

**PILING ANALYSIS
WAVE EQUATION COMPUTER PROGRAM
UTILIZATION MANUAL**

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Research Report Number 33-11

Piling Behavior

Research Project Number 2-5-62-33

Sponsored by

The Texas Highway Department

in Cooperation with the

**U. S. Department of Transportation
Federal Highway Administration
Bureau of Public Roads**

August 1967

**TEXAS TRANSPORTATION INSTITUTE
Texas A&M University
College Station, Texas**

Foreword

The information contained herein was developed on Research Project 2-5-62-33 entitled "Piling Behavior" which is a cooperative research project sponsored jointly by the Texas Highway Department and U. S. Department of Transportation, Federal Highway Administration, Bureau of Public Roads. The broad objective of this project is to fully develop the use of the computer solution of the wave equation so that it may be used to predict driving stresses in piling and be used to estimate the static load bearing capacity of a piling from driving resistance records and basic soils data. This report covers the specific objective of explaining the use of the wave equation computer program.

The opinions, findings, and conclusions expressed in this publication are those of the author and not necessarily those of the Bureau of Public Roads.

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WAVE EQUATION COMPUTER PROGRAM UTILIZATION MANUAL

Introduction

This manual describes the utilization of the computer program for the application of the one-dimensional wave equation to the investigation of a pile during driving. The program is based upon a procedure developed by E. A. L. Smith.¹

The program can be used to obtain the following information for one blow of the pile driver's ram for any specified soil resistance:

1. Stresses in the pile.
2. Displacement of the pile (penetration).
3. Static load capacity of the pile for a specified soil resistance and distribution. This capacity is the static resistance at the time of driving and does not reflect soil set-up due to consolidation.

The program is valuable in that system parameters heretofore ignored (in pile driving formulas) can be included and their effects investigated. It makes possible an engineering evaluation of driving equipment and pile type, rather than relying only upon experience and judgment.

General Pile System Simulation

Figure 1 shows a typical pile system and the idealization (discrete weight-spring idealization for use in the numerical solution of the wave equation) for this system. The idealization includes a simulation of the soil medium as well as the pile driver and pile. The pile hammer and pile are idealized as a system of discrete masses connected by massless springs. The springs represent the stiffness of the pile, cushion, and in some cases the pile driver's ram. The soil medium is assumed to be massless, i.e., the pile moves through the soil and does not move the adjacent soil mass, which is simulated by a spring and damper (dashpot) on each pile segment whose real counterpart is embedded in the medium. This system is completely general. Additions or deletions in the real system (for example, no pile cushion or addition of an anvil between the ram and capblock) can be handled.

In order to simulate a given system, the following information is essential:

1. Pile driver.
 - a) energy and efficiency of hammer,
 - b) weight and dimensions of ram,
 - c) weight and dimensions of anvil (if included),
 - d) dimensions and mechanical properties of capblocks,
 - e) weight and dimensions of pile cap helmet,
 - f) and dimensions and mechanical properties of cushion.
2. Dimensions, weight and mechanical properties of the pile.

3. Soil medium.

- a) embedment of pile,
- b) distribution of soil resistance over the embedded length of the pile expressed as a percentage of the total static soil resistance,
- c) point soil resistance expressed as a percentage of the total static soil resistance,
- d) ultimate elastic displacement for the soil on the side and point of pile.
- e) and the damping constant for the soil on the side and point of the pile.

It should be recognized that the solution obtained with the program represents the results for one blow of the hammer at the specified soil embedment and soil resistance.

The techniques for idealization can be categorized in three groups:

1. the hammer,
2. the pile, and
3. the soil.

Idealization of Hammers

The program is formulated to handle drop hammers, single, double, and differential acting steam hammers and diesel hammers that operate on the head of the

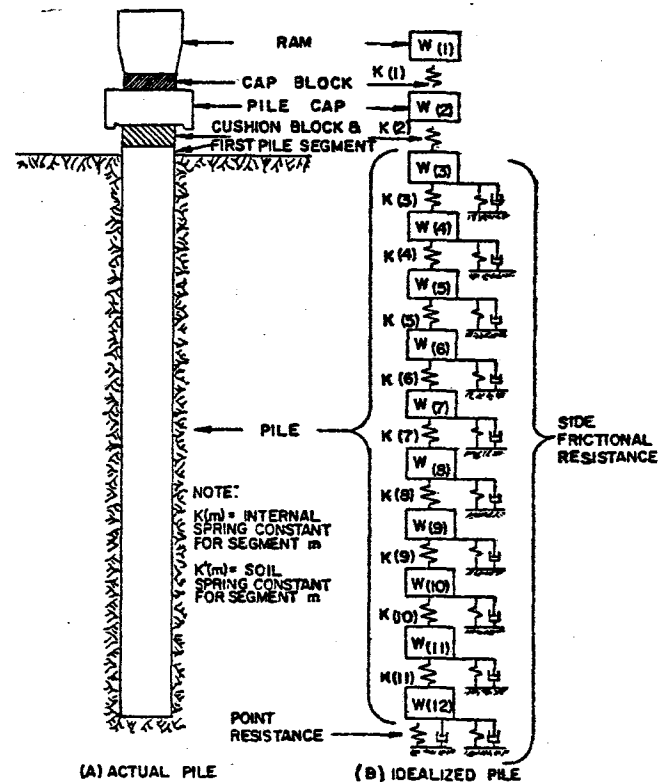
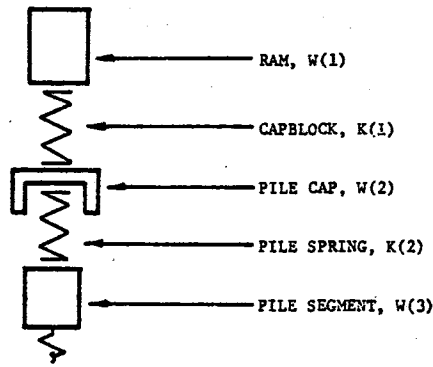


Figure 1. Idealization of a pile for purpose of analysis.



Calculations for idealization

W(1) = Weight of ram, (lb.)

$K(1) = \frac{A(1) E(1)}{L(1)}$, stiffness of the capblock, (lb./in.)

where A(1) = cross sectional area of the capblock, (in.²)

E(1) = modulus of elasticity of the capblock, (psi)

L(1) = thickness of the capblock, (in.)

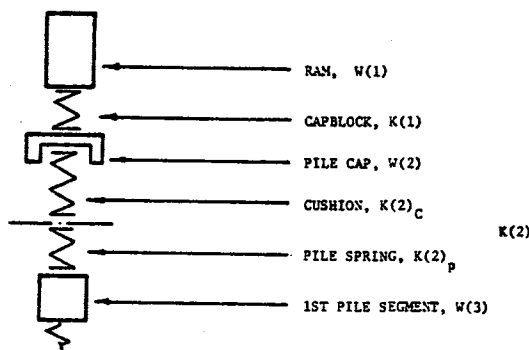
Note: See reference 2 for capblock properties.

Figure 2. Case I—Ram, capblock, and pile cap.

pile. The techniques presented in this section are general in scope and are presented for illustration. Appendix A gives the idealizations and pertinent information for the most common hammers.

Figures 2 through 4 describe the idealization for the following cases:

1. Case I—Ram, capblock, pile cap, and pile (Figure 1),



Calculations for idealization

W(1) = Weight of ram, (lb.)

K(1) = Stiffness of the capblock, (lb./in.)

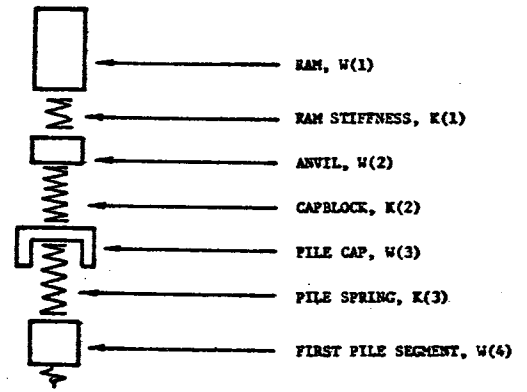
$K(2)_c$ = Stiffness of cushion, (lb./in.)

$K(2)_p$ = Stiffness of pile spring, (lb./in.)

$K(2) = \frac{K(2)_c K(2)_p}{K(2)_c + K(2)_p}$, combined stiffness of $K(2)_c$ and $K(2)_p$ in series.

Note: See reference 2, for capblock and cushion properties.

Figure 3. Case II—Ram, capblock, cushion, and pile spring.



Calculations for idealization

W(1) = Weight of ram, (lb.)

$K(1) = \frac{A(1) E(1)}{L(1)}$, the stiffness of the ram, (lb./in.)

where A(1) = ram cross sectional area, (in.)

E(1) = modulus of elasticity of ram material, (psi)

L(1) = length of ram, (in.)

This calculation assumes that the pile cap and anvil are rigid.

Figure 4. Case III—Ram, anvil, capblock, and pile cap.

2. Case II—Ram, capblock, pile cap, cushion and pile, (Figure 2), and

3. Case III—Ram, anvil, capblock, pile cap, and pile (Figure 3).

Methods of including coefficient of restitution in capblock and cushion springs. In the cases when K(1) is a capblock (Cases I and II), and K(2) is a cushion (Case II), it is desirable to include the energy loss due the coefficient of restitution of the particular material.

$$e = \sqrt{\frac{\text{Area BCD}}{\text{Area ABC}}} = \sqrt{\frac{\text{Energy output}}{\text{Energy input}}} \quad (1)$$

In Case II it is necessary to combine springs $K(2)_c$ and $K(2)_p$ to determine the equivalent spring $K(2)$. In this instance it is also necessary to determine the coefficient of restitution of the combined springs. The stiffness of the spring in the restitution phase is the slope of the line DB in Figure 5.

$$K_{DB} = \frac{F_B}{\Delta_C - \Delta_D} \quad (2)$$

Since,

$$\text{Energy output} = \text{Area BCD} = F_B (\Delta_C - \Delta_D) / 2$$

$$\text{Energy input} = \text{Area ABC} = F_B (\Delta_C) / 2$$

$$e^2 = \frac{F_B (\Delta_C - \Delta_D)}{F_B (\Delta_C)} = \frac{\frac{F_B}{\Delta_C}}{\frac{F_B}{(\Delta_C - \Delta_D)}} = \frac{K_{AB}}{K_{DB}}$$

$$\text{or } K_{DB} = \frac{K_{AB}}{e^2} \quad (3)$$

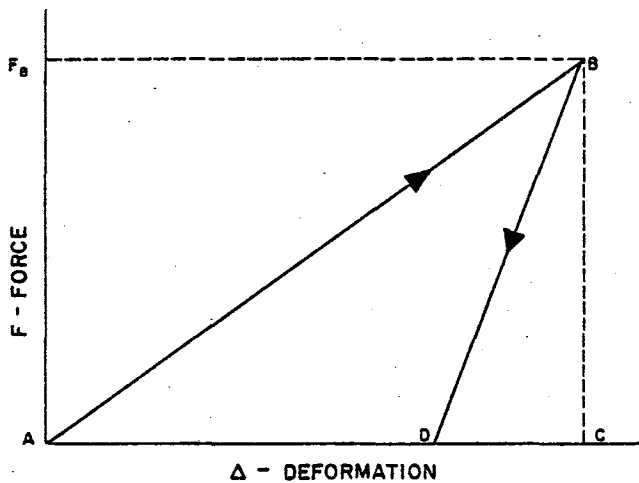


Figure 5. Definition of coefficient of restitution.

The combined restitution stiffness of $K(2)_c$ and $K(2)_p$ can be determined from,

$$\frac{1}{K(2)} = \frac{1}{K(2)_c} + \frac{1}{K(2)_p} \quad (\text{for restitution phase DB in Figure 5})$$

from Eq. 3,
$$\frac{e(2)^2}{K(2)} = \frac{e(2)_c^2}{K(2)_c} + \frac{e(2)_p^2}{K(2)_p}$$

$$e(2)^2 = \frac{K(2)}{K(2)_c K(2)_p} [e(2)_c^2 K(2)_p + e(2)_p^2 K(2)_c]$$

since
$$K(2) = \frac{K(2)_c K(2)_p}{K(2)_c + K(2)_p}$$

$$e(2) = \quad (4)$$

$$\sqrt{\frac{1}{K(2)_c + K(2)_p} [e(2)_c^2 K(2)_p + e(2)_p^2 K(2)_c]}$$

Ram kinetic energies. The kinetic energy of the ram for specific hammer types can be calculated as follows:

1. Drop hammers and single acting steam hammers;

$$E_H = W(1) \cdot h \cdot e_r \quad (5)$$

where

- E_H = ram kinetic energy, (ft.-lb.)
- $W(1)$ = ram weight, (lb.)
- h = ram stroke, (ft.)
- e_r = hammer mechanical efficiency (usually between 0.75 and 0.85 for most single acting hammers)

2. Differential and double-acting steam hammers;

$$E_H = h \left[1 + \frac{p \text{ actual}}{p \text{ rated}} \cdot \frac{W(h)}{W(1)} \right] W(1) \quad (6)$$

where

- h = actual ram stroke, (ft.)
- $p \text{ actual}$ = actual steam pressure, (psi)

$p \text{ rated}$ = manufacturers rated steam pressure, (psi)

$W(h)$ = hammer housing weight, (lb.)

$W(1)$ = ram weight, (lb.)

3. Diesel hammers;

$$E_H = W(1) (h_e - C) \cdot e_r \quad (7)$$

where

h_e = actual ram stroke for open-end hammers, and the effective stroke (includes effect of bounce chamber pressure) for closed-end hammers, (ft.)

C = distance from bottom-dead-center of ram to exhaust ports, (ft.)

Work done on the pile by the diesel explosive force is automatically accounted for by using an explosive pressure (see Sample Problem and Table A2).

In the hammer idealization, note that the parts composing the pile driver are physically separated, i.e., the ram is capable of transmitting compressive force to the anvil but not tension. The same is true of the interface between the anvil and pile cap, and the pile cap and the head of the pile. The program contains provisions for eliminating the capability of transmitting tensile forces between adjacent segments. The mechanics of this provision are more fully explained in the following section.

Idealization for Piles

The idealization of the pile is handled by breaking the continuum of the pile into discrete segments. Each segment is represented by its weight and spring representing the total segment stiffness. In Figure 6 the weight representing the segment is assumed to be concentrated at the end of the segment away from the impact. This places the spring on top of the weight whose stiffness it represents, i.e., $K(2)$ is associated with $W(3)$.

Piles should be broken into segments not to exceed approximately 10 feet in lengths, but into not less than five segments. The stiffness of each pile segment spring is calculated from

$$K(m-1) = \frac{A(m) E(m)}{L(m)} \quad (8)$$

where

- $K(m-1)$ = spring stiffness for segment m , (lb.-in.)
- $A(m)$ = cross sectional area of segment m , (in.²)
- $E(m)$ = modulus of elasticity of the material of segment m , (psi)
- $L(m)$ = length of segment m , (in.)

The weight of each pile segment is calculated by

$$W(m) = A(m) L(m) \alpha$$

where

α = unit weight of pile material, (lb./in.)

If the pile is tapered the average value of $A(m)$ should be used.

The program has provisions for handling cases where the physical construction of the pile prohibits the

transmission of tensile stresses or is capable of transmitting tensile stresses only after a specified movement of a mechanical joint (joint slack or looseness). These conditions occur with certain types of pile splices. The program provides for this eventuality by entering the following:

1. If a joint (a joint is defined as the interface between two segments) can transmit tension the slack or looseness is entered as SLACK (m) = 0. (Refer to Figure 6.)

2. If a joint is completely loose, no tension can be transmitted and SLACK (m) should be made a very large number, i.e., SLACK (m) = 1000.0.

3. If a joint is capable of moving 1.25 in. before transmitting tension, SLACK (m) = 1.25, i.e., the physical value of the slack or looseness in a joint is entered in inches.

The SLACK (m) values are always associated with spring K(m). In Figure 6 if tension can be transmitted

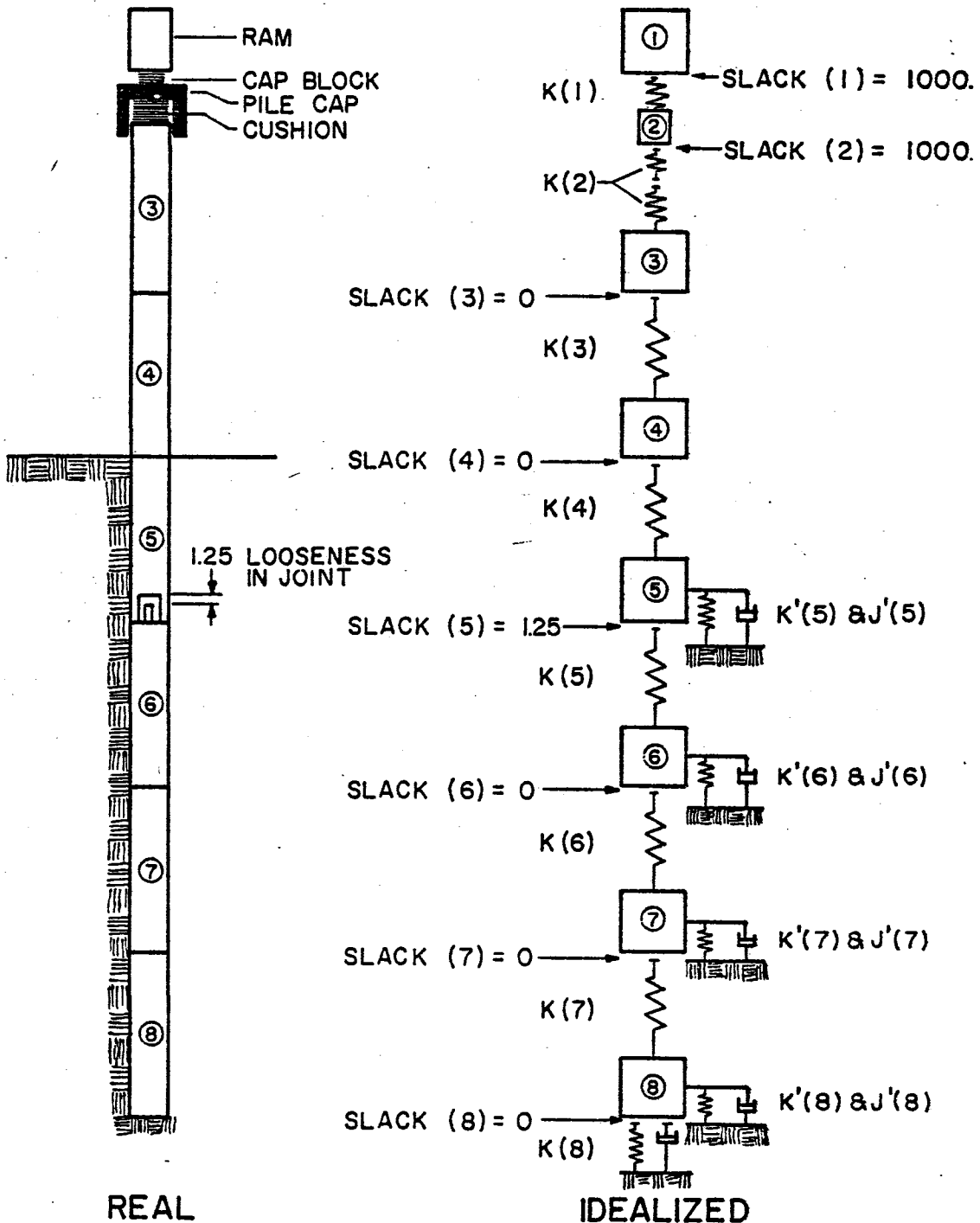


Figure 6. Pile idealization.

across the interface between segments 3 and 4 the slack value would be associated with spring $K(3)$, i.e., $SLACK(3) = 0$.

The interfaces between the various parts composing the pile driver (ram, capblock, pile cap, etc.) which cannot transmit tension are also handled by setting the $SLACK$ values equal to 1000.

Idealization for Soils

The true soil resistance to dynamic loading is not clearly understood. Simplifying assumptions are used in the program. Figure 7(a) shows the assumed static load deformation characteristic for the soil along the side of the pile or at the tip. For the soil on the side of the pile, path OABCDEFG represents the load-deformation that occurs as the pile moves in the soil. For the soil at the point only compressive loading can occur, since the point of the pile is free to rebound, and the load-deformation path is OABCO.

It can be seen that the characteristics of the spring representing the soil stiffness are defined by the quantities Q and RU . Q is the quake or maximum elastic deformation which occurs at the maximum elastic force RU . A load deformation diagram like that of Figure 7(a) can be established for each soil spring. The stiffness of a side soil spring is

$$K'(m) = \frac{RU(m)}{Q'(m)} \quad (9)$$

where

$RU(m)$ = side soil resistance on segment m , (lb.)

$Q'(m)$ = side soil quake, (in.)

For the soil at the point

$$K(p) = \frac{RU(p)}{Q(p)} \quad (10)$$

where

$RU(p)$ = soil resistance at the point (lb.)

$Q(p)$ = point soil quake, (in.)

The dynamic loading effects on the soil characteristics are included by assuming that the soil has a damper (dashpot) in parallel with the spring (see Figure 1). The resistance of the damper is assumed to be directly proportional to the velocity of the associated segment weight during the displacement. The total resistance of the spring and damper under dynamic load is expressed as³

$$R(m) = [D(m) - D'(m)] K'(m) [1 + J'V(m)], \text{ in the elastic range}$$

$$R(m) = RU(m) [1 + J'V(m)], \text{ in the plastic range}$$

where

$R(m)$ = total resistance (static plus dynamics)

$D(m)$ = displacement of $W(m)$

$D'(m)$ = plastic displacement of soil

$K'(m)$ = spring stiffness of side soil

J' = damping constant of side soil spring

$V(m)$ = velocity of $W(m)$

This equation will produce a dynamic load-deformation behavior as shown by path OABCDEFG in Figure 7(b) for the side soil and the path OABCO for the point soil. Note in Figure 7(b) that the dynamic and static parts of $R(m)$ are easily separated. This point is important to the development that follows.

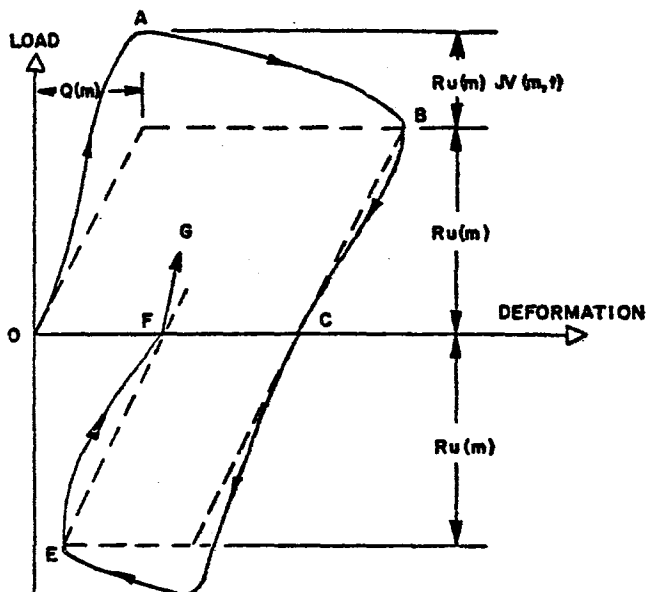
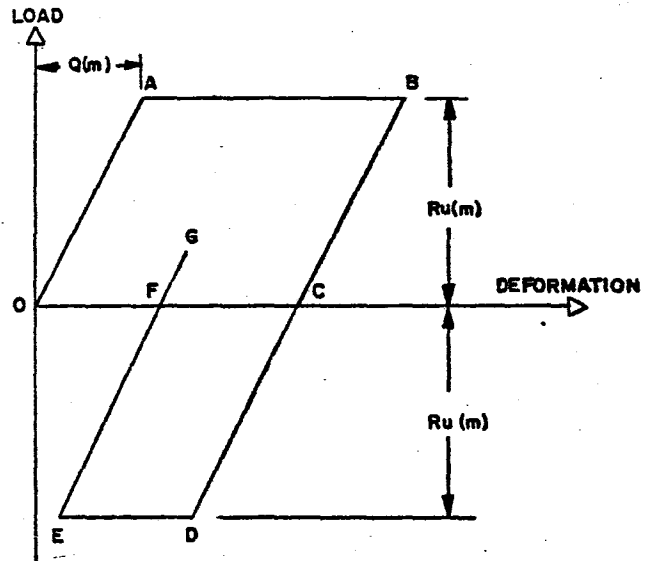


Figure 7. Soil load-deformation characteristics.

Use of the Program to Develop Dynamic-Static Soil Resistance Data

The engineer is most interested in the static load carrying capacity of the piles he drives. In the past he has had to rely on judgment based on empirical pile equations (Hiley, ENR, etc.) or static load tests. With the wave equation method of analysis of pile driving, a much more realistic engineering estimate can be made using data generated by the program. If it is assumed that the method of approximating the dynamic soil resistance is reasonably correct (this fact has yet to be proved), the static resistance of a pile, at the time of driving, can be determined from the dynamic driving behavior. This is accomplished in the following manner.

For any specified total static soil resistance and soil parameters, $Q(m)$, $J'(m)$, $Q(p)$, and $J(p)$, the penetration of the pile under one blow of the hammer can be determined. The penetration per blow (or its reciprocal, blows/in.) is a measurable parameter which indicates the dynamic behavior of the pile. By specifying various total static soil resistances a data table of total static soil resistance, $RU(TOTAL)$ and the corresponding penetrations under one blow of the hammer (expressed as blows/in.) can be developed for any constant set of pile and soil parameters. The graph of these values can be used as a field guide to estimate the total static soil resistance, at the time of driving, for any observed pile penetration (blows/in.). It should be recognized that any pile "set up" due to consolidation in cohesive soils is not included. The static values obtained should be viewed as a practical minimum of the static soil capacity that was achieved.

Explanation of Data Input Sheets

Data for the Pile Driving Analysis program is entered on two sheets. Page 1 contains data pertaining to the physical parameters of a particular pile. Page 2 is used to vary the soil, pile driver, or cushion characteristics for the pile described on page 1. Examples of the data sheets follow the explanation.

Page 1

- Case No. = Any combination of up to six alphabetic or numerical characters used for identifying information. These characters will identify all problems associated with the pile data entered on sheets 1 and 2.
- No. of Probs. = Total number of problems listed on page 2.
- 1/DELTA T = This space may be left blank in most cases as the program calculates the critical time interval from the parameters of the system. The value calculated is
- $$1/DELTA T = 2(19.698 \sqrt{K/W})$$
- If, however, one desires to use a specific 1/DELTA T, it may be entered. The program will then compare the entered value with the critical value calculated by the above formula and use the larger of the two. This is done so that the user cannot inadvertently enter a value too small and hence introduce instability into the numerical process.
- P = Total number of weights including ram of hammer, follower and helmet, etc.

- SLACK (1) = This indicates a specified looseness between $W(1)$ and $W(2)$ in inches. This is the amount of movement required before $K(1)$ will take tension. If there is complete tensile freedom of $K(1)$ then enter SLACK (1) = 1000. Leave blank if option 3 is "2".
- SLACK (2),
SLACK (3)
Option 1 = see notes on Slack (1).
= This is an option for the manual entry of the cross sectional area of each segment.
(a) Enter "1" and all AREAS will automatically be set equal to 1.00. In this case, draw a horizontal line through all AREA rows on the middle portion of page 1. If "1" is used, do not enter areas in AREA rows.
(b) Enter "2" if the cross sectional area of each segment is to be entered manually in the AREA rows. In this case enter AREAS (1) to (P) inclusive.
- Option 2 = This is an option for the manual entry of soil resistances.
(a) Enter "2" if the soil resistances (expressed as a percentage of the total soil resistance) are to be entered manually in the RU rows. The RU values are entered from (1) to (P + 1) inclusive. Note that (P + 1) is the point resistance and all others are side resistances. The total of all RU percentages entered must total 100%.
(b) Enter "1" if the soil resistances are not listed in the RU rows but are indicated under Option 12 on page 2.
- Option 3 = This is an option for manual entry of the SLACK values.
(a) Enter "1" if SLACK values from SLACK (4) to SLACK (P - 1) are all 0.00 (indicating $K(4)$ to $K(P - 1)$ can take tension). In this case only SLACK (1) to SLACK (3) are entered in row 1. Draw a horizontal line through all SLACK rows in the lower portion of page 1. In this case do not enter any values in the Slack rows.
(b) Enter "2" if SLACK values are to be entered manually. In this case, SLACK (1) to SLACK (3) in row 1 may be left blank.
- Option 4 = This is an option on the routine used to simulate the material behavior of springs $K(1)$, $K(2)$, and $K(3)$.
(a) Enter "1" for use of Smith's routine 3 and 4.
(b) Enter "2" for use of Texas A&M's routine. It is suggested that Option 4 = 2. Option 4 may be left blank in which case it is automatically set equal to 2.
- IPRINT = This is an option on the amount of data printed out when the long form output is used (Option 15 = 2). If Option 15 = 2, IPRINT is the print interval expressed as the number of time intervals. As an example, if a print out is required every 10th time interval, 10 would be entered for IPRINT. If Option 15 is "1" or "3" leave IPRINT blank.
- NSEG 1 = NSEG 1 is the mass number of the first pile segment. If NSEG 1 is left blank NSEG 1 = 2 will be used by the program.

The total weight of each segment, in pounds, is entered in the rows marked $W(2)$, $W(3)$,, $W(24)$. The weights, W 's, are entered from 2 to P inclusive. Note that $W(1)$ is not entered as it will be included on page 2.

The spring stiffness of each segment, in lb./in., is entered in the rows marked K(1), K(2),, K(24). The stiffness, K's, are entered from 1 to P - 1 inclusive. Spring K(P) is the soil spring at the pile tip and is calculated by the program from the soil data entered on page 2.

If Option 1 = 2, the average area of each segment must be entered in the rows marked A(1), A(2),, A(24). The units of A should be consistent with the stress units desired in the output. The basic force unit of the output is the pound. The areas, A's, are entered from 1 to P inclusive. A(P - 1) and A(P) in most instances will be the same. Areas of segments of the hammer are usually entered as A(1) = 1.00, etc., since stress values obtained for these segments are not usually of concern. If Option 1 = 1 the area row should be marked through with a solid horizontal line indicating no data cards are to be included.

If Option 2 = 2, the side soil resistance on each segment, expressed as a percentage of the total soil resistance, is entered in the rows marked RU(1), RU(2),, RU(24). The soil resistances, RU's, are entered from 1 to P + 1 inclusive. The value of RU (P + 1) is the pile tip resistance. Mark out all rows when Option 2 = 1.

If Option 3 = 2 the physical slack or looseness, expressed in inches, is entered in each row marked SLACK (1), SLACK (2),, SLACK (24). SLACK'S are entered from 1 to P - 1 inclusive. If there is no slack, enter 0.0, if there is complete looseness, enter 1000.0. SLACK (P) is automatically set equal to 1000.0 since the point soil spring cannot take tension. If Option 3 = 1, mark out all rows.

Note that the forms have 24 spaces for W's, K's, A's, RU's and SLACK's. The program is capable of handling a pile with a maximum of 149 segments. Additional cards may be added to each parameter as needed.

Page 2

- W(1) = The weight of the pile driver's ram in pounds.
- NC = The number of the spring for which K(NC) is being varied, see discussion on page 8.
- K(NC) = The spring constant of the spring being varied in pounds/inch. Only one spring can take on variable values per case.
- EFF = The efficiency of the pile hammer.
- ENERGY = The kinetic energy of the falling ram calculated by Equation 5, 6 or 7.
- ERES (1) = The coefficient of restitution of spring K(1)
- ERES (2) = The coefficient of restitution of spring K(2)
- ERES (3) = The coefficient of restitution of spring K(3).
- RU (TOTAL) = This space should be used only when Option 11 = 2. In this case RU(TOTAL) is the desired ultimate pile resistance in pounds. When Option 11 = 1 leave this entry blank.
- % RU (TOTAL) AT POINT = The percentage of the total pile soil resistance, RU(TOTAL), under the point of the pile. This value is entered as a percentage.

- MO = If Option 12 is "1" or "2" enter the number of the first pile segment acted upon by soil resistance. This space may be left blank if Option 12 = 3, i.e., RU's are read in on page 1.
- Q POINT = Quake of the soil at the point. Normally "0.10" is used.
- Q SIDE = Quake of the soil on the side of the pile. Normally "0.10" is used.
- J POINT = Damping constant for the soil at the point.
- J SIDE = Damping constant for the soil on the side of the pile.
- FEXP = The diesel explosive force (in pounds) which acts on the ram and anvil of a diesel hammer. In the case where no explosive force exists, as with drop hammers or steam hammers, leave FEXP blank.
- Option 11 = This option provides for single or multiple calculations.
 - (a) Enter "1" if multiple calculations for RU (TOTAL) VS. BLOW/IN., data are desired. The computer will assign suitable values of RU(TOTAL). Leave RU(TOTAL) space on page 2 blank.
 - (b) Enter "2" if single calculation is to be made with RU(TOTAL) value entered on page 2.
- Option 12 = This option is used for designation of the distribution of side friction on the pile.
 - (a) Enter "1" for a uniform distribution of side friction from segment MO to P.
 - (b) Enter "2" for a triangular distribution of side friction from segment MO to P.
 - (c) Enter "3" if Option 2 = 2, i.e., RU values are entered on page 1.
- Option 13 = This option provides for computer plotted curves using the data generated for RU (TOTAL) VS. BLOW/IN. (Option 11 = 1).
 - (a) Enter "1" for computer plot of data. If no plot is desired, leave blank.
- Option 14 = This is used to include or exclude gravity in the calculations.
 - (a) Enter "1" if the forces of gravity are to be included in the calculations.
 - (b) Enter "2" if the forces of gravity are to be excluded from the calculation. This alternate in effect excludes the weight of the pile from the calculations. It is used when the pile driver is in a horizontal position or for an extreme batter.
- Option 15 = This option provides for versatility in the output format.
 - (a) Enter "1" for a normal data printout.
 - (b) Enter "2" for extra detail in printout. This alternate gives pertinent stresses, deformations, velocities, etc., at the print interval, specified as IPRINT on page 1.
 - (c) Enter "3" for short output. This alternate gives only a tabular summary of BLOWS/IN. VS. RU(TOTAL). Option 15 = 3 should be used only when Option 11 = 2.

SPECIAL NOTE Where anything listed for Problem 1 is to be repeated for Problem 2, 3, etc., draw an arrow down through the last problem to indicate repetition.

Comments on data input. On page 2 of the input forms, provisions are made for varying the stiffness of any spring, K(1) through K(P - 1), in the hammer or pile idealization. This is accomplished by entering the number of the spring to be changed in the NC column and then the stiffness of spring K(NC) in the K(NC) column. As soon as this problem is completed, the spring stiffnesses, K(NC) will be reset automatically to the value listed on page 1 of the input form.

The various output options, Option 15, are discussed in Appendix B in conjunction with example problems.

The program is capable of handling pile idealizations with a maximum of 149 segments. There is no limit on the number of problems that can be run for each case.

Sample Problems and Program Listing

A sample problem showing the use of the input forms and the output forms is presented in Appendix B.

The program listed is presented in Appendix C. It is coded in IBM FORTRAN IV. A flow diagram of the program logic is also presented. An attempt has been made to use variable names which correspond to the nomenclature presented by Smith.⁵

PILE DRIVING ANALYSIS										OPTIONS				BY: _____	DATE: _____	PAGE # 1 OF 2
TEXAS A&M UNIVERSITY										AREAS	RU'S	SLACK	IF PRINT USED WHEN OPTION 15 = 2	NEEQ		
CASE NO.	NO. OF PROBS	1/Delta T	P	SLACK (1)	SLACK (2)	SLACK (3)	1	2	3	4	5	6	7	8	9	
W (1)		W (2)		W (3)		W (4)		W (5)		W (6)		W (7)		W (8)		
W (9)		W (10)		W (11)		W (12)		W (13)		W (14)		W (15)		W (16)		
W (17)		W (18)		W (19)		W (20)		W (21)		W (22)		W (23)		W (24)		
K (1)		K (2)		K (3)		K (4)		K (5)		K (6)		K (7)		K (8)		
K (9)		K (10)		K (11)		K (12)		K (13)		K (14)		K (15)		K (16)		
K (17)		K (18)		K (19)		K (20)		K (21)		K (22)		K (23)		K (24)		
AREA (1)		AREA (2)		AREA (3)		AREA (4)		AREA (5)		AREA (6)		AREA (7)		AREA (8)		
AREA (9)		AREA (10)		AREA (11)		AREA (12)		AREA (13)		AREA (14)		AREA (15)		AREA (16)		
AREA (17)		AREA (18)		AREA (19)		AREA (20)		AREA (21)		AREA (22)		AREA (23)		AREA (24)		
RU (1) %		RU (2) %		RU (3) %		RU (4) %		RU (5) %		RU (6) %		RU (7) %		RU (8) %		
RU (9) %		RU (10) %		RU (11) %		RU (12) %		RU (13) %		RU (14) %		RU (15) %		RU (16) %		
RU (17) %		RU (18) %		RU (19) %		RU (20) %		RU (21) %		RU (22) %		RU (23) %		RU (24) %		
SLACK (1)		SLACK (2)		SLACK (3)		SLACK (4)		SLACK (5)		SLACK (6)		SLACK (7)		SLACK (8)		
SLACK (9)		SLACK (10)		SLACK (11)		SLACK (12)		SLACK (13)		SLACK (14)		SLACK (15)		SLACK (16)		
SLACK (17)		SLACK (18)		SLACK (19)		SLACK (20)		SLACK (21)		SLACK (22)		SLACK (23)		SLACK (24)		

NOTES: ONE OR MORE PROBLEMS MUST BE LISTED ON PAGE 2
 W'S AND AREAS 1 TO P INCL.; K'S AND SLACK'S 1 TO P-1 INCL.; RU'S 1 TO P+1 INCL. (P+1 IS % RU UNDER POINT OF PILE.)

		PILE DRIVING ANALYSIS TEXAS A&M UNIVERSITY										BY: _____	DATE: _____	PAGE # 2 OF 2						
BO R P	W(I) POUNDS	N C	K(NC) POUNDS/INCH	EFF.	ENERGY	ERES	ERES	ERES	RU (TOTAL) POUNDS	% AT POINT	MO	Q POINT	Q SIDE	J POINT	J SIDE	FEXP	OPTIONS			
						(1)	(2)	(3)									11	12	13	14
1																				
2																				
3																				
4																				
5																				
6																				
7																				
8																				
9																				
10																				
11																				
12																				
13																				
14																				
15																				
16																				
17																				
18																				
19																				
20																				

NOTE: IF OPTION #11 = 1, RU (TOTAL) NOT REQUIRED

NO. OF CALCULATIONS
RESISTANCE
PLOT
QUALITY
PRINT OUT

References

1. Smith, E. A. L., "Pile Driving Analysis by the Wave Equation," Journal of the Soil Mechanics and Foundations Division, *Proceedings of the American Society of Civil Engineers*, Proceedings Paper 2574, SM 4, August, 1960, pp. 35-61.
2. Hirsch, T. J. and T. C. Edwards, "Impact Load-Deformation Properties of Pile Cushioning Materials," Research Report 33-4, Project 2-5-62-33, Piling Behavior, Texas Transportation Institute, Texas A&M University, College Station, Texas, May, 1966, 12 pgs.
3. Smith, *op. cit.*, p. 47.
4. Samson, C. H., Hirsch, T. J. and L. L. Lowery, "Computer Study of Dynamic Behavior of Piling," Journal of the Structural Division, *Proceedings of the American Society of Civil Engineers*, Proceedings Paper 3608, ST 4, August, 1963, p. 419.
5. Smith, *op. cit.*, p. 44.

Appendix A

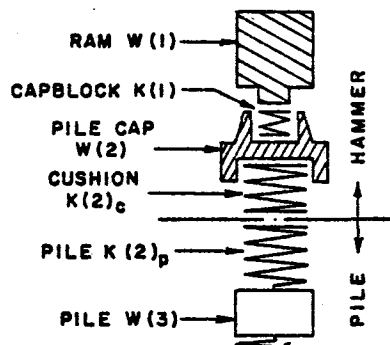
PILE DRIVING HAMMERS

Table A1 and A2 list the information needed for the simulation of the most common type of pile driving hammers.

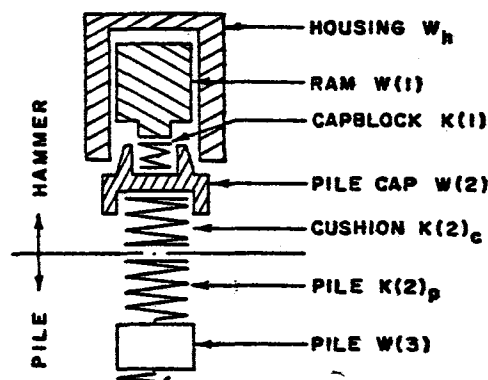
Equations (5) through (7), in the text, may be used to calculate the kinetic energy for a specific ram stroke. Capblock and cushion stiffness can be calculated using

the equation in Figures 2 through 4 of the text. When it is necessary to calculate the coefficient of restitution of combined spring stiffnesses, Equation (4) of the text should be used.

The mechanical properties of selected cushion and capblock materials can be found in Reference 2 of the text.



**DROP HAMMERS
SINGLE ACTING STEAM HAMMERS
(A)**

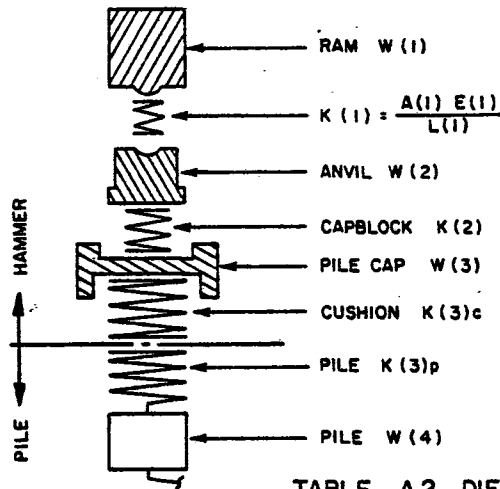


**DOUBLE AND DIFFERENTIAL ACTING
STEAM HAMMERS
(B)**

TABLE A 1 DROP HAMMERS AND STEAM HAMMERS

HAMMER	TYPE	W (1) (LB.)	W (2)* (LB.)	W (h) (LB.)	K (1) (LB./IN.)	K (2)c (LB./IN.)	K (2)p (LB./IN.)	STROKE h, (FT.)	Prated (PSI)	EFF. e _f
MKT S3	A	3000	—	—	DEPENDS ON MATERIAL PROPERTIES & DIMENSIONS	DEPENDS ON MATERIAL PROPERTIES & DIMENSIONS	$K(2)_p = \frac{A(2)E(2)}{L(2)}$	3.00	—	0.80
MKT S5	A	5000	—	—				3.25	—	0.80
VULCAN 1	A	5000	1000	—				3.00	—	0.80
VULCAN 2	A	3000	1000	—				2.42	—	0.80
VULCAN 30C	B	3000	1000	4036				1.04	120	0.85
VULCAN 50C	B	5000	1000	6800				1.29	120	0.85
VULCAN 80C	B	8000	2000	9885				1.38	120	0.85
VULCAN 140C	B	14000	—	13984	1.29	140	0.85			

* REPRESENTATIVE VALUES FOR PILE NORMALLY USED IN HIGHWAY CONSTRUCTION



NOTES FOR TABLE A2

* for actual stroke use field observations (may vary from 4.0 to 8.0 ft.)

** determine from bounce chamber pressure ($h_b = E/W(1)$ where E=indicated Energy)

† average values

TABLE A2 DIESEL HAMMERS

TYPE HAMMER	W(1) (LB.)	W(2) (LB.)	W(3) [†] (LB.)	K(1) x 10 ⁶ (LB./IN.)	K(2) x 10 ⁶ (LB./IN.)	K(3) _c	K(3) _p	MAX h _e (FT.)	C (FT.)	E _n (FT.-LB.)	EXPLOSIVE FORCE (LB.)	e _f
MKT DE-20	2000	640	1300 690 1000		14.2	VARIES	$K(3) = \frac{A(3)E(3)}{L(3)}$	8.00*	0.92	E _n = W(1)(h _b - C) DEPENDS ON RAM STROKE h _b	46300	1.00
MKT DE-30	2800	775		38.7	63.8			8.00*	1.04		98000	1.00
MKT DE-40	4000	1350			101.0			8.00*	1.16		138000	1.00
DELMAG D 5	1100	330	All sq. concrete All wood All H-bearing All pipe	18.5	13.6			8.00*	0.83		46300	1.00
DELMAG D 12	2750	816		31.5	18.6			8.00*	1.08		93700	1.00
DELMAG D 22	4850	1576		49.7	23.8			8.00*	1.08		158700	1.00
DELMAG D 44	9500	4081		106.2	56.5			8.00*	1.19		200000	1.00
LINK-BELT 180	1724	377		44.5	15.5			4.63**	0.64		81000	1.00
LINK-BELT 312	3857	1188		142.5	18.6			3.87**	0.50		98000	1.00
LINK-BELT 440	4000	705		138.0	18.6			4.55**	1.25		98000	1.00
LINK-BELT 520	5070	1179		108.5	18.6			5.20**	0.83		98000	1.00

Appendix B

SAMPLE PROBLEM

Consider the pile shown in Figure B-1.

Pile: 16 in. square prestressed concrete pile, 26 ft. in length. The modulus of the concrete is 7.82×10^6 psi and its unit weight is 154 lb./ft.³. The pile is assumed to be embedded for its full length.

Pile hammer: Hypothetical diesel hammer with 4850 lb. ram with an input ram kinetic energy of 39,800 ft. lb. The explosive force produced by the diesel fuel is 158,700 lb. The stiffness of the ram is given as 42.25×10^6 lb./in. The anvil is assumed rigid and weighs 1150 lb. The capblock stiffness is 24.5×10^6 lb./in.

In order to illustrate the utilization of the input data sheets and explain the output data sheets four problems are considered.

Problem 1 and Problem 2 are concerned with the driving effects produced by two different cushions. The object of these two cases is to determine the dynamic-static resistance curves (RU(TOTAL) VS. BLOWS/IN.) for one blow of the hammer. In Problem 1, the cushion is assumed to have a cross sectional area equal to that of the pile, is $6\frac{1}{4}$ in. thick and has a modulus of elasticity of 1.0×10^6 psi. In Problem 2 the cushion area and properties are the same as in Problem 1 but the thickness is $3\frac{3}{8}$ in. In Problems 1 and 2 the soil side friction is assumed to have a triangular distribution with 10% point resistance. The soil constants are:

- (a) $Q = Q' = 0.10$ in.
- (b) $J = 0.15$ sec./ft.
- (c) $J' = 0.05$ sec./ft.

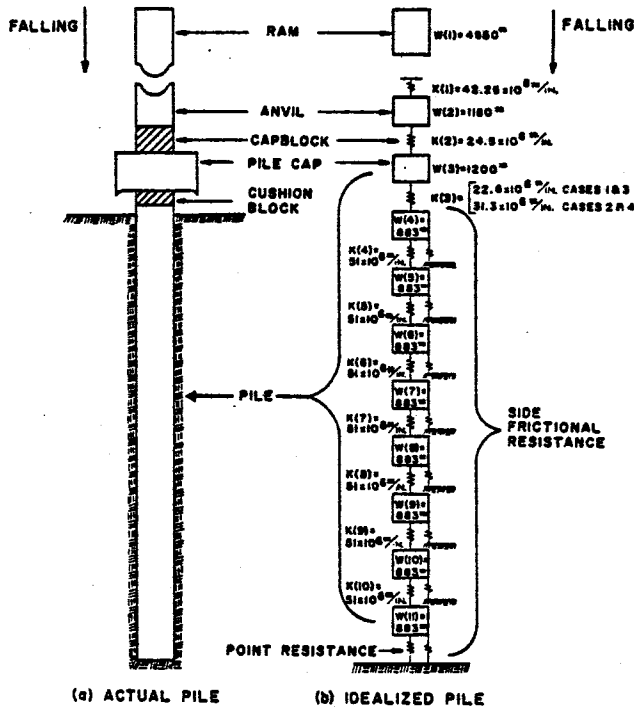


Figure B-1. Sample problem.

Problem 3 and 4 illustrate the use of program to investigate the penetration of a pile to 200 tons of static soil resistance produced by one blow of the hammer. In Problem 3 the soil resistance is distributed uniformly along the side with 10% at the point. The cushion is the same as in Case 1. In Problem 4 the soil has a triangular distribution along the side with 10% soil resistance (same as Problem 2). The cushion is the same as in Problem 2. Problem 4 will also illustrate the use of the output option (OPTION 15).

The following calculations illustrate the computations for the hammer and pile idealization.

- (a) **Pile:** The pile is broken into eight equal length segments of 39 in. The spring stiffness for each segment is,

$$K(3)_p = \frac{A(3)_p E(3)_p}{L(3)_p}$$

where

$$A(3)_p = 254 \text{ in.}^2$$

$$E(3)_p = 7.32 \times 10^6 \text{ psi}$$

$$L(3)_p = 39 \text{ in.}$$

therefore

$$K(3)_p = \frac{(254)(7.32 \times 10^6)}{39} = 51.0 \times 10^6 \text{ lb./in.}$$

- (b) **Cushion:** Spring $K(3)$ in Figure B1 (b) represents the combined stiffness of the cushion and first pile segment.

In Problem 1 and 3

$$K(3)_c = \frac{A(3)_c E(3)_c}{L(3)_c}$$

where

$$A(3)_c = 254 \text{ in.}^2$$

$$E(3)_c = 1.00 \times 10^6 \text{ psi}$$

$$L(3)_c = 6.25 \text{ in.}$$

then

$$K(3)_c = \frac{(254)(1 \times 10^6)}{6.25} = 40.5 \times 10^6 \text{ lb./in.}$$

The combined stiffness of $K(3)_c$ and $K(3)_p$ is

$$K(3) = \frac{K(3)_c \times K(3)_p}{K(3)_c + K(3)_p} = \frac{(40.5)(51.0)(10^6)}{(40.5 + 51.0)(10^6)}$$

$$K(3) = 22.6 \times 10^6 \text{ lb./in.}$$

The coefficient of restitution for the combined springs is assumed to be 0.50.

For Problem 2 and 4 similar calculation yields

$$K(3) = 31.3 \times 10^6 \text{ lb./in.}$$

The output data sheets are completed as follows:

Page 1 (Same for all 4 problems)

No. of Problems = 4, there are 4 problems to be solved on page 2.

1/DELTA T = 0.0, since the program will calculate the correct value.

P = 11, there are 11 weights (3 for the hammer and 8 for the pile).

SLACK'S = all set equal to 1000 since there is complete looseness between the ram, anvil, capblock, pile cap, cushion, and pile head.

OPTION 1 = 2, all areas are entered manually in AREA rows.

OPTION 2 = 1, since OPTION 12 is used to describe the soil distribution.

OPTION 3 = 1, all pile segments are connected, hence SLACK (4) to SLACK (10) = 0.0.

OPTION 4 = left blank since it is desired to use the A&M routine.

IPRINT = 10, in Problem 4, OPTION 15 = 2, it is desired to print output every 10 iterations.

NSEG1 = 4, the first pile segment, see Figure B1 (b).

W'S = enter the weight of each element in lb. Note that W(1) is blank since it will be entered on page 2.

K'S = enter all spring stiffnesses for the pile system considered to be basic, i.e., the program will automatically reset the stiffnesses to these values after each problem on page 2.

A'S = enter all cross sectional areas of pile segments only.

Page 2—Problem 1

W(1) = 4850 lb., the ram weight.

NC = 3, the cushion spring number, see Figure B1 (b).

K(NC) =

K(3) = 22,500,000, the stiffness of the combined springs.

EFF = 1.00, diesel hammers are considered to be 100% efficient.

ENERGY = 39,800, the input energy for this particular hammer blow.

ERES(1) = 0.60, coefficient of restitution of steel on steel impact.

ERES(2) = 0.80, coefficient of restitution of cap-block material.

ERES(3) = 0.50, coefficient of restitution of combined cushion and first pile spring.

RU(TOTAL) = leave blank, since OPTION 11 = 1, i.e., the program will generate suitable values for curve generation.

% AT POINT = 10%.

MO = 4, the first pile segment with side soil resistance.

QPOINT = 0.10, Q.

QSIDE = 0.10, Q'.

JPOINT = 0.15, J.

JSIDE = 0.05, J'.

FEXP = 158,700, lb. the diesel explosive force.

OPTION 11 = 1, for program generated RU(TOTAL) VS. BLOWS/IN. curve.

OPTION 12 = 2, for triangular side soil resistance distribution.

OPTION 13 = leave blank since computer plotted curve is not desired.

OPTION 14 = 1, to indicate gravity.

OPTION 15 = 1, for normal data output.

PILE DRIVING ANALYSIS										OPTIONS		BY: A. Aggie		DATE: 8/2/67		PAGE # 1	
TEXAS A & M UNIVERSITY										AREAS	BLKS	SLACK	PRINT USED WHEN OPTION=2	NSEG1	60	70	80
CASE NO.	NO. OF PROBS.	1/Delta T	P	SLACK (1)	SLACK (2)	SLACK (3)	1	2	3	4	5	6	7	8	9	10	
HPS 10	4	0.0		111000	1000	1000	2	1	1	1	1	1	1	1	1	1	
W (1)	W (2)	W (3)	W (4)	W (5)	W (6)	W (7)	W (8)	W (9)	W (10)	W (11)	W (12)	W (13)	W (14)	W (15)	W (16)	W (17)	
	1150	1200	883	883	883	883											
K (1)	K (2)	K (3)	K (4)	K (5)	K (6)	K (7)	K (8)	K (9)	K (10)	K (11)	K (12)	K (13)	K (14)	K (15)	K (16)	K (17)	
42200000	24500000	22600000	51000000	51000000	51000000	51000000											
AREA (1)	AREA (2)	AREA (3)	AREA (4)	AREA (5)	AREA (6)	AREA (7)	AREA (8)	AREA (9)	AREA (10)	AREA (11)	AREA (12)	AREA (13)	AREA (14)	AREA (15)	AREA (16)	AREA (17)	
10	10	254	254	254	254	254											
RU (1) %	RU (2) %	RU (3) %	RU (4) %	RU (5) %	RU (6) %	RU (7) %	RU (8) %	RU (9) %	RU (10) %	RU (11) %	RU (12) %	RU (13) %	RU (14) %	RU (15) %	RU (16) %	RU (17) %	
SLACK (1)	SLACK (2)	SLACK (3)	SLACK (4)	SLACK (5)	SLACK (6)	SLACK (7)	SLACK (8)	SLACK (9)	SLACK (10)	SLACK (11)	SLACK (12)	SLACK (13)	SLACK (14)	SLACK (15)	SLACK (16)	SLACK (17)	

NOTES: ONE OR MORE PROBLEMS MUST BE LISTED ON PAGE 2
 W's AND AREAS 1 TO P INCL.; K's AND SLACK's 1 TO P-1 INCL.; RU's 1 TO P+1 INCL. (P+1 IS % RU UNDER POINT OF PILE.)

Page 2, Problem 2

Only the value of K(3) is changed.

NC = 3.
 K(NC) =
 K(3) = 31,300,000.

Page 2, Problem 3

The value of K(3) and the OPTIONS are changed.

NC = 3.
 K(NC) =
 K(3) = 22,500,000.
 RU(TOTAL) = 400,000, lb. for a 200 ton total static soil resistance.
 OPTION 11 = 2, for single calculation using RU (TOTAL) = 400,000.
 OPTION 12 = 1, for uniform side soil resistance distribution.

Page 2, Problem 4

In this problem the cushion and the options are changed.

NC = 3.
 K(NC) =
 K(3) = 31,300,000.
 OPTION 12 = 2, for triangular side soil resistance distribution.
 OPTION 15 = 2, for output at interval expressed by IPRINT on page 1.

The output for the four sample problems are shown in Figures B2 through B6. Figure B2 is the output for one point on the RU(TOTAL) VS. BLOWS/INCH curve generated for Problem 1. The block of data on the

upper part of the figure is a printout of the input data. The RU(TOTAL) value of 1,040,962.1 is the total static soil resistance for which this problem was run. This value was generated by the program and is only one point of 10 used to develop the data for the total RU(TOTAL) VS. BLOWS/INCH curve shown in Figure B3. The second block of data shows the maximum compressive and tensile stresses and the maximum displacement of each segment. The column labeled TIME N is the time interval at which the maximum compressive stress (MAX C STRESS) occurred, i.e., the maximum compressive stress of 7432 psi occurred in segment 3 at time interval 11 (11/9443.9 sec.). Similar data is printed for each point on the RU(TOTAL) VS. BLOWS/INCH shown in Figure B3.

Figure B4 shows the summary of the data for the RU(TOTAL) VS. BLOWS/INCH for Problems 1 and 2. Data of this type can be used to construct curves like that shown in Figure B3. These curves can be used to compare the effects of cushion stiffness (the cushion stiffness, K(3)_c, in Problem 2 was twice that in Problem 1). Note the stiffer cushion (Problem 2) produces the most efficient driving since for a specified resistance the penetration per blow is larger (BLOWS/IN. is smaller).

Figure B5 is a typical output when RU(TOTAL) is specified. The maximum penetration of the point of the pile under one blow of the hammer is 0.473011 in., listed

FILE DRIVING ANALYSIS TEXAS A&M UNIVERSITY										BY: A. Aggie		DATE: 8/31/67		PAGE # 2 OF 2			
RECORD	W(I) POUNDS	N C	K(NC) POUNDS/INCH	EFF.	ENERGY	ERES (1)	ERES (2)	ERES (3)	RU (TOTAL) POUNDS	% AT POINT	MO	Q POINT	Q SIDE	J POINT	J SIDE	FEXP	OPTIONS 11 12 13 14 15
1	4850	3	22500000	100	39800	060	080	050		10		40100	100	15005		158706	12 11
2			31300000														12 11
3			22500000						400000								21 11
4			31300000						400000								22 12
5																	
6																	
7																	
8																	
9																	
10																	
11																	
12																	
13																	
14																	
15																	
16																	
17																	
18																	
19																	
20																	

NOTE: IF OPTION #11 = 1, RU(TOTAL) NOT REQUIRED

NO. OF CALCULATIONS
RESISTANCE
PLOT
GRAVITY
PRINT OUT

TEXAS A * M UNIVERSITY				PILE DRIVING ANALYSIS				CASE NO. HSP 10				PROBLEM NO. 1 OF 4			
1/Delta T P OPTIONS				1 2 3 4				11 12 13 14 15				EXP. FORCE			
9443.9 11				2 1 1 2				1 2 0 1				158700.			
ENERGY HAMMER EFFICIENCY				RU(TOTAL)				PERCENT UNDER POINT				MU Q(POINT)			
39800.00 1.00				1040962.1				10.0				0.10			
M W(M) K(M)				AREA(M)				SLACK(M)				ERES(M)			
1 4850.000 0.4220000E 08				1.000				0.0				1000.000 0.60			
2 1150.000 0.2450000E 08				1.000				0.0				1000.000 0.40			
3 1200.000 0.2250000E 08				254.000				0.0				1000.000 0.50			
4 883.000 0.5100000E 08				254.000				14638.531				0.0 1.00			
5 883.000 0.5100000E 08				254.000				43915.594				0.0 1.00			
6 883.000 0.5100000E 08				254.000				73192.425				0.0 1.00			
7 883.000 0.5100000E 08				254.000				102469.687				0.0 1.00			
8 883.000 0.5100000E 08				254.000				131746.750				0.0 1.00			
9 883.000 0.5100000E 08				254.000				161023.812				0.0 1.00			
10 883.000 0.5100000E 08				254.000				190300.875				0.0 1.00			
11 883.000 0.1040962E 07				254.000				219577.937				1000.000 1.00			
12 -0.0 -0.0				-0.0				104096.125				-0.0 -0.0			
SEGMENT				AREA				TIME N				MAX C STRESS			
1 1.000				4				2883699.				0 -0.0			
2 1.000				7				2245092.				0 -0.0			
3 254.000				11				7432.				42 -0.0			
4 254.000				13				7324.				0 -0.0			
5 254.000				15				7107.				0 -0.0			
6 254.000				17				6883.				0 -0.0			
7 254.000				19				6633.				34 1.			
8 254.000				21				6344.				34 172.			
9 254.000				23				5834.				29 1973.			
10 254.000				35				4195.				30 2491.			
11 254.000				27				1320.				0 -0.0			
PERMANENT SET OF PILE =				0.06760806 INCHES				NUMBER OF BLOWS PER INCH =				14.79113579			
												TOTAL INTERVALS = 49			

Figure B-2. Normal output (option 15 = 1) for prob. 1.

under DMAX(M), and the permanent set is 0.473011 (the ground quake Q) or 0.373911 in. Note that the input data is listed as well as the maximum stresses and the displacement of each segment.

Figure B6 is a sampling of the output when data is desired at some specified interval (OPTION 15 = 2, IPRINT = 1). The input information is listed in the first block of data. The next two blocks show the stresses at time interval N = 0 and N = 1. The data is defined as follows:

- D(M) = displacement of each mass point, (in.),
- C(M) = the compression in each spring, (in.),
- STRESS(M) = stress in each segment, (psi),
- F(M) = force in each spring, (lb.),
- R(M) = force in each soil spring, (lb.),
- W(M) = weight of each segment, (lb.),
- V(M) = velocity of each segment, (fps),
- DPRIME(M) = elastic displacement of soil, (in.),
- KPRIME(M) = soil spring stiffness, (lb./in.),
- FMAXC(M) = maximum compressive force in segment, (lb.), and
- FMAXT(M) = maximum tensile force in segment, (lb.).

Time interval N = 0 is for the pile under the influence of gravity alone. The particular output listed in Figure B6 shows that the point of the pile of Problem 4 would penetrate 0.002353 in. under gravity alone.

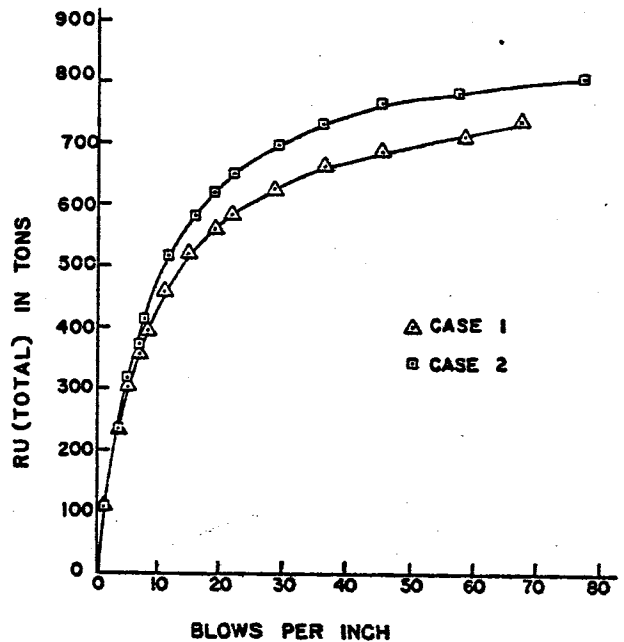


Figure B-3. Effect of varying cushion stiffness.

PILE DRIVING ANALYSIS				CASE NUMBER		HSP 10		PROBLEM NUMBER	
OPOINT = 0.10				JPOINT = 0.15					
BLCS PER IN.	RUTOTAL	POINT FORCE	MAX C STRESS	SEG	MAX T STRESS	SEG			
1.0733	213593.-	107.T	92981.	7321.	4	4411.	10		
3.3072	462346.-	231.T	185557.	7322.	4	3706.	10		
4.9401	601539.-	301.T	232773.	7322.	4	3358.	10		
6.6525	708095.-	354.T	266561.	7323.	4	3112.	10		
8.1351	785875.-	393.T	285066.	7323.	4	2955.	10		
10.7809	917031.-	459.T	312727.	7324.	4	2708.	10		
14.7911	1040962.-	520.T	335193.	7324.	4	2491.	10		
18.8100	1118220.-	559.T	350215.	7324.	4	2362.	10		
21.5075	1166279.-	583.T	359397.	7324.	4	2285.	10		
28.2760	1255360.-	628.T	375509.	7325.	4	2148.	10		
36.2405	1321145.-	661.T	386685.	7325.	4	2051.	10		
44.9512	1371145.-	686.T	394790.	7325.	4	1981.	10		
58.1772	1421145.-	711.T	402573.	7325.	4	1908.	10		
67.8860	1471145.-	736.T	410044.	7325.	4	1836.	10		

PILE DRIVING ANALYSIS				CASE NUMBER		HSP 10		PROBLEM NUMBER	
OPOINT = 0.10				JPOINT = 0.15				2	
BLCS PER IN.	RUTOTAL	POINT FORCE	MAX C STRESS	SEG	MAX T STRESS	SEG			
1.0377	213593.-	107.T	96645.	7664.	4	4171.	10		
3.1615	470888.-	235.T	196306.	7663.	4	3412.	10		
4.8150	622323.-	311.T	247556.	7662.	4	3083.	10		
6.5367	736804.-	368.T	283048.	7662.	4	2857.	10		
7.5466	819918.-	410.T	307190.	7661.	4	2703.	10		
11.4121	1025674.-	513.T	361561.	7661.	4	2352.	10		
15.8643	1158745.-	579.T	392988.	7660.	4	2145.	10		
18.7758	1233466.-	617.T	409456.	7660.	4	2035.	10		
22.0974	1297626.-	649.T	420576.	7659.	4	1950.	10		
29.3317	1394207.-	697.T	430297.	7659.	4	1840.	10		
36.8446	1460959.-	730.T	436023.	7659.	4	1766.	10		
45.1832	1510959.-	755.T	439816.	7659.	4	1713.	10		
57.8852	1560959.-	780.T	443210.	7658.	4	1663.	10		
77.6870	1610959.-	805.T	446224.	7658.	4	1613.	10		

Figure B-4. Summary output for RU(total) vs blows/in. (option 11 = 1) for prob. 1 and 2.

TEXAS A & M UNIVERSITY		PILE DRIVING ANALYSIS				CASE NO. HSP 10			PROBLEM NO.			3 OF 4	
1/DELTA T	P	OPTIONS	1	2	3	4	11	12	13	14	15	EXP. FORCE	
9443.9	11		2	1	1	2	2	1	0	1	1	158700.	
ENERGY	HAMMER EFFICIENCY		RUI(TOTAL)	PERCENT UNDER POINT		MO	Q(POINT)	Q(SIDE)	J(POINT)	J(SIDE)	NZ		
39800.00	1.00		400000.0	10.0		4	0.10	0.10	0.15	0.05	142		
M	W(M)	K(M)	AREA(M)	RU(M)	SLACK(M)	ERES(M)	VSTART(M)	KPRIME(M)					
1	4850.000	0.4220000E 08	1.000	0.0	1000.000	0.60	22.99	0.0					
2	1150.000	0.2450000E 08	1.000	0.0	1000.000	0.80	0.0	0.0					
3	1200.000	0.2250000E 08	254.000	0.0	1008.000	0.50	0.0	0.0					
4	883.000	0.5100000E 08	254.000	45000.000	0.0	1.00	0.0	0.4500001E 06					
5	883.000	0.5100000E 08	254.000	45000.000	0.0	1.00	0.0	0.4500001E 06					
6	883.000	0.5100000E 08	254.000	45000.000	0.0	1.00	0.0	0.4500001E 06					
7	883.000	0.5100000E 08	254.000	45000.000	0.0	1.00	0.0	0.4500001E 06					
8	883.000	0.5100000E 08	254.000	45000.000	0.0	1.00	0.0	0.4500001E 06					
9	883.000	0.5100000E 08	254.000	45000.000	0.0	1.00	0.0	0.4500001E 06					
10	883.000	0.5100000E 08	254.000	45000.000	0.0	1.00	0.0	0.4500001E 06					
11	883.000	0.3999999E 06	254.000	45000.000	1000.000	1.00	0.0	0.4500001E 06					
12	-0.0	-0.0	-0.0	39999.984	-0.0	-0.0	-0.0	0.0					
SEGMENT	AREA	TIME N	MAX C STRESS	TIME N	MAX T STRESS	DMAX(M)	D(M)	V(M)					
1	1.000	4	2883701.	0	-0.0	0.502888	0.375493	-4.67					
2	1.000	7	2245095.	93	-0.0	0.688212	0.688212	1.00					
3	254.000	11	7445.	97	-0.0	0.608394	0.608307	1.22					
4	254.000	13	7258.	41	2537.	0.497042	0.495229	-2.50					
5	254.000	15	7017.	39	3001.	0.489747	0.489520	-0.17					
6	254.000	17	6826.	38	2655.	0.484540	0.484376	1.46					
7	254.000	19	6656.	35	2477.	0.481653	0.481653	0.52					
8	254.000	21	6493.	34	3081.	0.479475	0.479301	-0.90					
9	254.000	23	6133.	29	3078.	0.474198	0.473951	0.21					
10	254.000	24	4278.	30	4194.	0.475263	0.470868	1.83					
11	254.000	27	647.	0	-0.0	0.473911	0.473011	-1.43					
PERMANENT SET OF PILE =			0.37391138 INCHES	NUMBER OF BLOWS PER INCH =		2.67442989		TOTAL INTERVALS =					

Figure B-5. Normal output for single RU(total) (option 11 = 2) for prob. 3.

TEXAS A & M UNIVERSITY		PILE DRIVING ANALYSIS				CASE NO. HSP 10			PROBLEM NO.			4 QF 4		
1/Delta T P OPTIONS		1	2	3	4	11	12	13	14	15	EXP. FORCE			
9443.9 11		2	1	1	2	2	2	0	1	2	159700.			
ENERGY	HAMMER EFFICIENCY	RU(TOTAL)	PERCENT UNDER	POINT	NO	Q(POINT)	Q(SIDE)	J(POINT)	J(SIDE)	N2				
39900.00	1.00	400000.0	10.0		4	0.10	0.10	0.15	0.05	140				
M	W(M)	K(M)	AREA(M)	RU(M)	SLACK(M)	ERES(M)	VSTART(M)	KPRIME(M)						
1	4850.000	0.4220000E 08	1.000	0.0	1000.000	0.60	22.99	0.0						
2	1150.000	0.2450000E 08	1.000	0.0	1000.000	0.80	0.0	0.0						
3	1200.000	0.3130000E 08	254.000	0.0	1000.000	0.50	0.0	0.0						
4	883.000	0.5100000E 08	254.000	5625.000	0.0	1.00	0.0	0.5625002E 05						
5	883.000	0.5100000E 08	254.000	16875.000	0.0	1.00	0.0	0.1687500E 06						
6	883.000	0.5100000E 08	254.000	28125.000	0.0	1.00	0.0	0.2812501E 06						
7	883.000	0.5100000E 08	254.000	39375.000	0.0	1.00	0.0	0.3937501E 06						
8	883.000	0.5100000E 08	254.000	50625.000	0.0	1.00	0.0	0.5062501E 06						
9	883.000	0.5100000E 08	254.000	61875.000	0.0	1.00	0.0	0.6187502E 06						
10	883.000	0.5100000E 08	254.000	73125.000	0.0	1.00	0.0	0.7312502E 06						
11	883.000	0.3999999E 06	254.000	84375.000	1000.000	1.00	0.0	0.8437502E 06						
12	-0.0	-0.0	-0.0	39999.984	-0.0	-0.0	-0.0	0.0						

TIME INTERVAL N = 0		NET PENETRATION = 0.0				N1 = 140		N2 =					
SEGMENT M	D(M)	C(M)	STRESS(M)	F(M)	R(M)	W(M)	V(M)	CPN(M)	KPRIME(M)	FMAX(C)	FMAX(M)		
1	0.002919	0.0	0.0	0.0	0.0	4850.00	22.988647	0.000566	0.0	0.0	0.0		
2	0.002919	0.000047	1150.	1150.	0.0	1150.00	0.0	0.000566	0.0	0.0	0.0		
3	0.002873	0.000075	9.	2350.	0.0	1200.00	0.0	0.000519	0.0	0.0	0.0		
4	0.002797	0.000061	12.	3101.	132.	883.00	0.0	0.000444	56250.	0.0	0.0		
5	0.002737	0.000070	14.	3586.	397.	883.00	0.0	0.000383	168750.	0.0	0.0		
6	0.002666	0.000075	15.	3808.	662.	883.00	0.0	0.000313	281250.	0.0	0.0		
7	0.002592	0.000074	15.	3764.	927.	883.00	0.0	0.000238	393750.	0.0	0.0		
8	0.002518	0.000068	14.	3455.	1191.	883.00	0.0	0.000164	506250.	0.0	0.0		
9	0.002450	0.000057	11.	2882.	1456.	883.00	0.0	0.000097	618750.	0.0	0.0		
10	0.002394	0.000040	8.	2044.	1721.	883.00	0.0	0.000040	731250.	0.0	0.0		
11	0.002353	0.0	4.	941.	1986.	883.00	0.0	0.0	843750.	0.0	0.0		

TIME INTERVAL N = 1		NET PENETRATION = 0.0				N1 = 140		N2 =					
SEGMENT M	D(M)	C(M)	STRESS(M)	F(M)	R(M)	W(M)	V(M)	CPN(M)	KPRIME(M)	FMAX(C)	FMAX(M)		
1	0.032130	0.029211	1232689.	1232689.	0.0	4850.00	22.126266	0.000566	0.0	1232689.	0.0		
2	0.002919	0.000047	1150.	1150.	0.0	1150.00	3.651348	0.000566	0.0	1150.	0.0		
3	0.002873	0.000075	9.	2350.	0.0	1200.00	0.000000	0.000519	0.0	2350.	0.0		
4	0.002797	0.000061	12.	3101.	132.	883.00	-0.000000	0.000444	56250.	3101.	0.0		
5	0.002737	0.000070	14.	3586.	397.	883.00	-0.000000	0.000383	168750.	3586.	0.0		
6	0.002666	0.000075	15.	3808.	662.	883.00	0.000000	0.000313	281250.	3808.	0.0		
7	0.002592	0.000074	15.	3764.	927.	883.00	0.000000	0.000238	393750.	3764.	0.0		
8	0.002518	0.000068	14.	3455.	1191.	883.00	0.000000	0.000164	506250.	3455.	0.0		
9	0.002450	0.000057	11.	2882.	1456.	883.00	-0.000000	0.000097	618750.	2882.	0.0		
10	0.002394	0.000040	8.	2044.	1721.	883.00	0.000000	0.000040	731250.	2044.	0.0		
11	0.002353	0.0	4.	941.	1986.	883.00	-0.000000	0.0	843750.	941.	0.0		

Figure B-6. Detailed output for single RU(total) (option 15 = 2) for prob. 4.

Appendix C

FORTRAN IV. PROGRAM STATEMENT

The listing that follows is known as an UNRAVL listing. Each statement is numbered, for reference, consecutively from the first to the last statement. The variables and program statement numbers are indexed by

their reference number. This listing facilitates finding each variable in the program and makes the logic much easier to follow.

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C      TEXAS A * M UNIVERSITY
C      PILE DRIVING ANALYSIS BY THE WAVE EQUATION
C      TEXAS A AND M PROGRAM REVISED 12/1/65 BY EAS
C      PERMANENT SET, BLOWS PER INCH
C      LCCSE, TIGHT, OK LIMITED MOTION AT JOINTS
C      MAXIMUM STRESSES OR FORCES
C      IOPT USED FOR OPTION.
C      I, JT, JTM, LAMP, LAY, LT, LACK, ARE USED FOR CONTROL
C      X AT END OF NAME = LAST PRECEDING VALUE EXCEPT IN MAX = MAXIMUM
C      N ALWAYS MEANS NUMBER OF TIME INTERVAL.
C      NOTATION FOLLOWS SMITHS ASCE PAPER CLOSELY. TO DECODE NOTE THAT
C      NFMAXT = NO. OF TIME INTERVAL WHERE FORCE = MAXIMUM IN TENSION
ISN 0002 5000 REAL JPOINT, JSIDE, K, KPRIME, NPASS, NPI, K HOLD, CASE=0
ISN 0003 5001 INTEGER P, PPLUS1, PLESS1, PRUB, PROBS
ISN 0004 5002 DIMENSION AREA(150), C(150), CX(150), CMAX(150), D(150), DX(150)
      1, CMAX(150), OPRIME(150), ERES(150), F(150), FX(150), FMAXC(150),
      2 FMAXT(150), K(150), KPRIME(150), LAM(150), NDMAX(150),
      3 NFMAXC(150), NFMAXT(150), R(150), RU(150), SLACK(150),
      4 LBLOWS(150), UFMAXC(150), URUTTL(150), V(150),
      5 W(150), WULIST(150), RUMIL(30), WENR(30), RWHICH(30),
      6 XPLT(150), YPLT(150), STRESS(150), K HOLD(150),
      7 FCMAX(150), ACMAX(150), FTMAX(150), NIMAX(150)
C      24 OF EACH OF ABOVE SUFFICIENT FOR USUAL PROBLEMS
C----- INPUT -- GENERAL
ISN 0005 5010 READ(5,5113) CASE, PROBS, TTDDEL, P, SLACK(1), SLACK(2), SLACK(3), IOPT1,
      1 ICPT2, IOPT3, IOPT4, IPRINT, NSEGI,
      WRITE(6,5003)
ISN 0006 5003 FORMAT(IH1)
ISN 0007 IF(TTDDEL.LE.0.) TTDDEL=1.0
ISN 0008 IF(IOPT4.LE.0) IOPT4=2
ISN 0010 IF(IPRINT.LE.0) IPRINT=1
ISN 0012 IF(NSEGI.LE.0) NSEGI=2
ISN 0014 TDELTA=TTDEL
ISN 0016 5020 UELTAT = 1./TDELTA
ISN 0017 5021 PPLUS1 = P+1
ISN 0018 5022 PLESS1 = P-1
ISN 0019 5030 READ (5,5114)(W(M),M=1,P)
ISN 0020 5031 W(PPLUS1) = -0.0
ISN 0021 C-----CALCULATE PILE WEIGHT
      WPILE=0.
      DO 6 JT=NSEGI,P
      6 WPILE=WPILE+W(JT)
ISN 0022 5040 READ(5,5115)(K(M),M=1,PLESS1)
ISN 0023 K(P) IS DETERMINED AT 5184
ISN 0024 5041 KIPPLUS1) = -0.0
ISN 0025 5083 DO 5084 M=1,P
      KMCLD(M)=K(M)
ISN 0026 5084 AREA(M) = 1.0
ISN 0027 5086 AREA(PPLUS1) = -0.0
ISN 0028 5087 IF(IOPT1-2)5090,5088,5088
ISN 0029 5088 READ (5,5114)(AREA(M),M=1,P)
ISN 0030 IF(AREA(1).LE.0.) AREA(1)=1.0
ISN 0031 IF(AREA(P).LE.0.) AREA(P)=1.0
ISN 0032 5090 IF(IOPT2-2)5100,5092,5092
ISN 0033 5092 READ (5,5116)(KULIST(M),M=1,PPLUS1)
ISN 0034 5100 IF(IOPT3-2)5101,5104,5104
ISN 0035 5101 DO 5102 M=4,PLESS1
ISN 0036 5102 SLACK(M) = 0.0
ISN 0037 5103 GO TO 5105
ISN 0038 5104 READ (5,5114)(SLACK(M),M=1,PLESS1)
ISN 0039 5105 SLACK(P) = 1000.0
ISN 0040 5106 SLACK(PPLUS1) = -0.0
ISN 0041 5110 UC 5111 M=4,P
ISN 0042 5111 ERES(M) = 1.0
ISN 0043 5112 ERES(PPLUS1) = -0.0
ISN 0044 5113 FORMAT(A6,I3,F10.4,I3,3F7.3,4I1,1X13,I2)
ISN 0045 5114 FORMAT(8F10.3)
ISN 0046 5115 FORMAT(8F10.0)
ISN 0047 5116 FORMAT(8F10.7)
ISN 0048 5117 FORMAT (I2,F8.2,I1,F9.0,F3.2,F6.0,3F3.2,F9.1,F4.1,I3,4F3.2,F9.0
ISN 0049 1,511)
ISN 0050 5118 FORMAT(1H0,5H CASE,4X,5H PROB,46,74H RU PERCENTAGES ON DATA SHE
ISN 0051 LET PAGE 1 SHOULD TOTAL 100.0 BUT ACTUALLY TOTAL,F15.7)
ISN 0052 C----- DO 5570 SOLVES PROBLEMS ONE AFTER ANOTHER
ISN 0053 NC=1
ISN 0054 5120 UC 5570 I=1,PROBS
ISN 0055 K(INC)=K HOLD(INC)
ISN 0056 5121 READ(5,5117) PROB,W(1),NC,K(INC), EFF, ENERGY, ERES(1), ERES(2), ERES(3)
ISN 0057 1 ,RUSUM, PERCNT, MO, QPOINT, QSIDE, JPOINT, JSIDE, FEXP, IOPT11,
      2 ICPT12, IOPT13, IOPT14, IOPT15
ISN 0058 IF(IOPT12.LE.0) IOPT12=3
ISN 0059 VSTART= SQRT(64.4*EFF*(ENERGY/W(1)))
ISN 0060 DO 9009 M=1,50
ISN 0061 FTMAX(M)=0.
ISN 0062 9009 FCMAX(M)=0.
ISN 0063 NKCNT=0
ISN 0064 5140 KUTTLX = 0.0
ISN 0065 5141 BLCMSX = 0.0
ISN 0066 5150 V(1) = VSTART
ISN 0067 5152 LT = 0
ISN 0068 C----- FIRST DETERMINE VALUE OF RUTOTL
ISN 0069 5154 IF(IOPT11-2)5151,5160,5151
ISN 0070

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C      FOR CURVE PLOTTING
ISN 0071 5151 RUTOTL = W(1)* V(1)**2/12.0
ISN 0072 5153 GO TO 5170
C      FOR SINGLE PROBLEM
ISN 0073 5160 RUTOTL=RUSUM
ISN 0074 GO TO 5170
C      COMPUTER CYCLES FROM 707 NEAR END OF PROGRAM
ISN 0075 701 SLCPE = (RUTOTL-RUTTLX)/(BLOWS-BLOWSX)
ISN 0076 SLCPE=AMAX1(10000.,SLCPE)
ISN 0077 IF(BLOWS-7.0)5164,702,702
ISN 0078 5164 IF(IOPT4-2)5165,703,703
ISN 0079 702 IF(HLOWS-20.0)704,704,705
ISN 0080 5165 DB = 1.00
ISN 0081 GO TO 706
ISN 0082 703 DB = 1.25
ISN 0083 GO TO 706
ISN 0084 704 DB = 2.5
ISN 0085 GO TO 706
ISN 0086 705 DB = 5.0
ISN 0087 GO TO 706
ISN 0088 706 RUTTLX = RUTOTL
ISN 0089 RUTOTL = RUTTLX+(DB*SLOPE)
ISN 0090 BLCWSX = BLOWS
C----- SECOND DETERMINE ALL VALUES OF RU(M)
ISN 0091 5170 DO 13 M=1,MO
ISN 0092 13 RU(M) = 0.0
ISN 0093 5171 RUPINT = (PERCNT/100.0)*RUTOTL
ISN 0094 5172 IF(IOPT12-2)1143,146,5176
C      FOR UNIFORM DISTRIBUTION
ISN 0095 143 DO 144 M=MO,P
ISN 0096 144 RU(M) = (RUTOTL-RUPINT)/FLOAT(P-MO+1)
ISN 0097 5173 RU(PPLUS1) = RUPINT
ISN 0098 GO TO 713
C      FOR TRIANGULAR DISTRIBUTION
ISN 0099 146 DO 145 M=MO,P
ISN 0100 145 RU(M) = (2.0*(RUTOTL-RUPINT)*(FLOAT(M-MO)+0.5))/(FLOAT(P-MO+1))**2
ISN 0101 5175 RU(PPLUS1) = RUPINT
ISN 0102 GO TO 713
C      FOR DISTRIBUTION PER RU LIST ON DATA SHEET
ISN 0103 5176 TOTAL = 0.0
ISN 0104 DO 5177 M=1,PPLUS1
ISN 0105 5177 TCTAL = TOTAL+RULIST(M)
ISN 0106 5178 IF((ABS(TOTAL-100.0))-2.0)5180,5180,5179
ISN 0107 5179 WRITE (6,5118)CASE,PROB,TOTAL
ISN 0108 GO TO 5570
ISN 0109 5180 DO 5181 M=1,PPLUS1
ISN 0110 5181 RU(M) = (RULIST(M)/100.0)*RUTOTL
ISN 0111 GO TO 713
C----- THIRD DETERMINE STARTING VALUES OF V(M)
ISN 0112 713 V(1)=VSTART
ISN 0113 DO 180 M=2,P
ISN 0114 180 V(M) = 0.0
ISN 0115 5183 V(PPLUS1) = -0.0
ISN 0116 C----- FOURTH DETERMINE VALUE FOR K(P)
ISN 0116 5184 K(P) = RU(PPLUS1)/QPOINT
ISN 0117 C      FIFTH CHANGE CYCLE COUNT
ISN 0117 5186 LT = LT + 1
ISN 0118 C-----CHECK ON DELTAT
ISN 0118 CALL DELTCK(NPASS,TDELTA,P,W,K,TDELTA,DELTAT,N2)
ISN 0118 C-----END DELTAT CHECK
ISN 0118 C----- ASSIGN OTHER VALUES REQUIRED (TEXAS A AND M REPI)
ISN 0119 DO 5218 M=1,P
ISN 0120 32 KPRIME(M) =RU(M)/QSIDE
ISN 0121 C(M) = 0.0
ISN 0122 F(M) = 0.0
ISN 0123 CMAX(M) = 0.0
ISN 0124 LAP(M)=1
ISN 0125 D(M) = 0.0
ISN 0126 NFMAX(M) = 0
ISN 0127 NFMXT(M) = 0
ISN 0128 UMAX(M) = 0.0
ISN 0129 NDMAX(M) = 0
ISN 0130 FMAX(M) = 0.0
ISN 0131 FMAXT(M) = 0.0
ISN 0132 K(P) = 0.0
ISN 0133 5218 DPRIME(M) = 0.0
ISN 0134 KPRIME(PPLUS1)=0.
ISN 0135 DPRIMP = 0.0
ISN 0136 LAPP = 1
C----- SIXTH PRINT INPUT FOR ONE PROBLEM
ISN 0137 5190 WRITE (6,5200)CASE,PROB,PROBS
ISN 0138 5191 WRITE (6,5201)
ISN 0139 5192 WRITE(6,5202) TDELTA,P,IOPT1,IOPT2,IOPT3,IOPT4, IOPT11,
1 IOPT12,IOPT13,IOPT14,IOPT15,FEXP
ISN 0140 5193 WRITE (6,5203)
ISN 0141 5194 WRITE(6,5204) ENERGY,EFF,RUTOTL,PERCNT,MO,QPOINT,QSIDE,JPOINT,JSI
IDE,N2
ISN 0142 5195 WRITE (6,5205)
ISN 0143 5196 WRITE (6,5206)(M,W(M),K(M),AREA(M),RU(M),SLACK(M),ENES(M),
1 V(M),KPRIME(M),M=1,PPLUS1)
ISN 0144 5200 FORMAT(///27H TEXAS A * M UNIVERSITY ,3X,22H PILE DRIVING ANALY
1SIS,4X,9H CASE NO.,A7,3X,12H PROBLEM NO.,14,3H OF,14)
ISN 0145 5201 FORMAT(2X10H 1/DELTA T3X1HP4X62HOPTIONS 1 2 3 4
1 11 12 13 14 1510X10MEXP FORCE)
ISN 0146 5202 FORMAT(F11.1,15,11X415,10X515,F18.0)
ISN 0147 5203 FORMAT(113H ENERGY HAMMER EFFICIENCY RU(TOTAL) PERCENT UN

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IDEN POINT MO OIPOINT Q(SIDE) J(POINT) J(SIDE) N2)
ISN 0148 5204 FORMAT(2F10.2,10XF12.1,F16.1,111,F10.2,F9.2,F10.2,F9.2,17)
ISN 0149 5206 FORMAT(I3, F14.3,E15.7,F10.3,2F14.3,F9.2,F11.2,E15.7)
ISN 0150 5205 FORMAT(3H M,7X,5H W(M),7X,5H K(M),7X,8H AREA(M),6X,6H RU(M),7X,4I
IM SLACK(M) ERES(M) VSTART(M) KPRIME(M))
C---- EFFECT OF GRAVITY BEFORE RAM STRIKES--TEXAS A AND M SMITHS GRAVITY
ISN 0151 5258 IF(IOPT14-2)5220,5221,5221
ISN 0152 5270 WTCAL = 0.0
ISN 0153 RTOTAL = 0.0
ISN 0154 DO 5 JT=2,PPLUS1
ISN 0155 WTCAL = WTOTAL + W(JT)
ISN 0156 5 WTCAL = RTOTAL + RU(JT)
ISN 0157 DO 8 JT = 2,PLESS1
ISN 0158 R(JT) = (RU(JT)*WTOTAL)/RTOTAL
ISN 0159 8 F(JT) = F(JT-1)+W(JT)-R(JT)
ISN 0160 IF(K(P))67,66,67
ISN 0161 66 IF(KPRIME(P))67,63,67
ISN 0162 67 U(P) = (F(PLESS1)+W(P))/(KPRIME(P)+K(P))
ISN 0163 IF(QS(OE-O(P))64,65,65
ISN 0164 64 R(P) = RU(P)
ISN 0165 F(P) = F(PLESS1) + W(P) - R(P)
ISN 0166 U(P) = F(P)/K(P)
ISN 0167 GC TO 63
ISN 0168 65 R(P) = U(P)*KPRIME(P)
ISN 0169 F(P) = D(P)+K(P)
ISN 0170 63 CONTINUE
ISN 0171 DO 111 JT = 1,PLESS1
ISN 0172 JTM = P-JT
ISN 0173 C(JTM) = F(JTM)/K(JTM)
ISN 0174 D(JTM) = D(JTM+1)+C(JTM)
ISN 0175 DPRIME(JTM) = D(JTM)-WTOTAL*QSIDE/RTOTAL
ISN 0176 111 CONTINUE
ISN 0177 DO 8000 M=1,P
ISN 0178 8000 STRESS(M)=F(M)/AREA(M)
ISN 0179 5271 N=0
ISN 0180 LAY = 1
ISN 0181 5230 IF(IOPT15-2)5240,5231,5240.
ISN 0182 5231 WRITE(6,5234)N,DPRIME,M2
ISN 0183 5232 WRITE (6,5235)
ISN 0184 5233 WRITE(6,5236)(M,D(M),C(M),STRESS(M),F(M),R(M),W(M),V(M),DKPRIME(M),
IKPRIME(M),FMAXC(M),FMAXT(M),M=1,P)
NKCNT=0
ISN 0185
ISN 0186 5234 FORMAT(/,18H TIME INTERVAL N =16,7X18HNET PENETRATION = F10.6,
17X5HNL = 15,5X5HN2 = 15)
ISN 0187 5235 FORMAT(120H SEGMENT M D(M) C(M) STRESS(M) F(M)
1 R(M) W(M) V(M) DKPRIME(M) KPRIME(M) FMAXC(M) FMAXT(M))
ISN 0188 5236 FORMAT(I8,F11.6,F10.6,F11.0,2F10.0,F10.2,2F10.6,3F10.0)
C---- DYNAMIC COMPUTATION BASED ON SMITHS PAPER MODIFIED (TEXAS REPN)
ISN 0189 5240 LACK = 1
ISN 0190 5241 DO 68 M=1,P
C
ISN 0191 68 IS BETWEEN 5439 AND 5440
ISN 0192 U(M) = D(M)+V(M)*12.0*DELTAT
ISN 0193 IF(DMAX(M)-U(M))20,21,21
ISN 0194 20 UMAX(M) = D(M)
ISN 0195 NDMAX(M) = N + 1
ISN 0196 21 CX(M) = C(M)
ISN 0197 IF(X-P)34,5400,34
ISN 0198 34 C(M) = D(M)-U(M+1)-V(M+1)*12.0*DELTAT
ISN 0199 C STATEMENT 34 MUST USE A COMPUTED VALUE FOR THE ACTUAL D(M+1)
ISN 0200 5242 IF(C(M))5243,30,30
ISN 0201 5243 IF(ABS(C(M))-SLACK(M))5244,5244,5246
ISN 0202 5244 C(M) = 0.0
ISN 0203 5245 GC TO 30
ISN 0204 5246 C(M) = C(M)+SLACK(M)
C
ISN 0205 NOTE THAT ONLY A NEGATIVE VALUE OF C(M) RESULTS FROM 5246
ISN 0206 30 FX(M) = F(M)
C
ISN 0207 A TEXAS ROUTINE FOR B(M) IS OMITTED HERE
ISN 0208 5250 IF(IOPT4-2)5300,36,5300
C---- 36 TO 35 IS A TEXAS ROUTINE REPLACING SMITH ROUTINE 3 OR 4
ISN 0209 36 IF(ABS(ERES(M)-1.0)-.00001)38,38,14
ISN 0210 38 F(M) = C(M)*K(M)
ISN 0211 GC TO 5400
ISN 0212 14 IF(C(M)-CX(M))12,35,15
ISN 0213 15 F(M) = FX(M)+((C(M)-CX(M))*K(M))
ISN 0214 GC TO 35
ISN 0215 12 F(M) = FX(M)+((C(M)-CX(M))*K(M)/ERES(M)**2)
ISN 0216 35 F(M) = AMAX1(0.0,F(M))
ISN 0217 GC TO 5400
C
ISN 0218 A TEXAS ROUTINE FOR GAMMA IS OMITTED HERE
ISN 0219 C---- SMITH ROUTINE 3 OR 4
ISN 0220 5300 IF(ERES(M)-1.00)5302,5301,5301
ISN 0221 5301 F(M) = C(M)*K(M)
ISN 0222 GC TO 5400
ISN 0223 5302 IF(C(M))5303,5303,5304
ISN 0224 5303 F(M) = 0.0
ISN 0225 GC TO 5400
ISN 0226 5304 IF(C(M)-CMAX(M))5306,5305,5305
ISN 0227 5305 CMAX(M) = C(M)
ISN 0228 F(M) = C(M)*K(M)
ISN 0229 GC TO 5400
ISN 0230 5306 F(M)=(K(M)/ERES(M)**2)*C(M)-(1./ERES(M)**2-1.)*K(M)*CMAX(M)
ISN 0231 F(M) = AMAX1(F(M),0.0)
ISN 0232 GC TO 5400
ISN 0233 5400 IF(M.GT.1) GO TO 48
ISN 0234 IF(FEXP.LE.0.) GO TO 48
ISN 0235 NPI=N+1

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ISN 0232      IF(NP1.GT.(0.0125/DELTAT)) GO TO 46
ISN 0234      IF(NP1-0.01/DELTAT)46,46,90
ISN 0235      46 IF(F1)-FX(1)147,48,48
ISN 0236      47 F(1)=AMAX1(F(1),FEXP,0.)
ISN 0237      GO TO 48
ISN 0238      90 F(1)=AMAX1(0.0,FEXP*(1.0-(DELTAT*(NP1-0.01/DELTAT)/0.0025)))
ISN 0239      48 IF(KPRIME(M))50,55,50
ISN 0240      50 IF(OPRIME(M)-DIM)+QSIDE)51,52,52
ISN 0241      51 OPRIME(M) = DIM)-QSIDE
ISN 0242      52 CONTINUE
ISN 0243      IF(OPRIME(M)-DIM)-QSIDE)53,53,54
ISN 0244      54 OPRIME(M) = DIM)+CSIDE
ISN 0245      53 CONTINUE
ISN 0246      5410 LAP = LAM(M)
ISN 0247      GC TO(10,57),LAP
ISN 0248      10 IF(U(M)-OPRIME(M)-QSIDE)56,57,57
ISN 0249      56 K(P) = (D(M)-OPRIME(M))*KPRIME(M)*(1.0+JSIDE*V(M))
ISN 0250      GC TO 55
ISN 0251      57 K(P) = (U(M)-OPRIME(M)+JSIDE*QSIDE*V(M))*KPRIME(M)
ISN 0252      LAP(M) = 2
ISN 0253      55 CONTINUE
ISN 0254      73 IF(M-P)71,74,71
ISN 0255      74 IF(OPRIMP-D(P)+QPOINT)75,76,76
ISN 0256      75 OPRIMP = D(P)-QPOINT
ISN 0257      76 CONTINUE
ISN 0258      LAMP = LAMP
ISN 0259      GC TO (77,78),LAMP
ISN 0260      77 IF(U(P)-OPRIMP-QPOINT)79,78,78
ISN 0261      79 F(P) = (U(P)-OPRIMP)*K(P)*(1.0+JPOINT*V(P))
ISN 0262      GC TO 171
ISN 0263      78 F(P) = (U(P)-OPRIMP+JPOINT*QPOINT*V(P))*K(P)
ISN 0264      LAMP = 2
ISN 0265      171 F(P) = AMAX1(0.0,F(P))
ISN 0266      71 CONTINUE
C
ISN 0267      GRAVITY OPTION
ISN 0268      5420 IF(IOPT14-2)5421,5423,5423
ISN 0269      5421 IF(LACK-2)5424,5427,5427
ISN 0270      58 V(1) = V(1)-(F(1)+R(1)-W(1))*32.17*DELTAT/W(1)
ISN 0271      LACK = 2
ISN 0272      GO TO 5429
ISN 0273      72 V(P) = V(M)+(F(M-1)-F(M)-R(M)+W(M))*32.17*DELTAT/W(M)
ISN 0274      5422 GO TO 5429
ISN 0275      5423 IF(LACK-2)5424,5427,5427
ISN 0276      5424 V(1) = V(1)-(F(1)+R(1))*32.17*DELTAT/W(1)
ISN 0277      5425 LACK = 2
ISN 0278      GO TO 5429
ISN 0279      5427 V(P) = V(M)+(F(M-1)-F(M)-R(M))*32.17*DELTAT/W(M)
ISN 0280      5429 CONTINUE
ISN 0281      IF(M.GT.1) GO TO 5430
ISN 0282      IF(F(1).LE.0..AND.V(1).LE.-0.1) V(1)=-VSTART
ISN 0283      5430 FMAX(M) = AMAX1(FMAX(M),F(M))
ISN 0284      FMAX(M) = AMIN1(FMAX(M),F(M))
ISN 0285      5439 IF(FMAX(M)-F(M))166,167,166
ISN 0286      167 NFMAX(M) = N+1
ISN 0287      166 IF(FMAX(M)-F(M))168,69,68
ISN 0288      168 NFMAX(M) = N+1
ISN 0289      68 STRESS(M)=F(M)/AREA(M)
ISN 0290      N=N+1
ISN 0291      C
ISN 0292      THIS IS END OF DO 68 STARTING AT 5241
ISN 0293      5440 IF(IOPT15-2)5444,5441,5444
ISN 0294      5441 IF(N-1)7000,7001,7000
ISN 0295      7000 NKCNT=NKONT+1
ISN 0296      IF(NKONT-IPRINT)5444,7001,5444
ISN 0297      7001 WRITE (6,5234)N,OPRIMP,N2
ISN 0298      WRITE (6,5235)
ISN 0299      WRITE(6,5236)(M,U(M),C(M),STRESS(M),F(M),R(M),W(M),V(M),OPRIME(M),
ISN 0300      KPRIME(M),FMAX(M),FMAX(M),M=1,P)
ISN 0301      7003 NKCNT=0
ISN 0302      5444 GC TO (5443,192),LAY
ISN 0303      5443 IF((V(P)+0.1).GT.0.) GO TO 192
ISN 0304      WV=0.0
ISN 0305      GO 193 JA=NSEG1,P
ISN 0306      193 WV=W+V(JA)*V(JA)
ISN 0307      IF(V(1).LT.0..AND.WV-LT.0..AND.DMAX(P).GT.DIP)) GO TO 190
ISN 0308      GO TO 192
ISN 0309      190 LAY=2
ISN 0310      GC TO (192,194,192),IOPT15
ISN 0311      194 WRITE(6,5234) N,OPRIMP,N2
ISN 0312      WRITE(6,5235)
ISN 0313      WRITE(6,5236)(M,U(M),C(M),STRESS(M),F(M),R(M),W(M),V(M),OPRIME(M),
ISN 0314      KPRIME(M),FMAX(M),FMAX(M),M=1,P)
ISN 0315      192 IF(V(2)/VSTART-3.1)161,60,60
ISN 0316      GO WRITE (6, 105)
ISN 0317      105 FORMAT(74H THE RATIO OF THE VELOCITY OF W(2) TO THE VELOCITY OF T
ISN 0318      HE RAM EXCEEDS 3.1)
ISN 0319      GO TO 5570
ISN 0320      61 IF(V(P)/VSTAKT-3.1)163,62,62
ISN 0321      62 WRITE (6, 106)
ISN 0322      GC TO 5570
ISN 0323      106 FORMAT(74H THE RATIO OF THE VELOCITY OF W(P) TO THE VELOCITY OF T
ISN 0324      HE RAM EXCEEDS 3.1)
C --- END OF TEXAS REPN
ISN 0325      163 CONTINUE
ISN 0326      IF(LAY.EC.2) GO TO 5447
ISN 0327      IF(N-2)5240,5447,5447
ISN 0328      C---- 5240 CYCLES FOR NEXT TIME INTERVAL

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ISN 0326 5447 UC 5449 M=1,P
ISN 0327 5448 FMAXC(M) = FMAXC(M)/AREA(M)
ISN 0328 5449 FMAXT(M)=FMAXT(M)/(-AREA(M))
ISN 0329 GC TO(5442,5442,5553),IOPT15
ISN 0330 5442 WRITE (6,2105)
ISN 0331 5550 WRITE(6,2106)(M,AREA(M),NFMAXC(M),FMAXC(M),NFMAXT(M),FMAXT(M),DMAX
      L(M),U(M),V(M),M=1,P)
      BLCHS=0.0
ISN 0332 5553 IF(DPRIMP.GT.0.0) BLOWS=1.0/DPRIMP
ISN 0333 5551 UBLOWS(LT) = BLOWS
ISN 0335 UHLTTL(LT) = RUTOTL
ISN 0336 UFMAX(LT) = FMAXC(P)*AREA(P)
ISN 0337 C INITIAL U ABOVE IDENTIFIES FIGURES USED IN SUMMARY
ISN 0338 GO TO(5552,5552, 150),IOPT15
ISN 0339 5552 WRITE (6,2107)DPRIMP,BLOWS,N
ISN 0340 2105 FCRMAT(//103H SEGMENT AREA TIME N MAX C STRESS TIME N
      I MAX T STRESS CMAX(M) DIM) VIM))
ISN 0341 2106 FCRMAT(13,F15.3,18,F12.0,114,F12.0,F16.6,F10.6,F13.2)
ISN 0342 2107 FCRMAT(24H PERMANENT SET UP PILE =F15.8, 38H INCHES NUMBER OF B
      LLOWS PER INCH = F16.8,22H TOTAL INTERVALS = 18)
ISN 0343 150 CCNTINUE
ISN 0344 5558 UC 5563 M=NSEG1,P
ISN 0345 FTMAX(LT)=AMAX1(FTMAX(LT),FMAXT(M))
ISN 0346 FCPAX(LT)=AMAX1(FCMAX(LT),FMAXC(M))
ISN 0347 IF(FCMAX(LT)-FMAXC(M))5560,5561,5560
ISN 0348 5561 NCMAX(LT)=M
ISN 0349 5560 IF(FTMAX(LT)-FMAXT(M))5563,5562,5563
ISN 0350 5562 NTPAX(LT)=M
ISN 0351 5563 CCNTINUE
ISN 0352 5555 IF(10PT11-2)5556,5570,5570
ISN 0353 5556 IF (UPRIMP-0.001)59,707,707
ISN 0354 707 IF (HLOWS-60.0)701,701,59
ISN 0355 59 CONTINUE
ISN 0356 WRITE (6,803) CASE,PROB
ISN 0357 WRITE (6,804) QPOINT,JPOINT
ISN 0358 WRITE (6,805)
ISN 0359 DO 801 J=1,LT
ISN 0360 URUTIN=URUTTL(J)/2000.
ISN 0361 801 WRITE(6,802) UBLOWS(J),URUTTL(J),URUTON,UFMAXC(J),FCMAX(J),NCMAX(J)
      P,FTMAX(J),NFMAX(J)
ISN 0362 802 FORMAT(4X F7.4,F10.0,1H-F5.0,1HTF13.0,F13.0,4X I2,F13.0,4X I2)
ISN 0363 803 FORMAT (1H0,10X,22H PILE DRIVING ANALYSIS,
      I 10X,17H CASE NUMBER,3X,A6,10X,15H PROBLEM NUMBER,3X,I3)
ISN 0364 804 FORMAT(19X,9H QPOINT = F5.2,11X,9H JPOINT = F5.2)
ISN 0365 805 FORMAT(2X13H BLOWS PER IN.2X7H RUTOTAL7X11H POINT FORCE2X12H MAX C STR
      ESS2X3H SEG2X12H MAX T STRESS2X3H SEG//)
ISN 0366 C-----PLOTING ROUTINE
ISN 0367 IF(10PT13-1)5570,5574,5574
      5574 CALL DRAW(WTCTA,PROB,UBLOWS,LT,CASE,PROB)
ISN 0368 C-----END PLOTING ROUTINE
ISN 0369 5570 WRITE(6,5572)
ISN 0370 C DO 5570 STARTS AT 5120
ISN 0371 5572 FCRMAT(1H)
      5571 GO TO 5010
      END

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*****FURTRAN CROSS REFERENCE LISTING*****

SYMBOL	INTERNAL STATEMENT NUMBERS																
TOTAL	0103	0105	0105	J106	0107												
WFILE	0022	0024	0024														
XPLOF	0004																
YPLUF	0004																
HLOWSX	0067	0075	0090														
DELTA	0017	0118	0191	0197	0232	0234	0238	0238	0269	0272	0275	0278					
DELTCX	0118																
UPRIME	0004	0133	0175	0184	0240	0241	0243	0244	0248	0249	0251	0298	0313				
DPKIMP	0135	0182	0255	0256	0260	0261	0263	0296	0311	0333	0333	0339	0353				
ENERGY	0058	0061	0141														
IGPT11	0058	0070	0139	0352													
IGPT12	0058	0059	0059	0094	0139												
IGPT13	0058	0139	0366														
IGPT14	0058	0139	0151	0267													
IGPT15	0058	0139	0181	0292	0310	0329	0338										
IPRINT	0005	0017	0012	0295													
JPRINT	0002	0058	0141	0261	0263	0357											
KPRIME	0002	0004	0120	0134	0143	0161	0162	0168	0184	0239	0249	0251	0298	0313			
NMAXC	0004	0126	0287	0331													
NMAXT	0004	0127	0289	0331													
PERCNT	0058	0093	0141														
PLESSI	0003	0019	0025	0040	0043	0157	0162	0165	0171								
PPLUS1	0003	0018	0021	0025	0030	0038	0045	0048	0097	0101	0104	0109	0115	0116	0134	0143	0154
QPOINT	0058	0116	0141	0255	0256	0260	0263	0357									
RTOTAL	0153	0156	0156	0158	0175												
RULIST	0004	0038	0105	0110													
RUPINT	0093	0096	0097	0100	0101												
RUTITL	0071	0073	0075	0088	0089	0093	0096	0100	0110	0141	0336						
RUTLX	0068	0075	0088	0089													
RWITCH	0004																
STRESS	0004	0178	0184	0293	0298	0313											
TDELTA	0016	0017	0118	0139													
TDELTA	0005	0008	0008	0016	0118												
UNLWDS	0004	0335	0361	0367													
UNMAXC	0004	0337	0361														
UNTON	0360	0361															
UNTTL	0004	0336	0360	0361	0367												
VSTART	0061	0068	0112	0282	0314	0318											
WGTAL	0132	0155	0155	0158	0175	0367											

*****FURTRAN CROSS REFERENCE LISTING*****

LABEL	DEFINED	REFERENCES
5	0156	0154
6	0024	0024
8	0159	0157
10	0248	0247
12	0211	0208
13	0092	0091
14	0208	0205
15	0209	0208
20	0193	0192
21	0195	0192 0192
30	0203	0190 0198 0201
32	0120	
34	0197	0196 0196
35	0212	0208 0210
36	0205	0204
38	0206	0205 0205
46	0235	0234 0234
47	0236	0235
48	0239	0227 0229 0232 0235 0235 0237
50	0240	0239 0239
51	0241	0240
52	0242	0240 0240
53	0245	0243 0243
54	0244	0243
55	0253	0239 0250
56	0249	0248
57	0251	0247 0248 0248
58	0269	0268
59	0355	0353 0354
60	0315	0314 0314
61	0318	0314
62	0319	0318 0318
63	0170	0161 0167
64	0164	0163
65	0168	0163 0163
66	0161	0160
67	0162	0160 0160 0161 0161
68	0290	0190 0298 0288
69	0289	0288
71	0266	0254 0254
72	0272	0268 0268
73	0254	
74	0255	0254
75	0256	0255
76	0257	0255 0255
77	0260	0259

*****FORTRAN CROSS REFERENCE LISTING*****

LABEL	DEFINED	REFERENCES
78	0263	0259 0260 0260
79	0261	0260
90	0238	0234
105	0316	0315
106	0321	0319
111	0176	0171
143	0095	0094
144	0096	0095
145	0100	0099
146	0099	0094
150	0343	0338
163	0322	0318
166	0288	0286 0286
167	0287	0286
171	0265	0262
180	0114	0113
190	0309	0306
192	0314	0300 0301 0308 0310 0310
193	0305	0304
194	0311	0310
701	0075	0354 0354
702	0079	0077 0077
703	0082	0078 0078
704	0084	0079 0079
705	0086	0079
706	0088	0081 0083 0085 0087
707	0354	0353 0353
713	0112	0098 0102 0111
801	0361	0359
802	0362	0361
803	0363	0356
804	0364	0357
805	0365	0358
2105	0340	0330
2106	0341	0331
2107	0342	0339
5003	0007	0006
5010	0005	0370
5020	0017	
5021	0018	
5022	0019	
5030	0020	
5031	0021	
5040	0025	
5041	0026	
5083	0027	

*****FORTRAN CROSS REFERENCE LISTING*****

LABEL	DEFINED	REFERENCES
5084	0029	0027
5086	0030	
5087	0031	
5088	0032	0031 0031
5090	0037	0031
5092	0038	0037 0037
5100	0039	0037
5101	0040	0039
5102	0041	0040
5103	0042	
5104	0043	0039 0039
5105	0044	0042
5106	0045	
5110	0046	
5111	0047	0046
5112	0048	
5113	0049	0005
5114	0050	0020 0032 0043
5115	0051	0025
5116	0052	0038
5117	0053	0058
5118	0054	0107
5120	0056	
5121	0058	
5140	0066	
5141	0067	
5150	0068	
5151	0071	0070 0070
5152	0069	
5153	0072	
5154	0070	
5160	0073	0070
5164	0078	0077
5165	0080	0078
5170	0091	0072 0074
5171	0093	
5172	0094	
5173	0097	
5175	0101	
5176	0103	0094
5177	0105	0104
5178	0106	
5179	0107	0106
5180	0109	0106 0106
5181	0110	0109
5183	0115	

*****FORTRAN CROSS REFERENCE LISTING*****

LABEL	DEFINED	REFERENCES
5184	0116	
5186	0117	
5190	0137	
5191	0138	
5192	0139	
5193	0140	
5194	0141	
5195	0142	
5196	0143	
5200	0144	0137
5201	0145	0138
5202	0146	0139
5203	0147	0140
5204	0148	0141
5205	0150	0142
5206	0149	0143
5218	0133	0119
5220	0152	0151
5221	0179	0151 0151
5230	0181	
5231	0182	0181
5232	0183	
5233	0184	
5234	0186	0182 0296 0311
5235	0187	0183 0297 0312
5236	0188	0184 0298 0313
5240	0189	0181 0181 0325
5241	0190	
5242	0198	
5243	0199	0198
5244	0200	0199 0199
5245	0201	
5246	0202	0199
5250	0204	
5258	0151	
5300	0214	0204 0204
5301	0215	0214 0214
5302	0217	0214
5303	0218	0217 0217
5304	0220	0217
5305	0221	0220 0220
5306	0224	0220
5400	0227	0196 0207 0213 0216 0219 0223 0226
5410	0246	
5420	0267	
5421	0268	0267

*****FORTRAN CROSS REFERENCE LISTING*****

LABEL	DEFINED	REFERENCES
5422	0273	
5423	0274	0267 0267
5424	0275	0274
5425	0276	
5427	0278	0274 0274
5429	0279	0271 0273 0277
5430	0284	0280
5439	0286	
5440	0292	
5441	0293	0292
5442	0330	0329 0329
5443	0301	0300
5444	0300	0292 0292 0295 0295
5447	0326	0323 0325 0325
5448	0327	
5449	0328	0326
5550	0331	
5551	0335	
5552	0339	0338 0338
5553	0333	0324
5555	0352	
5556	0353	0352
5558	0344	
5560	0349	0347 0347
5561	0348	0347
5562	0350	0349
5563	0351	0344 0349 0349
5570	0368	0056 0108 0317 0320 0352 0352 0366
5571	0370	
5572	0369	0368
5574	0367	0366 0366
7000	0294	0293 0293
7001	0296	0293 0295
7003	0299	
8000	0178	0177
9009	0064	0062

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COMPILER OPTICNS - NAME= MAIN,OPT=00,LINECNT=50,SCURCE,EBCDIC,NOLIST,NODECK,LOAD,NOMAP,NOEDIT,ID,XREF
ISN 0002      SUBROUTINE DRAW(WTOTAL,URUTTL,UBLWS,LT,CASE,PROB)
ISN 0003      DIMENSION URUTTL(150),UBLWS(150),YPLOT(51),XPLOT(51)
ISN 0004      5574 YPLUT(1)=WTOTAL
ISN 0005      XPLUT(1)=0.
ISN 0006      LTP1=LT+1
ISN 0007      DC 5573 IP=1,LT
ISN 0008      YPLOT(IP+1)=URUTTL(IP)/2000.
ISN 0009      5573 XPLUT(IP+1)=UBLWS(IP)
ISN 0010      YMAX=YPLUT(LTP1)
ISN 0011      N2=N2
ISN 0012      IF(YMAX.LE.400.) GO TO 3
ISN 0014      IF(YMAX.LE.800.) GO TO 4
ISN 0016      IF(YMAX.LE.1600.) GO TO 5
ISN 0018      IF(YMAX.LE.3200.) GO TO 6
ISN 0020      3 DY=50.
ISN 0021      GC TO 10
ISN 0022      4 DY=100.
ISN 0023      GC TO 10
ISN 0024      5 DY=200.
ISN 0025      GC TO 10
ISN 0026      6 DY=400.
ISN 0027      10 DX=10.
ISN 0028      PPRUN=PROB
ISN 0029      RETURN
ISN 0030      END
    
```

***** FORTRAN CROSS REFERENCE LISTING *****

SYMBOL	INTERNAL STATEMENT NUMBERS
DX	0027
DY	0020 0022 0024 0026
IP	0007 0008 0008 0009 0009
LT	0002 0006 0007
N2	0011 0011
CASE	0002
DRAW	0002
LTP1	0006 0010
PROB	0002 0028
YMAX	0010 0012 0014 0016 0018
PPRUN	0028
XPLUT	0003 0003 0009
YPLUT	0003 0004 0008 0010
UBLWS	0002 0003 0009
URUTTL	0002 0003 0008
WTOTAL	0002 0004

***** FORTRAN CROSS REFERENCE LISTING *****

LABEL	DEFINED	REFERENCES
3	0020	0012
4	0022	0014
5	0024	0016
6	0026	0018
10	0027	0021 0023 0025
5573	0009	0007
5574	0004	

***** END OF COMPILATION *****

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```

COMPILER OPTICNS - NAME= MAIN,OPT=00,LINECNT=50,SCURCE,EBCDIC,NOLIST,NODECK,LOAD,NOMAP,NOEDIT,ID,XREF
ISN 0002      SUBROUTINE DELTCK(NPASS,TDELTA,P,W,K,TDELTA,DELTA,N2)
ISN 0003      REAL K,NPASS
ISN 0004      INTEGER P,PLESS1
ISN 0005      DIMENSION W(150),K(150),DELTA(300)
ISN 0006      PLESS1=P-1
ISN 0007      N2=N2-1
ISN 0008      SUM=0.
ISN 0009      TMIN=1.
ISN 0010      TDELTA=TDELTA
ISN 0011      DELTA=1./TDELTA
ISN 0012      DC 1 M=1,PLESS1
ISN 0013      DELTA(M)=SQRT(W(M)/K(M))/19.648
ISN 0014      NN=PLESS1*M
ISN 0015      1 DELTA(NN)=SQRT(W(M)/K(M))/19.648
ISN 0016      IF(K(P).GT.0.) GO TO 2
ISN 0018      DELTA(NN)=1.0
ISN 0019      GC TO 3
ISN 0020      2 DELTA(NN)=SQRT(W(P)/K(P))/19.648
ISN 0021      3 DC 4 M=1,N
ISN 0022      4 TMIN=AMIN1(TMIN,DELTA(M))
ISN 0023      IF(TMIN/2.-DELTA(M).LE.6.6
ISN 0024      5 DELTA=TMIN/2.
ISN 0025      TDELTA=1.0/DELTA
ISN 0026      6 DC 7 M=1,N
ISN 0027      7 SUM=SUM+DELTA(M)
ISN 0028      N2=4.0*SUM/(2.0*DELTA)
ISN 0029      RETURN
ISN 0030      END
    
```

***** J K T R A N C R O S S R E F E R E N C E L I S T I N G *****

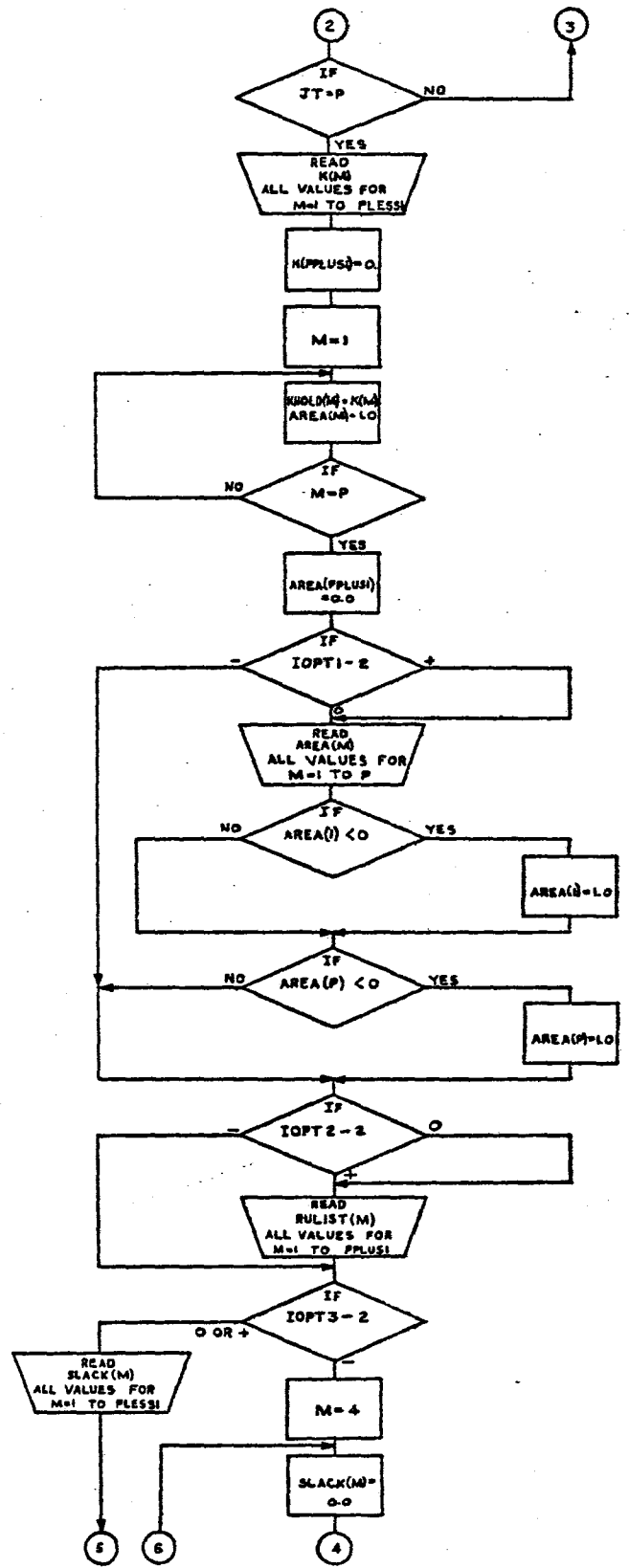
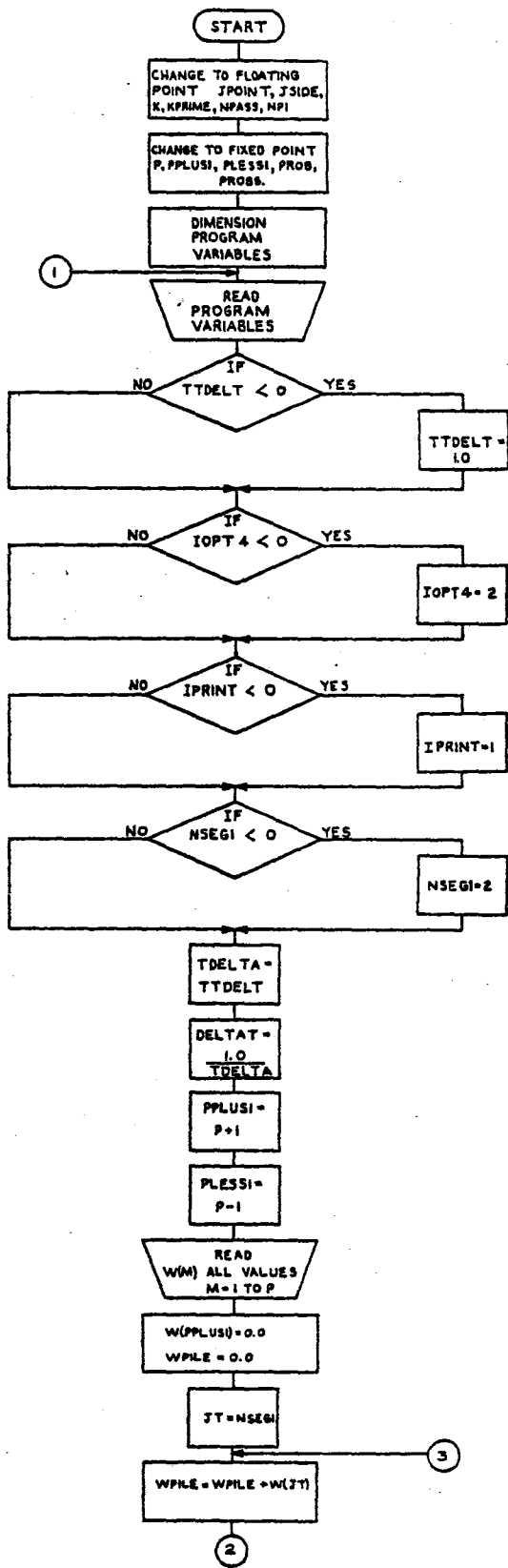
SYMBOL	INTERNAL STATEMENT NUMBERS										
K	0007	0003	0005	0013	0015	0016	0020				
M	0012	0013	0013	0013	0014	0015	0015	0021	0022	0026	0027
N	0007	0019	0020	0021	0026						
P	0002	0004	0026	0007	0016	0020					
W	0007	0005	0013	0014	0020						
N1	0014	0015									
N2	0002	0028									
SUM	0008	0027	0027	0028							
SGRT	0013	0015	0020								
TMIN	0009	0022	0022	0023	0024						
AMIN1	0022										
DELTA	0005	0013	0015	0019	0020	0022	0027				
MPASS	0002	0003									
DELTA1	0002	0011	0023	0024	0025	0028					
DELTA2	0002										
PLLS1	0004	0006	0012	0014							
TDELTA	0002	0010	0011	0025							
TDELTA	0002	0010									

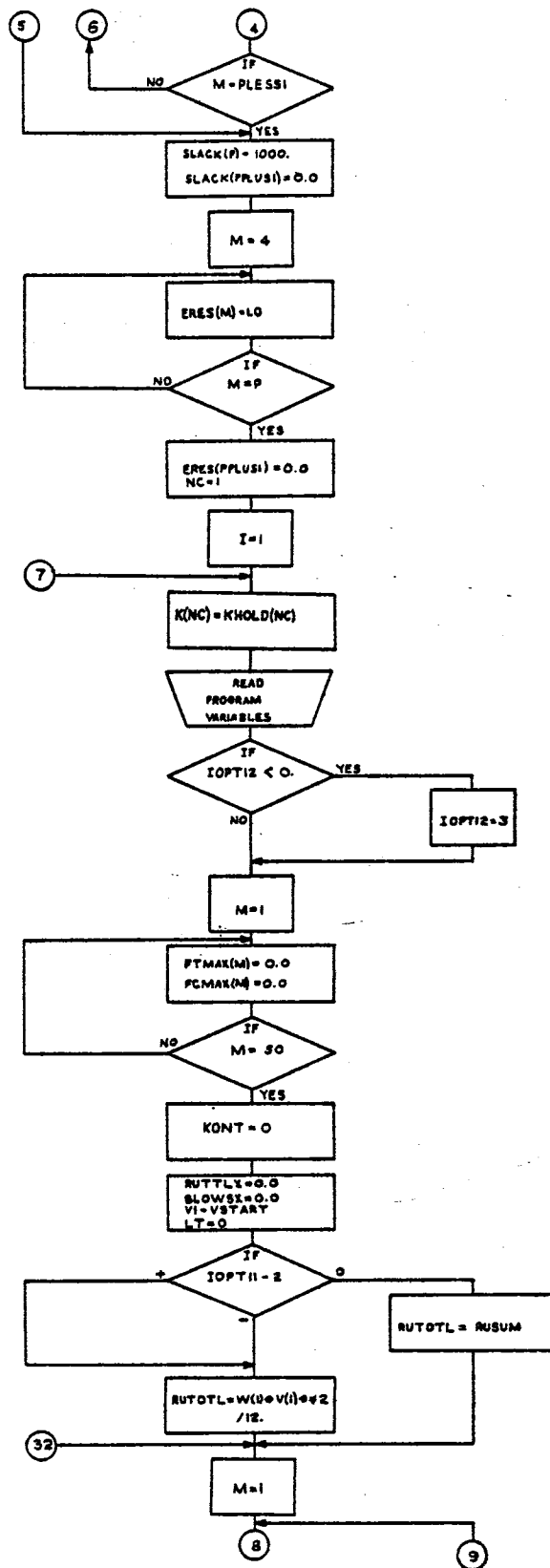
***** J K T R A N C R O S S R E F E R E N C E L I S T I N G *****

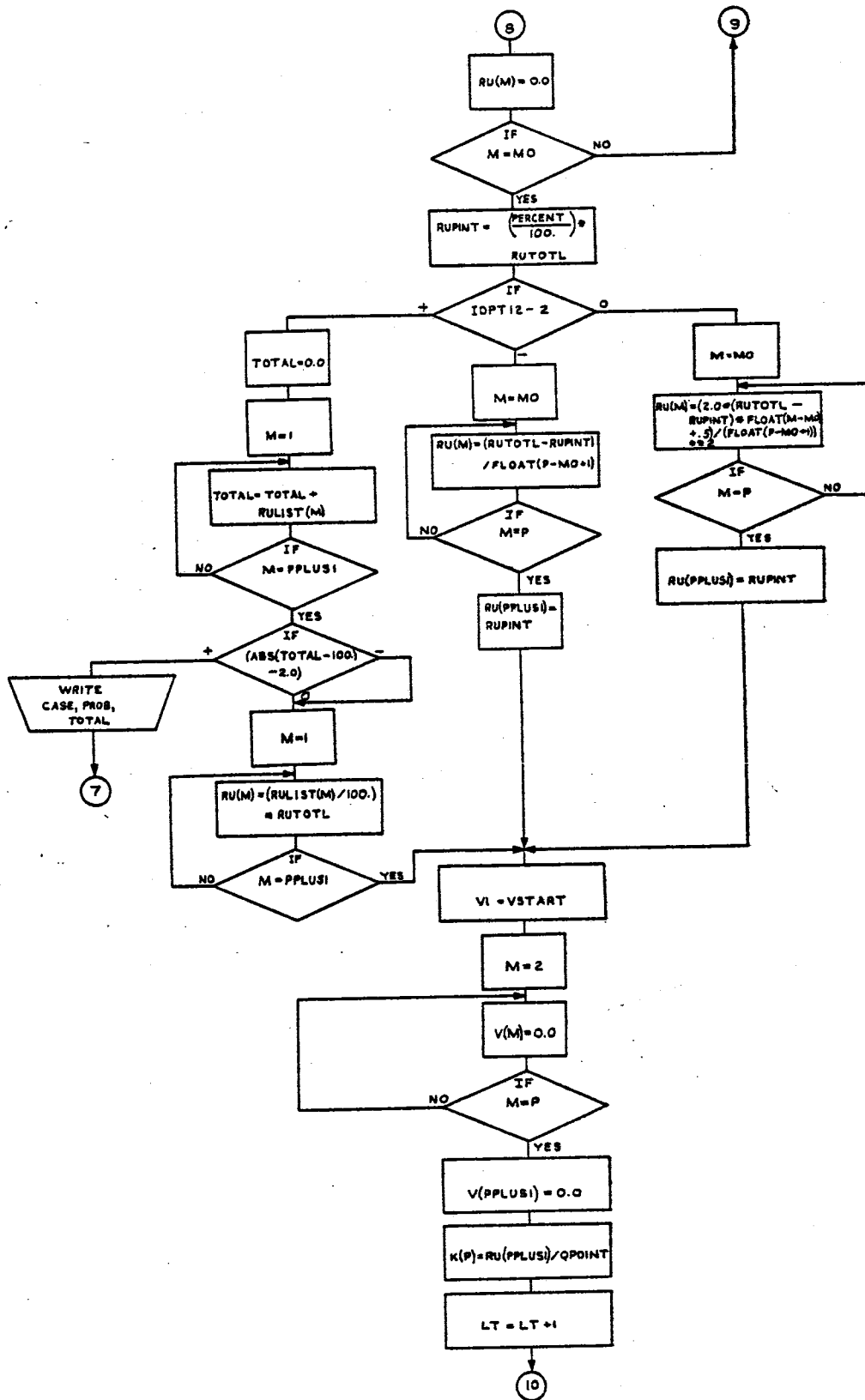
LABEL	DEFINED	REFERENCES
1	0015	0012
2	0020	0016
3	0021	0019
4	0022	0021
5	0024	0023
6	0026	0023 0023
7	0027	0026

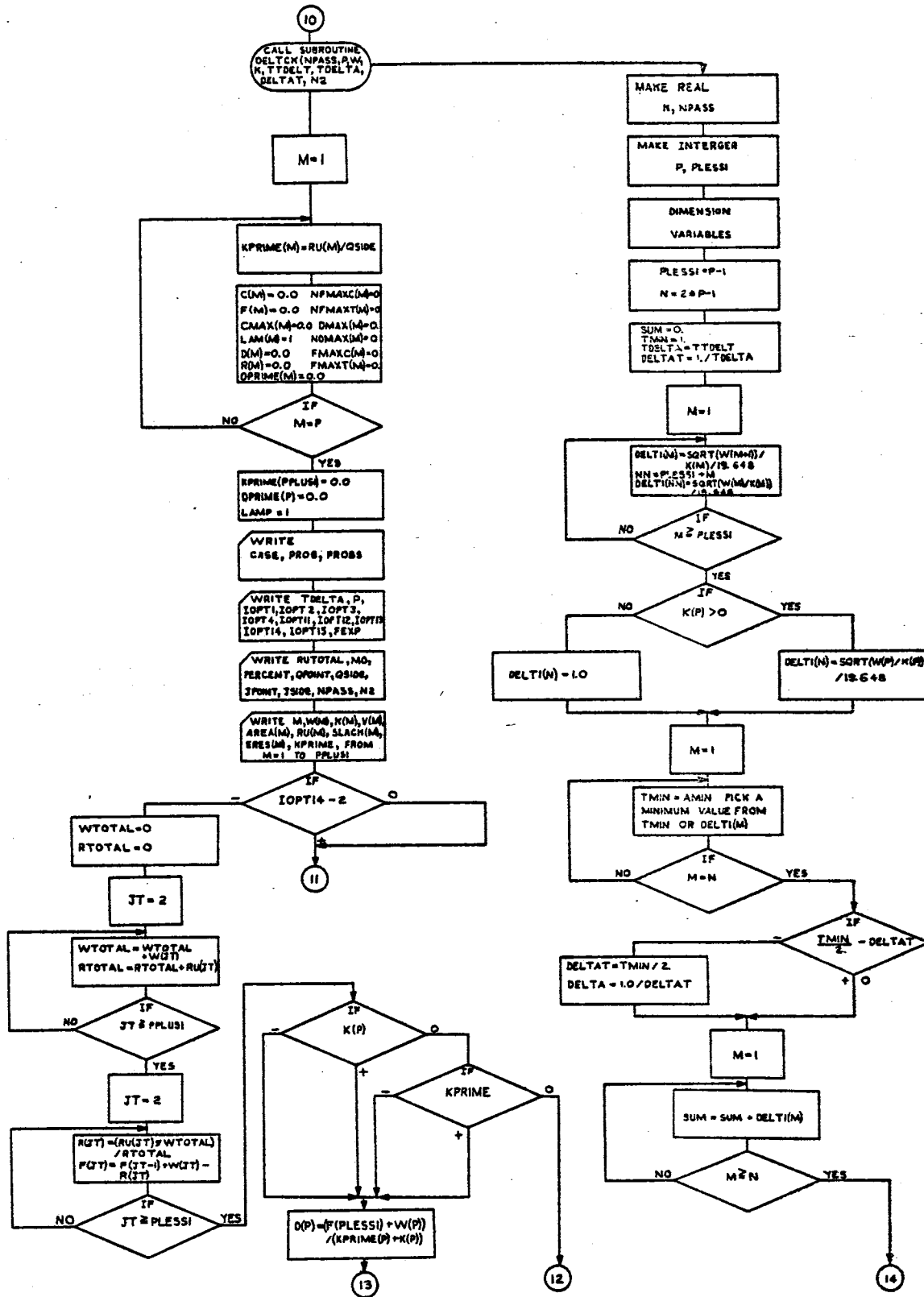
***** END OF COMPILATION *****

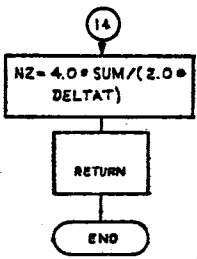
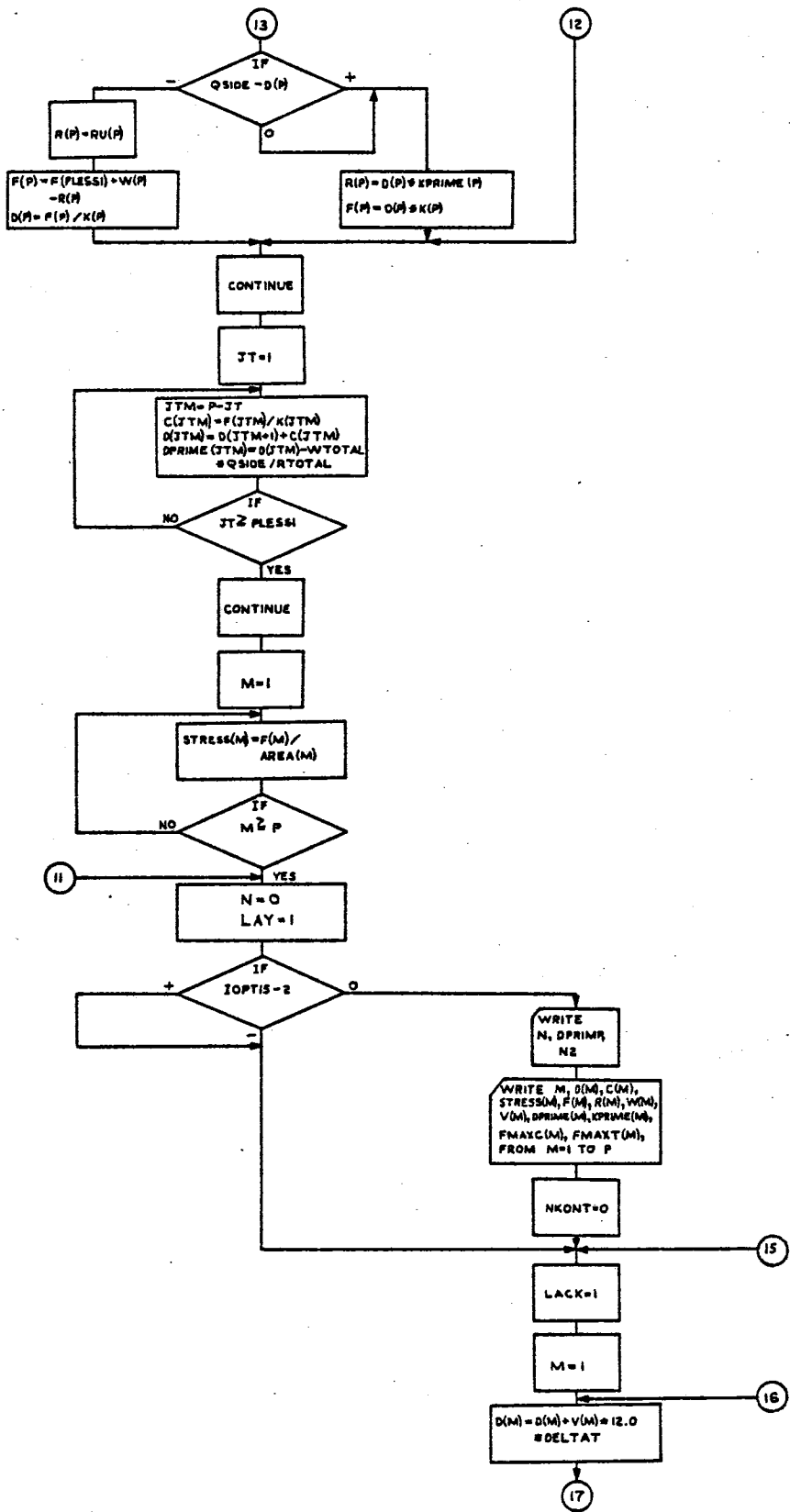
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IEF285I	VOL SER NOS= 555555.	
IEF285I	SYS68134.T145405.RP001.A49399.R0000445	DELETED
IEF285I	VOL SER NOS= STUBAU.	
IEF285I	SYSOUT	SYSOUT
IEF285I	VOL SER NOS=	
IEF285I	SYS68134.T145405.RP001.A49399.LCA0SET	PASSED
IEF285I	VOL SER NOS= 666666.	
IEF285I	SYS68134.T145405.RP001.A49399.LCA0SET	DELETED
IEF285I	VOL SER NOS= 666666.	

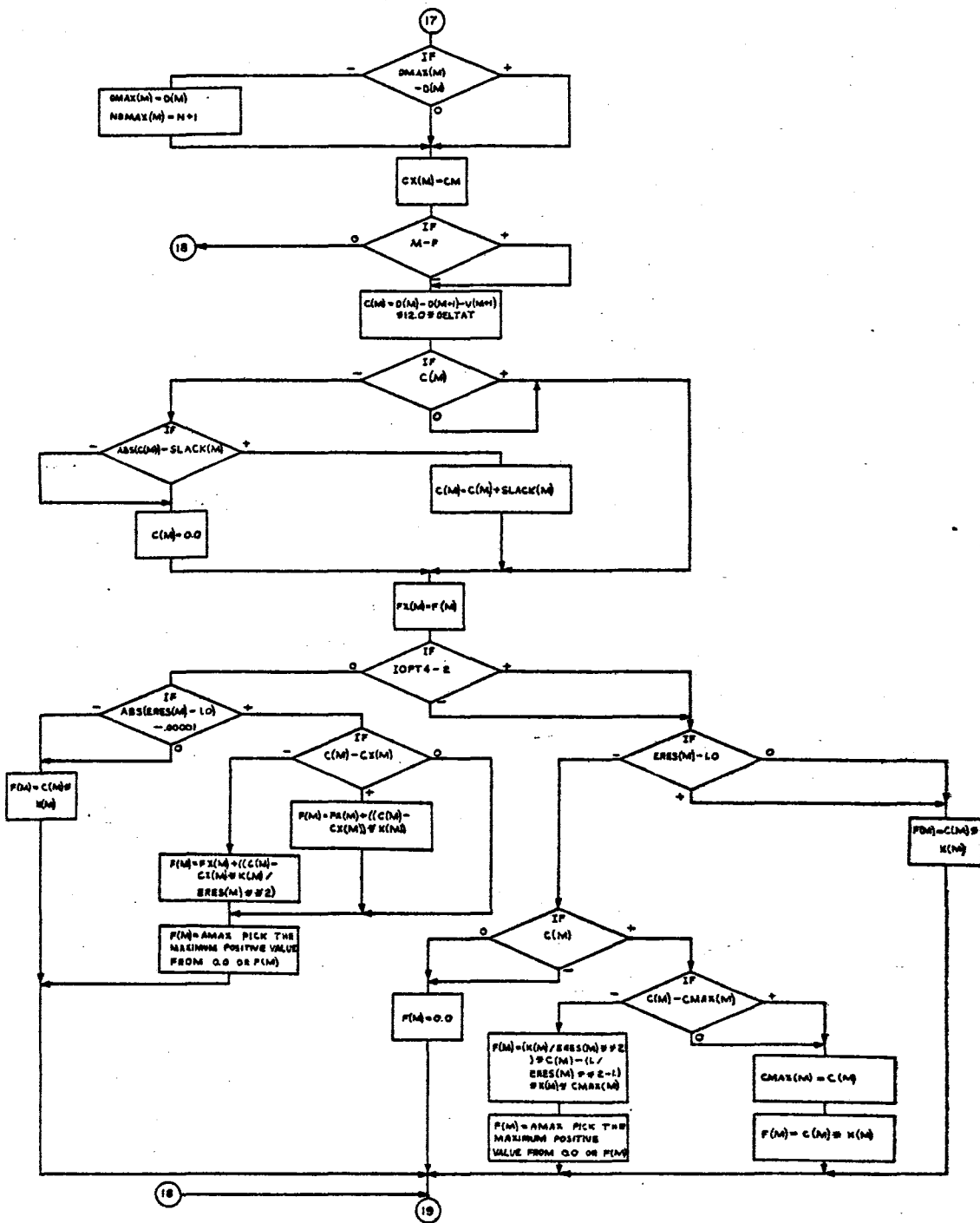


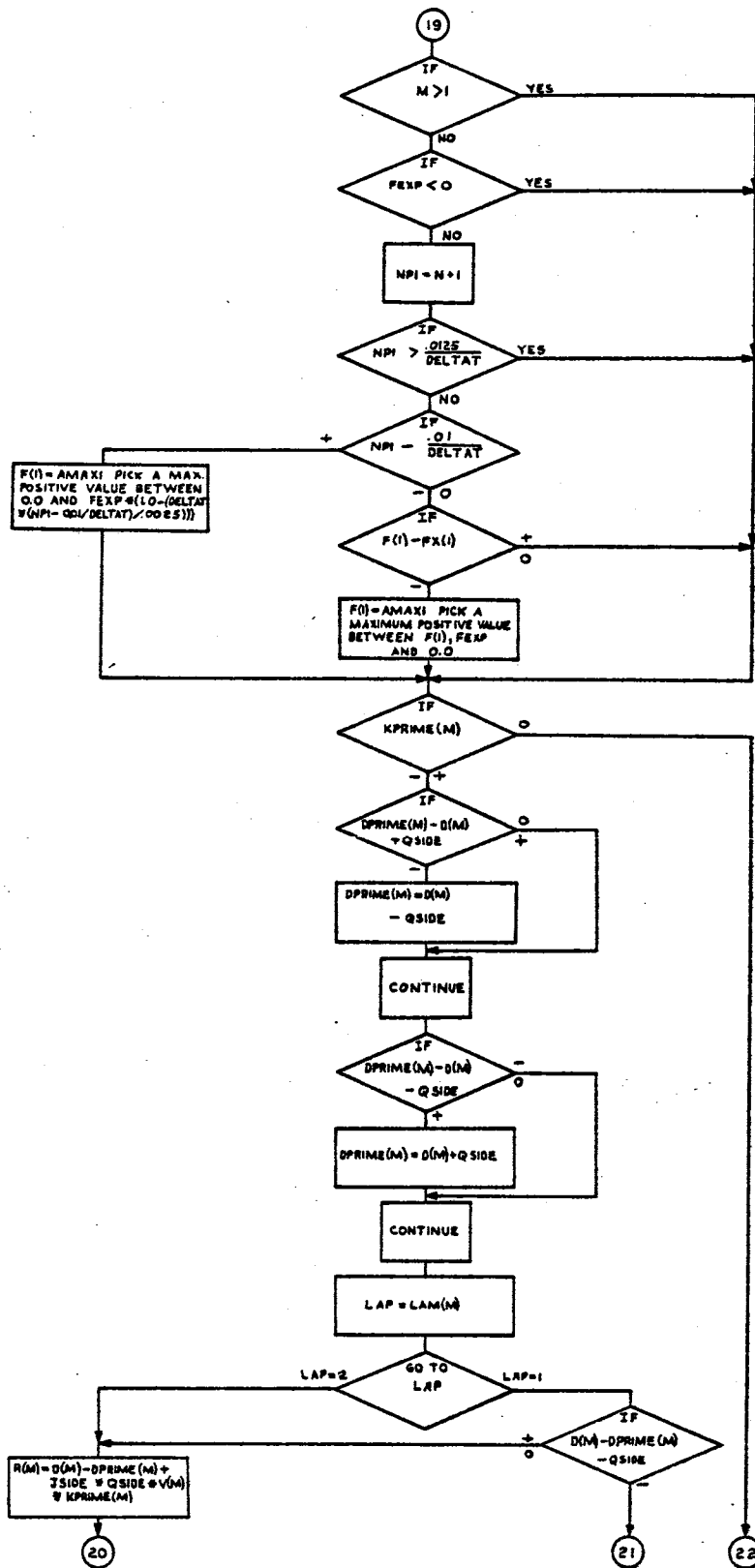


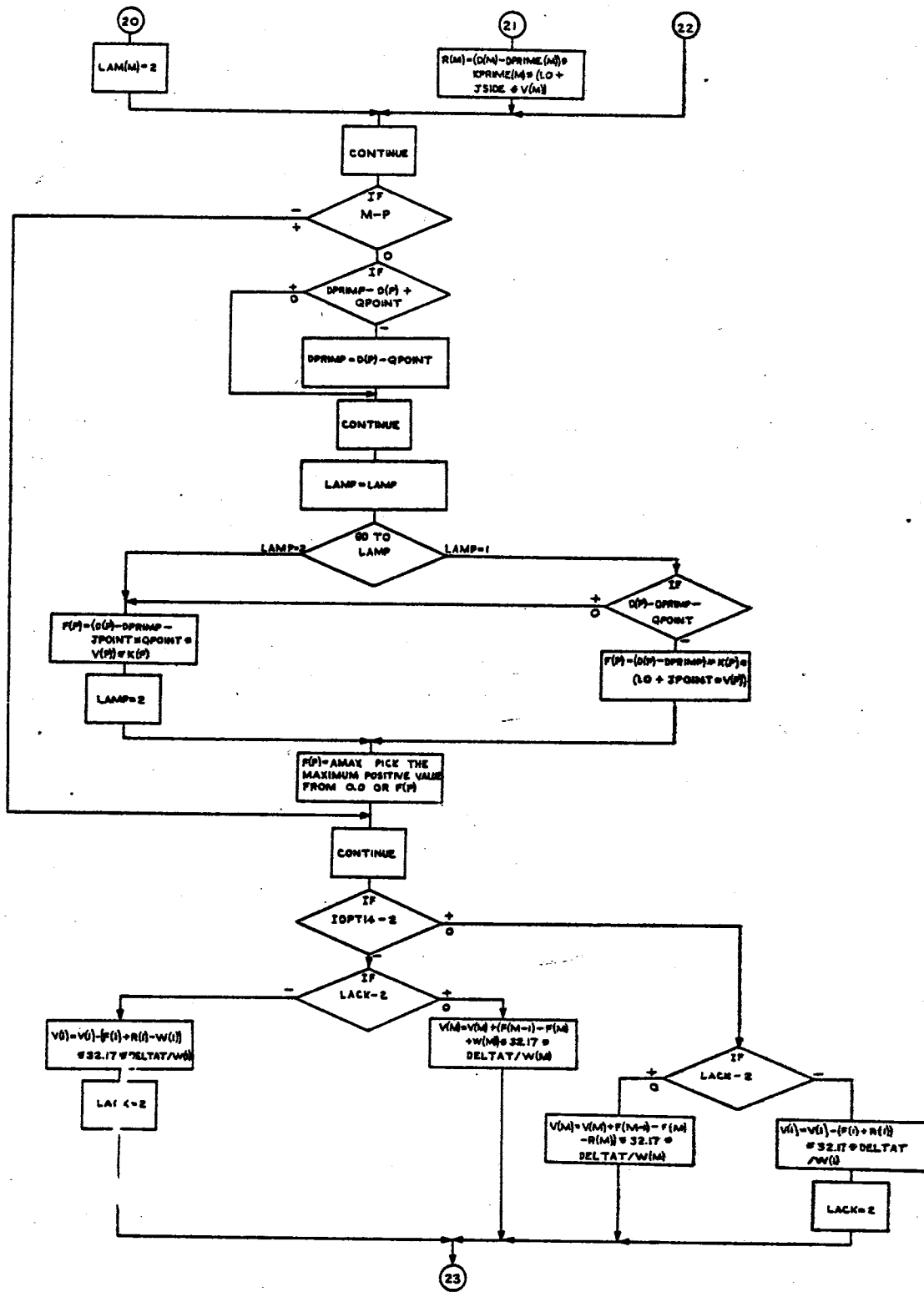


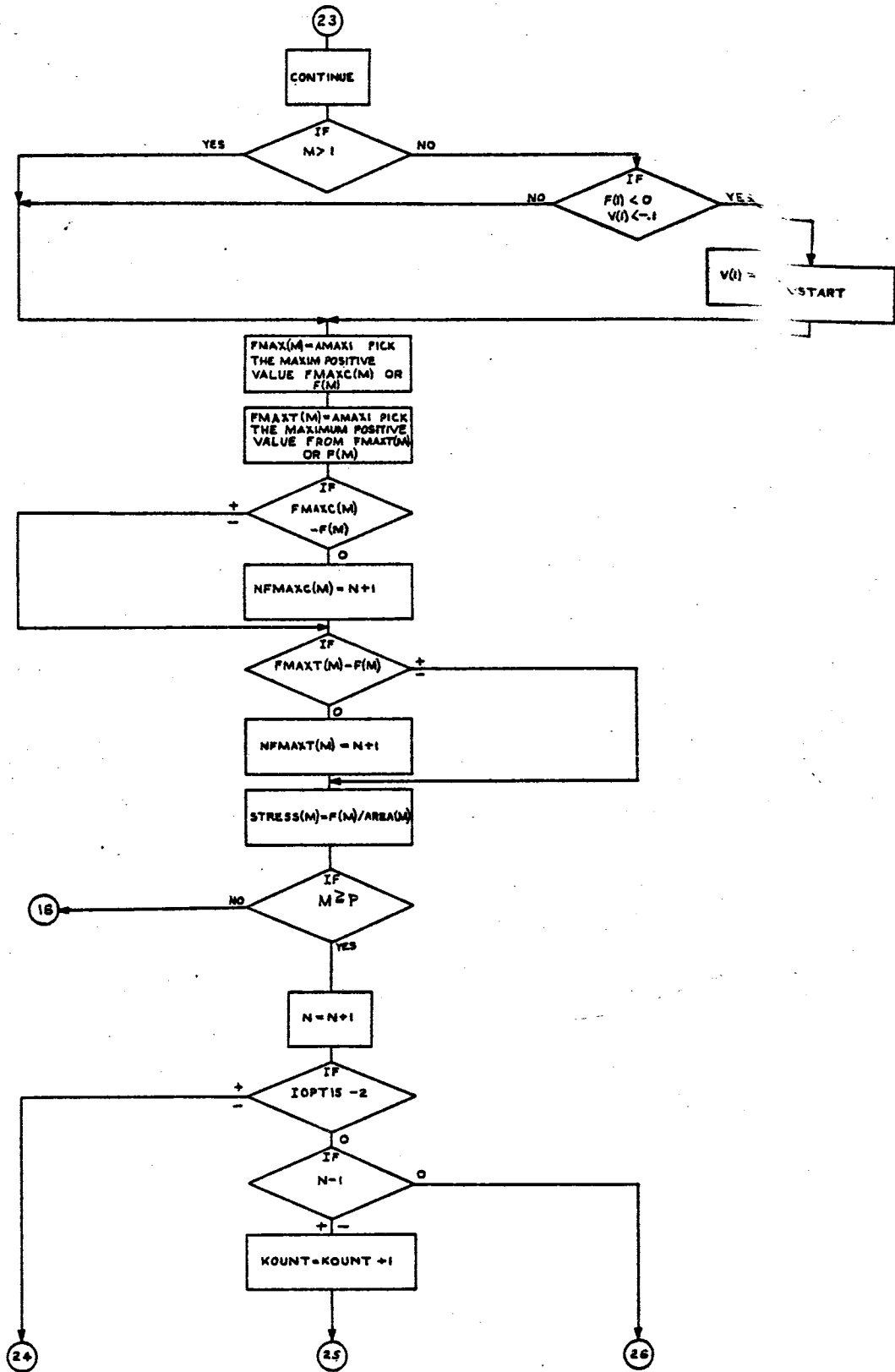


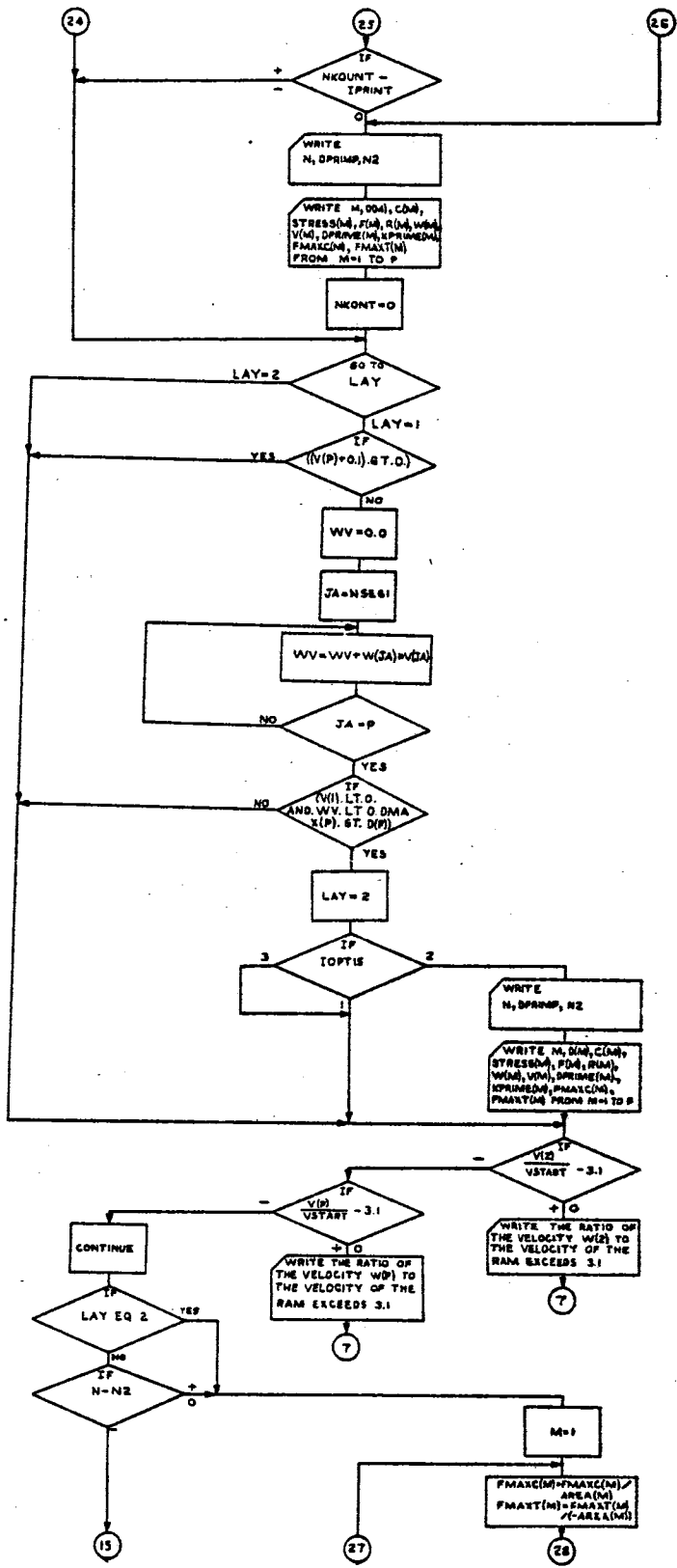


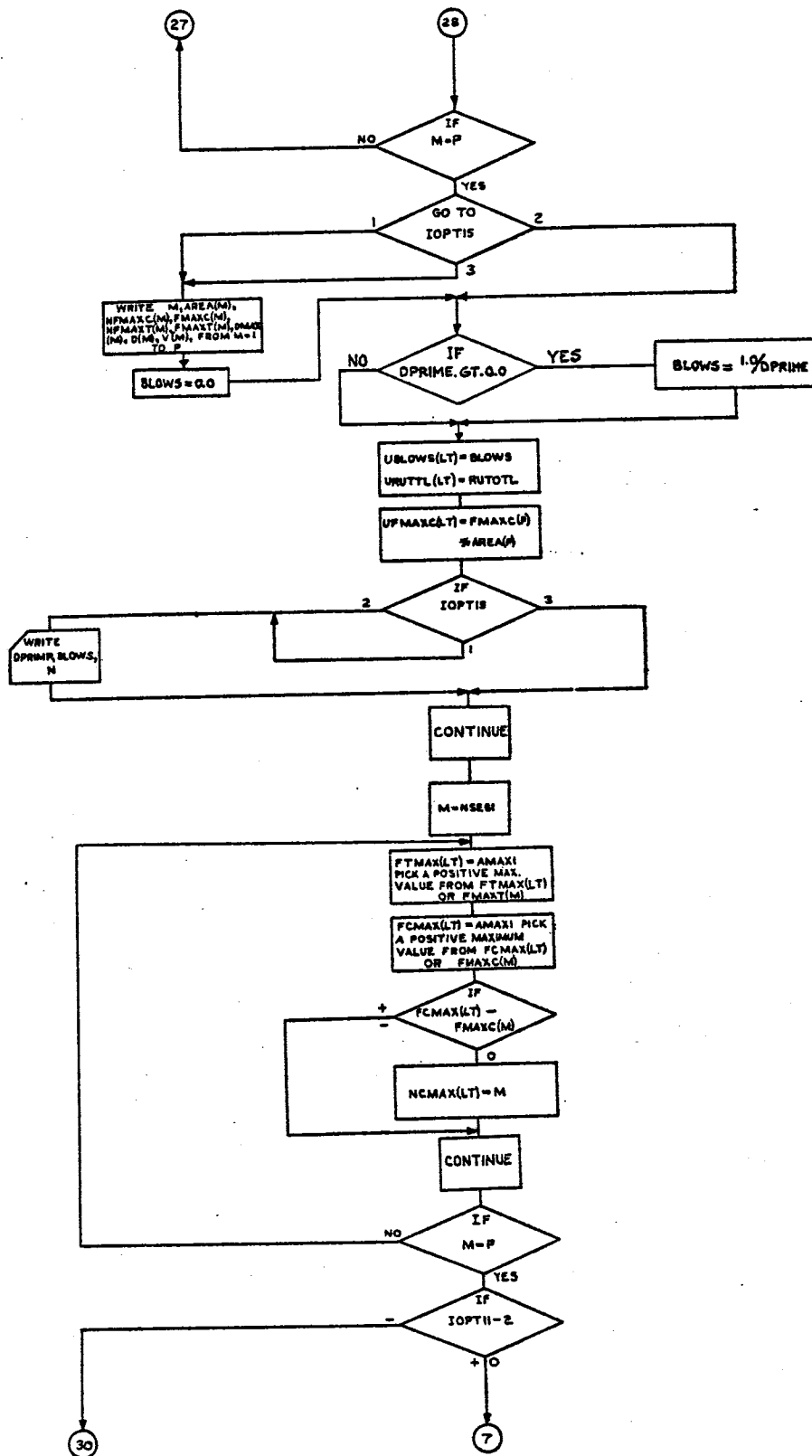


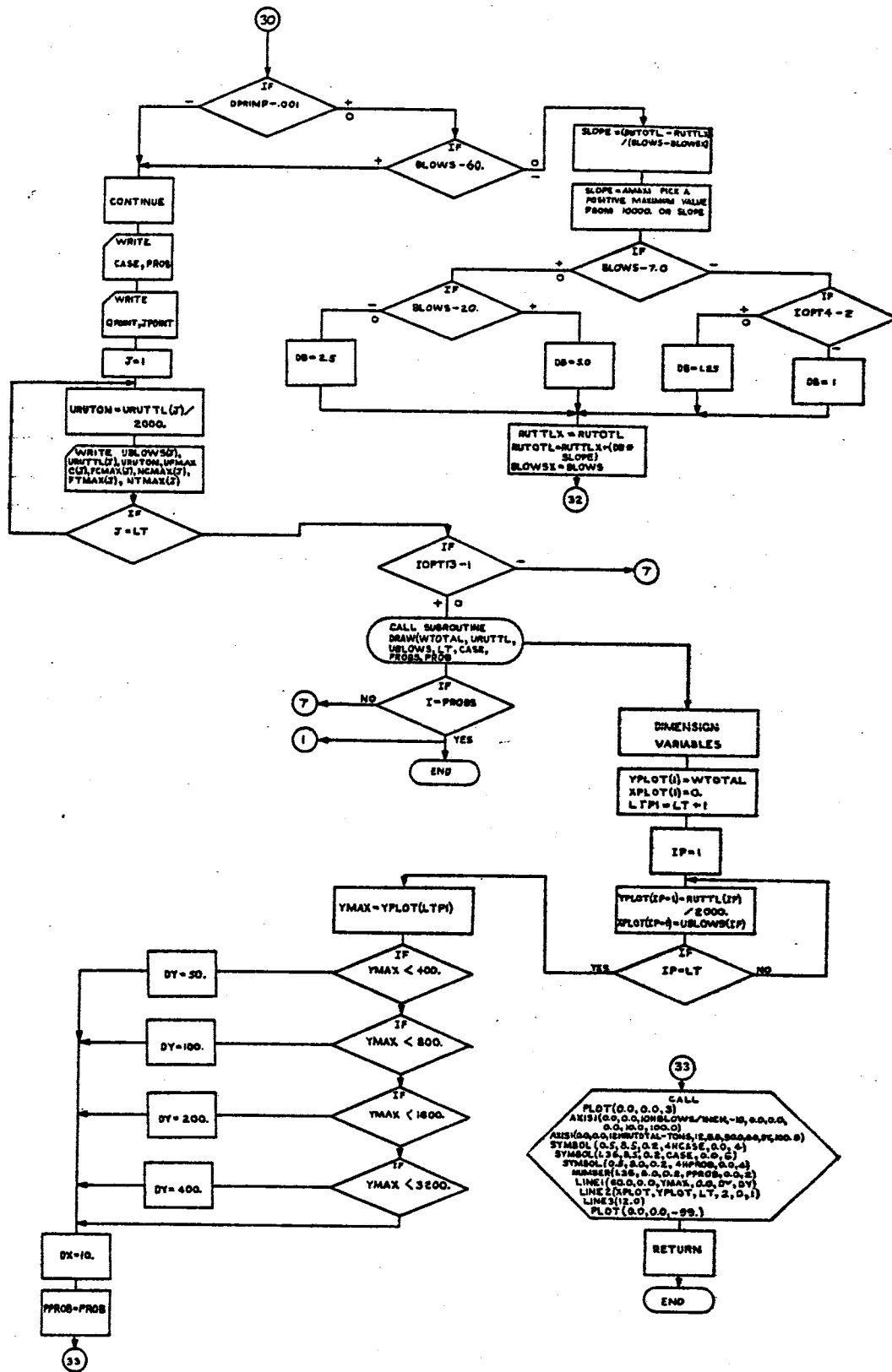












```

SUBROUTINE DELTCK(NPASS,TTDELT,P,W,K,TDELTA,DELTAT,N2)
REAL K,NPASS
INTEGER P,PLESS1
DIMENSION W(150),K(150),DELT1(300)
PLESS1=P-1
N=2*P-1
SUM=0.
TMIN=1.
TDELTA=TTDELT
DELTAT=1./TDELTA
DO 1 M=1,PLESS1
DELT1(M)=SQRT(W(M+1)/K(M))/19.648
NN=PLESS1+M
1 DELT1(NN)=SQRT(W(M)/K(M))/19.648
DELT1(N)=1.0
IF(K(P).GT.0.) DELT1(N)=SQRT(W(P)/K(P))/19.648
3 DO 4 M=1,N
4 TMIN=AMIN1(TMIN,DELT1(M))
IF(TMIN/2.-DELTAT)5,6,6
5 DELTAT=TMIN/2.
TDELTA=1.0/DELTAT
IF(K(P).EQ.0.0) DELT1(N)=0.0
6 DO 7 M=1,N
7 SUM=SUM+DELT1(M)
N2=4.0*SUM/(2.0*DELTAT)
RETURN
END

```

```

SUBROUTINE DELTCK(NPASS, TTDELT, P, W, K, TDELTA, DELTAT, N2)
REAL K, NPASS
INTEGER P, PLESS1
DIMENSION W(150), K(150), DELT1(300)
PLESS1=P-1
N=2*P-1
SUM=0.
TMIN=1.
TDELTA=TTDELT
DELTAT=1./TDELTA
DO 1 M=1, PLESS1
DELT1(M)=SQRT(W(M+1)/K(M))/19.648
NN=PLESS1+M
1 DELT1(NN)=SQRT(W(M)/K(M))/19.648
DELT1(N)=1.0
IF(K(P).GT.0.) DELT1(N)=SQRT(W(P)/K(P))/19.648
3 DO 4 M=1, N
4 TMIN=AMIN1(TMIN, DELT1(M))
IF(TMIN/2.-DELTAT) 5, 6, 6
5 DELTAT=TMIN/2.
TDELTA=1.0/DELTAT
IF(K(P).EQ.0.0) DELT1(N)=0.0
6 DO 7 M=1, N
7 SUM=SUM+DELT1(M)
N2=4.0*SUM/(2.0*DELTAT)
RETURN
END

```