
		PIS(J) = PS(J)	25050
		IF(PS(J).GE.PO) PIS(J) = PO - .001	25060
		B1 = (PO-PIS(J))/(PO-PF)	25070
		B2 = ALOG(B1)	25080
		B3 = B2 * (-1.0)	25090
		IF (B3.LT.0.0) GO TO 99	25100
		ANW = B3**(1/BETAP)	25110
		N1 = (1/ANW)*RHOP	25120
		N2 = N1*1000000.0/N18MTH	25130
		N2 = N2 + 12	25140
		N18 = N2 * N18MTH/1000000.0	25150
		PWR = (RHOP/N18)**BETAP	25160

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      PSIL(J) = PO - (PO - PF) * EXP( -PWR)
      IF (PSIL(J).GT.PS(J)) PSIL(J) = PS(J) - 0.15
C     PSTE = PSIL(J)
      GO TO 29
39  PSIL(J) = 2.65
      GO TO 29
99  PSIL(J) = PIS(J)
29  CONTINUE
40  CONTINUE

      WRITE(6,251) XTI, FTC, AVT, PI, ASPH, BINDER, HPR2, HPR3, N18MTH
251  FORMAT(T10,'DATA INPUTS:'//T10,'TI+50  FTC  AVT  PI  ASPH'
      &,' BINDER  HPR2  HPR3  N18/MTH'/T7,4F7.0,2F7.1,F7.0,F7.2,F10.0)

      WRITE(6,252)
252  FORMAT(/T26,'DISTRESS', T60, 'DISTRESS' /T12, 'N          RUTTING',
      & 'RAVL FLUSH FAIL ALIG LONG TRNS ' / T11,
      S'ACT      ONE  TWO  AREA AREA AREA AREA AREA AREA')

      DO 41 J = 1, 100

      WRITE(6,255) W(J), (DISL(I,J), I = 1, 8)
255  FORMAT( F14.2, 2X, 8F6.2)
41  CONTINUE
      DO 44 J = 1, 50
      WRITE(6,257) PS(J), PSIL(J)
257  FORMAT(T8,F5.3,10X,F5.3)
44  CONTINUE

      KNT = KNT + 1
      IF( KNT .EQ. 1 ) WRITE(6,200)
      IF( KNT .EQ. 1 ) KNT = 0

      RETURN
      END

      SUBROUTINE ST(CNTY, IT, PESC, TIN, FRTH, AVTP, DMD, PLSX, FLEXL, DISL,
1     PSIL, EALT, AADT, AKIP)
      *****
      THIS SUBROUTINE USES THE SURVIVAL CURVES TO GENERATE
      TRANSITION MATRIX FOR SURFACE TREATED PAVEMENTS.
      *****

      CNTY - COUNTY NUMBER
      IT - PAVEMENT TYPE
      PESC - PAVEMENT SCORE
      TIN - THORNTHWAITE INDEX
      FRTH - FREEZE/THAW CYCLES
      AVTP - AVERAGE TEMPERATURE
      FLEXL - THICKNESS OF FLEX BASE IN INCHES
      DMD - DYNAPLECT MEAN DEFLECTION
      LL - SUBGRADE LIQUID LIMIT
      PLSX - PLASTICITY INDEX
      DISL - (8 X 100)-ELEMENT ARRAY WHICH HOLDS THE TRANSITION
      MATRIX FOR THE DISTRESSES OF THE SECTION IN ANALYSIS
      PSIL - TRANSITION MATRIX FOR THE PSI OF THE SECTION IN
      ANALYSIS

      INTEGER CNTY, EALT
      REAL W(100), N18, N18MTH, N, NX, NS, PS(50), N1, N2, ADT, NADT
      REAL LL
      DIMENSION DISL(8,100), HEADER(10), X(17), PIS(50)
      DIMENSION TIN(254), FRTH(254), AVTP(254), RAIN(254), PSIL(50)

      WRITE (6,200)

```


C			26650
		IF(X(9).GT. 172.0) X(9)= 172.0	26660
		IF(X(9).LT. 30.0) X(9)= 30.0	26670
C			26680
		IF(X(10).GT. 2.65) X(10)= 2.65	26690
		IF(X(10).LT. 0.68) X(10)= 0.68	26700
C			26710
			26720
			26730
			26740
			26750
			26760
		TRANS AREA	26770
		X(11)= -66.4 + 2.156*TI50 + 10.12*FLEXL + 0.718*FTC	26780
		X(12)= 2.059 + 0.0734*FLEXL - 0.06*LL + 0.0607*PI - 0.00375*FTC	26790
		IF(X(11).GT. 176.0) X(11) = 176.0	26800
		IF(X(11).LT. 41.0) X(11) = 41.0	26810
C			26820
		IF(X(12).GT. 2.65) X(12)= 2.65	26830
		IF(X(12).LT. 0.61) X(12)= 0.61	26840
C			26850
			26860
			26870
		PATCHING	26880
		X(13)= 0.00799 + 0.00252*AVT50 + 0.000218*TI50 + 0.00166*FLEXL	26890
		1 - 0.00125*PI	26900
		X(14)= 1.75	26910
C			26920
		IF(X(13).GT. 0.104) X(13) = 0.104	26930
		IF(X(13).LT. 0.0036) X(13) = 0.0036	26940
C			26950
		IF(X(14).GT. 5.36) X(14)= 5.36	26960
		IF(X(14).LT. 0.63) X(14)= 0.63	26970
C			26980
			26990
			27000
			27010
		RHORA = X(1)	27020
		BETRA = X(2)	27030
		RHORV = X(3)	27040
		BETRV = X(4)	27050
		RHOFL = X(5)	27060
		BETFL = X(6)	27070
		RHOAA = X(7)	27080
		BETAA = X(8)	27090
		RHOLA = X(9)	27100
		BETLA = X(10)	27110
		RHOTA = X(11)	27120
		BETTA = X(12)	27130
		RHOPT = X(13)	27140
		BETPT = X(14)	27150
		RHOP = X(15)	27160
		BETAP = X(16)	27170
		PF = X(17)	27180
C			27190
		WRITE(6,300) RHORA,BETRA,RHORV,BETRV,RHOFL,BETFL,RHOAA, BETAA,	27200
		& RHOTA, BETTA, RHOLA, BETLA,RHOPT,BETPT,	27210
		\$ RHOP, BETAP, PF	27220
C		300 FORMAT(// 1X, 10G13.5 / 1X, 7G13.5/)	27230
			27240
		DO 15 I = 1, 5	27250
		DO 15 J = 1, 100	27260
		15 DISL(I,J) = 0.0	27270
C			27280
			27290
		CALCULATE DISTRESS	27300
		DO 30 J = 1, 100	27310
		IF (J.EQ.100) GO TO 507	27320
C			27330
		W(J)= J	27340
		TO =W(J)/100.0	27350
		GO TO 508	27360
		507 TO = .9910	27370
			27380

508 CONTINUE

C		27390
C	RUTTING AREA NOW	27400
C		27410
	SO = ALOG (TO)	27420
	RO = ABS (SO)	27430
	ANW = RO**(1/BETRA)	27440
	N = (1/ANW)*RHORA	27450
C		27460
C	RUTTING AREA NEXT YEAR	27470
		27480
	N18 = N + (N18MTH * 12.0/1000000.0)	27490
	PWR = (RHORA/N18)**BETRA	27500
	DISL(1,J) = EXP(-PWR) * 100.0	27510
	DISL(2,J) = EXP(-PWR) * 100.0	27520
C		27530
C	RAVELLING	27540
		27550
	ANW = RO**(1/BETRV)	27560
	ADT = (1/ANW)*RHORV	27570
	NADT = ADT + (AADT * 365./1000000.)	27580
	PWR = (RHORV/NADT)**BETRV	27590
	DISL(3,J) = EXP(-PWR) * 100.0	27600
C		27610
C	FLUSHING	27620
		27630
	ANW = RO**(1/BETFL)	27640
	ADT = (1/ANW)*RHOFL	27650
	NADT = ADT + (AADT * 365./1000000.)	27660
	PWR = (RHOFL/NADT)**BETFL	27670
	DISL(4,J) = EXP(-PWR) * 100.0	27680
C		27690
C	PATCHING	27700
	ANW = RO**(1/BETPT)	27710
	N = (1/ANW)*RHOPT	27720
		27730
	PATCHING AREA NEXT YEAR	27740
		27750
	N18 = N + (N18MTH * 12.0/1000000.0)	27760
	PWR = (RHOPT/N18)**BETPT	27770
		27780
	DISL(5,J) = J + 3.0	27790
C		27800
C	ALLIGATOR CRACKING NOW	27810
		27820
	BA = 1/BETAA	27830
	ANW = RO**BA	27840
	N = (1/ANW)*RHOAA	27850
C		27860
C	ALLIGATOR CRACKING NEXT YEAR	27870
		27880
	N18 = N + (N18MTH * 12.0/1000000.0)	27890
	PWR = (RHOAA/N18)**BETAA	27900
	DISL(6,J) = EXP(-PWR) * 100.0	27910
C		27920
C	LONGITUDINAL CRACKING NOW	27930
		27940
	BD = 1/BETLA	27950
	ANW = RO**BD	27960
	N = (1/ANW)*RHOLA	27970
C		27980
C	LONGITUDINAL CRACKING NEXT YEAR	27990
		28000
	IN = INT(N)	28010
	IN = IN + 12	28020
	PWR = (RHOLA/IN)**BETLA	28030
	DISL(7,J) = EXP(-PWR) * 100.0	28040
C		28050
C	TRANSVERSAL CRACKING NOW	28060
		28070
	BL = 1/BETTA	28080
	ANW = RO**BL	28090
	N = (1/ANW)*RHOTA	28100
		28110
		28120

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		28140
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		28170
		28180
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		28800
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		28860

TRANSVERSAL CRACKING NEXT YEAR

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IN = INT(N)
IN = IN + 12
PWR = (RHOTA/IN)**BETTA
DISL(8,J) = EXP(-PWR) * 100.0
30 CONTINUE

```

PSI

```

DO 40 J = 1,50
PSIL(J) = PO
IF( RHOP .LE. 0.0 ) GO TO 40
IF(PF.GE.PO) GO TO 39
PS(J) = J/10.0
PIS(J) = PS(J)
IF(PS(J).GE.PO) PIS(J) = PO - .001
B1 = (PO-PIS(J))/(PO-PF)
B2 = ALOG(B1)
B3 = B2 * (-1.0)
IF (B3.LT.0.0) GO TO 99
ANW = B3**(1/BETAP)
N1 = (1/ANW)*RHOP
N2 = N1*1000000.0/N18MTH
N2 = N2 + 12
N18 = N2 * N18MTH/1000000.0
PWR = (RHOP/N18)**BETAP
PSIL(J) = PO - (PO - PF) * EXP(-PWR)
IF (PSIL(J).GT.PS(J)) PSIL(J) = PS(J) - 0.15
PSTE = PSIL(J)
GO TO 29
39 PSIL(J) = 2.65
GO TO 29
99 PSIL(J) = PIS(J)
29 CONTINUE
40 CONTINUE

```

```

WRITE(6,251) T150, FTC, AVT, PI, FLEXL, DMD, LL, N18MTH
251 FORMAT(T10, 'DATA INPUTS: '//T10, 'T1+50 FTC AVT PI FLEXL'
&, ' DMD LL N18/MTH'/T7,4F7.0,2F7.1,F7.0,F10.0)

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```

WRITE(6,252)
252 FORMAT(/T26, 'DISTRESS', T60, 'DISTRESS' /T12, 'N RUTTING',
& ' RAVL FLUSH PATCH ALIG LONG TRNS ' / T11,
& ' S'ACT ONE TWO AREA AREA AREA AREA AREA')

```

```

DO 41 J = 1, 100
WRITE(6,255) W(J), (DISL(I,J), I = 1, 8)
255 FORMAT( F14.2, 2X, 8F6.2)
41 CONTINUE
DO 44 J = 1, 50
WRITE(6,257) PS(J), PSIL(J)
257 FORMAT(T8,F5.3,10X,F5.3)
44 CONTINUE

```

```

KNT = KNT + 1
IF( KNT .EQ. 1 ) WRITE(6,200)
IF( KNT .EQ. 1 ) KNT = 0

```

RETURN
END

```

SUBROUTINE SURVTA (CNTY,JX,IT,PESC,TIN,FRTH,AVTP,PLSX,
OV2,OV3,BB2,BB3,OVTH,ASPH,DMD,DISL,
FLEXL,PSIL,EALT,HPR2,HPR3,AADT,AKIP,HMAC)

```

THIS SUBROUTINE CALCULATES THE SURVIVAL CURVES OF

SECTIONS OF ROAD THAT HAVE BEEN REHABILITATED		
		28870
		28880
		28890
		28900
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		29000
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		29580
		29590
		29600

	HPR3 = BB3(4, IC)	29610
	GO TO 25	29620
C	23 IF (JX.NE.2) GO TO 24	29630
	ASPH = ASPH + 2.0	29640
	HPR2 = HPR2 + 2.0	29650
	IF (HPR2.GT.BB2(4, IC)) HPR2 = BB2(4, IC)	29660
	HPR3 = HPR3 - 0.4	29670
	IF (HPR3.LT.BB3(4, IC)) HPR3 = BB3(4, IC)	29680
	GO TO 25	29690
C	24 ASPH = ASPH + 0.75	29700
		29710
		29720
	25 CALL BB (CNTY, IT, PESC, TIN, FRTH, AVTP, ASPH, HPR2, HPR3, PLSX,	
	1 DISL, PSIL, EALT)	29740
	GO TO 50	29750
C		29760
C		29770
C	HOT MIX	29780
C		29790
	30 IF (IT.NE.5.AND.IT.NE.6) GO TO 40	29800
	IF (JX.NE.1) GO TO 40	29810
	HMAC = HMAC + 0.75	29820
	CALL HM(CNTY, IT, PESC, TIN, FRTH, AVTP, HMAC, HPR2, HPR3, PLSX,	29830
	1 DISL, PSIL, EALT)	29840
	GO TO 50	29850
C		29860
C		29870
C	OVERLAY	29880
C		29890
	40 IF (IT.NE.5. AND .IT.NE.6) GO TO 41	29900
	IC = 1	29910
	GO TO 42	29920
	41 IC = IT - 5	29930
	42 CONTINUE	29940
	IF (JX.NE.5) GO TO 43	29950
	OVTH = 6.0	29960
	HPR2 = OV2(4, IC)	29970
	HPR3 = OV3(4, IC)	29980
	GO TO 47	29990
C		30000
	43 IF (JX.NE.4) GO TO 44	30010
	OVTH = 4.5	30020
	HPR2 = OV2(4, IC)	30030
	HPR3 = OV3(4, IC)	30040
	GO TO 47	30050
C		30060
	44 IF (JX.NE.3) GO TO 45	30070
	OVTH = 3.0	30080
	HPR2 = OV2(4, IC)	30090
	HPR3 = OV3(4, IC)	30100
	GO TO 47	30110
C		30120
	45 IF (JX.NE.2) GO TO 46	30130
	OVTH = 2.0	30140
	HPR2 = HPR2 + 2.0	30150
	IF (HPR2.GT.OV2(4, IC)) HPR2 = OV2(4, IC)	30160
	HPR3 = HPR3 - 0.4	30170
	IF (HPR3.LT.OV3(4, IC)) HPR3 = OV3(4, IC)	30180
	GO TO 47	30190
C		30200
	46 OVTH = 0.75	30210
	47 CALL OV (CNTY, IT, PESC, TIN, FRTH, AVTP, HPR2, HPR3, OVTH, PLSX, DISL,	30220
	1 PSIL, EALT)	30230
C		30240
	50 RETURN	30250
	END	30260
C		30270
		30280
	SUBROUTINE HM(CNTY, IT, PESC, TIN, FRTH, AVTP, HMAC, HPR2, HPR3, PLSX,	30290
	1 DISL, PSIL, EALT)	30300
C		30310
C	*****	30320
C		30330
C	THIS SUBROUTINE USES THE SURVIVAL CURVES TO GENERATE	30340


```

      GO TO 223
      LOG RHO & BETA , RUTTING AREA
222 X(1) = PI**(-0.3303)*HMAC**(0.5756)
      X(2) = AVT**(0.2454)*HPR2**(-0.1132)
      LINEAR RHO & BETA , RUTTING SEVERITY
223 Y(1) = -0.0077*PI+0.386*HMAC
      Y(2) = -0.00072*FTC+0.0273*AVT-0.00267*HMAC-0.000418*HPR2
      IF( Y(1) .LT. 0.0 .OR. Y(2) .LT. 0.0 ) GO TO 232
      GO TO 233
      LOG RHO & BETA , RUTTING SEVERITY
232 Y(1) = PI**(-0.4591)*HMAC**(0.826)
      Y(2) = 2.6063
      LINEAR RHO & BETA , ALLIGATOR CRACK AREA
233 X(3) = 5.315 + 0.825*HMAC + 1.189*HPR3 - 0.0162*XTI - 0.114*AVT
      X(4) = 7.927 + 1.124*HMAC - 0.191*HPR2 - 0.198*PI
      IF( X(3) .LT. 0.0 .OR. X(4) .LT. 0.0 ) GO TO 242
      GO TO 243
      LOG RHO & BETA , ALLIGATOR CRACK AREA
242 X(3) = PI**(-0.4155)*HMAC**(0.8605)
      X(4) = 2.8661
      LINEAR RHO & BETA , ALLIGATOR CRACK SEVERITY
243 Y(3) = -0.000075*PI+0.291*HMAC
      Y(4) = 3.145*HPR3
      IF( Y(3) .LT. 0.0 .OR. Y(4) .LT. 0.0 ) GO TO 252
      GO TO 253
      LOG RHO & BETA , ALLIGATOR CRACK SEVERITY
252 Y(3) = PI**(-0.5858)*HMAC**(1.1462)
      Y(4) = 3.7293
      LINEAR RHO & BETA LONG. CRACK AREA
253 X(5) = -0.988*FTC+4.38*AVT-2.99*PI+7.21*HMAC
      X(6) = 0.0422*FTC + 0.359*HPR3
      IF( X(5) .LT. 0.0 .OR. X(6) .LT. 0.0 ) GO TO 262
      GO TO 263
      LOG RHO & BETA , LONG. CRACK AREA
262 X(5) = XTI**(-0.1089)*FTC**(-0.1605)*AVT**(1.6203)
      & *PI**(-0.2219)*HPR3**(-0.0088)
      X(6) = FTC**(0.241)*HMAC**(-0.0653)
      LINEAR RHO & BETA , LONG. CRACK SEVERITY
263 Y(5) = -0.144*XTI+3.018*AVT-3.155*PI+8.331*HMAC
      Y(6) = 0.0343*XTI + 0.0502*FTC
      IF( Y(5) .LT. 0.0 .OR. Y(6) .LT. 0.0 ) GO TO 272
      GO TO 273
      LOG RHO & BETA , LONG. CRACK SEVERITY
272 Y(5) = XTI**(-0.1232)*FTC**(-0.119)*AVT**(1.6797)
      & *PI**(-0.4653)*HMAC**(0.1199)
      Y(6) = FTC**(0.3494)
      LINEAR RHO & BETA , TRANS. CRACK AREA
273 X(7) = -1.97*TI -0.826*FTC+5.193*AVT-1.768*PI-26.3*HPR3
      X(8) = 0.017*TI +0.0433*FTC-0.115*HMAC-0.0159*HPR2
      & + 0.259*HPR3

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IF( X(7) .LT. 0.0 .OR. X(8) .LT. 0.0 ) GO TO 282
GO TO 283
LOG RHO & BETA, TRANS. CRACK AREA
282 X(7) = XTI**(-0.4432)*FTC**(-0.2055)*AVT**(1.8972)*PI**(-0.2218)
X(8) = XTI**(0.0702)*FTC**(0.311)*HMAC**(-0.494)*HPR2**(-0.1237)
LINEAR RHO & BETA, TRANS. CRACK SEVERITY
283 Y(7) = -0.196*XTI+2.9*AVT-2.69**PI+5.475*HMAC
Y(8) = 0.0519*FTC + 0.537*HPR3
IF( Y(7) .LT. 0.0 .OR. Y(8) .LT. 0.0 ) GO TO 292
GO TO 293
LOG RHO & BETA, TRANS. CRACK SEVERITY
292 Y(7) = XTI**(-0.1399)*FTC**(-0.157)*AVT**(1.7128)
& *PI**(-0.5024)*HMAC**(0.0348)*HPR2**(0.0606)
Y(8) = FTC**(0.2462) * HPR2**(-0.0049)
293 CONTINUE
RHORA = X(1)
BETRA = X(2)
RHOAA = X(3)
BETAA = X(4)
RHOLA = X(5)
BETLA = X(6)
RHOTA = X(7)
BETTA = X(8)
RHOP = X(9)
BETAP = X(10)
PF = X(11)
RHORS = Y(1)
BETRS = Y(2)
RHOAS = Y(3)
BETAS = Y(4)
RHOLS = Y(5)
BETLS = Y(6)
RHOTS = Y(7)
BETTS = Y(8)
WRITE(6,300) RHORA, BETRA, RHOAA, BETAA,
& RHOTA, BETTA, RHOLA, BETLA,
& RHOP, BETAP, PF
300 FORMAT( // 1X, 10G13.5 / 1X, 1G13.5 / )
DO 15 I = 1, 5
DO 15 J = 1, 100
15 DISL(I,J) = 0.0
CALCULATE DISTRESS
DO 30 J = 1, 100
IF (J.EQ.100) GO TO 507
W(J)= J
TO =W(J)/100.0
GO TO 508
507 TO = .9910
508 CONTINUE
RUTTING AREA NOW
SO = ALOG (TO)
RO = ABS (SO)
ANW = RO**(1/BETRA)

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	N = (1/ANW)*RHORA	32570
	NX = N*1000000.0/N18MTH	32580
000	RUTTING AREA NEXT YEAR	32590
	IN = INT(NX)	32600
	IN = IN + 12	32610
	N18 = N18MTH*IN/1000000.0	32620
	PWR = (RHORA/N18)**BETRA	32630
	DISL(1,J) = EXP(-PWR) * 100.0	32640
	DISL(2,J) = EXP(-PWR) * 100.0	32650
		32660
		32670
000	RAVELLING	32680
	DISL(3,J) = J + 1.0	32690
000	FLUSHING	32700
	DISL(4,J) = J + 1.0	32710
000	FAILURES	32720
	DISL(5,J) = J + 1.0	32730
000	ALLIGATOR CRACKING NOW	32740
	BA = 1/BETAA	32750
	ANW = RO**BA	32760
	N = (1/ANW)*RHOAA	32770
	NX = N*1000000.0/N18MTH	32780
000	ALLIGATOR CRACKING NEXT YEAR	32790
	IN = INT(NX)	32800
	IN = IN + 12	32810
	N18 = N18MTH*IN/1000000.0	32820
	PWR = (RHOAA/N18)**BETAA	32830
	DISL(6,J) = EXP(-PWR) * 100.0	32840
000	LONGITUDINAL CRACKING NOW	32850
	BD = 1/BETLA	32860
	ANW = RO**BD	32870
	N = (1/ANW)*RHOLA	32880
000	LONGITUDINAL CRACKING NEXT YEAR	32890
	IN = INT(N)	32900
	IN = IN + 12	32910
	PWR = (RHOLA/IN)**BETLA	32920
	DISL(7,J) = EXP(-PWR) * 100.0	32930
000	TRANSVERSAL CRACKING NOW	32940
	BL = 1/BETTA	32950
	ANW = RO**BL	32960
	N = (1/ANW)*RHOTA	32970
000	TRANSVERSAL CRACKING NEXT YEAR	32980
	IN = INT(N)	32990
	IN = IN + 12	33000
	IF (ICK.EQ.1) GO TO 27	33010
	PWR = (RHOTA/IN)**BETTA	33020
	IF (PWR.LT.0.090) ICK = 1	33030
	GO TO 28	33040
27	PWR = 0.073	33050
28	DISL(8,J) = EXP(-PWR) * 100.0	33060
30	CONTINUE	33070
000	PSI	33080
		33090
		33100
		33110
		33120
		33130
		33140
		33150
		33160
		33170
		33180
		33190
		33200
		33210
		33220
		33230
		33240
		33250
		33260
		33270
		33280
		33290
		33300

C	1.- SEAL CRACKS	34050
C	2.- PATCHING	34060
C	3.- FULL DEPTH REPAIR	34070
C	4.- FOG SEAL	34080
C	5.- STRIP SEAL	34090
C	6.- SEAL COAT	34100
C	7.- ASPHALT-RUBBER SEAL	34110
C	8.- SLURRY SEAL	34120
C	9.- LEVEL UP	34130
C	10.- THIN OVERLAY	34140
C	11.- ROTOMILL	34150
C	12.- SPOT SEAL	34160
C	13.- ROTOMILL + SEAL	34170
C	14.- ROTOMILL + OVERLAY	34180
C		34190
C		34200
C	DIMENSION IVIS(7)	34210
C	DIMENSION TOTALS(9,7), TOTLN(7), IEXT(6)	34220
C	DIMENSION MTREE(7, 28, 4), CSQYD(12,7), CCOST(12,7)	34230
C	DIMENSION IST(9), AREA(9), DST(6), DAREA(6), DCOST(6)	34240
C		34250
C	INTEGER DIST, CNTY, ADTL, EALT, PESC	34260
C	INTEGER BMIL, EMIL, HWFC, DST	34270
C	REAL LNTH	34280
C		34290
C	REAL*8 HWAY	34300
C	REAL*8 UNIT2	34310
C		34320
C	DATA MSTRAT / 'SEAL CRA' 'CKS	34330
C	1 'PATCH	34340
C	2 'FULL DEP' 'TH RPAIR	34350
C	3 'FOG SEAL	34360
C	4 'STRIP SE' 'AL	34370
C	5 'SEAL COA' 'T	34380
C	6 'ASP-RUBB' 'ER SEAL	34390
C	7 'SLURRY S' 'EAL	34400
C	8 'LEVEL UP	34410
C	9 'THIN OVE' 'RLAY	34420
C	1 'ROTOMILL	34430
C	2 'SPOT SEA' 'L	34440
C	3 'ROTOMILL' '+ SEAL	34450
C	'ROTOMILL' '+ OVERLY' /	34460
C		34470
C	DATA DSTRES / 'SLIGHT R' 'UTTING	34480
C	1 'SEVERE R' 'UTTING	34490
C	2 'RAVELLIN' 'G	34500
C	3 'FLUSHING	34510
C	4 'FAILURES	34520
C	5 'ALLIGATO' 'R CRACKI' 'NG	34530
C	6 'LONGITUD' 'INAL CRA' 'CKING	34540
C	7 'TRANSVER' 'SE CRACK' 'ING	34550
C	8 'RIDING' 'QUALITY' /	34560
C		34570
C	DATA UNITS / 'SQ YDS' 'SQ YDS' 'SQ YDS' 'SQ YDS' /	34580
C	1 'PER LN/M' 'SQ YDS' 'LN FEET' 'LN FEET' /	34590
C	DATA UNIT2 / 'SQ YDS' 'SQ YDS' 'SQ YDS' 'SQ YDS' /	34600
C	1 'TOTAL' 'SQ YDS' 'LN FEET' 'LN FEET' 'MEAN PSI' /	34610
C		34620
C	DATA RSTRAT / 'SEAL COA' 'T	34630
C	1 'THIN OVE' 'RLAY	34640
C	2 'MEDIUM O' 'VERLAY	34650
C	3 'THICK OV' 'ERLAY	34660
C	4 'RECONSTR' 'UCTION' /	34670
C		34680
C	DATA RST10 / 'SEAL COA' 'T	34690
C	1 'SECTIONA' 'L RECONS' 'TRUCTION' /	34700
C	2 'FULL REC' 'ONSTRUCT' 'ION' /	34710
C		34720
C	DATA PTTYPE / 'THICK HO' 'T MIX' '>5.5INS' /	34730
C	1 'INTERMED' 'IATE HOT' 'MIX' /	34740
C	2 'THIN HOT' 'MIX' '<2.5INS' /	34750
C	3 'COMPOSIT' 'E' /	34760
C	4 'WIDENED' 'OLD CONC' 'ETE' /	34770
C	5 'WIDENED' 'OLD FLEX' 'IBLE' /	34780

	20	LNTH = 2.0	35530
C		RCOST = RCOST * 100.0	35540
C		CALL STRAT (IT, IVIS, LOWHI, SRVC, NLANES, WPTH, LNTH, MTRTE,	35550
	1	IST, AREA, DST, DAREA, DCOST, JS)	35560
C		IF (JS .EQ. 0) GOTO 61	35570
		DO 60 I = 1, JS	35580
C		CSQYD(DST(I),IC) = CSQYD(DST(I),IC) + DAREA(I)	35590
C		CCOST(DST(I),IC) = CCOST(DST(I),IC) + DCOST(I)	35600
C	60	CONTINUE	35610
C	61	CONTINUE	35620
		DO 62 I = 1, 8	35630
C	62	TOTALS(I,IC) = TOTALS(I,IC) + AREA(I)	35640
C		TOTALS(9,IC) = TOTALS(9,IC) + SRVC	35650
C		TOTLN(IC) = TOTLN(IC) + 1.0	35660
C		WRITE (6, 600) DIST	35670
C		WRITE (6, 610) CNTY, HWAY	35680
C		WRITE (6, 620) BMIL, BSIGN, BDISP, EMIL, ESIGN, EDISP,	35690
C	1	LANE, PESC	35700
		K = IT - 3	35710
C		WRITE (6, 630) (PTYPE(J,K),J=1,3), ADTL, EALT	35720
		WRITE (6, 635)	35730
		DO 100 I = 1, 8	35740
C		IF (AREA(I) .EQ. 0.0) GOTO 100	35750
C		IF (IST(I) .EQ. 0) GOTO 95	35760
C		WRITE (6, 640) (DSTRES(J,I),J=1,3), AREA(I), UNITS(I),	35770
C	1	(MSTRAT(J,IST(I)),J=1,2)	35780
		GOTO 100	35790
C	95	CONTINUE	35800
C	95	WRITE (6, 641) (DSTRES(J,I),J=1,3), AREA(I), UNITS(I)	35810
C	100	CONTINUE	35820
		IF (IST(9) .EQ. 0) GOTO 102	35830
C		WRITE (6, 650) (DSTRES(J,9),J=1,3), SRVC,	35840
C	1	(MSTRAT(J,9),J=1,2)	35850
		GOTO 103	35860
C	102	CONTINUE	35870
C	102	WRITE (6, 651) (DSTRES(J,9),J=1,3), SRVC	35880
C	103	CONTINUE	35890
		WRITE (6, 670)	35900
		IF (JX .EQ. 0) GOTO 160	35910
C		IF (.IT .EQ. 10) GOTO 140	35920
C		WRITE (6, 675) (RSTRAT(J,JX),J=1,2), RCOST	35930
C		GOTO 160	35940
C	140	CONTINUE	35950
C	140	WRITE (6, 680) (RST10(J,JX),J=1,3), RCOST	35960
C		GOTO 160	35970
C	150	CONTINUE	35980
C	150	WRITE (6, 690)	35990
C	160	CONTINUE	36000
C		WRITE (6, 700)	36010
		TOT = 0.0	36020
		IF (JS .EQ. 0) GOTO 175	36030
		ISTE = 0	36040
		DO 170 I = 1, JS	36050
C		WRITE (6, 710) (MSTRAT(J,DST(I)),J=1,2), DAREA(I), DCOST(I)	36060
		TOT = TOT + DCOST(I)	36070
C	170	CONTINUE	36080
C		WRITE (6, 720) TOT	36090
C		DO 176 I = 1,JS	36100
		IEXT(I) = DST(I)	36110
		ISTE = ISTE + 1	36120
C	176	CONTINUE	36130
		GOTO 180	36140
C	175	CONTINUE	36150
C	175	WRITE (6, 730)	36160
			36170
			36180
			36190
			36200
			36210
			36220
			36230
			36240
			36250
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      IEXT(1) = 0
      ISTE = 1
C 177 CONTINUE
      180 CONTINUE
C
      INDIST = DIST
      INCNTY = CNTY
      DO 302 I = 1, 7
      IF (TOTLN(I) .EQ. 0.0) GOTO 302
      TOTALS(9,I) = TOTALS(9,I) / TOTLN(I)
C 302 CONTINUE
      DO 304 I = 1, 8
      WRITE( 6, 734) (DSTRES(J,I), J=1,3), UNIT2(I),
      1 (TOTALS(I,J), J=1,7)
C 304 CONTINUE
      WRITE( 6, 735) (DSTRES(J,9), J=1,3), UNIT2(9),
      1 (TOTALS(9,J), J=1,7)
      WRITE ( 6, 600) INDIST
      WRITE ( 6, 733)
      WRITE ( 6, 740) INDIST
      DO 310 I = 1, 12
      WRITE ( 6, 750) (MSTRAT(J,I), J=1,2), UNITS(IORDER(I)),
      1 (CSQYD(I,J), J=1,7)
C 310 CONTINUE
      PRINT ESTIMATED COST DATA
      ANY SMALL DOLLAR AMOUNTS LT 1000 SET EQUAL TO 0
      DO 315 I = 1, 12
      DO 316 J = 1, 7
      IF ( CCOST(I,J) .LT. 1000.0 ) CCOST(I,J) = 0.0
C 316 CONTINUE
C 315 CONTINUE
      WRITE ( 6, 600) INDIST
      WRITE ( 6, 733)
      WRITE ( 6, 760) INDIST
      DO 320 I = 1, 12
      WRITE ( 6, 770) (MSTRAT(J,I), J=1,2), (CCOST(I,J),J=1,7)
C 320 CONTINUE
      IF ( DIST .NE. 99) GOTO 10
      RETURN
      END
C
      SUBROUTINE SETUP(MTREE)
C
      *****
      THIS SUBROUTINE ASSIGN REHABILITATION STRATEGIES TO EVERY
      BRANCH OF THE DECISION TREE.
      *****
C
      DIMENSION MTREE( 7, 28, 4)
      READ ( 3, 100) ((MTREE(I,J,K),K=1,4),J=1,28),I=1,7)
C 100 FORMAT (4(I2,1X))
      RETURN
      END
C
      SUBROUTINE STRAT(IT,IVIS,LOWHI,SRVC,NLANES,WDTH,LNTH,MTREE,
      1 IST, AREA, DST, DAREA, DCOST, JS)
C
      *****
      THIS SUBROUTINE IS USED BY SUBROUTINE TRE IN THE SELECTION
      OF THE BEST MAINTENANCE STRATEGY.
      *****

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C			37010
		DIMENSION RUT(7), DIS(4), DPATCH(7), SPATCH(7), JSEQ1(4)	37020
		DIMENSION JSEQ2(3), PAREA(3,8), COST(12)	37030
		DIMENSION MTREE(7,28,4), IVIS(7), JMS(14), RAREA(14)	37040
		INTEGER DST, RUT, DIS	37050
		REAL LNTH	37060
C			37070
		DIMENSION IST(9), AREA(9), DST(6), DAREA(6), DCOST(6)	37080
C			37090
		DATA RUT / 000, 100, 010, 001, 200, 020, 002 /	37100
		DATA DIS / 000, 100, 010, 001 /	37110
		DATA DPATCH / 12.0, 10.0, 8.0, 10.0, 10.0, 10.0, 4.0 /	37120
		DATA SPATCH / 6.0, 4.5, 2.5, 2.5, 2.5, 1.5 /	37130
		DATA JSEQ1 / 7, 6, 8, 4 /	37140
		DATA JSEQ2 / 5, 12, 1 /	37150
		DATA PAREA / 5.0, 30.0, 50.0,	37160
	2	5.0, 30.0, 50.0,	37170
	3	5.0, 30.0, 50.0,	37180
	4	5.0, 30.0, 50.0,	37190
	5	1.0, 3.0, 5.0,	37200
	6	2.0, 10.0, 20.0,	37210
	7	50.0, 100.0, 200.0,	37220
	8	2.0, 5.0, 10.0 /	37230
		DATA COST / 0.25, 1.5, 2.5, 0.5, 0.5, 0.95, 1.2, 0.6,	37240
	1	1.5, 2.4, 1.7, 0.5 /	37250
			37260
C			37270
			37280
		DO 10 N = 1, 9	37290
		AREA(N) = 0.0	37300
		IST(N) = 0	37310
C	10	CONTINUE	37320
			37330
		DO 11 N = 1, 5	37340
		DST(N) = 0	37350
		DAREA(N) = 0.0	37360
		DCOST(N) = 0.0	37370
C	11	CONTINUE	37380
			37390
		K = LOWHI	37400
			37410
C			37420
		HANDLE RUTTING SEPERATELY	37430
			37440
		DO 20 NR = 1, 7	37450
		IF (IVIS(1) .EQ. RUT(NR)) GOTO 30	37460
C	20	CONTINUE	37470
		IF (NR .EQ. 1) GOTO 50	37480
C	30	J = NR - 1	37490
		IF (J .LT. 4) GOTO 40	37500
		IST(2) = MTREE(IT-3,J,K)	37510
		AREA(2) = PAREA(J-3,2) * 0.01 * LNTH * 1760. * WIDTH * 0.33	37520
		GOTO 50	37530
	40	CONTINUE	37540
		IST(1) = MTREE(IT-3,J,K)	37550
		AREA(1) = PAREA(J,1) * 0.01 * LNTH * 1760. * WIDTH * 0.33	37560
C	50	CONTINUE	37570
			37580
C			37590
		OTHER DISTESS TYPES	37600
			37610
		DO 100 M = 2, 7	37620
		L = M + 1	37630
		DO 110 N = 1, 4	37640
		IF (IVIS(M) .EQ. DIS(N)) GOTO 120	37650
C	110	CONTINUE	37660
		IF (N .EQ. 1) GOTO 100	37670
	120	J = 6 + (M-2)*3 + N-1	37680
		IST(L) = MTREE(IT-3,J,K)	37690
		AREA(L) = PAREA(N-1,L) * 0.01 * LNTH * 1760. * WIDTH * 0.33	37700
		IF (M.EQ.4) AREA(L) = PAREA(N-1,L) * NLANES	37710
		IF (M.EQ.6) AREA(L) = PAREA(N-1,L) * LNTH * 5280. * 0.01	37720
		IF (M.EQ.7) AREA(L) = PAREA(N-1,L) * WIDTH * 5280. * 0.01 * LNTH	37730
C			37740

C	PUT IN CASE OF M = 6 AND 7	37750
C	100 CONTINUE	37760
C		37770
C		37780
	J = 25	37790
	IF (SRVC .GT. 3.0) GOTO 200	37800
	J = 26	37810
	IF (SRVC .GT. 2.5) GOTO 200	37820
	J = 27	37830
	IF (SRVC .GT. 1.5) GOTO 200	37840
	J = 28	37850
	200 IST(9) = MTREE(IT-3,J,K)	37860
	AREA(9) = LNTH * 1760. * WPTH * 0.33	37870
C		37880
C	WRITE(6,201) (IST(I),I=1,9)	37890
C	WRITE(6,202) (AREA(I),I=1,8)	37900
C201	FORMAT(10X, 9I6)	37910
C202	FORMAT(10X, 8F6.1)	37920
C	DOMINANT STRATEGY CALCCULATION	37930
C		37940
	DO 300 I = 1, 14	37950
300	JMS(I) = 0	37960
	DO 310 I = 1,9	37970
	IF (IST(I) .EQ. 0) GOTO 310	37980
	J = IST(I)	37990
	JMS(J) = 1	38000
	RAREA(J) = AREA(I)	38010
310	CONTINUE	38020
C		38030
	JS = 0	38040
	IF (JMS(3) .EQ. 0) GOTO 395	38050
	JS = JS + 1	38060
	DST(JS) = 3	38070
	DAREA(JS) = RAREA(3) * 20.0	38080
	DCOST(JS) = DAREA(JS) * DPATCH(IT-3) * COST(3)	38090
C		38100
395	IF (JMS(2) .EQ. 0) GOTO 400	38110
	JS = JS + 1	38120
	DST(JS) = 2	38130
	DAREA(JS) = RAREA(2) * 20.0	38140
	DCOST(JS) = DAREA(JS) * SPATCH(IT-3) * COST(2)	38150
C		38160
400	IF (JMS(13) .EQ. 0) GOTO 440	38170
	JS = JS + 2	38180
	DST(JS-1) = 11	38190
	DST(JS) = 9	38200
	DAREA(JS-1) = RAREA(11)	38210
	DCOST(JS-1) = DAREA(JS-1) * COST(11)	38220
	DAREA(JS) = RAREA(9)	38230
	DCOST(JS) = DAREA(JS) * COST(9)	38240
	GOTO 999	38250
C		38260
440	IF (JMS(14) .EQ. 0) GOTO 450	38270
	JS = JS + 2	38280
	DST(JS-1) = 11	38290
	DST(JS) = 10	38300
	DAREA(JS-1) = RAREA(11)	38310
	DAREA(JS) = LNTH * 1760. * WPTH * 0.33	38320
	DCOST(JS-1) = DAREA(JS-1) * COST(11)	38330
	DCOST(JS) = DAREA(JS) * COST(10) * 1.5	38340
	GOTO 999	38350
C		38360
450	IF (JMS(11) .EQ. 0) GOTO 460	38370
	JS = JS + 1	38380
	DST(JS) = 11	38390
	DAREA(JS) = RAREA(11)	38400
	DCOST(JS) = DAREA(JS) * COST(11)	38410
C		38420
460	IF (JMS(10) .EQ. 0) GOTO 470	38430
	JS = JS + 1	38440
	DST(JS) = 10	38450
	DAREA(JS) = LNTH*1760. * WPTH*0.33	38460
	DCOST(JS) = DAREA(JS) * COST(10)	38470
		38480

	GOTO 999	38490
C	470 IF (JMS(9) .EQ. 0) GOTO 480	38500
	JS = JS + 1	38510
	DST(JS) = 9	38520
	DAREA(JS) = RAREA(9)	38530
	DCOST(JS) = DAREA(JS) * COST(9)	38540
	GOTO 999	38550
		38560
C	480 DO 490 I = 1, 4	38570
	IF (JMS(JSEQ1(I)) .EQ. 0) GOTO 490	38580
	JS = JS + 1	38590
	DST(JS) = JSEQ1(I)	38600
	DAREA(JS) = LNTH*1760. * WPTH*0.33	38610
	DCOST(JS) = DAREA(JS) * COST(JSEQ1(I))	38620
	GOTO 999	38630
		38640
C	490 CONTINUE	38650
		38660
	DO 500 I = 1, 3	38670
	IF (JMS(JSEQ2(I)) .EQ. 0) GOTO 500	38680
	JS = JS + 1	38690
	DST(JS) = JSEQ2(I)	38700
	DAREA(JS) = RAREA(JSEQ2(I))	38710
	DCOST(JS) = DAREA(JS) * COST(JSEQ2(I))	38720
	GOTO 999	38730
		38740
C	500 CONTINUE	38750
		38760
	999 CONTINUE	38770
	IF (IT .EQ. 10) GOTO 1010	38780
	IF (JS .EQ. 0) GOTO 1010	38790
	DO 1000 IX = 1, JS	38800
1000	IF (DST(IX) .EQ. 2) GOTO 1002	38810
	IF (IVIS(5) .EQ. 000 .OR. IVIS(5) .EQ. 100) GOTO 1002	38820
	JS = JS + 1	38830
	J = 2	38840
	IF (IVIS(5) .EQ. 001) J = 3	38850
	DST(JS) = 2	38860
	DAREA(JS) = PAREA(J,6)*0.01 * LNTH*1760. * WPTH*0.33	38870
	DCOST(JS) = DAREA(JS) * COST(2) * SPATCH(IT-3)	38880
1002	CONTINUE	38890
		38900
		38910
	LOOK FOR LONGITUDINAL CRACKS	38920
		38930
		38940
	DO 1004 IX = 1, JS	38950
1004	IF (DST(IX) .EQ. 1) GOTO 1010	38960
	IF (IVIS(6) .EQ. 000 .OR. IVIS(6) .EQ. 100) GOTO 1006	38970
	JS = JS + 1	38980
	J = 2	38990
	IF (IVIS(6) .EQ. 001) J = 3	39000
	DST(JS) = 1	39010
	DAREA(JS) = PAREA(J,7)*LNTH*5280.*0.01	39020
	DCOST(JS) = DAREA(JS) * COST(1)	39030
1006	CONTINUE	39040
		39050
		39060
	LOOK FOR TRANSVERSE CRACKING	39070
		39080
		39090
	IF (IVIS(7) .EQ. 000 .OR. IVIS(7) .EQ. 100) GOTO 1010	39100
	JS = JS + 1	39110
	J = 2	39120
	IF (IVIS(7) .EQ. 001) J = 3	39130
	DST(JS) = 1	39140
	DAREA(JS) = PAREA(J,8) * WPTH * LNTH * 5280. * 0.01	39150
	DCOST(JS) = DAREA(JS) * COST(1)	39160
1010	CONTINUE	39170
	RETURN	39180
	END	39190
C		39200
	SUBROUTINE RECODE(RVIS,IVIS)	39210
C		39220


```

    IRE = 0
    ICHEK = 0
    J = 0
    DO 5 I=1,8
5  REVIS(I) = RVIS(I)
    SERVC = SRVC
    IF ( IENT.EQ.1) GO TO 20
10 CALL SCORE(REVIS,SERV,V,FLEXSC,ADTS,SIBNRY,FUNC,IC,IAVUC,
    1   ISIUC,PESC,IT)
    KPESC = (PESC + 0.001) * 100
    IF (PESC.LT.PESM(IC)) GO TO 20
    IAS = IAS + 1
    CALL AGING (JX,IT,RVISO,TCLS,INX,ISWITH,REVIS,SERV,DISL,PSIL)
    GO TO 10
20 IF (IAS.EQ.0) ICHEK = 1
    J = 3
    CALL FINAVU(IT,J,AVUC,SRVC,SKID,FAVU,FSIU,FSKU,
    1   AVU,SIV,SNV,RVIS,ENDVIS)
    DO 30 I=1,8
    VI(I) = ENDVIS(I)
30 CONTINUE
    SIV1 = SIV
    JX = J
    AREA = LGTH * WPTH
    ACOST = AREA * ECFS(DIST,4)
    CALL SCORE (ENDVIS,SIV,V,FLEXSC,ADTS,SIBNRY,FUNC,IC,IAVUC,ISIUC,
    1   PESF,IT)
    PESF = PESF
    PESX = PESF
    IF (ICHEK.EQ.1) GO TO 70
    OVTHI = OVTH
    ASPHI = ASPH
    HMACI = HMACH
    FLEXLI = FLEXL
    CALL SURVTA (CNTY,JX,IT,PESC,TIN,FRTH,AVTP,PLSX,
    1   OV2,OV3,BB2,BB3,OVTHI,ASPHI,DMD,DOSL,
    2   FLEXLI,PSOL,EALT,THP2,THP3,AADT,AKIP,HMACI)
40 CALL SCORE (ENDVIS,SIV,V,FLEXSC,ADTS,SIBNRY,FUNC,IC,IAVUC,ISIUC,
    1   PESX,IT)
    IF (PESX.LT.PESM(IC)) GO TO 50
    IRE = IRE + 1
    CALL AGING (JX,IT,RVISO,TCLS,INX,ISWITH,ENDVIS,SIV,DOSL,PSOL)
    GO TO 40
50 CONTINUE
    REC = IRE/IAS
    IF(REC.LE.1.0) GO TO 60
    AMTOT = REC * TOT
    IF (AMTOT.GT.ACOST) GO TO 70
60 J = 0
    GO TO 80
70 J = 3
80 CONTINUE
    RETURN
    END

    SUBROUTINE SORT(A,N)
    .....
    THIS SUBROUTINE SORTS IN AN INCREASING MANNER ANY NUMERICAL
    ONE DIMENSIONAL ARRAY.
    .....
    DIMENSION A(N)
    IF (N.LE.1) RETURN
    LAST = N - 1
    DO 20 I = 1, LAST

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41450
41460
41470
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41990
42000
42010
42020
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42050
42060
42070
42080
42090
42100
42110
42120
42130
42140
42150
42160
42170
42180

```

AMIN = A(I)	42190
JMIN = I	42200
JFIRST = I + 1	42210
DO 10 J = JFIRST, N	42220
IF (AMIN.LE.A(J)) GO TO 10	42230
AMIN = A(J)	42240
JMIN = J	42250
10 CONTINUE	42260
A(JMIN) = A(I)	42270
A(I) = AMIN	42280
20 CONTINUE	42290
RETURN	42300
END	42310