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16. Abstract This report presents a new approach of determining the damage that overweight vehicles can do to light pavement structures. This computerized procedure uses results obtained from the Dynaflect or the Falling Weight Deflectometer to determine the number of passes of a specific load that will cause a critical level of rut depth in a light pavement structure. This method was based on field observations and ILLI-PAVE, a finite element pavement analysis program. In the study, hyperbolic curve is used to describe both the stress softening and stress hardening form of load-deflection characteristics observed on light pavements. A method of determining the nonlinear elastic material models for the base course and the subgrade using the Falling Weight Deflectometer or the Dynaflect was developed. From the data collected with the Pavement Dynamic Cone Penetrometer, it appears that the stiffness of the granular base course depends to a large extent on the degree of compaction of the material. The model adopted for repetitive loading follows a hyperbolic-shaped loading and reloading load deflection curve with a linear unloading path. Thick pavement which is usually the stress hardening type appears to be more resistant to rutting. The new approach is shown to be accurate in predicting the development of rut depth with repeated loads applied by a variety of different vehicles. A computer program is written to incorporate the complete analysis method and a convenient data coding form is provided to make data entry more convenient.		13. Type of Report and Period Covered Interim - September 1979 December 1983	
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LOAD RATING OF LIGHT PAVEMENT STRUCTURES

by

Koon Meng Chua and Robert L. Lytton

Research Report No. 284-6F

Flexible Pavement Data Base and Design

Research Study 2-8-80-284

Conducted for

The Texas State Department of Highways
and Public Transportation

in cooperation with the
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by the

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ABSTRACT

This report presents a new approach of determining the damage that overweight vehicles can do to light pavement structures. This computerized procedure uses results obtained from the Dynaflect or the Falling Weight Deflectometer to determine the number of passes of a specific load that will cause a critical level of rut depth in a light pavement structure. This method was based on field observations and ILLI-PAVE, a finite element pavement analysis program.

In the study, a hyperbolic curve is used to describe both the stress softening and stress hardening form of load-deflection characteristics observed on light pavements. A method of determining the nonlinear elastic material models for the base course and the subgrade using the Falling Weight Deflectometer or the Dynaflect was developed. From the data collected with the Pavement Dynamic Cone Penetrometer, it appears that the stiffness of the granular base course depends to a large extent on the degree of compaction of the material. The model adopted for repetitive loading follows a hyperbolic-shaped loading and reloading load deflection curve with a linear unloading path. Thick pavement which is usually the stress hardening type appears to be more resistant to rutting. The new approach is shown to be accurate in predicting the development of rut depth with repeated loads applied by a variety of different vehicles.

A computer program is written to incorporate the complete analysis method and a convenient data coding form is provided to make data entry more convenient.

SUMMARY

An increase in volume of overweight vehicle permit applications has caused the Texas Highway Department to look for a more efficient way of determining the damage that can be done to light pavement structures.

A new approach is presented here. This computerized procedure uses results obtained from non-destructive testing methods, namely, the Dynaflect or the Falling Weight Deflectometer to determine the number of passes of a specific set of loads that will cause a critical level of rut depth in a light pavement structure. Conversely, the maximum allowable load can be determined using the rut depth as a criterion for unacceptability. This method was based on field observations and ILLI-PAVE, a finite element pavement analysis program.

In the study, it is found that a hyperbolic curve can be used to describe both the stress softening and stress hardening form of load-deflection characteristics observed on light pavements. It is shown that nonlinear elastic material models for the base course and the subgrade can be determined from the Falling Weight Deflectometer or the Dynaflect. From the data collected with the Pavement Dynamic Cone Penetrometer, it appears that the stiffness of the granular base course depends largely on the degree of compaction of the material. The model adopted for repetitive loading follows a hyperbolic-shaped loading and reloading load-deflection curve with a linear unloading path. Thick pavement which is usually the stress hardening type appears to be more resistant to rutting.

The new approach is shown to be accurate in predicting the development of rut depth with repeated loads applied by a variety of different vehicles.

A computer program is written to incorporate the complete analysis method and a convenient data coding form is provided to make data entry more convenient. A number of example problems are worked to illustrate the use of the program. With the aid of the program, and having in hand field deflection data and the thickness of the base course, it is possible to do the following: (a) determine the maximum load that can be carried by a particular pavement; (b) determine how many passes of a specified vehicle will cause a particular pavement to have an unacceptable level of rutting; (c) determine the effect on rutting of a particular pavement that a specified traffic stream will have.

These capabilities provide the Texas SDHPT with a versatile tool for load rating and load zoning the low volume roads in the State of Texas.

IMPLEMENTATION STATEMENT

This report describes the development of a new load rating method for light pavement structures. The computer program uses results obtained from the Dynaflect or the Falling Weight Deflectometer and can be used exactly as it is presented in this report to determine what is the maximum load a particular pavement can carry and also how many passes of a specified vehicle will cause an unacceptable level of rut depth. The program can be used with new pavements or pavements that already show evidence of rutting. The program input is simple and straight-forward and is expected to be useful to D-18 and D-8 immediately.

DISCLAIMER

The contents of this report reflect the view of the authors who are responsible for the facts and the accuracy of the data presented within. The contents do not necessarily reflect the official views or policies of the Federal Highway Administration. This report is not a standard, a specification or a regulation.

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CHAPTER I

INTRODUCTION

Overweight truck operations in the State of Texas have increased from 7.75% in 1974 to 26.33% in 1976 (1) and this trend is still true at the present time. As a result of the increasing industrial and agricultural activities, heavier trucks and higher traffic volume require the Texas State Department of Highways and Public Transportation (SDHPT) to look into the problem of load zoning of various Farm-to-Market roads which are of the light pavement structure type.

With regard to Farm-to-Market roads in Texas, studies have shown (2) that the Gross Vehicle Weight [GVW] of trucks can range from 33,000 lbs to over 80,000 lbs of which the latter contributes as much as 59% of truck traffic. Many studies (3) are being conducted on the effects of truck size and weight on pavements by various states and the results show that the economic implication is significant.

In evaluating overweight vehicle permit applications, the present practice of the Texas SDHPT is to determine the gross allowable loads on the light pavement structure by testing on undisturbed samples of the subgrade. Texas Triaxial Tests (4) are performed on cored samples. This method requires a considerable amount of labor in the laboratory and the coring process also interrupts traffic. It is obvious that a more efficient method of determining damage to pavement by overweight vehicles is needed.

Presently, no method of load rating of light pavement structures such as the one proposed here has been developed. Among the states that have done load rating of some sort, the AASHO Road Tests results or the AASHO Interim Guide (5) is often consulted.

This report presents a new method which will alleviate the above-mentioned problem. The new approach is a computerized procedure which uses results obtained from non-destructive testing methods, namely, the Dynaflect or the Falling Weight Deflectometer [FWD], to determine the number of passes of a specified load that will cause a critical level of rut depth in a light pavement structure.

Conversely, the maximum allowable load on a light pavement structure can be determined using the rut depth as a criterion for unacceptability. Rut depths are caused by accumulating pavement deformation under repeated load applications. Each time a load passes, the pavement fails to rebound as much as it was deflected under load. Establishing the difference between the loading and the unloading path is critical to making a reliable prediction of this rut depth. Some of the advantages of the new approach are:

1. Non-destructive testing will reduce the time and manpower currently required to determine the maximum load allowed on a pavement, will expedite permit evaluation, and will reduce the costs of the overall process.

2. Estimating the maximum allowable number of applications of load on a pavement will assist in planning and budgeting decisions that are related to patterns of future development.

3. The method will assist in evaluating the economic impact of load intensive industries upon the local road maintenance and rehabilitation budget.

This report is divided into five chapters and three appendices. The first chapter serves as an introduction. The second chapter describes the location of the test sites used in the study and the characteristics of the test sections. It also gives a detailed description of the functioning and the use of the Falling Weight Deflectometer and the Dynaflect. The third chapter gives an account of the analysis approach taken. It describes the finite element program ILLI-PAVE and the material models that were used in the computer analysis and also how deflection basins were generated to verify the adequacy of the program as well as to form a data pool to formulate the load rating procedure. It further describes the load deflection model assumed, the load rating model or rutting model and also the curve fitting techniques used in this study. The fourth chapter describes, discusses and also gives an evaluation of the proposed load rating procedure. The computer program written for this purpose is also introduced. The final chapter includes the conclusions and recommendations that arise out of this study.

Appendix A lists the data from non-destructive testing of the selected pavement sections using the Falling Weight Deflectometer and the Dynaflect. Appendix B includes the data obtained from a previous study to formulate a multiplier used in the load deflection model of pavements under repetitive loading. Appendix C gives the

documentation of the computer program. This includes the program flow-charts, the input instructions, the program listing, a sample input and also a sample output. Appendic D includes the coding forms used with the computer program. Input is self-explanatory.

CHAPTER II

DATA COLLECTION

This task involved the non-destructive testing of 78 pavement sections from 14 Farm-to-Market roads using the Dynaflect and the Falling Weight Deflectometer. In addition, construction drawings were referred to for those sections tested. These data formed the basis for the development of the determination of the load deflection model using the two non-destructive testing methods.

Location of Test Sites

The State of Texas consists of 254 counties divided into 25 highway districts, as shown in Figure 1. In view of the size of the state, a wide variation in climate can be expected. Average annual rainfall varies from about 8 inches at El Paso in West Texas to about 56 inches at the extreme east of the state (6). About 38 inches is recorded for the Bryan-College Station area. Average annual temperature ranges from 53⁰ F in the northwestern edge of the High Plains to 74⁰F along the Rio Grande in the southernmost section of the state. The pavement sections tested are located in the Brazos and the Burleson Counties of District 17 for the reasons that they are moderately representative of the State as well as their proximity to the TTI. Figures 2 and 3 show the portions of the Farm-to-Market roads that were tested.

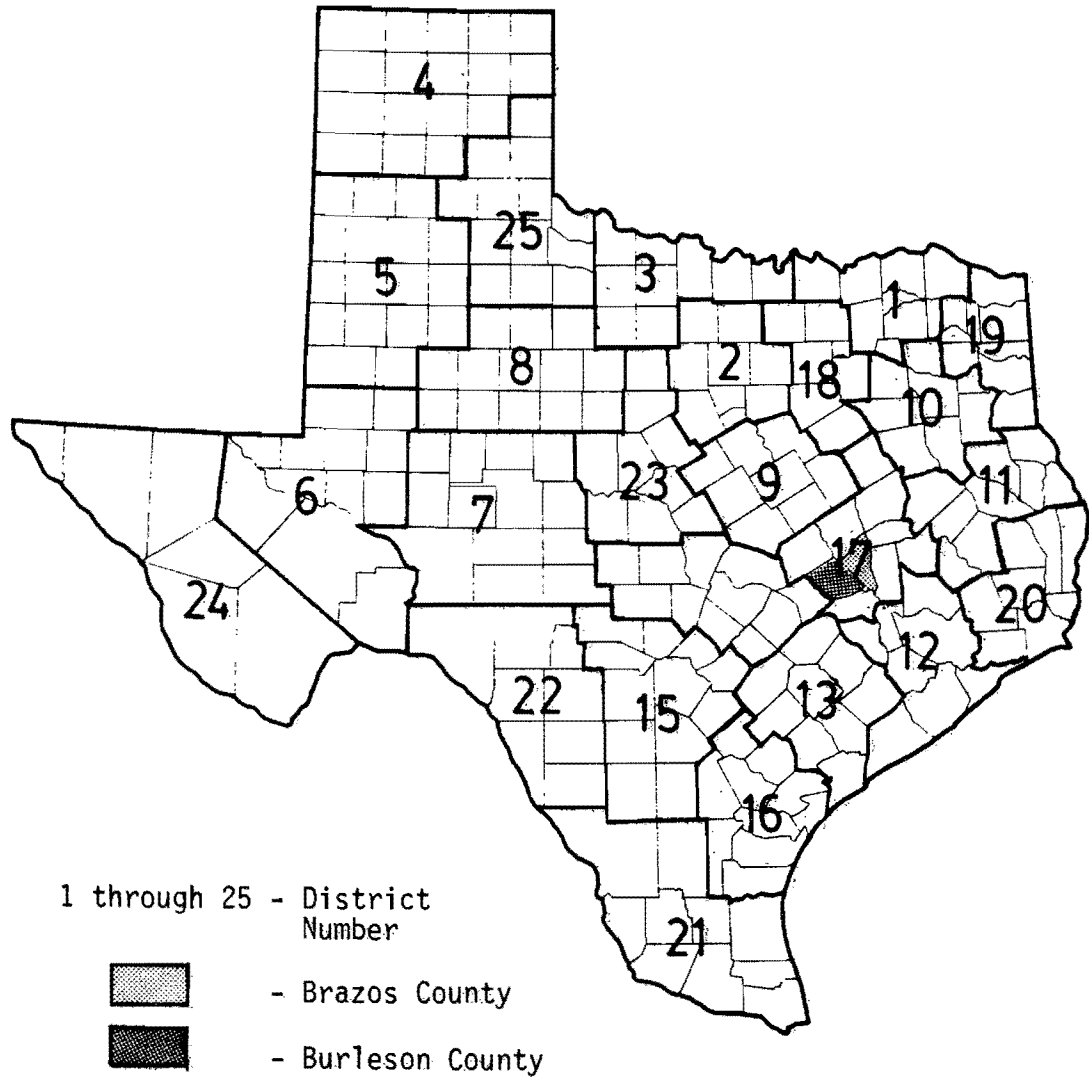


FIGURE 1. Texas District and County Outline Map

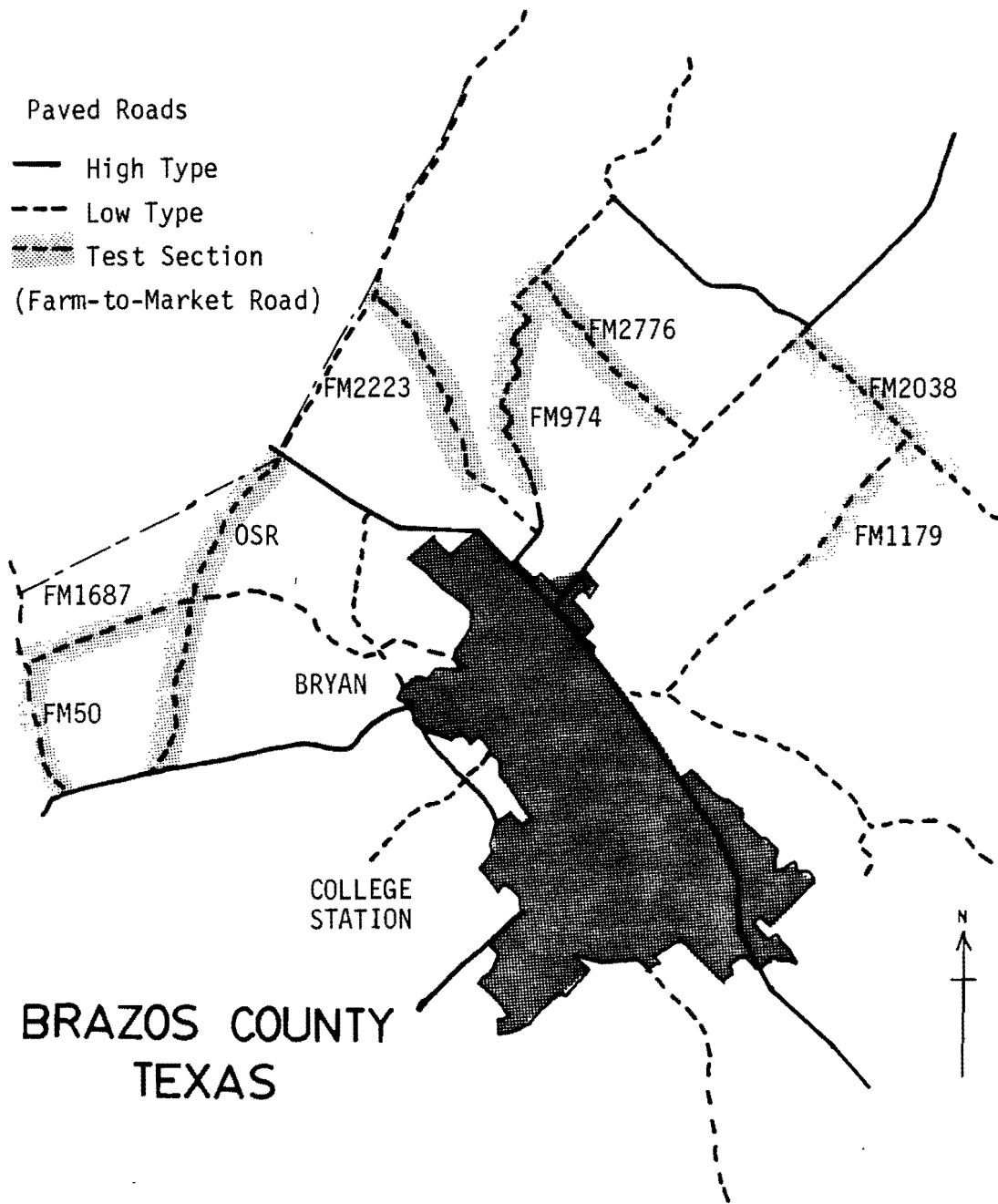


FIGURE 2. Location of Test Sections in Brazos County

BURLESON COUNTY
TEXAS

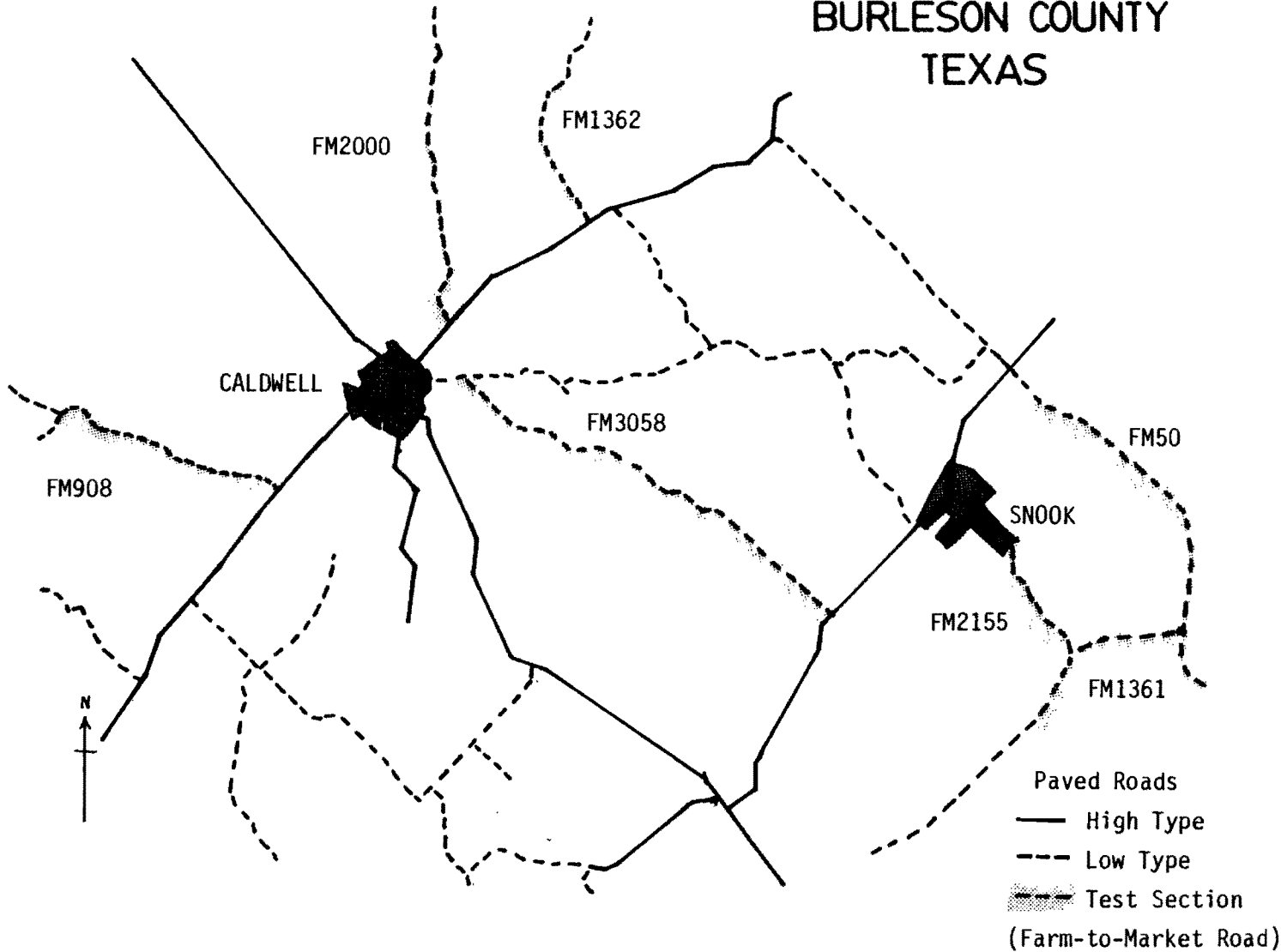


FIGURE 3. Location of Test Sections in Burleson County

Test Sections

The test sections were chosen to be at mile posts (spaced two miles apart) along the Farm-to-Market roads for easy identification and also because it allows the roads to be tested at regular intervals. These sections represent a diverse sampling. Some were constructed or reconstructed as early as 1953 and as late as 1981. Table 1 lists the Farm-to-Market roads that were tested with the corresponding references of construction drawing that were available from the District Office. The base course thicknesses and the field observed base course material type are also given. Figure 4 illustrates the typical cross sections of these roads. Base course thicknesses range from 4 inches to 14 inches. Base course materials were found to consist of crushed stone, river gravel, sandstone and iron-ore. Other types of material, for example oyster shells, are found in other parts of the State. The surface courses or wearing courses, although originally intended to be only a surface treatment course, were measured to be about an inch thick. This is due to numerous seal coat applications.

The pavement sections were first tested with the Falling Weight Deflectometer on the 20th and the 21st of December in 1982. Usually 2 or 3 sections spaced about 10 feet apart were tested at each of the selected mile-posts. The exact spot of each load application was then marked with paint. Subsequently in March of 1983, the Dynaflect was used on these marked sections.

It had been observed (7) that pavements show a constant value

TABLE 1. Relevant Construction Details of Test Sections

District No. 17					
Burleson County					
Road Name	Mile Post No.	Relevant Construction Details		Dated	Field Identified Base Course Material Type
		Base Course Thickness (ins)	Drawing No.		
FM 3058	2 to 4	6	S3021(1)A Sheet 2	10/30/67	Crushed Stone (Caliche)
FM 3058	6 to 8	6	A3119-1-4 Sheet 2	7/17/72	Crushed Stone (Caliche)
	10	6	A3119-1-6 Sheet 2	2/10/77	Crushed Stone (Caliche)
FM 908	10	8	S2216(1) Sheet 2	1/2/58	--
FM 1361	6 to 10	8	C1399-1-9 Sheet 2	3/31/66	--
FM 1362	4 to 8	-- No records - - - - -			
FM 2000	8 to 10	7	A-1129-2-5 Sheet 2	10/29/65	Crushed Stone and Sand Stone
	12	6	833-11C Sheet 2	3/04/75	Gravel
FM 2155	2 to 4	6	R-506-4-2 Sheet 2	8/17/55	River Gravel
FM 50	2 to 4	7.5	CSB457-1-28 Sheet 2	2/19/81	River Gravel
	6 to 16	7.5	CSB457-1-28 Sheet 2	2/19/81	Crushed Stone
Brazos County					
OSR	2 to 4	14	--	3/31/67	Crushed Stone (Caliche)
FM 974	6 to 8	4	C540-3-5 Sheet 2	6/24/58	Crushed Stone (iron ore)
FM 1179	4	6	R-1361-1-3 Sheet 2	10/27/53	Crushed Stone Gravel
FM1687	2	9	C-1560-1 Sheet 2	2/17/59	Gravel
FM 2038	8 to 10	10	CSB2236-1-8 Sheet 2	5/25/78	River Gravel
FM 2776	0 to 2	6	S2654(1)A Sheet 3	1/04/63	River Gravel

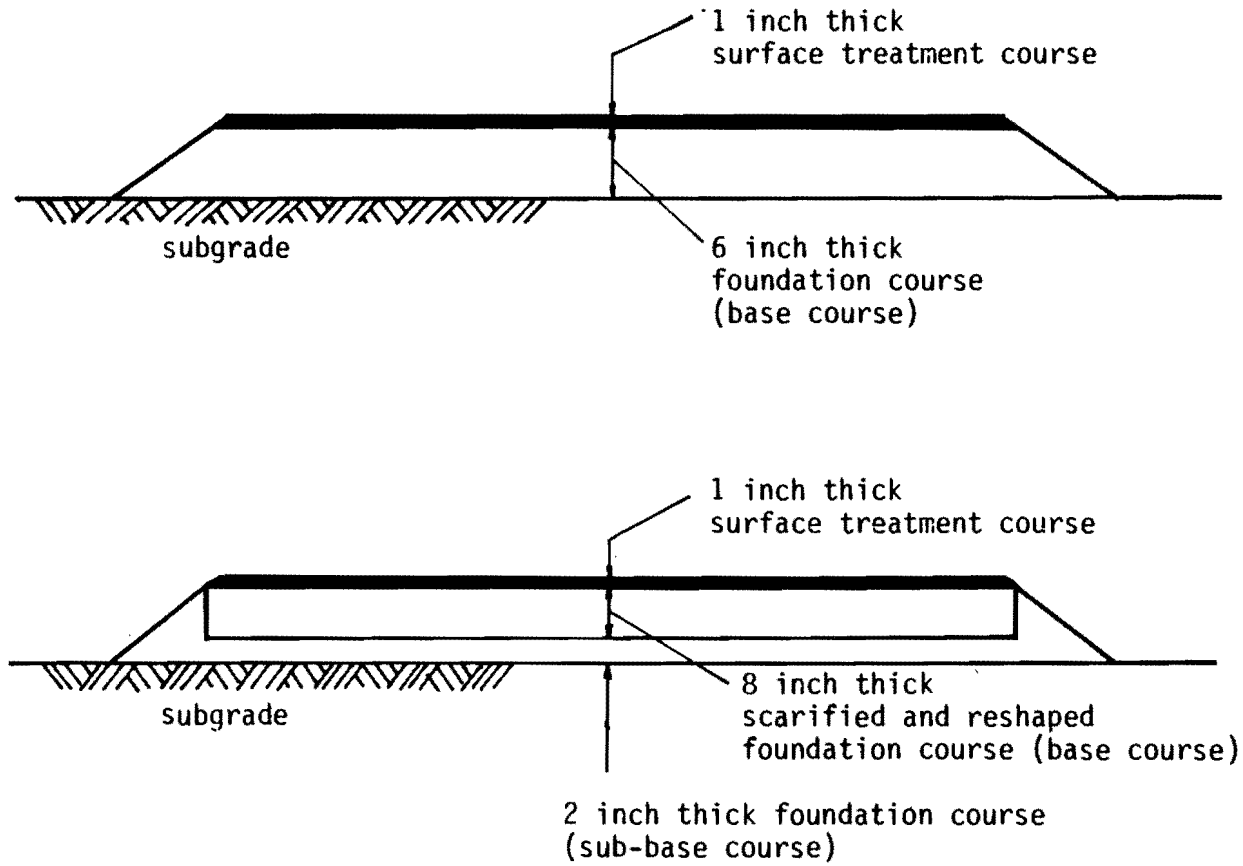


FIGURE 4. Typical Cross-Section of Farm-to-Market Roads

of deflection in response to the same magnitude of loading but when approaching the end of the design life, this value increases sharply. For this reason, the 'standard FWD practice' was to make measurements at points between the wheel paths where the traffic is slight. This was done to obtain a more consistent evaluation of the pavement integrity.

Falling Weight Deflectometer

The Dynatest 8000 FWD Test System which was used in this project consisted of the Dynatest 8002 FWD (8,9) and a complement of system processor and a device which recorded and interpreted the measured loads and deflections. The FWD itself is a light-weight trailer mounted unit, as can be seen in Figure 5.

The FWD can deliver an impulse load of 1,500 lbs to 24,000 lbs to a pavement. The impulse is essentially a half sine curve with a duration of 25 to 30 milliseconds. The load is transmitted to the pavement through a 12 inch diameter loading plate which rests on a thick rubber pad which is in contact with the pavement surface. In principle, the force applied to the pavement is dependent on the mass of the drop-weights used, the height of the drop and the spring constants of the rubber pad as well as that of the overall pavement. In practice however, only the mass of the drop-weights and/or the height of drop is varied. The actual load relayed to the pavement is measured by the load cell located just above the loading plate.

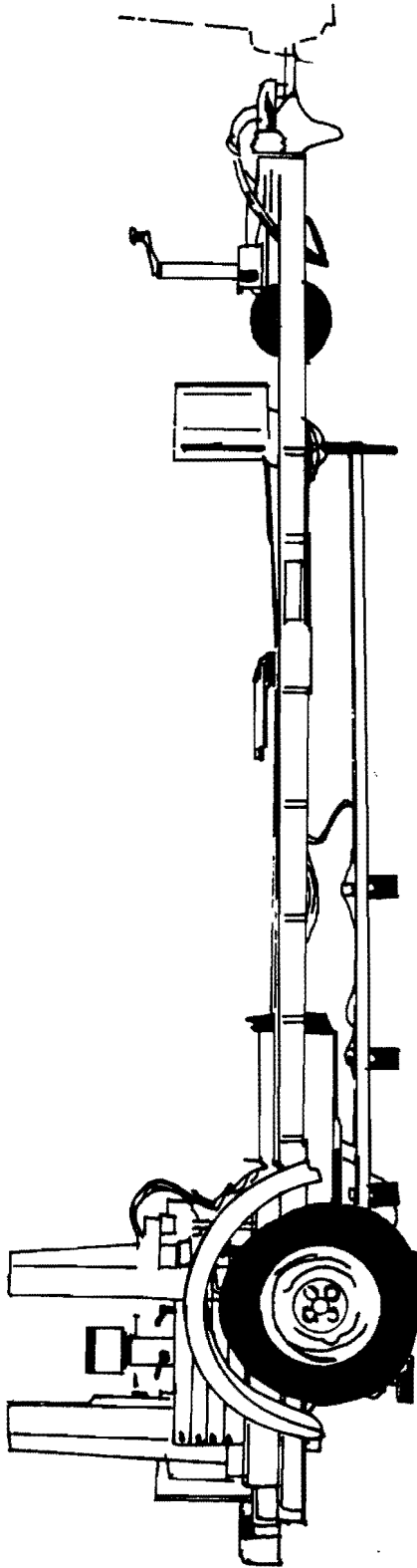


FIGURE 5. The Dynatest Falling Weight Deflectometer

The deflection basin is obtained by monitoring the deflections at seven locations on the pavement surface using velocity transducers. One of these is located in an opening in the center of the loading plate.

In the tests, the height of drop and weight were adjusted to produce four different load levels - 9,000 lbs, 11,000 lbs, 15,000 lbs and 23,000 lbs with the exact magnitude being registered by the load cell. Figure 6 shows the locations of the deflection sensors and a set of typical deflection basins observed at the four different load levels.

Dynalect

The Dynalect (18) is currently the most commonly used NDT device in the United States for the purpose of pavement evaluation and design (10). This equipment is a dynamic force generator mounted on a covered trailer, as can be seen in Figure 7. The cyclic force is produced by a pair of counter-rotating unbalanced flywheels and this force oscillates in a sine-wave fashion with an amplitude of 500 lbs at a cycle frequency of 8 cycles per second. This force together with the dead weight of the trailer, which is about 1,600 lbs, is transmitted to the ground via two steel wheels placed 20 inches apart. The peak-to-peak deflections are measured by five geophones placed at 1 foot intervals with the first directly between the wheels. A typical deflection basin obtained is shown in Figure 8.

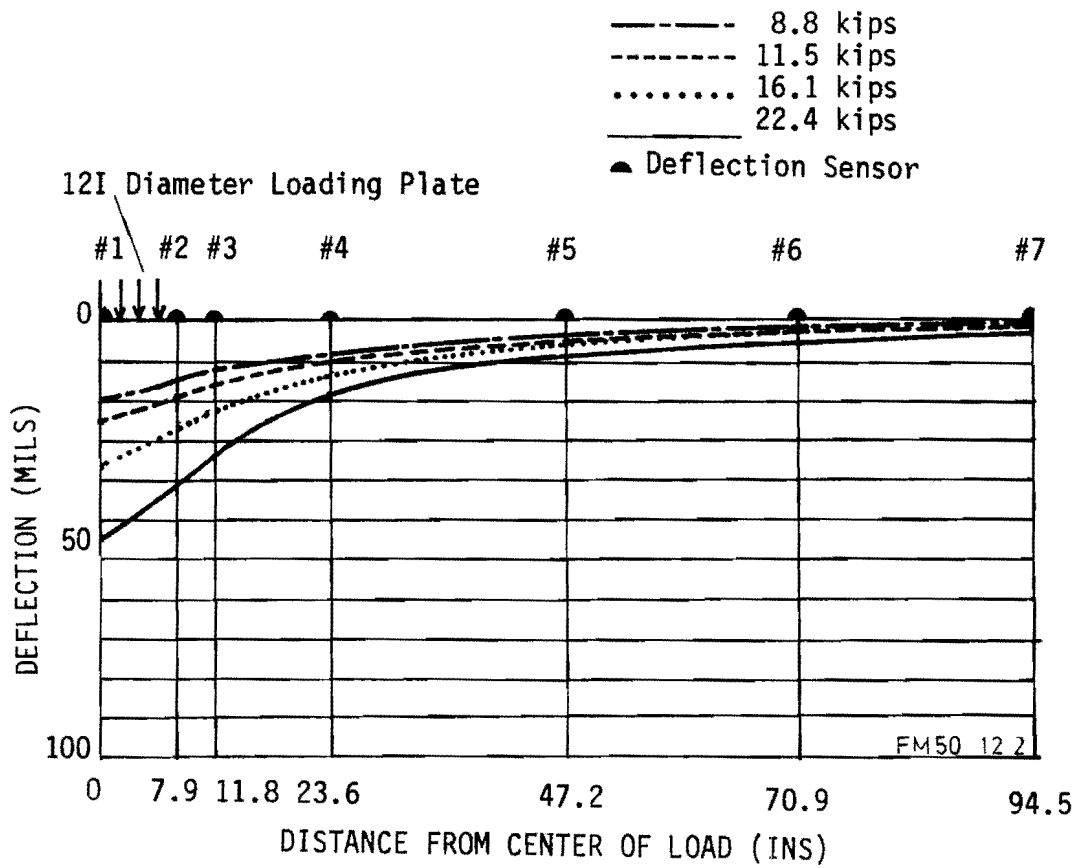


FIGURE 6. Typical Deflection Basin - Falling Weight Deflectometer

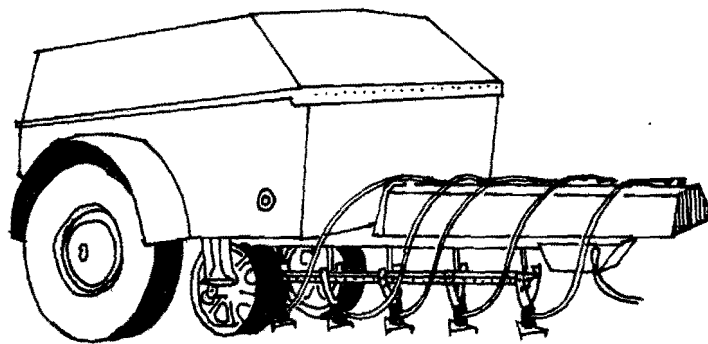


FIGURE 7. The Dynaflect

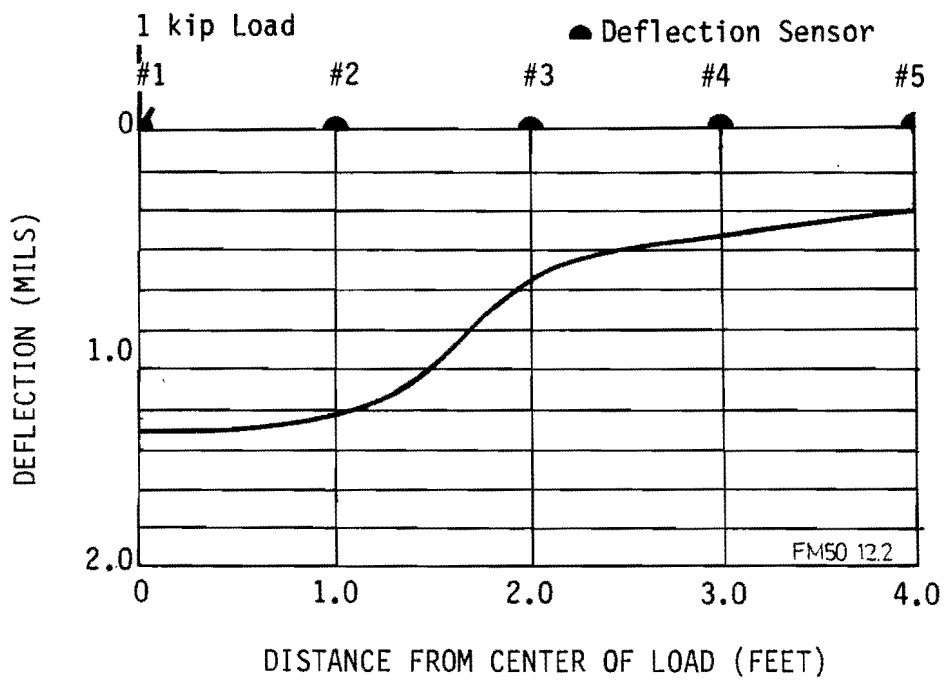


FIGURE 8. Typical Deflection Basin - Dynaflect



CHAPTER III

DATA ANALYSIS

After the data had been collected, it was necessary to verify that the ILLI-PAVE computer program with appropriately assumed material models can reproduce measured deflection basins. Then, ILLI-PAVE was used to generate deflection basins for four different load levels with different combinations of assumed material models, particularly those of the base course and the subgrade, and at the same time using different thicknesses of the base course. These finite element computations were made simulating tests done with an FWD. It was presumed that the last deflection sensor reading, which is 94.5 inches from the center of the loading plate, is related to subgrade material type.

Next, having developed a procedure of identifying material models from FWD deflection sensor readings, a load-deformation equation was formulated for each set of deflection sensor readings. A hyperbolic load-deflection model was adopted and a means of identifying the unknown coefficients was established.

The load rating or rutting model assumed was one that allows for a linear unloading path in the load-deformation curve. The reloading path was assumed to be the same as the loading path. The gradient of the unloading path was determined from actual rut depth and the number of passes of a known load, or estimated from a formulation presented in this study which was based on the backcalculation from observed rut depths. Finally, from the comparisons of deflection basins from the FWD and Dynaflect, a correlation between the first and the last

deflection sensors reading of both instruments was made.

The following section discusses the analytical approach adopted, the analytical tools used and the assumptions made.

ILLI-PAVE: Finite Element Analysis

The load-deflection relationship of layered systems was investigated by Burmister (11,12) in the 1940's. He adapted Boussinesq's (1885) theory of distribution of stresses in an elastic half-space under the compressive action of a rigid body to include a layered system. Subsequently, many computerized systems of the closed form solution were developed. These solutions assume linearly elastic material properties.

The finite element approach is now being used to analyze the load-deflection relationships of pavement structures. In the analysis, the body under consideration is divided into a set of elements which are connected at their nodal points. From the material property assumed, that is, the force-displacement relationship, the stiffness at each nodal point is specified. By expressing the nodal forces in terms of displacements and stiffnesses, the equilibrium equations for each nodal point can then be solved to obtain the displacements. The stresses and strains in each element can then be computed.

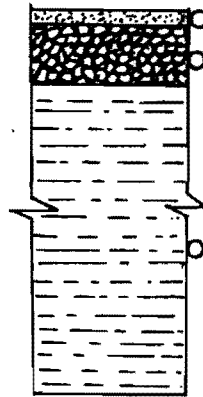
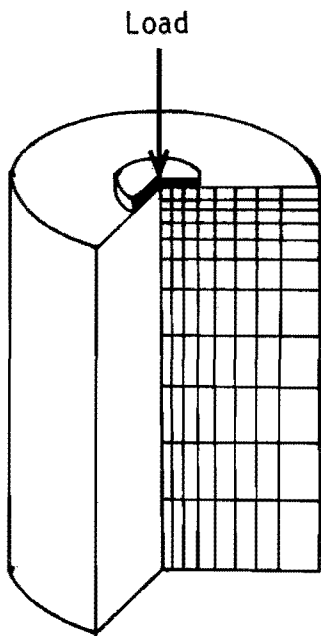
A finite element program for flexible pavements was being developed by Wilson as early as 1963. Later, he and others (13) presented the technique which can take into account the nonlinear properties of materials in their response to traffic loads.

Modifications were then made by Raad and Figueroa (14) to include a failure model for granular and subgrade soils based on the Mohr-Coulomb theory.

ILLI-PAVE (15,16) is an alternative finite element program. It models an asymmetrical solid of revolution and allows for linear as well as nonlinear stress-dependent elastic moduli for granular and fine-grained soil. This program has been shown to be adequate in predicting the flexible pavement response to load by comparing the results of computer modelling and field test data (17). A similar program, ILLI-CALC (21), allows for the backcalculation of nonlinear resilient moduli from deflection data.

Figure 9 shows the ILLI-PAVE finite element model as an asymmetrical solid of revolution. For the analyses done in this study, a mesh of 121 elements was used. The sizes of the elements were made to be smallest nearest the pavement surface so as to allow for greater accuracy in the computation. To allow for an adequate simulation of the boundary conditions, it was suggested (13) that the depth of the mesh be at least 50 times the radius of the circular loading plate of the FWD which is 6 inches and that the horizontal extent be at least 12 times that radius away from the center of the loading plate. In this case, to accommodate for the FWD last deflection sensor, a width of 96 inches was used. However, from the analyses made at about 11,000 lbs loading, vertical stresses caused by the load input seem to be negligible beyond a depth of about 12 times the radii of the loading plate.

The following paragraphs will describe how ILLI-PAVE was used in



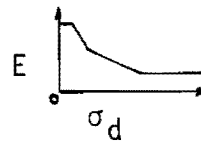
Surface Course
- linearly elastic modulus

Base Course
- elastic modulus

$$E = K_1 \theta^{K_2}$$

where θ = bulk stress
 K_1, K_2 = constants

Subgrade
- elastic modulus E



where σ_d = deviator stress

FIGURE 9. The ILLI-PAVE Model:
Finite Element Pavement Analysis

this study and the material models that were input.

A. Pavement Material Models

The Farm-to-Market roads encountered generally show three distinct layers: a surface course, a base course and a subgrade. Some older roads were found to have a subbase consisting of the old road base which was partially scarified and then overlain with new base course material. The subgrade material was found to vary greatly even along the same road.

As an extraneous part of the study of pavement materials, the Pavement Dynamic Cone Penetrometer [DCP] (19) was introduced. This basically consists of a steel rod with a 60⁰ cone of tempered steel at one end. A sliding hammer of about 17.6 lbs falling over a height of 22.6 inches provided the consistent impact load required to penetrate the pavement. The penetration given as inches per blow gives an indication of the stiffnesses of the pavement layers. This instrument was found to be useful in comparing the stiffnesses of the base courses encountered in this study. Figure 10 shows the DCP.

The one-inch thick surface courses did not contribute much to the pavement in terms of rigidity but were nevertheless included in the material modelling in recognition of their presence. The material was assumed to be linearly elastic and to have a modulus of 30,000 psi. The determination of an actual value of the modulus is superfluous as its influence on the analysis was insignificant.

The base course thickness used in the simulations were taken from

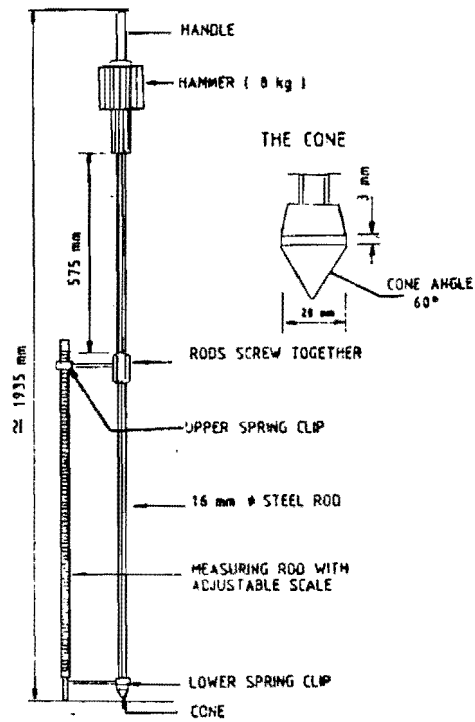


FIGURE 10. The Dynamic Cone Penetrometer (49)

construction drawings. However, more direct means such as using sample coring or the DCP was also used to enhance the accuracy in the simulation. However, the thicknesses found using the DCP differed from the design value by as much as 5 inches for an 8-inch thick base course. However, in most cases the difference was much less. In the ILLI-PAVE analyses, the subbase course, if any, was considered as part of the base course since its material type did not appear to be different. As a point of interest, from the DCP data, it appeared that most old pavements show a distinct interfacial layer between the base course and the subgrade. This might be due to infiltration of fines from the subgrade into the base course layer as well as the presence of moisture.

Base course materials were found to be of the granular, unbound type. Using the DCP it was found that knowledge of the material hardness and shape is not sufficient to categorize its load deflection behavior. Figure 11 shows the rate of penetration of the DCP into a few pavements with different base course materials. It appeared that the major determining factor of the stiffness of the material is the unit weight, that is, the degree of compaction of the material. This characteristic had been realized earlier (20). In view of this, the elastic modulus of the base course material can be expressed as

$$E = K_1 \theta^{K_2} \quad (1)$$

where

θ is the bulk stress or the first stress invariant, and K_1 the unknown coefficient defining the material.

TABLE 2. Material Properties used in ILLI-PAVE

Property	Surface Course	Base Course	Subgrade			
			Stiff	Medium	Soft	Very Soft
Unit Weight (PCF)	145.00	135.00	125.00	120.00	115.00	110.00
Lateral pressure coefficient at rest	0.87	0.60	0.82	0.82	0.82	0.82
Poisson's Ratio	--	0.38	0.45	0.45	0.45	0.45
Unconfined compressive strength (PSI)	--	--	32.80	22.85	12.90	6.21
Deviator Stress (PSI)						
Upper Limit	--	--	32.80	22.85	12.90	6.21
Lower Limit	--	--	2.00	2.00	2.00	2.00
Deviator Stress at 'breakpoint' (PSI)	--	--	6.20	6.20	6.20	6.20
Initial Elastic Modulus (KSI)	--	--	12.34	7.68	3.02	1.00
Elastic Modulus at Failure (KSI)	--	--	7.605	4.716	1.827	1.00
Constant Elastic Modulus (PSI)	30,000	--	--	--	--	--
Elastic Modulus Model	Linear	(see Fig.12)	(see Fig. 13)			
Friction Angle ($^{\circ}$)	--	40.0	0.0	0.0	0.0	0.0
Cohesion (PSI)	--	0.0	16.4	11.425	6.45	3.105

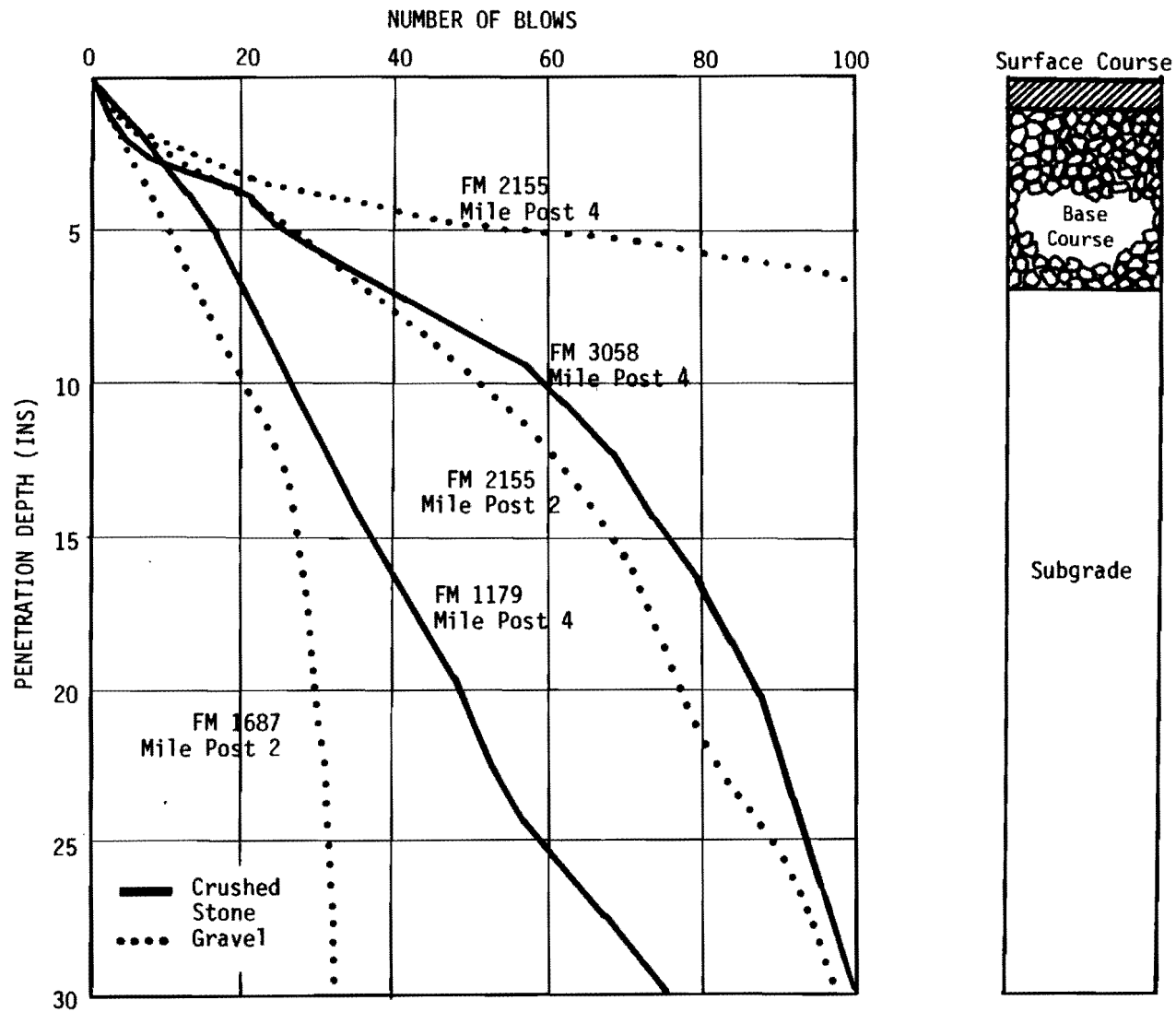


FIGURE 11. Comparison of the Pavement Stiffnesses using the Dynamic Cone Penetrometer

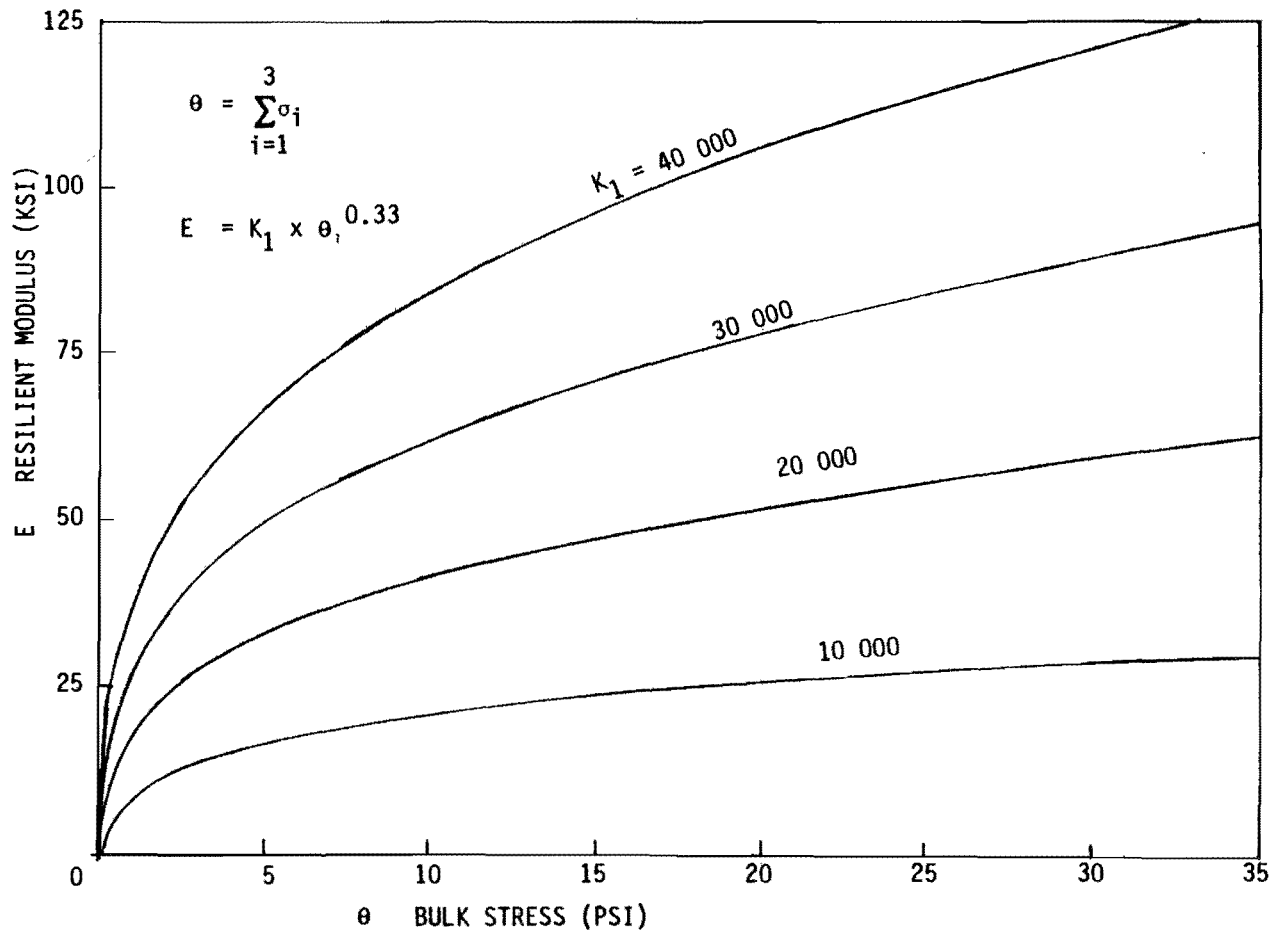


FIGURE 12. Base Course Material Models

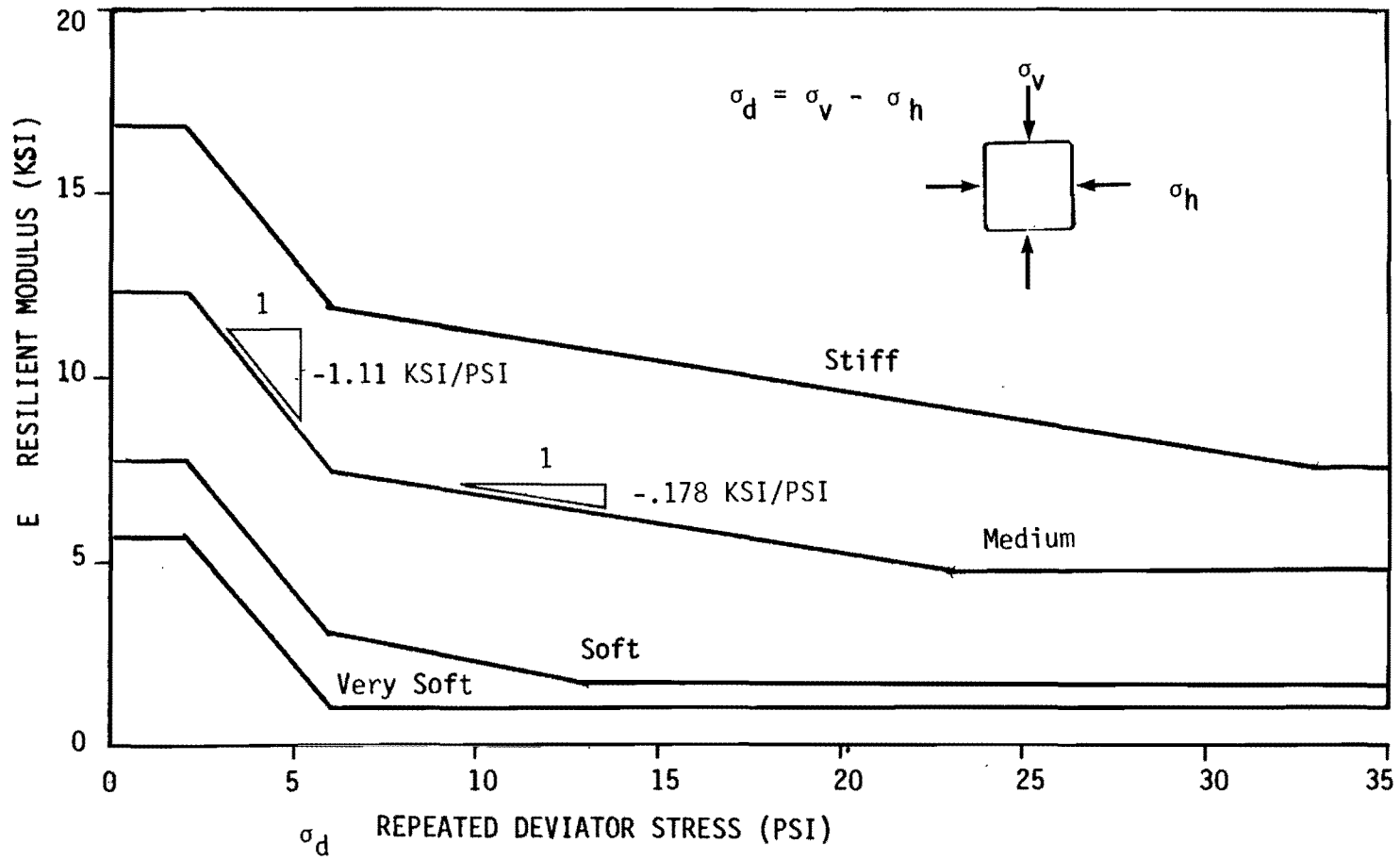


FIGURE 13. Subgrade Soil Material Models

This value shall be referred to as the K_1 -value hereafter. The range of K_2 -values was reported to be between 0.30 to 0.60 (21,22). Most analyses using ILLI-PAVE (15,21) adopt a range of 0.30 to 0.33 for this value. For practical reasons, in this study a value of 0.33 is assumed. This reduces to one the number of factors to be identified in the base course material. Subsequent analyses showed that this is an adequate assumption. Figure 12 shows the assumed base material model.

Four nonlinear elastic moduli, shown in Figure 13, were used to describe the subgrade properties. They are for the Very Soft, Soft, Medium, and Stiff subgrades. These models had been successfully used before with ILLI-PAVE (15,21)

Table 2 gives a summary of the pavement material properties used in the analyses with ILLI-PAVE.

B. Generation of Deflection Basin using ILLI-PAVE

In order to obtain enough load-deflection data to cover a wide spectrum of light pavement structures with different materials, a series of finite element computer runs were made. These simulations included a combination of four subgrade types, that is, the Very Soft, Soft, Medium and Stiff, and four base course material types with K_1 -values at 10,000, 100,000, 200,000, and 300,000. In addition, four different base course thicknesses were used: 2-inch, 6-inch, 12-inch and 18-inch thick. For all of the above combinations, four FWD loadings of 80 psi, 100 psi, 140 psi, and 200 psi were used. The corresponding loads were 8765 lbs, 10956 lbs, 15339 lbs and 21913 lbs. In addition to the above framework, other combinations were used as

when was necessary. The results of these simulations were found to form a more than adequate pool of data whereby important correlations of various parameters were identified.

C. Matching of Measured Deflection Basin Using ILLI-PAVE

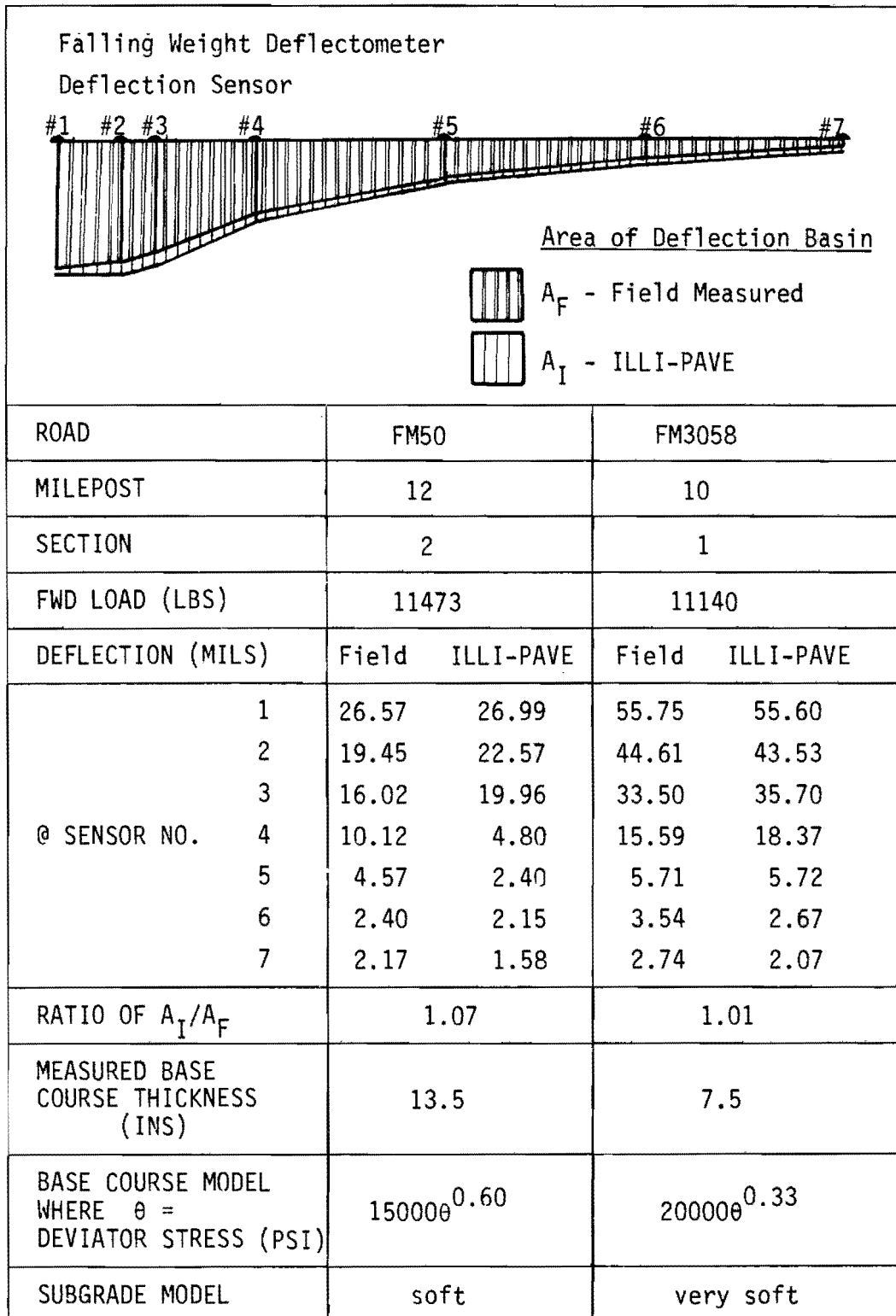
Previous study (17) had shown ILLI-PAVE to be adequate in predicting the response of flexible pavement to loads. That presupposes that use of appropriate material models can actually simulate the response of real pavements.

In this study, measured deflection basins of Farm-to-Market road sections were successfully matched to further show that the program is valid. The procedure was to adjust the input for subgrade and base course material characteristics to obtain field measured deflection basins. This was an iterative process. In this process, if the simulated last deflection sensor value of the FWD was underestimated, it implied that the subgrade assumed was too stiff. And if the first deflection sensor value was found to be too high, a stiffer material model would be used for the base course. Some difference in the curvature of the deflection basin was observed, probably due to the non-uniformity in the base and the subgrade materials. Table 3 shows the results obtained for two of the sections matched.

Load Deflection Model

A hyperbolic relationship between the load and the deflection of

TABLE 3. Comparisons of Measured Deflection Basins with ILLI-PAVE Results



the light pavement structure was assumed. As the hyperbolic stress-strain relationship is true of most soil materials (28,29), and since the light pavement structures considered are composed of soil materials, it is reasonable to adopt this as the load deflection model. The general equation is

$$P = \Delta / (A + B\Delta) \quad (2)$$

where P = load and Δ = deflections

The constants A and B will hereafter be termed Coefficient A and Coefficient B.

Rewriting equation (2) results in

$$\Delta/P = A + B\Delta \quad (3)$$

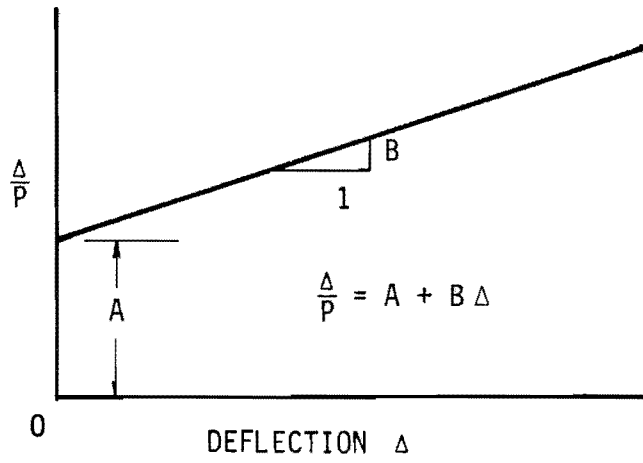
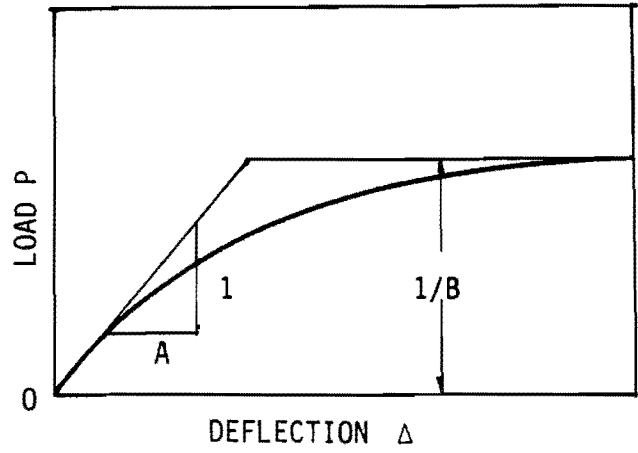
A plot of Δ/P versus Δ yields the straight line as shown in Figure 14 from which the Coefficients A and B are found. The above equation assumes a stress softening behavior. However, extrapolation of field measured maximum deflections for different loads showed that some pavement do stress harden. A typical set of load deflection curves for a Farm-to-Market road is shown in Figure 15. To allow for this, a modified hyperbolic load deflection equation was used.

This expression is

$$\frac{P}{\Delta} = \frac{1}{A} + \frac{1}{C}P \quad (4)$$

where C is a constant.

A plot of P/Δ versus P yields a straight line as is shown in Figure 16 from which A and C are found. Careful examination of the hyperbolic equation shows that by putting as $B = -A / C$ into Equation (2), a stress hardening form of the load deflection behavior



Load Deflection Equation :

$$P = \frac{\Delta}{(A + B \Delta)}$$

FIGURE 14. Load Deflection Model - Stress Softening Form

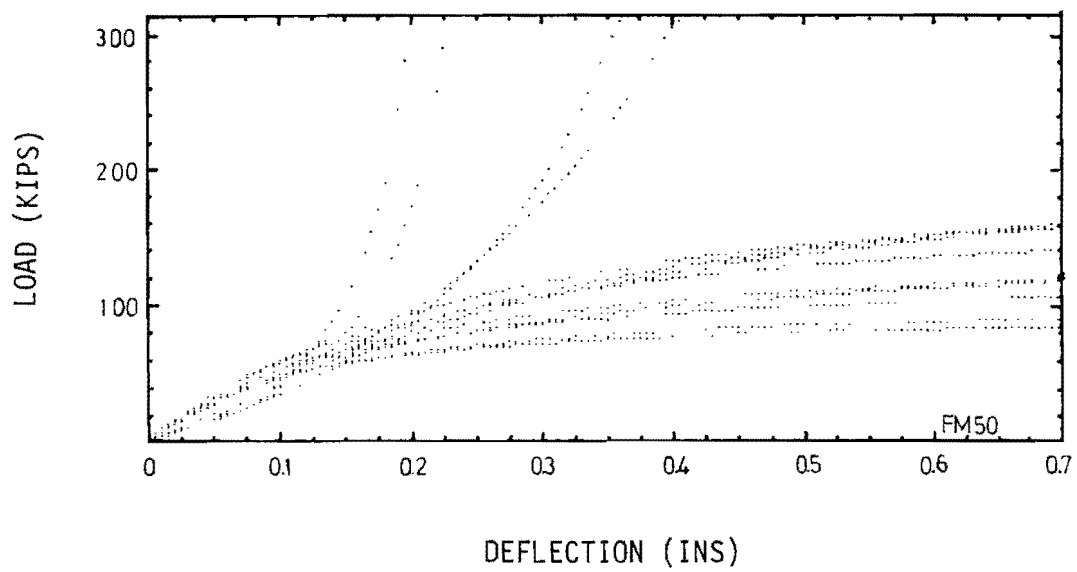
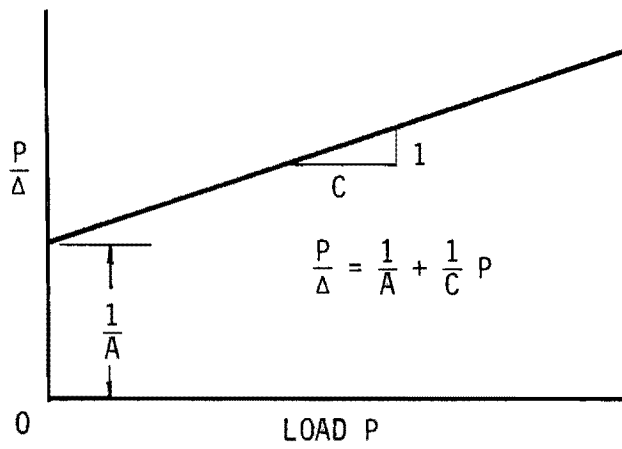
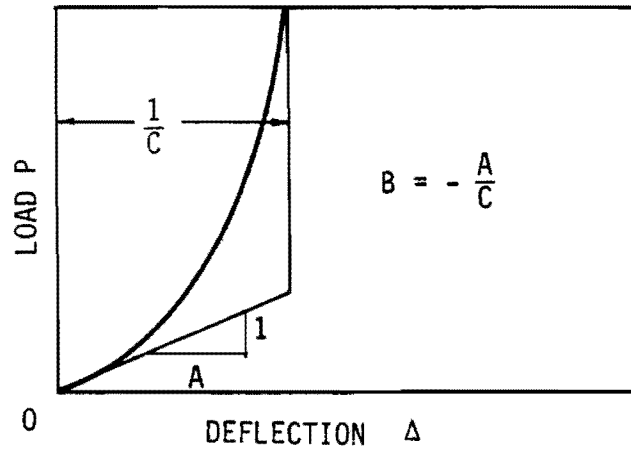


FIGURE 15. Typical Load Deflection Curves
of Farm-to-Market Roads



Load Deflection Equation :

$$P = \frac{\Delta}{(A + B \Delta)}$$

FIGURE 16. Load Deflection Model - Stress Hardening Form

results. Henceforth, the above expressions are described as the stress hardening and the stress softening form of the hyperbolic load deflection equation for the pavements considered.

Load Rating/Rutting Model

A rut can be formally defined as (23) "a permanent deformation in and of the pavement layers or subgrades caused by consolidation or lateral movement of the materials due to traffic loads". As the Farm-to-Market roads being considered do not contain much thickness of asphaltic material to move laterally under loads, rutting due to consolidation is the primary concern.

In considering the problem of rebound deformation under repetitive loading, the following information is of some relevance. In the loading and reloading of silica sand, Duncan and Chang (24) found that after the initial loading, the path of which was hyperbolic, the unloading and reloading path could be approximated with a high degree of accuracy as being linear and elastic. In another study Raad and Figueroa (14) observed that the resilient behavior of granular base and subgrade were maintained even after large deformations. Larew and Leonards (25) suggested that the rebound reached an equilibrium value after approximately one thousand repetitions. Thompson and Robnett (40) thought that rebound was related to the moisture level. A widely accepted model for cyclical loading of pavements is yet to be found.

To determine permanent deformation in pavement, Yandell and

Lytton (26) successfully used a three-dimensional mechano-lattice analysis with translating loads. This involved a computer simulation of translating rather than pulsating wheel loads over a pavement section whose material properties were determined in the laboratory. However, the costs of computer time and laboratory testing are too expensive for the immediate objective.

For the purpose of developing a load rating model, the rutting models shown in Figure 17 were assumed. The Type I model shows a stress softening load deflection behavior and the Type II a stress hardening one. The unloading path was assumed to be linear. This path is expressed in terms of the initial slope or initial stiffness of the pavement, by using a multiplier. This multiplier is assumed to be independent of the load level and can be found if information of the measured rut depth as caused by a known number of passes of a certain load is available. In the development of the procedure, by using measured rut depths and the corresponding number of 18-kip Equivalent Single Axle Loads [ESAL] on Farm-to-Market roads obtained from a previous TTI project (27), estimated values for the multiplier can be obtained. These are found to depend on the initial stiffnesses of the pavements.

Curve Fitting Techniques

Curve fitting techniques used in this study were based on the least squares method of regression analysis. The criterion of this method is to find an expression of a curve such that the sum of the

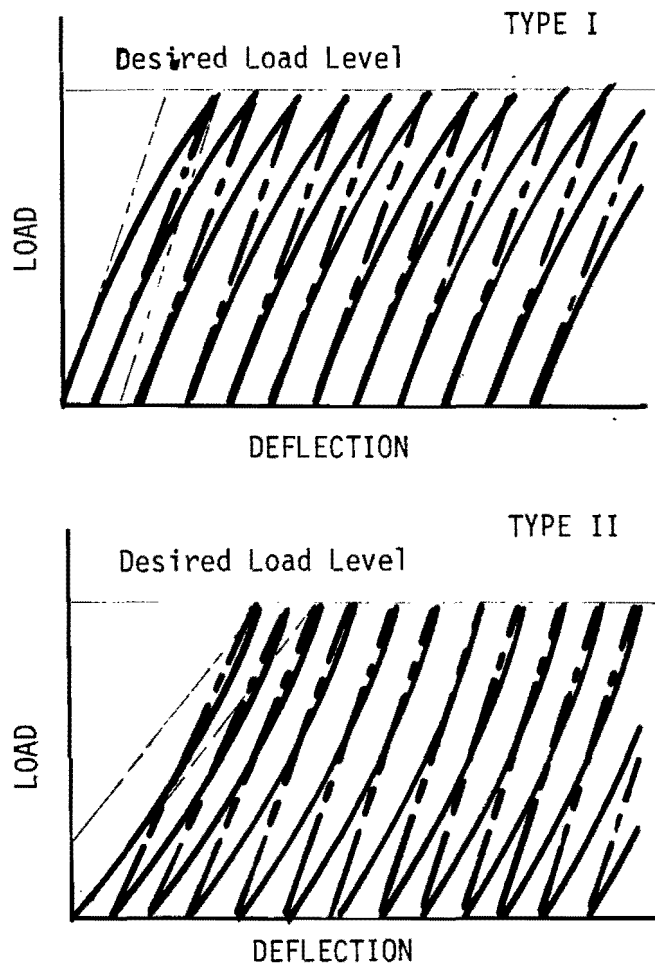


FIGURE 17. Load Deflection Model for Repetitive Loading (Rutting) on Pavement

squared vertical deviations between the curve and the scatter of points is minimized. Regression approaches employed include those to fit a line function, an exponential function, a geometric function, and an Nth order polynomial. For the last, only a second order polynomial was used. In addition, the technique presented earlier for fitting a hyperbolic function was used.

CHAPTER IV

SUMMARY OF RESULTS

Description and Discussion of the Load Rating Procedure

Two approaches to the load rating procedure were developed. One is for use with a Falling Weight Deflectometer and the other, which is based on the first, is for use with a Dynaflect. The following sections present the two approaches in depth.

A. Procedure Using the Falling Weight Deflectometer

1. Obtain field measured response of pavement to FWD pressure of about 100 psi which corresponds to a load of about 10,956 lbs. This loading shall be referred to as the Standard FWD Load. The condition is necessary because much of this procedure was developed based on that load level.

2. Adjust measured deflections at Sensor Nos.1 and 7 to their equivalent values at the Standard FWD Load. This can be done by multiplying the values by the ratio of 10,956 lbs over the registered load transmitted to the pavement. A linear variation can be assumed as the departure will not be expected to be large. These corrected deflections shall be referred to as the FWD deflections for the rest of the procedural outline.

3. Determine Coefficient A of the load-deflection equation. The stiffness of a pavement structure refers to the value obtained by dividing the applied load with the corresponding deflection at the

point of loading. The Overall Stiffness is then the division of the Standard FWD Load by the maximum FWD deflection which will be at Sensor No.1 . The Initial Stiffness, which is the slope of the load-deflection curve near a zero load, is then read off Figure 18 and the inverse of this is the value of Coefficient A. Figure 18 was derived from field-measured deflections.

4. Determine the type of subgrade. With the FWD deflection at Sensor No.7, from Figure 19, the type of subgrade soil model can be determined. Figure 19 was based on field measured deflections.

5. Determine the Standard Deflection. This is the maximum deflection that will be obtained if the particular pavement structure is resting on Very Soft subgrade and loaded with a Standard FWD Load. This value can be obtained from Figure 20. This correlation was derived from the ILLI-PAVE analyses and was found to match the field measured values.

6. Determine the base course material model. By interpolating from the curves shown in Figure 21, the K_1 -value of the base course material can be found. Necessary inputs will be the base course thickness and the FWD deflection at Sensor No.1 . These curves were based on the ILLI-PAVE analyses.

7. Determine the Coefficient B of the load deflection equation. As can be seen from Figure 22, the Coefficient B is dependent on the K_1 -value of the base course material and the subgrade type. The positive value can be interpolated from the curves shown in the figure. Different scales for the value of Coefficient B are given to adjust for the different subgrade encountered. This figure was based

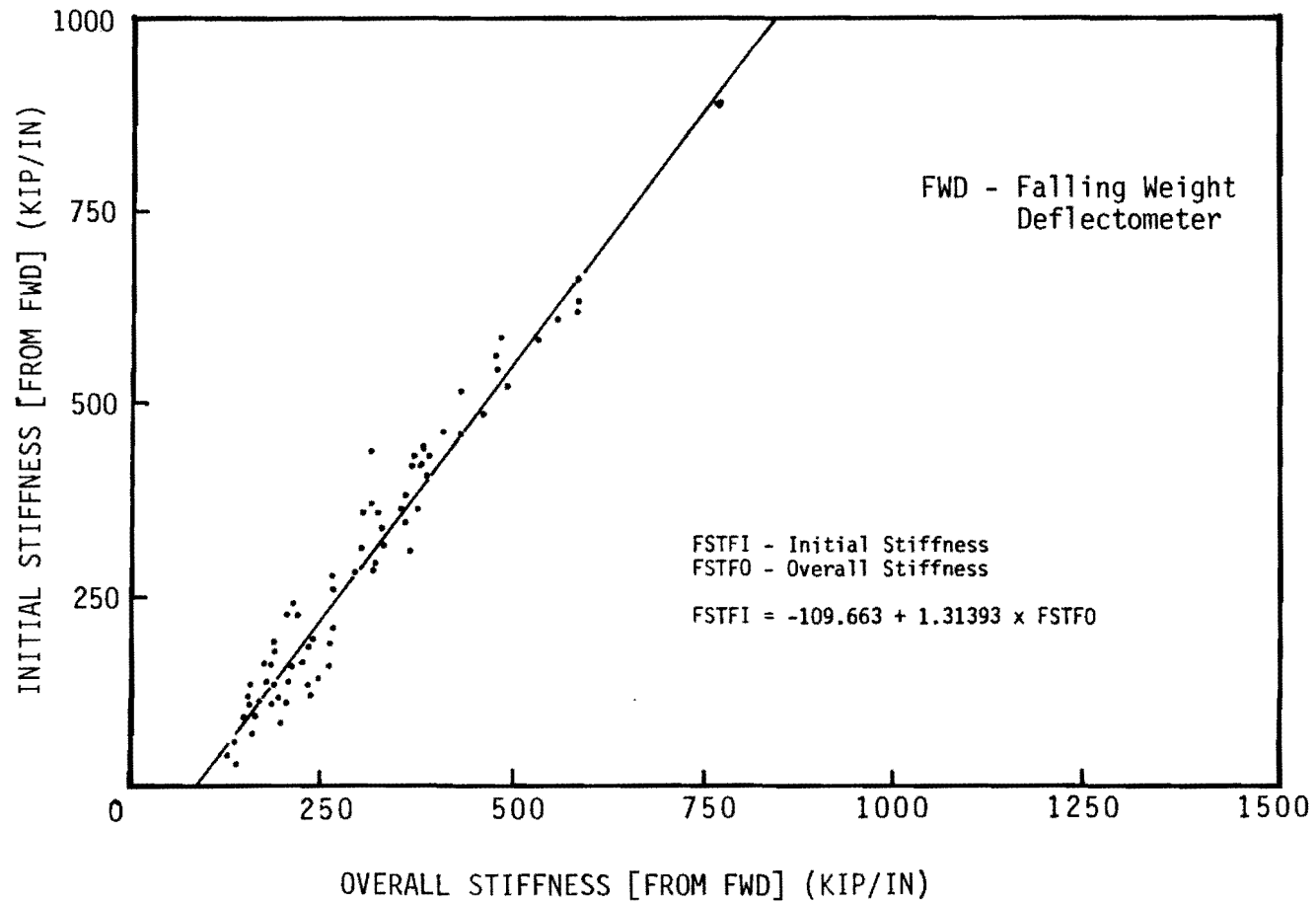


FIGURE 18. Determination of Initial Slope (Stiffness) of Load Deflection Curve

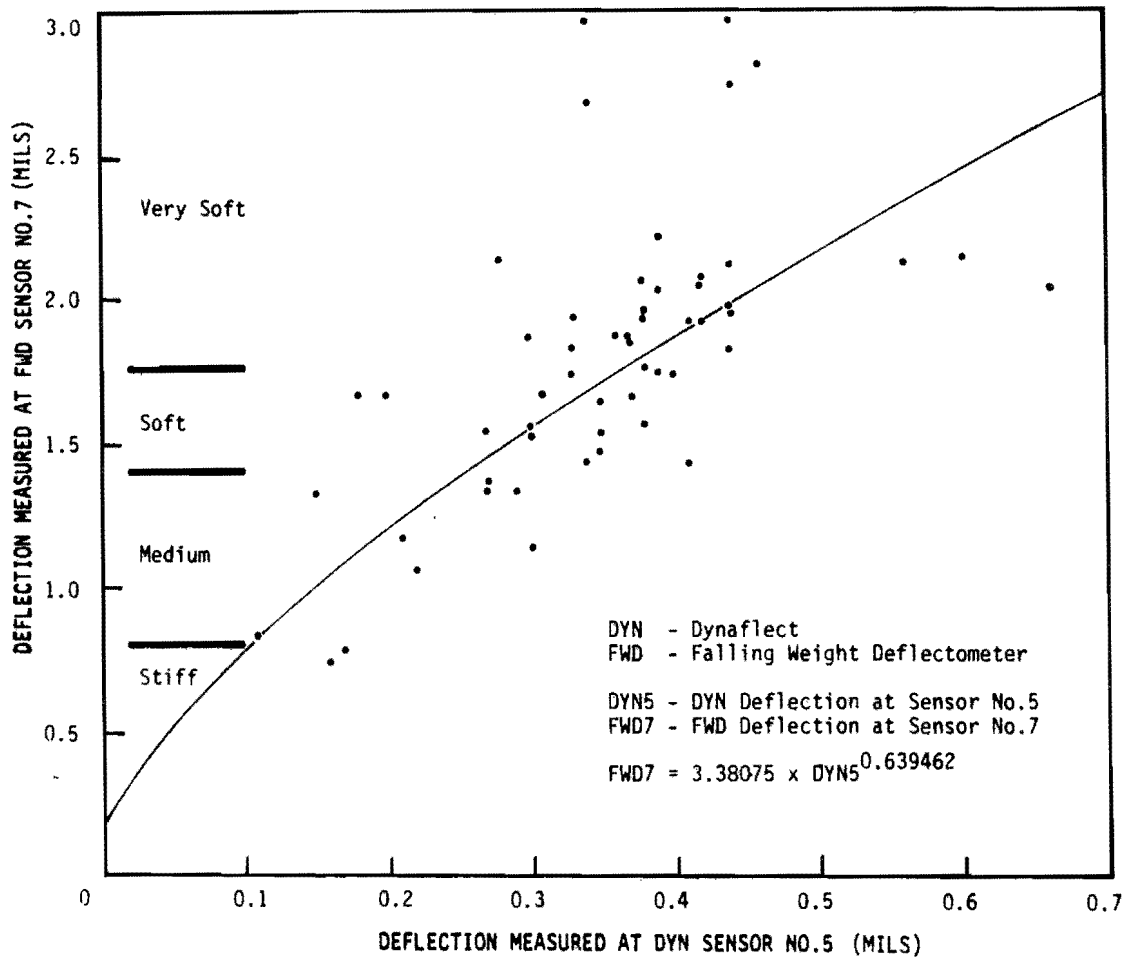


FIGURE 19. Determination of Subgrade Soil Model from Deflection

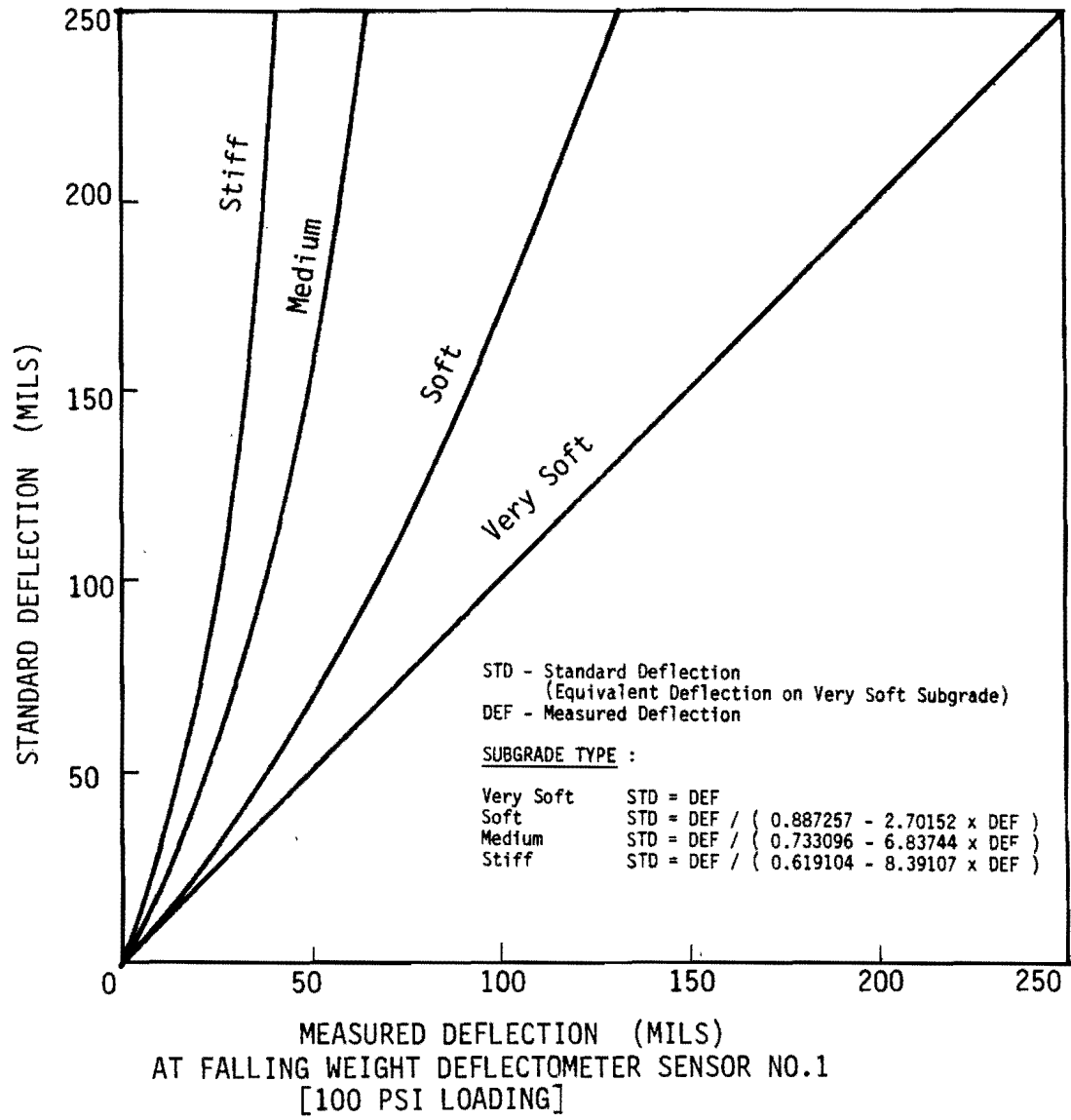


FIGURE 20. Determination of Standard Deflection

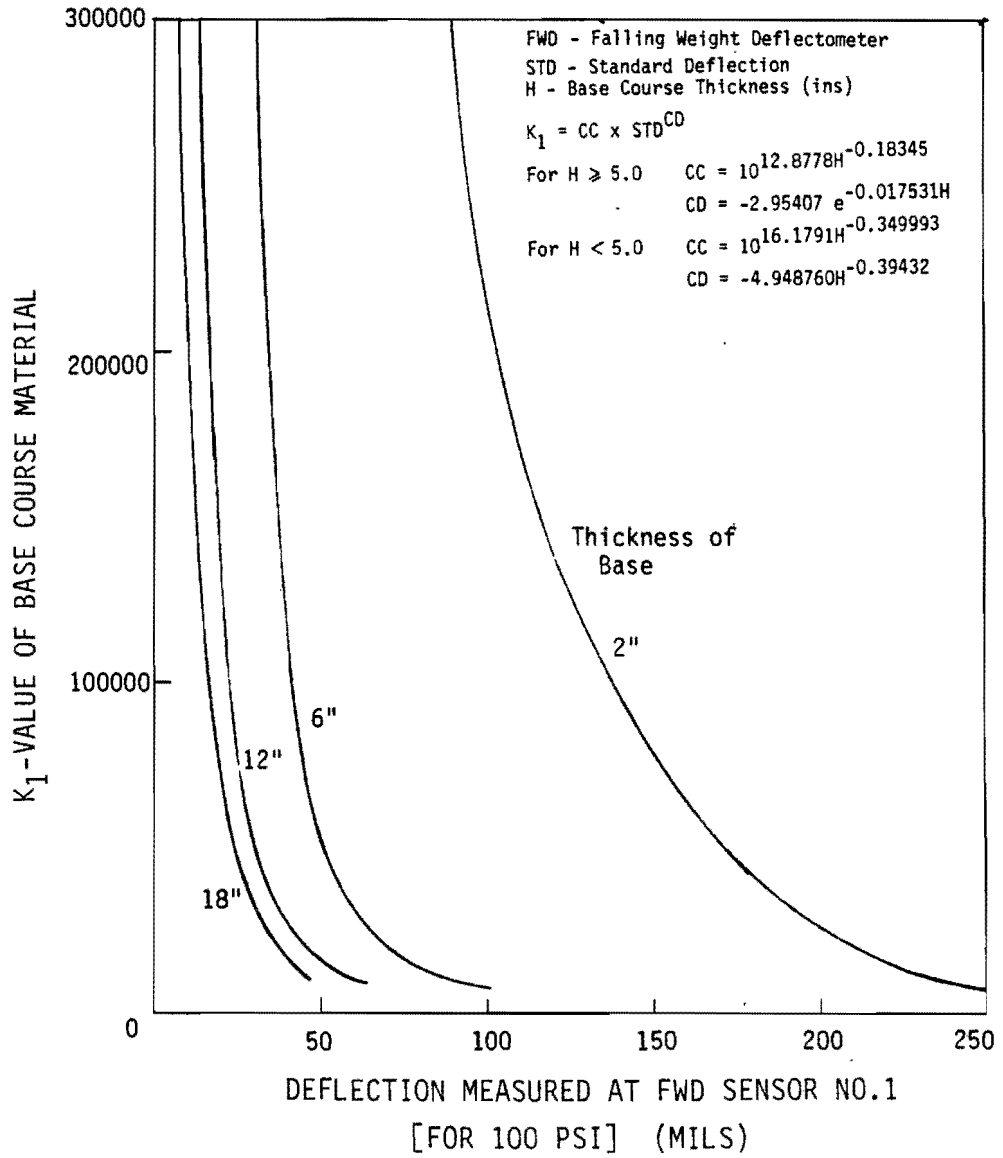
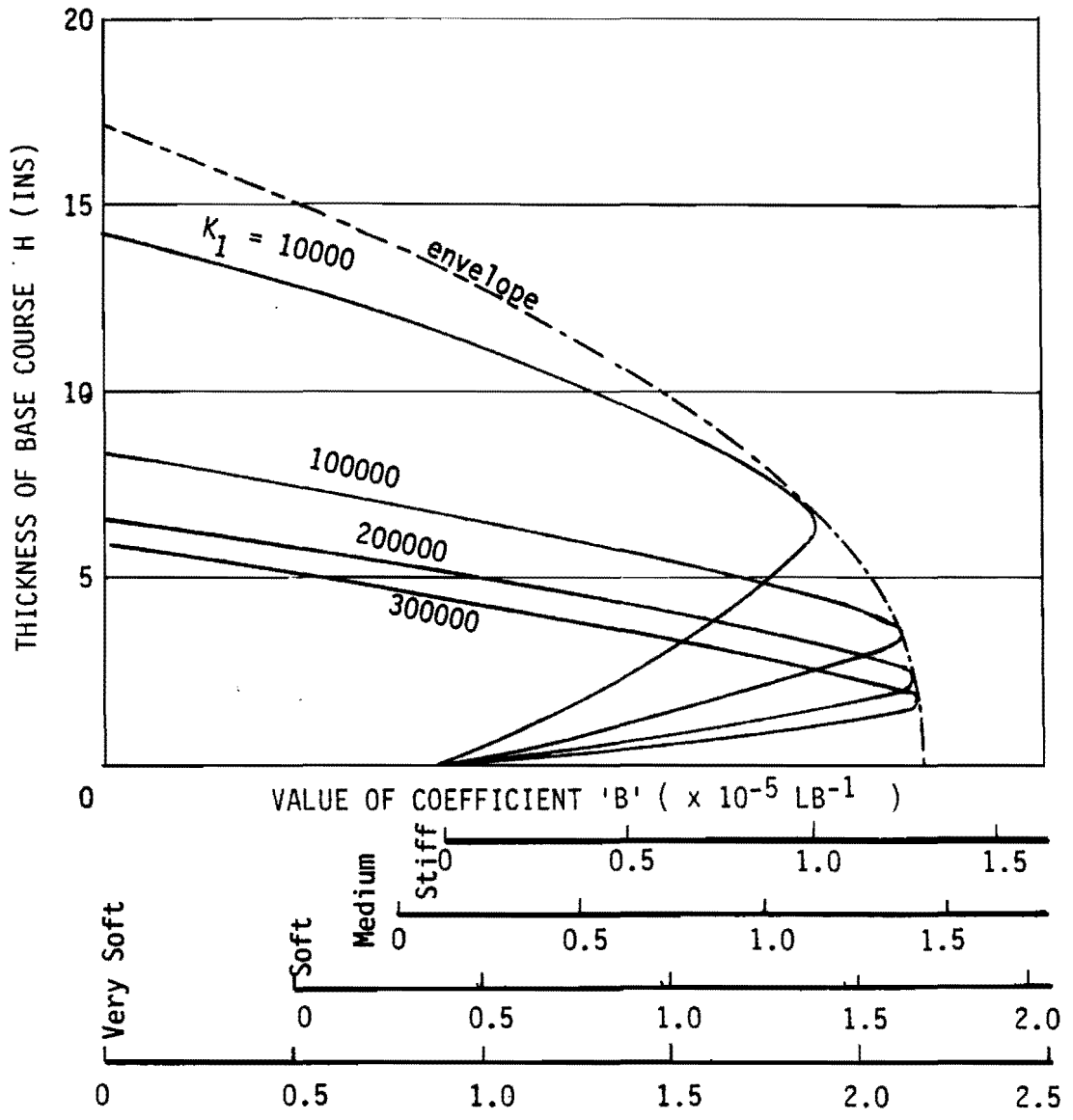


FIGURE 21. Determination of Base Course Material Model from FWD Deflection



B - Coefficient 'B' of Load Deflection Equation
 H - Thickness of Base Course
 K_1 - Coefficient for Base Course Material Model
 $B = CE + CF \times H + CG \times H^2$
 where $CE = 1.36543 \times 10^{-6} \times K_1^{0.185895}$
 $CF = 3.15679 \times 10^{-6} + 3.24823 \times 10^{-11} \times K_1 - 1.05093 \times 10^{-16} \times K_1^2$
 $CG = -1.74866 \times 10^{-7} + 1.00162 \times 10^{-11} \times K_1 + 2.3941 \times 10^{-17} \times K_1^2$

FIGURE 22. Determination of Positive Value of Coefficient B

on ILLI-PAVE analyses. For the negative value of Coefficient B, refer to Figure 23. This value is a linear function of the value of Coefficient A of the load deflection equation. As a check, it was observed that for a positive value of Coefficient B, if the calculated maximum deflection differs from the measured value by more than 20%, then it should be replaced with a negative value which can be found from Figure 23. This step was always found to provide a satisfactory load deflection equation. Figure 23 shows a linear relation between the negative values of Coefficient B and Coefficient A, both of which were calculated from measured deflections.

8. Determination of the Multiplier for the initial slope. This Multiplier when applied to the initial slope (stiffness) of the load deflection curve is the slope of the unloading path describing the deflection of the pavement after the passage of a wheel load. Sixty four light pavement sections from five Farm-to-Market roads, namely FM 418 and FM 365 from District 20, FM 665 in District 16, FM 612 in District 8 and FM 1381 in District 13 were used to backcalculate this Multiplier. Values of this Multiplier from these sections were found to vary from about 0.90 to 1.7. Figure 24 shows a method of estimating this value. However, if the rut depth and the number of passes of a known load is available, for that particular road, the Multiplier can be back-figured from the equation

$$\text{Multiplier} = \Delta P_m \left(\frac{A P_m}{1 - B P_m} \right) - \Delta_m \quad (6)$$

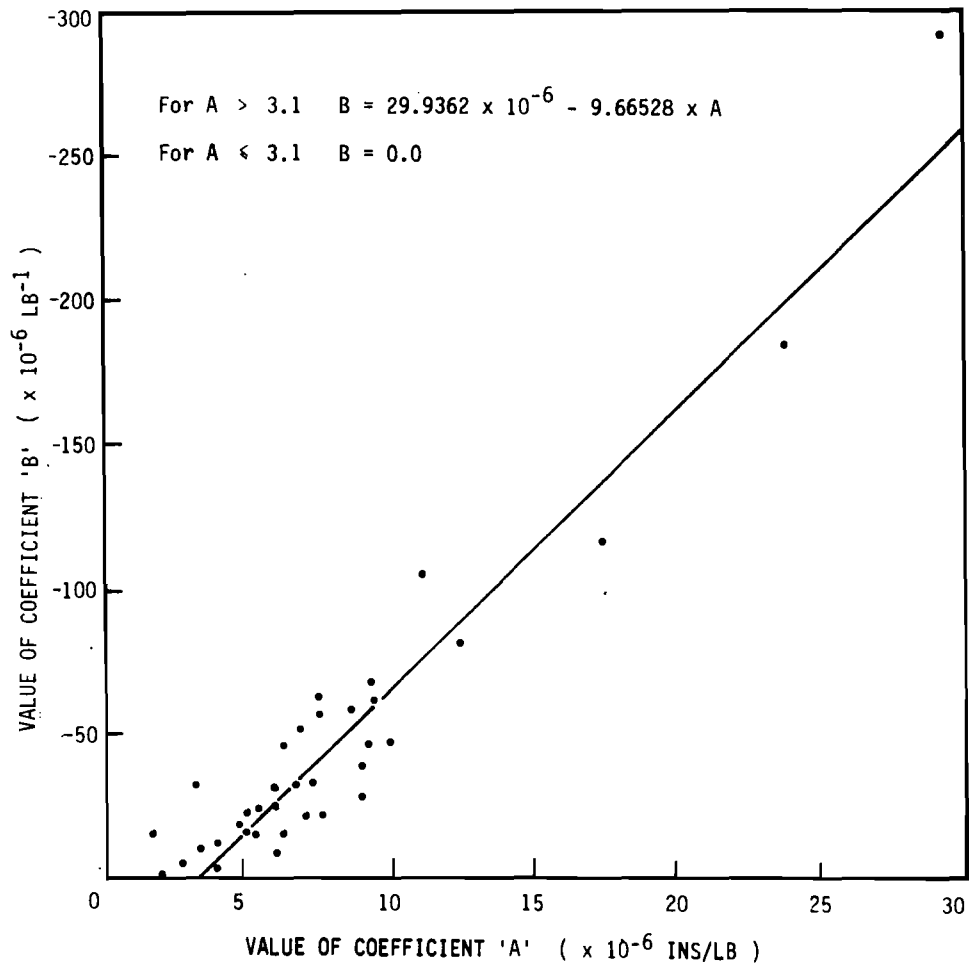


FIGURE 23. Determination of Negative Value of Coefficient B

AMULT - Multiplier of the Initial Stiffness
 [Slope of the Load Deflection Curve at near Zero Load]
 B - Coefficient 'B' in the Load Deflection Curve

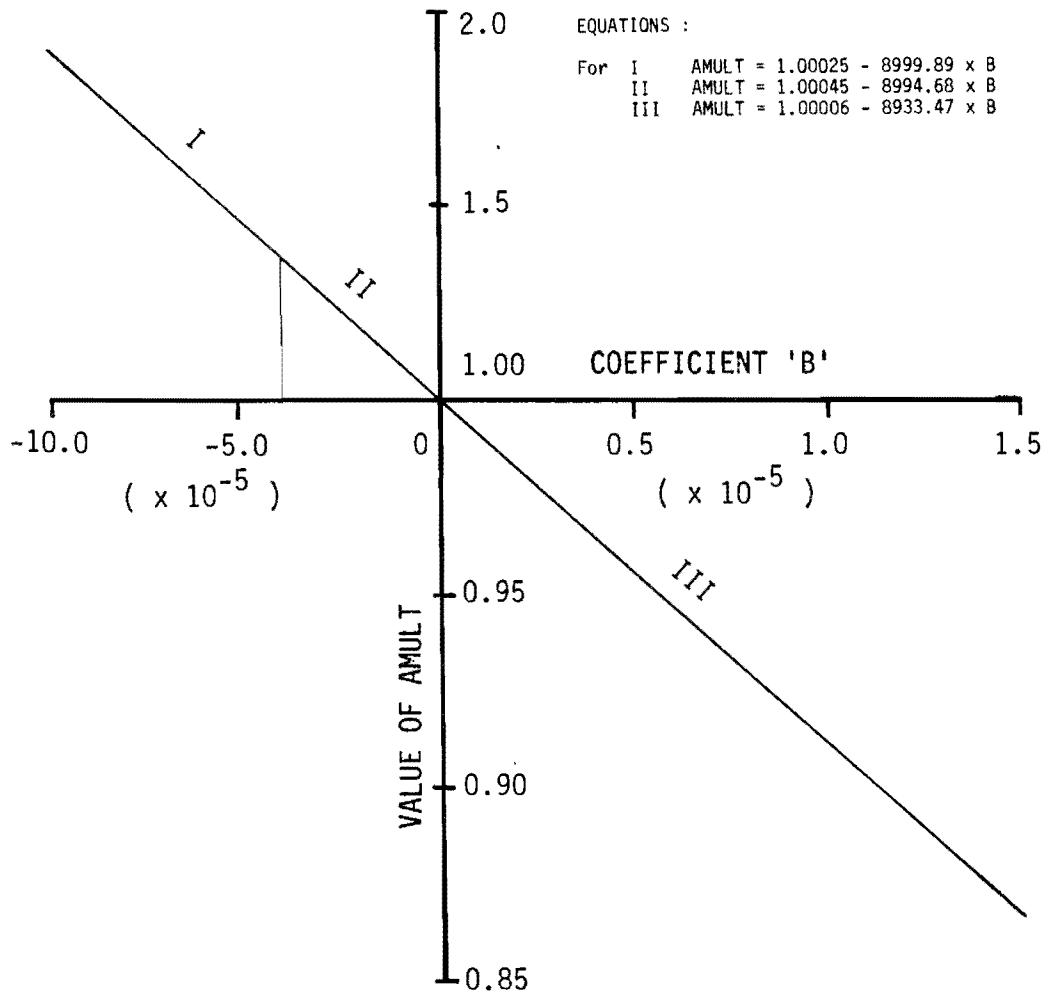


FIGURE 24. Determination of Multiplier of the Initial Slope (Stiffness) for the Unloading Path

where

P_m = measured load

Δ_m = measured rut depth/measured number of passes of P_m

9. Determine the allowable number of passes. The number of passes of a desired load that will cause a specified rut depth can be easily found from the following expression.

$$N_x = \frac{R_x}{\left(\Delta_N - \frac{A P_x}{\text{Multiplier}}\right)} \quad (7)$$

where

N_x = the allowable number of passes of a load equal to P_x

R_x = specified rut depth

P_x = specified load and

$$\Delta_N = \frac{A P_x}{(1 - B P_x)}$$

In the case of a set of different loads considered as a single pass, as for that of a multiple axle truck,

$$N_x = \frac{R_x}{\sum_{i=1}^n \left(\Delta N_i - \frac{A P_{xi}}{\text{Multiplier}}\right)}$$

where

n = the number of loads in the set

B. Procedure Using the Dynaflect

1. Obtain field measured response of pavement with a Dynaflect.

2. Determine the equivalent FWD deflection for the reading at Dynaflect Sensor No.1. As this approach is based on that described earlier for the FWD, the maximum Dynaflect deflection must be correlated with that of the FWD. Figure 25 shows the relationship between the pavement Overall Stiffness as measured with a Dynaflect with that obtained from the FWD. The equivalent FWD deflection can be calculated from the following expression:

$$\text{FWD deflection} = - 7.24474 + (29.6906 \times \text{Dynaflect Deflection}) \quad (8)$$

3. Determine Coefficient A of the load deflection equation. The equivalent FWD Overall Stiffness can be obtained from Figure 25. The Initial Stiffness which is the slope of the load deflection curve near a zero load, is then read off Figure 18 and the inverse of this is the value of Coefficient A.

4. Determine the type of subgrade. This is found from Figure 19 using the Dynaflect reading at Sensor No.5 .

5. Determine the Standard Deflection. This value can be obtained from Figure 20 using the equivalent FWD deflection.

6. Determine the base course material model. By interpolating from the curves shown in Figure 21, using the equivalent FWD deflection, the K_1 -value of the base course material can be found. The base course thickness must necessarily be known.

7. Determine the Coefficient B of the load deflection equation. The positive value of the Coefficient B can be interpolated from the curves shown in Figure 22. Different scales for the value of Coefficient B are given to adjust for the different subgrade encountered. For the negative value of Coefficient B, refer to Figure

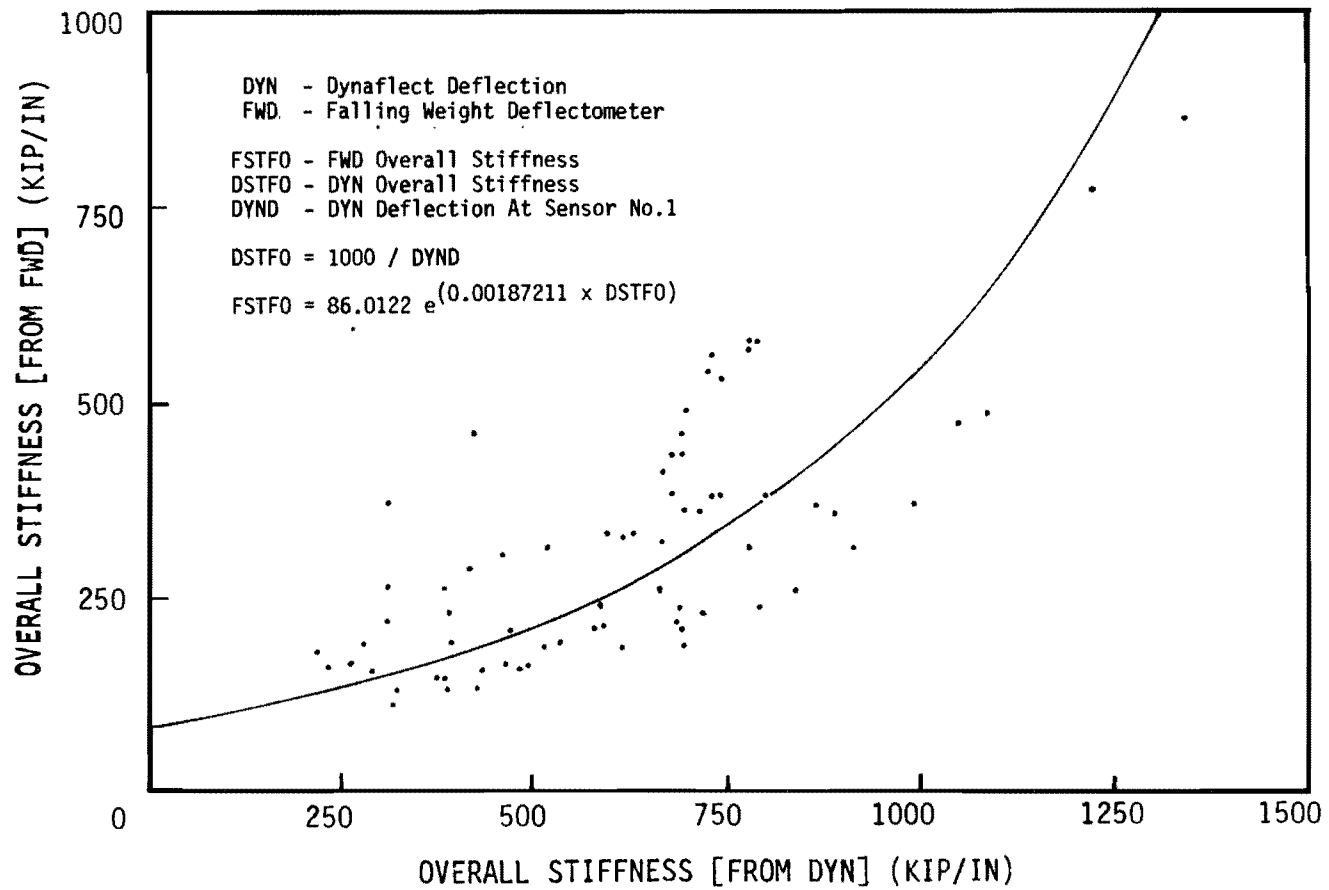


FIGURE 25. Determination of Equivalent FWD Overall Stiffness from Dynaflect Overall Stiffness

23. As a check, it was observed that for a positive value of Coefficient B, if the the calculated maximum deflection differs from the measured value by more than 20%, then it should be replaced with a negative value from Figure 23.

8. Determination of the Multiplier for the initial slope. Figure 24 shows a method of estimating this value. If the rut depth and the number of passes of a known load is available, for that particular road, the Multiplier can be back-figured from the equation

$$\text{Multiplier} = A P_m \left(\frac{A P_m}{1 - B P_m} \right) - \Delta_m \quad (6)$$

where

P_m = measured load

Δ_m = measured rut depth/measured of no. of passes of P_m .

9. Determine the allowable number of passes. The number of passes of a desired load that will cause a specified rut depth can be easily found from the following expression.

$$N_x = \frac{R_x}{\left(\Delta N - \frac{A P_x}{\text{Multiplier}} \right)} \quad (7)$$

where

N_x = the allowable number of passes

R_x = specified rut depth

P_x = specified load and

$$\Delta_N = \frac{A P_x}{(1 - B P_x)}$$

In the case of a set of different loads considered as a single pass, as for that of a multiple axle truck,

$$N_x = \frac{R_x}{\sum_{i=1}^n \left(\Delta N_i - \frac{A P_{xi}}{\text{Multiplier}} \right)}$$

where

n = the number of loads in the set.

Summary of Load Rating Procedure

Tables 4 and 5 give a summary of the load rating procedure with the use of a Falling Weight Deflectometer and a Dynaflect, respectively.

The Computer Program

A computer program LOADRATE, written in FORTRAN language, facilitates the load rating procedure developed in this study. With slight modifications, this program can also be executed on micro-computers.

This program can calculate the number of passes of a specified load that will cause a specified critical level of rut depth for every section where a deflection basin is input, and then give the average of the number of passes allowed for that particular road. The deflection basin can either be that obtained using a Falling Weight Deflectometer or the Dynaflect. It also has an option to print the

TABLE 4. Summary of Load Rating Procedure Using
a Falling Weight Deflectometer

LOAD RATING PROCEDURE USING A FALLING WEIGHT DEFLECTOMETER

1. Obtain deflection basin of pavement to a FWD pressure of about 100 psi which corresponds to a 10956 lbs load.
2. Adjust measured deflections at Sensor Nos. 1 and 7 to their equivalent values at the Standard FWD Load.
Multiply by $10956 \div \text{applied load measured}$
3. Determine Coefficient A of the Load Deflection Equation.
Overall Stiffness = applied load \div maximum deflection
Read Initial Stiffness from Figure 18.
Coefficient A = $1 \div \text{Initial Stiffness}$
4. Determine the type of subgrade from Figure 19.
5. Determine the Standard Deflection from Figure 20.
6. Determine the K_1 -value of the base course material model from Figure 21.
7. Determine the Coefficient B of the Load Deflection Equation from Figure 22. If the value is negative, use Figure 23.
Check : for positive B,
if $10956 \times A \div (1 - 10956 \times B)$
differs more than 20% from the Standard Deflection,
use Figure 23 for a new value of B.
8. Determine the Multiplier for the Initial Stiffness.
If the rut depth R_m and the number of passes N of a load P_m are known,
use $A P_m \left(\frac{A P_m}{1 - B P_m} \right) - \Delta_m$
and $\Delta_m = R_m \div N$
9. Determine the allowable number of passes N_x of a desired load P_x that will cause a specified rut R_x .
$$N_x = \frac{R_x}{\left(\Delta_N - \frac{A P_x}{\text{Multiplier}} \right)}$$

and $\Delta_N = \frac{A P_x}{1 - B P_x}$

TABLE 5. Summary of Load Rating Procedure Using a Dynaflect

LOAD RATING PROCEDURE USING A DYNAFLECT

1. Obtain deflection basin of pavement using a Dynaflect.
2. Calculate the equivalent maximum FWD deflection.
Use $-7.24474 + (29.6906 \times \text{Dynaflect maximum deflection})$
3. Determine Coefficient A of the Load Deflection Equation.
Overall Stiffness = applied load \div max. FWD deflection
Read Initial Stiffness from Figure 18.
Coefficient A = $1 \div$ Initial Stiffness
4. Determine the type of subgrade from Figure 19.
5. Determine the Standard Deflection from Figure 20.
6. Determine the K_1 -value of the base course material model from Figure 21.
7. Determine the Coefficient B of the Load Deflection Equation from Figure 22. If the value is negative, use Figure 23.
Check : for positive B,
if $10956 \times A \div (1 - 10956 \times B)$
differs more than 20% from the Standard Deflection,
use Figure 23 for a new value of B.
8. Determine the Multiplier for the Initial Stiffness.
If the rut depth R_m and the number of passes N of a load P_m are known,
use
$$A P_m \left(\frac{A P_m}{1 - B P_m} \right) - \Delta_m$$

and $\Delta_m = R_m \div N$
9. Determine the allowable number of passes N_x of a desired load P_x that will cause a specified rut R_x .
$$N_x = \frac{R_x}{\left(\Delta_N - \frac{A P_x}{\text{Multiplier}} \right)}$$

and
$$\Delta_N = \frac{A P_x}{1 - B P_x}$$

material model of the base course and the subgrade for each section considered. The program uses English Units for computation.

Appendix C gives the program documentation including the flow-charts for the main program and the four subroutines. The descriptions of input parameters and the input format is given as a preprocessor in the program listing. A sample input follows the listing of the main program and the subroutines. The output to the sample problem run is also given.

Evaluation of the Accuracy of the Procedure

In the development of this procedure, inaccuracy due to human error is minimized. Readings from the FWD are in the form of a computer read-out. However, those from the Dynaflect are from dial readings. In the correlation of data, valid statistical methods of regression were used to get the best-fit. Despite this, departures observed must be accepted as the normal variation in pavements.

With regards to regression analyses done with field data, Table 6 shows the degree of correlation obtained. It can be seen that those parameters describing the behavior of the pavement structure are better correlated than those between the readings from the FWD and the Dynaflect.

The degree of accuracy of the simulated load deflection model can be seen from Figures 26 through 29. The figures compare the measured maximum deflections of the test sections with those obtained in the procedure at four different load levels using FWD readings. It can be

TABLE 6. Degree of Correlation of Regression Analyses

Description	No. of Data Points	Coefficient of Determination (R^2)	Coefficient of Correlation
FIGURE 18. Determination of Initial Slope (Stiffness) of Load Deflection Curve	72	0.9668	0.9833
FIGURE 19. Determination of Subgrade Soil Model from Deflection	69	0.4936	0.7025
FIGURE 23. Determination of Negative Value of Coefficient B	36	0.9105	0.9542
FIGURE 25. Determination of Equivalent FWD Overall Stiffness from Dynaflect Overall Stiffness	68	0.5031	0.7093

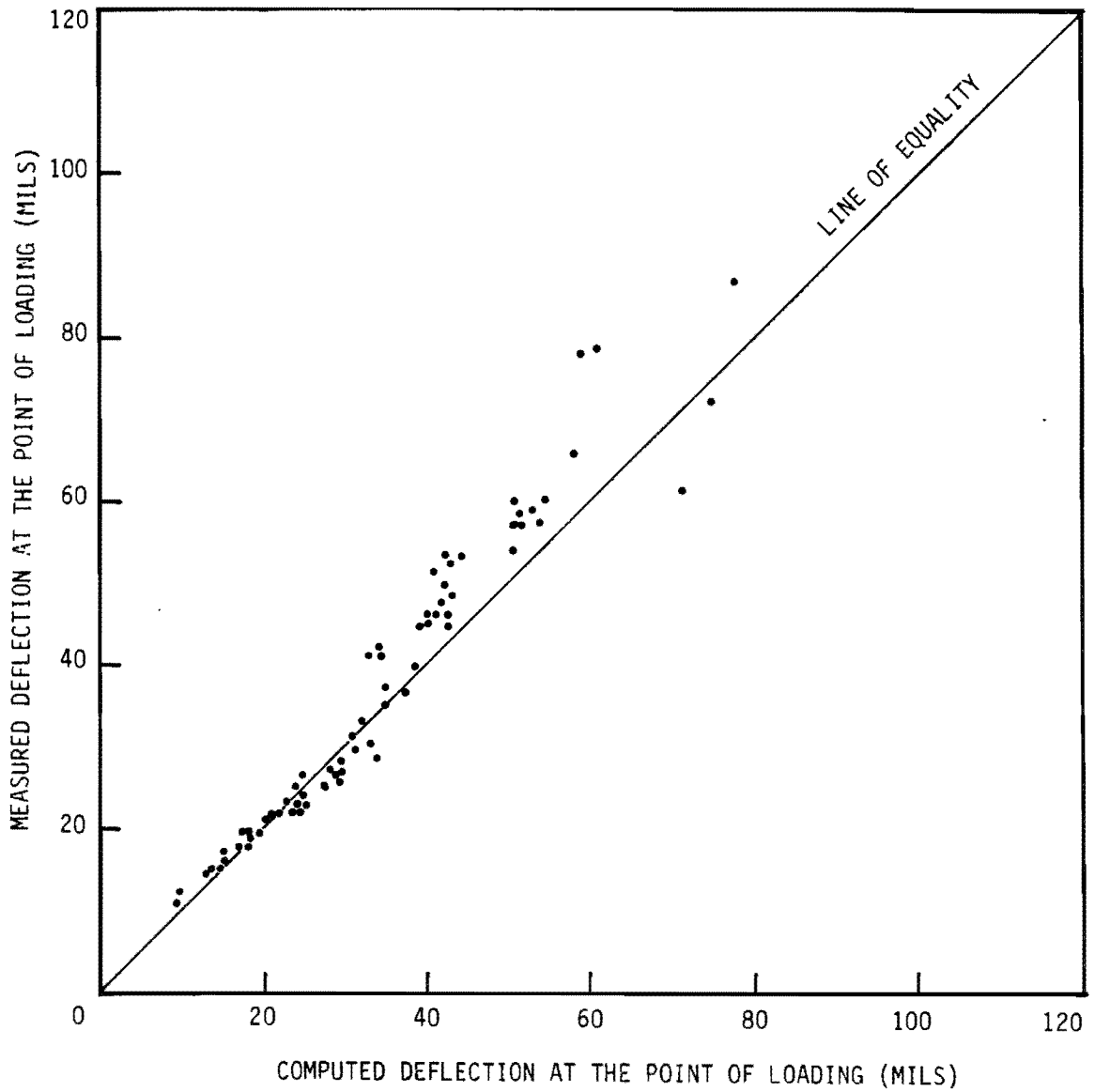


FIGURE 26. Comparison of Measured Deflections with Computed Deflections at about 9000 lbs. Loading

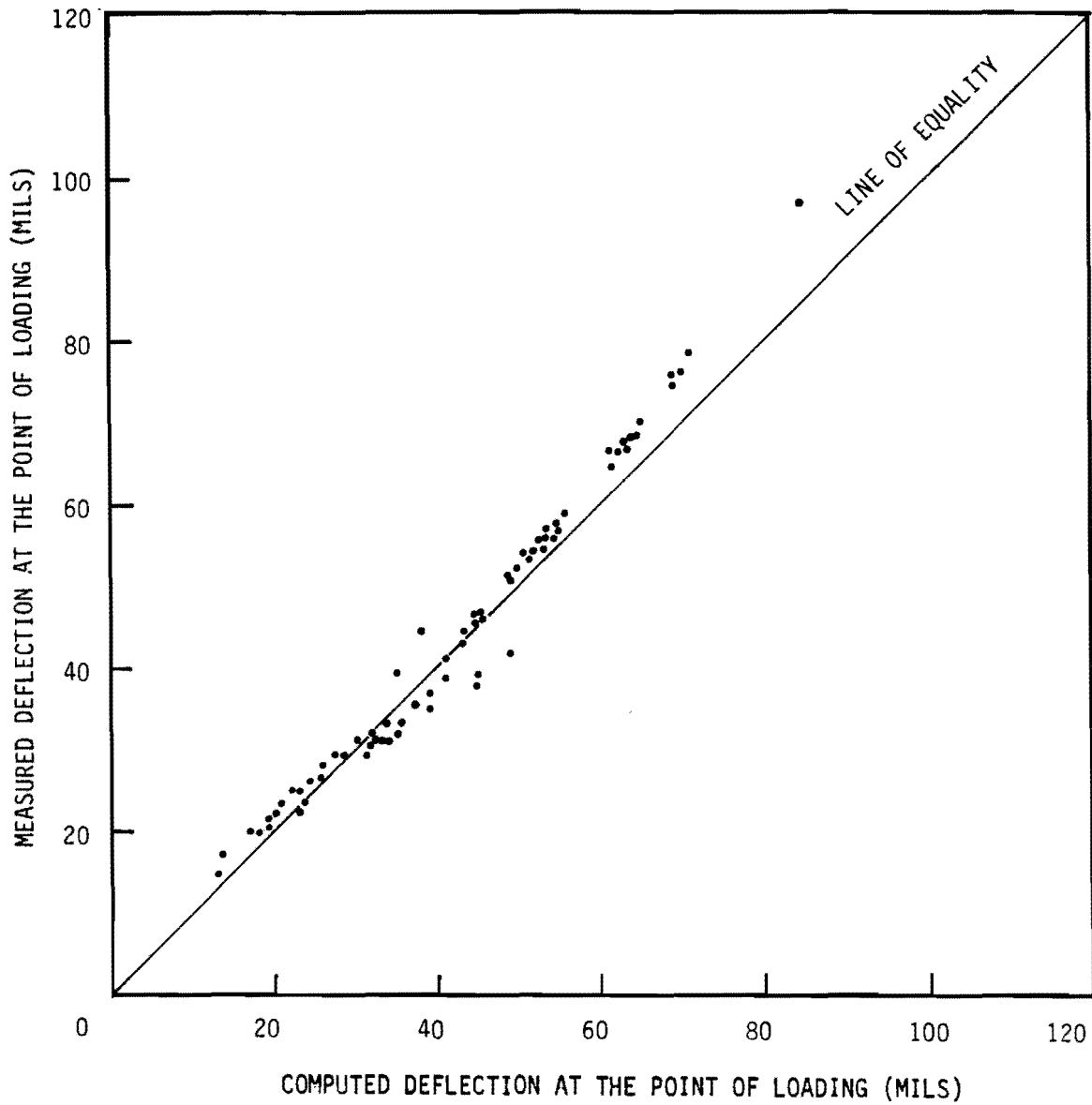


FIGURE 27. Comparison of Measured Deflections with Computed Deflections at about 11000 lbs. Loading

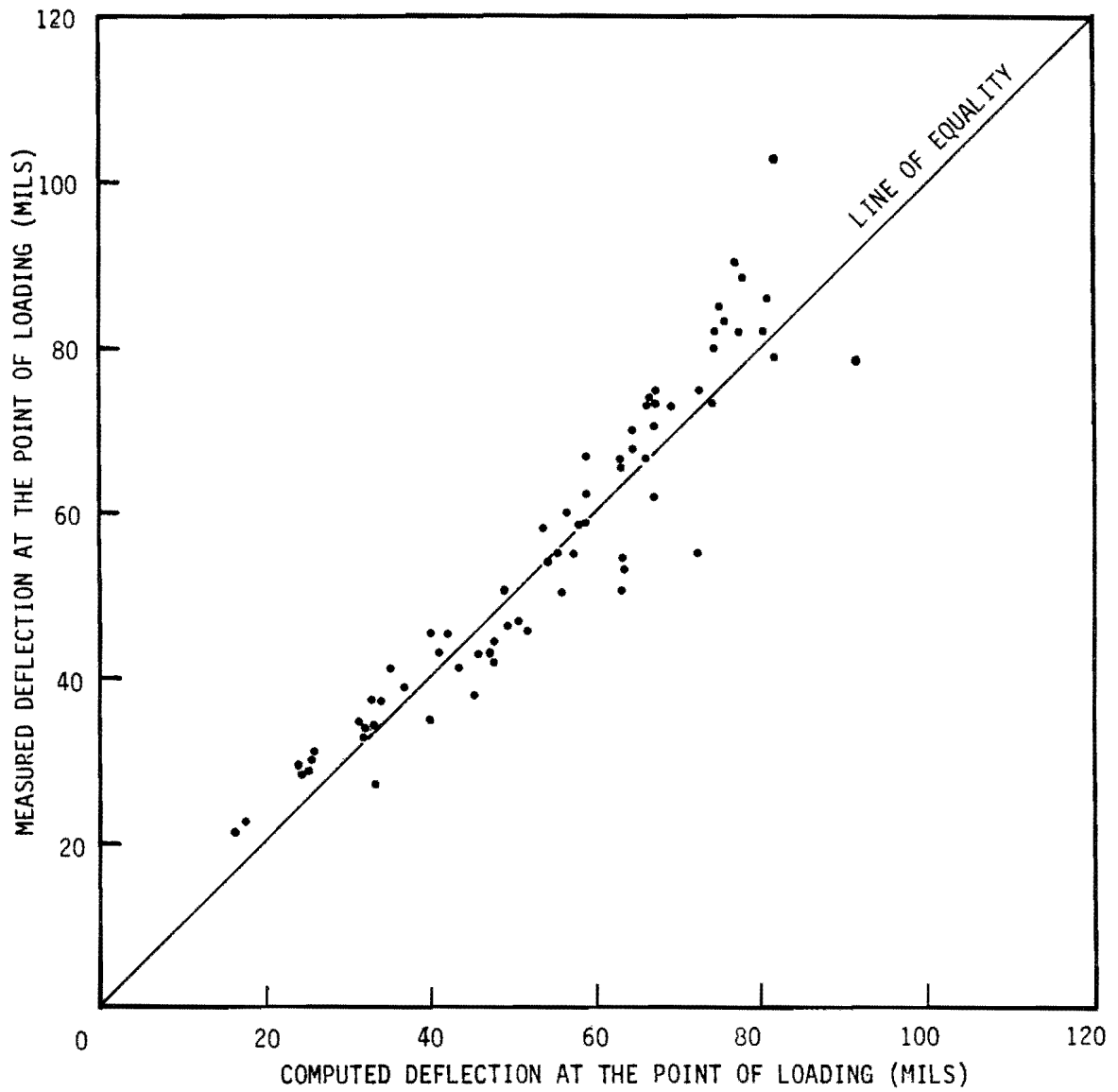


FIGURE 28. Comparison of Measured Deflections with Computed Deflections at about 15000 lbs. Loading

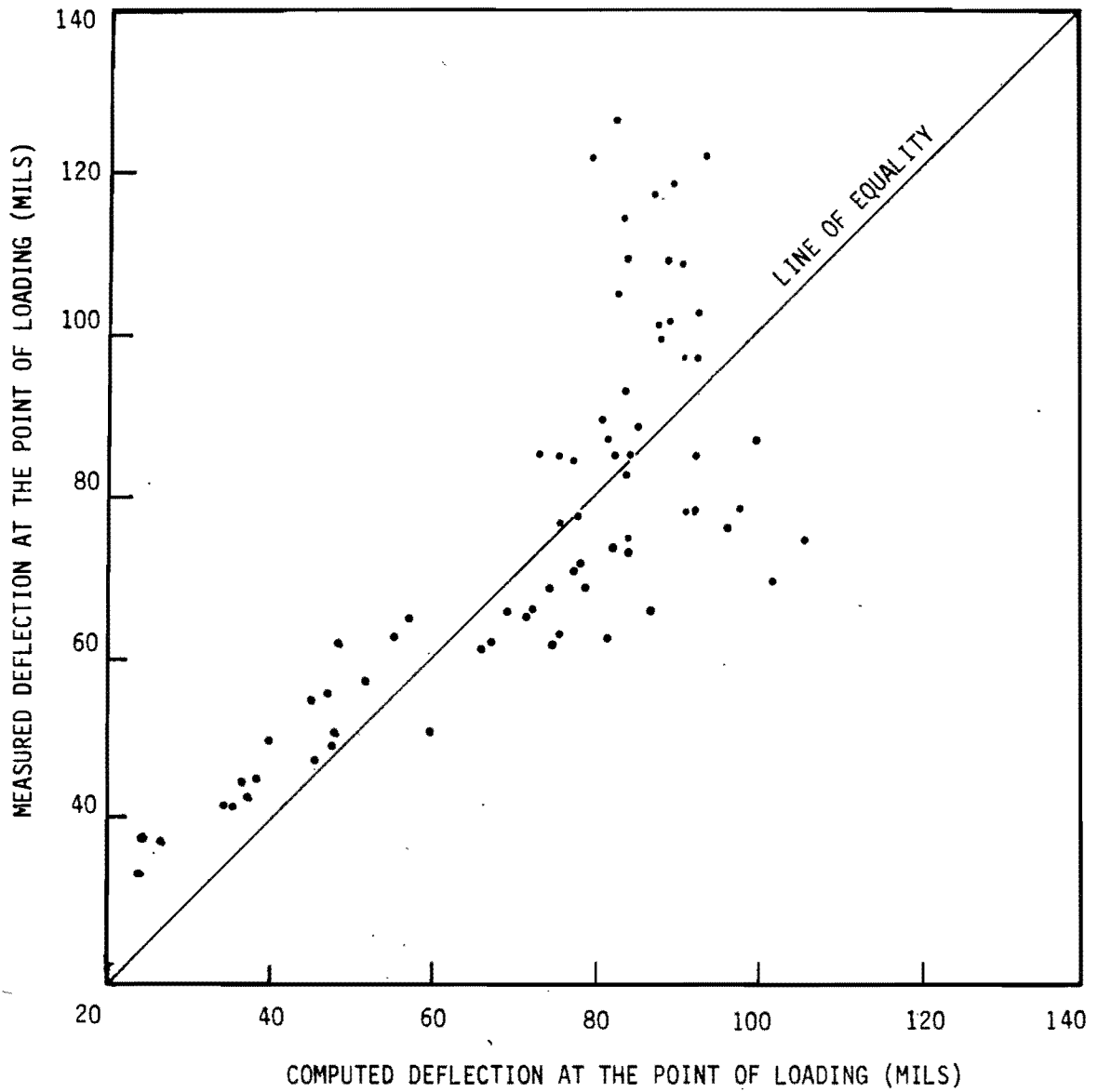


FIGURE 29. Comparison of Measured Deflections with Computed Deflections at about 23000 lbs. Loading

seen that the best result was obtained at the 11000 lbs load level. This was because the material models for the base course and the subgrade were determined based on a 100 psi loading from a FWD. At the 24,000 lbs load level, the deviations were more pronounced. At a lower load level, the load deflection curve seems to closely match that obtained from the field data. It should be noted that the procedure presented uses only one deflection basin. The accuracy of the approach using the FWD is an indication of the accuracy of that approach when using the Dynaflect as the latter was based on the former.

When evaluating the accuracy of the rutting model, it was observed that the analysis is very sensitive to the value of the slope Multiplier. Backcalculation of the number of passes for those sections used to derive the expression for the Multiplier showed that for certain cases, only the order of magnitude could be reproduced. This might be avoided if some rut history were available to compute the Multiplier.

CHAPTER V

CONCLUSIONS AND RECOMMENDATIONS

This report presents a new procedure for the load rating of light pavement structures using the Falling Weight Deflectometer or the Dynaflect. A computer program was developed for the procedure. In the course of the study, the following were accomplished.

1. It was found that light pavement structures such as those commonly found in the Farm-to-Market roads, show either a stress-softening form or a stress-hardening form of load-deflection behavior.

2. It was shown that a hyperbolic stress-strain relationship or load-deflection may be used to describe both the stress-softening as well as the stress-hardening form of the load-deflection characteristics of the light pavements.

3. The ILLI-PAVE finite element pavement analysis program was again verified to show that it was capable of simulating deflection basins of flexible light pavement structures to match those measured in the field.

4. A procedure for determining the nonlinear elastic material models for the base course and the subgrade using the Falling Weight Deflectometer or the Dynaflect was developed.

5. It was shown from the Pavement Dynamic Cone Penetrometer data that the stiffness of the granular base course may depend more on the degree of compaction of the material than the shape or hardness of the aggregates.

6. A model of the repetitive loading on pavements was proposed, which assumes a hyperbolic-shaped load-deflection curve with a linear unloading path. The slope of this unloading line was found to be smaller than the initial slope of the load deflection curve for the stress-softening type of pavement but was larger for the stress-hardening type.

7. Pavements with a thicker base course were usually found to show a stress-hardening form of load deflection behavior. This form is more resistant to rutting than the stress-softening form.

8. It was shown that the proposed procedure is capable of reproducing the load-deflection characteristics of the pavement sections tested.

It is recommended that this new procedure of load rating of light pavement structures should be implemented in the State of Texas to alleviate the problem faced in the evaluation of overweight vehicle permit applications.

The following studies may be considered to allow a better understanding of the problem of rutting:

1. The determination of factors that are causing some pavements to show a stress-hardening form of load-deflection behavior should be attempted. This may lead to a design procedure of pavements that will be more resistant to rutting.

2. A more rigorous study of the loading, unloading and reloading characteristic of pavements in the field should be carried out. Such

a study will involve the monitoring of deflections in the pavement layers in the field under repeated loading and unloading.

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APPENDICES

APPENDIX A - FIELD DATA

Falling Weight Deflectometer Readings



TEXAS TRANSPORTATION INSTITUTE
 DYNATEST 8000 FALLING WEIGHT DEFLECTOMETER
 DATA OF TEST DONE IN DECEMBER 1982

NOTATIONS : (U) REFERS TO MICROMETERS
 : (KPA) IS KILOPASCAL

DISTRICT: 17 COUNTY: BURLESON SECTION: FM3058 4.0L
 LD.(KPA) 566 732 992 1408
 DEF: 1 (U) 606 751 1036 1558
 DEF: 2 (U) 305 389 625 842
 DEF: 3 (U) 198 264 385 595
 DEF: 4 (U) 73 97 139 200
 DEF: 5 (U) 32 42 63 87
 DEF: 6 (U) 26 35 42 53
 DEF: 7 (U) 17 22 35 50

DISTRICT: 17 COUNTY: BURLESON SECTION: FM3058 4.1L
 LD.(KPA) 559 722 993 1407
 DEF: 1 (U) 462 590 831 1238
 DEF: 2 (U) 285 385 570 969
 DEF: 3 (U) 196 263 383 600
 DEF: 4 (U) 72 96 142 201
 DEF: 5 (U) 33 40 58 85
 DEF: 6 (U) 19 25 38 56
 DEF: 7 (U) 17 21 31 47

DISTRICT: 17 COUNTY: BURLESON SECTION: FM3058 4.2L
 LD.(KPA) 570 735 999 1409
 DEF: 1 (U) 463 606 864 1304
 DEF: 2 (U) 270 358 487 776
 DEF: 3 (U) 170 234 341 587
 DEF: 4 (U) 81 87 126 185
 DEF: 5 (U) 28 36 56 83
 DEF: 6 (U) 33 28 36 50
 DEF: 7 (U) 16 20 29 44

DISTRICT: 17 COUNTY: BURLESON SECTION: FM3058 8.0L
 LD.(KPA) 570 737 1008 1448
 DEF: 1 (U) 650 834 1159 1592
 DEF: 2 (U) 368 483 690 1010
 DEF: 3 (U) 259 352 505 746
 DEF: 4 (U) 119 151 235 334
 DEF: 5 (U) 48 70 102 146
 DEF: 6 (U) 19 40 59 66
 DEF: 7 (U) 22 32 43 58

DISTRICT: 17 COUNTY: BURLESON SECTION: FM3058 8.1L				
LD.(KPA)	559	733	984	1434
DEF: 1 (U)	772	958	1280	1756
DEF: 2 (U)	378	494	658	997
DEF: 3 (U)	243	331	463	688
DEF: 4 (U)	103	183	206	371
DEF: 5 (U)	41	64	89	128
DEF: 6 (U)	20	38	38	106
DEF: 7 (U)	19	29	40	58

DISTRICT: 17 COUNTY: BURLESON SECTION: FM3058 8.2L				
LD.(KPA)	551	716	989	1439
DEF: 1 (U)	608	778	1069	1554
DEF: 2 (U)	373	486	683	972
DEF: 3 (U)	261	350	495	724
DEF: 4 (U)	117	166	242	350
DEF: 5 (U)	52	74	104	150
DEF: 6 (U)	28	40	59	90
DEF: 7 (U)	23	30	45	62

DISTRICT: 17 COUNTY: BURLESON SECTION: FM3058 2.0				
LD.(KPA)	604	783	1012	1423
DEF: 1 (U)	483	639	915	1393
DEF: 2 (U)	364	484	694	1051
DEF: 3 (U)	283	382	559	846
DEF: 4 (U)	161	225	327	487
DEF: 5 (U)	81	115	168	242
DEF: 6 (U)	47	70	103	142
DEF: 7 (U)	35	51	74	108

DISTRICT: 17 COUNTY: BURLESON SECTION: FM3058 2.1				
LD.(KPA)	571	751	984	1384
DEF: 1 (U)	637	822	1155	1308
DEF: 2 (U)	379	523	737	1160
DEF: 3 (U)	299	409	593	1001
DEF: 4 (U)	163	234	333	511
DEF: 5 (U)	77	107	157	225
DEF: 6 (U)	47	67	98	143
DEF: 7 (U)	37	49	73	104

DISTRICT: 17 COUNTY: BURLESON SECTION: FM3058 2.2				
LD.(KPA)	567	752	974	1358
DEF: 1 (U)	702	906	1282	1643
DEF: 2 (U)	450	602	856	1298
DEF: 3 (U)	341	468	686	1047
DEF: 4 (U)	177	245	348	539
DEF: 5 (U)	90	122	173	262
DEF: 6 (U)	59	76	109	160
DEF: 7 (U)	41	53	75	110

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM3058	4.0H
LD.(KPA)	582	752	1010 1416
DEF: 1 (U)	530	665	915 1303
DEF: 2 (U)	341	436	603 866
DEF: 3 (U)	224	295	427 1235
DEF: 4 (U)	77	108	158 242
DEF: 5 (U)	32	45	64 90
DEF: 6 (U)	24	32	57 61
DEF: 7 (U)	18	26	36 51

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM3058	4.1H
LD.(KPA)	578	747	1000 1413
DEF: 1 (U)	440	574	793 1167
DEF: 2 (U)	314	411	572 815
DEF: 3 (U)	192	263	377 553
DEF: 4 (U)	75	105	150 217
DEF: 5 (U)	30	43	58 86
DEF: 6 (U)	21	29	42 59
DEF: 7 (U)	19	25	32 49

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM3058	4.2H
LD.(KPA)	564	732	1009 1415
DEF: 1 (U)	441	581	826 1150
DEF: 2 (U)	258	349	502 761
DEF: 3 (U)	170	237	348 525
DEF: 4 (U)	63	91	134 194
DEF: 5 (U)	25	38	53 75
DEF: 6 (U)	17	29	37 52
DEF: 7 (U)	14	23	31 44

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM3058	6.0
LD.(KPA)	571	740	998 1417
DEF: 1 (U)	684	887	1250 1682
DEF: 2 (U)	416	554	796 1202
DEF: 3 (U)	302	415	602 890
DEF: 4 (U)	140	200	288 427
DEF: 5 (U)	58	83	120 171
DEF: 6 (U)	35	53	72 106
DEF: 7 (U)	28	36	52 77

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM3058	6.1
LD.(KPA)	563	729	992 1400
DEF: 1 (U)	720	963	1394 2221
DEF: 2 (U)	345	478	696 1053
DEF: 3 (U)	248	349	506 747
DEF: 4 (U)	132	185	268 388
DEF: 5 (U)	53	76	110 157
DEF: 6 (U)	35	48	69 102
DEF: 7 (U)	30	37	58 79

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM3058	6.2
LD.(KPA)	558	736	996 1430
DEF: 1 (U)	579	785	1109 1673
DEF: 2 (U)	358	502	724 1106
DEF: 3 (U)	263	371	541 808
DEF: 4 (U)	151	212	309 461
DEF: 5 (U)	63	91	132 208
DEF: 6 (U)	43	59	83 128
DEF: 7 (U)	48	37	61 115

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM3058	8.0H
LD.(KPA)	590	760	1161 1463
DEF: 1 (U)	672	859	1330 1515
DEF: 2 (U)	376	503	806 1032
DEF: 3 (U)	230	320	573 726
DEF: 4 (U)	113	167	281 359
DEF: 5 (U)	43	69	118 146
DEF: 6 (U)	24	44	82 76
DEF: 7 (U)	61	65	51 72

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM3058	8.1H
LD.(KPA)	573	748	1004 1441
DEF: 1 (U)	796	976	1303 1859
DEF: 2 (U)	385	500	711 1034
DEF: 3 (U)	245	329	464 643
DEF: 4 (U)	112	158	227 340
DEF: 5 (U)	44	58	90 141
DEF: 6 (U)	28	35	54 83
DEF: 7 (U)	21	28	41 68

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM3058	8.2H
LD.(KPA)	568	761	1017 1464
DEF: 1 (U)	597	794	1090 1522
DEF: 2 (U)	394	541	797 1190
DEF: 3 (U)	269	374	527 770
DEF: 4 (U)	125	181	258 374
DEF: 5 (U)	52	76	109 150
DEF: 6 (U)	32	43	59 87
DEF: 7 (U)	22	32	45 289

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM3058	10.1
LD.(KPA)	519	701	973 1359
DEF: 1 (U)	1146	1416	1877 2917
DEF: 2 (U)	848	1133	1505 2062
DEF: 3 (U)	634	851	1232 1574
DEF: 4 (U)	291	396	544 773
DEF: 5 (U)	110	145	198 286
DEF: 6 (U)	64	90	121 175
DEF: 7 (U)	52	69	94 138

DISTRICT:	17	COUNTY:	BURLESON	SECTION:	FM3058 10.2
LD.(KPA)		502	669	949	1333
DEF: 1	(U)	1460	1761	2079	2603
DEF: 2	(U)	979	1244	1720	2251
DEF: 3	(U)	693	892	1200	1667
DEF: 4	(U)	278	357	484	692
DEF: 5	(U)	126	165	219	314
DEF: 6	(U)	78	106	137	195
DEF: 7	(U)	55	73	90	123

DISTRICT:	17	COUNTY:	BURLESON	SECTION:	FM908 10.0
LD.(KPA)		521	689	964	1383
DEF: 1	(U)	1528	1687	2043	2773
DEF: 2	(U)	714	797	941	1252
DEF: 3	(U)	408	476	640	886
DEF: 4	(U)	168	219	271	364
DEF: 5	(U)	88	117	164	222
DEF: 6	(U)	51	148	202	312
DEF: 7	(U)	66	50	69	94

DISTRICT:	17	COUNTY:	BURLESON	SECTION:	FM908 10.1
LD.(KPA)		502	686	977	1396
DEF: 1	(U)	1293	1381	1713	1901
DEF: 2	(U)	832	916	1074	1282
DEF: 3	(U)	456	535	703	929
DEF: 4	(U)	148	190	260	359
DEF: 5	(U)	94	121	170	236
DEF: 6	(U)	63	79	113	157
DEF: 7	(U)	47	56	80	110

DISTRICT:	17	COUNTY:	BURLESON	SECTION:	FM908 10.2
LD.(KPA)		502	680	965	1377
DEF: 1	(U)	1355	1468	1843	2251
DEF: 2	(U)	785	903	1109	1473
DEF: 3	(U)	423	506	657	883
DEF: 4	(U)	149	187	253	371
DEF: 5	(U)	91	118	165	231
DEF: 6	(U)	66	83	115	157
DEF: 7	(U)	42	51	75	102

DISTRICT:	17	COUNTY:	BURLESON	SECTION:	FM1361 6.0
LD.(KPA)		495	677	948	1363
DEF: 1	(U)	1686	1938	2199	2146
DEF: 2	(U)	1123	1284	1641	2196
DEF: 3	(U)	675	864	1181	1640
DEF: 4	(U)	184	255	332	539
DEF: 5	(U)	70	95	140	205
DEF: 6	(U)	44	64	75	121
DEF: 7	(U)	33	47	63	92

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM1361 6.1
LD.(KPA)	520	693 971 1372
DEF: 1 (U)	1443	1663 2082 2552
DEF: 2 (U)	799	886 1540 1941
DEF: 3 (U)	491	646 903 1259
DEF: 4 (U)	92	188 332 485
DEF: 5 (U)	60	86 131 189
DEF: 6 (U)	4	47 97 91
DEF: 7 (U)	31	40 61 149

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM1361 6.2
LD.(KPA)	502	673 954 1368
DEF: 1 (U)	1534	1792 2248 2475
DEF: 2 (U)	977	1167 1324 1927
DEF: 3 (U)	538	725 1044 1328
DEF: 4 (U)	196	216 288 467
DEF: 5 (U)	54	77 113 159
DEF: 6 (U)	62	116 58 102
DEF: 7 (U)	33	53 72 136

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM1361 8.0
LD.(KPA)	480	666 929 1322
DEF: 1 (U)	1821	2015 2928 2459
DEF: 2 (U)	1281	1428 1871 2343
DEF: 3 (U)	700	864 1205 1687
DEF: 4 (U)	221	287 420 598
DEF: 5 (U)	75	101 143 202
DEF: 6 (U)	32	52 76 120
DEF: 7 (U)	32	41 60 80

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM1361 8.1
LD.(KPA)	478	666 958 1341
DEF: 1 (U)	1832	1918 2628 2631
DEF: 2 (U)	1284	1559 1969 2207
DEF: 3 (U)	830	1005 1349 1790
DEF: 4 (U)	271	329 444 626
DEF: 5 (U)	72	94 133 191
DEF: 6 (U)	36	53 76 105
DEF: 7 (U)	27	41 57 82

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM1361 8.2
LD.(KPA)	481	659 930 1359
DEF: 1 (U)	1505	1722 2116 2772
DEF: 2 (U)	1194	1279 1685 2246
DEF: 3 (U)	661	798 1071 1489
DEF: 4 (U)	177	211 300 451
DEF: 5 (U)	53	71 99 138
DEF: 6 (U)	72	60 76 104
DEF: 7 (U)	8	32 48 73

DISTRICT:	17	COUNTY:	BURLESON	SECTION:	FM1361 10.0
LD.(KPA)		473	649	903	1295
DEF: 1	(U)	2006	1980	2006	2004
DEF: 2	(U)	1310	1600	1956	1938
DEF: 3	(U)	758	992	1388	1976
DEF: 4	(U)	254	358	495	617
DEF: 5	(U)	99	137	198	278
DEF: 6	(U)	67	91	110	240
DEF: 7	(U)	49	71	100	168

DISTRICT:	17	COUNTY:	BURLESON	SECTION:	FM1361 10.1
LD.(KPA)		479	658	908	1288
DEF: 1	(U)	2231	2473	1981	2008
DEF: 2	(U)	1297	1513	2121	3021
DEF: 3	(U)	772	1010	1401	1969
DEF: 4	(U)	211	333	429	548
DEF: 5	(U)	101	143	203	291
DEF: 6	(U)	35	84	105	230
DEF: 7	(U)	46	68	89	135

DISTRICT:	17	COUNTY:	BURLESON	SECTION:	FM1361 10.2
LD.(KPA)		483	662	915	1306
DEF: 1	(U)	1995	1937	2086	2017
DEF: 2	(U)	1183	1377	1805	2464
DEF: 3	(U)	622	833	1226	1733
DEF: 4	(U)	203	310	408	553
DEF: 5	(U)	87	121	171	243
DEF: 6	(U)	50	99	84	180
DEF: 7	(U)	43	67	81	143

DISTRICT:	17	COUNTY:	BURLESON	SECTION:	FM1362 4.0
LD.(KPA)		530	725	984	1410
DEF: 1	(U)	991	1091	1438	1986
DEF: 2	(U)	661	743	995	1411
DEF: 3	(U)	457	522	709	1010
DEF: 4	(U)	198	249	349	498
DEF: 5	(U)	95	126	176	244
DEF: 6	(U)	50	75	96	138
DEF: 7	(U)	237	231	331	606

DISTRICT:	17	COUNTY:	BURLESON	SECTION:	FM1362 4.1
LD.(KPA)		521	714	976	1400
DEF: 1	(U)	1139	1221	1591	1877
DEF: 2	(U)	612	692	941	1332
DEF: 3	(U)	408	476	650	913
DEF: 4	(U)	187	242	335	467
DEF: 5	(U)	86	123	165	236
DEF: 6	(U)	44	69	93	125
DEF: 7	(U)	187	193	304	524

DISTRICT: 17		COUNTY: BURLESON		SECTION: FM1362 6.0	
LD.(KPA)		545	701	946	1323
DEF: 1	(U)	964	1322	1902	2764
DEF: 2	(U)	559	786	1183	1771
DEF: 3	(U)	396	551	827	1220
DEF: 4	(U)	151	206	293	410
DEF: 5	(U)	70	101	145	202
DEF: 6	(U)	231	187	325	515
DEF: 7	(U)	123	118	164	268

DISTRICT: 17		COUNTY: BURLESON		SECTION: FM1362 6.1	
LD.(KPA)		526	676	920	1297
DEF: 1	(U)	1082	1472	2146	2336
DEF: 2	(U)	625	881	1319	2005
DEF: 3	(U)	406	576	861	1310
DEF: 4	(U)	134	193	269	370
DEF: 5	(U)	71	100	143	199
DEF: 6	(U)	366	343	437	611
DEF: 7	(U)	152	157	244	350

DISTRICT: 17		COUNTY: BURLESON		SECTION: FM1362 8.0	
LD.(KPA)		591	787	1008	1448
DEF: 1	(U)	504	669	940	1561
DEF: 2	(U)	291	389	547	835
DEF: 3	(U)	204	275	385	558
DEF: 4	(U)	87	124	175	265
DEF: 5	(U)	37	52	75	110
DEF: 6	(U)	23	35	46	69
DEF: 7	(U)	70	90	147	186

DISTRICT: 17		COUNTY: BURLESON		SECTION: FM1362 8.1	
LD.(KPA)		557	748	977	1410
DEF: 1	(U)	659	848	1175	1721
DEF: 2	(U)	409	531	771	1382
DEF: 3	(U)	275	360	504	726
DEF: 4	(U)	110	149	211	306
DEF: 5	(U)	41	59	82	122
DEF: 6	(U)	30	40	53	78
DEF: 7	(U)	124	138	202	286

DISTRICT: 17		COUNTY: BURLESON		SECTION: FM2000 8.0	
LD.(KPA)		526	699	985	1403
DEF: 1	(U)	1272	1394	1798	2103
DEF: 2	(U)	751	836	1128	2452
DEF: 3	(U)	469	538	735	1041
DEF: 4	(U)	214	262	366	506
DEF: 5	(U)	93	128	179	237
DEF: 6	(U)	49	73	97	122
DEF: 7	(U)	38	55	62	84

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM2000	8.1
LD.(KPA)	513	691	977 1386
DEF: 1 (U)	1381	1422	1560 2186
DEF: 2 (U)	761	834	1133 1577
DEF: 3 (U)	479	551	766 1107
DEF: 4 (U)	223	284	396 554
DEF: 5 (U)	100	142	197 258
DEF: 6 (U)	55	82	113 152
DEF: 7 (U)	40	52	66 60

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM2000	10.0
LD.(KPA)	558	731	1004 1429
DEF: 1 (U)	1147	1319	1432 2179
DEF: 2 (U)	710	834	1091 1476
DEF: 3 (U)	479	570	746 1004
DEF: 4 (U)	239	296	395 536
DEF: 5 (U)	94	127	174 237
DEF: 6 (U)	46	62	95 120
DEF: 7 (U)	249	254	384 459

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM2000	10.1
LD.(KPA)	554	736	992 1416
DEF: 1 (U)	1175	1365	1783 3240
DEF: 2 (U)	737	866	1130 1532
DEF: 3 (U)	501	591	769 1020
DEF: 4 (U)	256	314	415 552
DEF: 5 (U)	106	141	187 250
DEF: 6 (U)	53	75	102 141
DEF: 7 (U)	175	206	270 330

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM2000	12.0
LD.(KPA)	556	740	1011 1440
DEF: 1 (U)	582	801	1167 1768
DEF: 2 (U)	349	490	716 1094
DEF: 3 (U)	244	344	502 760
DEF: 4 (U)	130	183	260 381
DEF: 5 (U)	74	102	141 202
DEF: 6 (U)	49	66	92 131
DEF: 7 (U)	126	175	242 336

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM2000	12.1
LD.(KPA)	549	742	1015 1428
DEF: 1 (U)	601	819	1185 1619
DEF: 2 (U)	345	476	694 1055
DEF: 3 (U)	234	326	475 720
DEF: 4 (U)	131	178	255 376
DEF: 5 (U)	74	99	140 204
DEF: 6 (U)	48	62	90 130
DEF: 7 (U)	129	166	241 337

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM2155 2.0
LD.(KPA)	519 717 980 1409	
DEF: 1 (U)	1044 1122 1405 1818	
DEF: 2 (U)	627 674 887 1163	
DEF: 3 (U)	444 512 666 890	
DEF: 4 (U)	146 160 193 288	
DEF: 5 (U)	36 43 98 87	
DEF: 6 (U)	0 37 44 97	
DEF: 7 (U)	9 26 39 53	

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM2155 2.1
LD.(KPA)	515 709 981 1398	
DEF: 1 (U)	1057 1169 1493 1825	
DEF: 2 (U)	683 744 960 1207	
DEF: 3 (U)	420 503 674 909	
DEF: 4 (U)	135 158 216 336	
DEF: 5 (U)	22 38 51 73	
DEF: 6 (U)	22 40 47 70	
DEF: 7 (U)	21 27 43 56	

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM2155 2.2
LD.(KPA)	512 705 964 1407	
DEF: 1 (U)	1056 1173 1515 1748	
DEF: 2 (U)	717 748 932 1290	
DEF: 3 (U)	429 517 700 970	
DEF: 4 (U)	107 142 185 284	
DEF: 5 (U)	21 35 47 70	
DEF: 6 (U)	9 24 33 67	
DEF: 7 (U)	15 27 34 61	

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM2155 4.0
LD.(KPA)	563 727 1000 1391	
DEF: 1 (U)	564 757 1085 1656	
DEF: 2 (U)	421 578 836 1318	
DEF: 3 (U)	330 459 668 1032	
DEF: 4 (U)	167 209 313 480	
DEF: 5 (U)	61 83 118 176	
DEF: 6 (U)	28 47 58 104	
DEF: 7 (U)	31 38 49 81	

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM2155 4.1
LD.(KPA)	555 717 994 1375	
DEF: 1 (U)	568 766 1109 1682	
DEF: 2 (U)	413 569 825 1136	
DEF: 3 (U)	318 448 657 1000	
DEF: 4 (U)	164 215 322 469	
DEF: 5 (U)	62 83 121 174	
DEF: 6 (U)	48 48 68 118	
DEF: 7 (U)	28 35 52 74	

DISTRICT:	17	COUNTY:	BURLESON	SECTION:	FM2155 4.2
LD.(KPA)		550	713	991	1380
DEF:	1	(U) 670	896	1281	1888
DEF:	2	(U) 464	643	910	1172
DEF:	3	(U) 346	489	722	1211
DEF:	4	(U) 165	236	328	482
DEF:	5	(U) 61	85	120	174
DEF:	6	(U) 38	52	69	100
DEF:	7	(U) 28	38	52	77

DISTRICT:	17	COUNTY:	BRAZO	SECTION:	OSR 2.0
LD.(KPA)		608	785	997	1387
DEF:	1	(U) 555	740	1054	1586
DEF:	2	(U) 423	577	829	1268
DEF:	3	(U) 344	452	622	892
DEF:	4	(U) 202	281	402	593
DEF:	5	(U) 87	123	171	246
DEF:	6	(U) 53	73	101	144
DEF:	7	(U) 41	54	78	108

DISTRICT:	17	COUNTY:	BRAZO	SECTION:	OSR 2.1
LD.(KPA)		575	755	991	1364
DEF:	1	(U) 588	794	1144	1607
DEF:	2	(U) 419	581	839	1428
DEF:	3	(U) 356	495	770	2133
DEF:	4	(U) 224	313	452	677
DEF:	5	(U) 102	140	202	290
DEF:	6	(U) 58	80	112	162
DEF:	7	(U) 43	59	82	119

DISTRICT:	17	COUNTY:	BRAZO	SECTION:	OSR 4.0
LD.(KPA)		646	837	1066	1530
DEF:	1	(U) 291	389	545	853
DEF:	2	(U) 219	293	421	626
DEF:	3	(U) 186	253	362	542
DEF:	4	(U) 120	167	238	356
DEF:	5	(U) 69	92	131	187
DEF:	6	(U) 43	58	82	116
DEF:	7	(U) 34	44	66	77

DISTRICT:	17	COUNTY:	BRAZO	SECTION:	OSR 4.1
LD.(KPA)		606	781	1010	1485
DEF:	1	(U) 315	409	579	956
DEF:	2	(U) 223	302	431	660
DEF:	3	(U) 187	257	371	559
DEF:	4	(U) 123	172	248	358
DEF:	5	(U) 66	92	132	188
DEF:	6	(U) 41	59	82	115
DEF:	7	(U) 35	50	66	79

DISTRICT:	17	COUNTY:	BRAZO	SECTION:	FM974 6.0
LD.(KPA)		476	662	918	1312
DEF: 1	(U)	1504	1759	2299	3037
DEF: 2	(U)	800	1025	1402	1955
DEF: 3	(U)	449	580	774	1067
DEF: 4	(U)	162	242	350	513
DEF: 5	(U)	67	102	150	218
DEF: 6	(U)	42	69	96	149
DEF: 7	(U)	32	54	71	109

DISTRICT:	17	COUNTY:	BRAZO	SECTION:	FM974 6.1
LD.(KPA)		471	648	914	1295
DEF: 1	(U)	1474	4670	2154	2557
DEF: 2	(U)	826	1011	1388	1870
DEF: 3	(U)	470	610	884	1186
DEF: 4	(U)	166	234	340	480
DEF: 5	(U)	67	93	146	211
DEF: 6	(U)	50	67	116	152
DEF: 7	(U)	28	37	50	99

DISTRICT:	17	COUNTY:	BRAZO	SECTION:	FM974 8.0
LD.(KPA)		485	674	931	1341
DEF: 1	(U)	1313	1487	1906	2793
DEF: 2	(U)	812	969	1252	1708
DEF: 3	(U)	455	576	766	1000
DEF: 4	(U)	159	232	310	432
DEF: 5	(U)	75	86	130	172
DEF: 6	(U)	81	193	74	106
DEF: 7	(U)	26	43	75	84

DISTRICT:	17	COUNTY:	BRAZO	SECTION:	FM974 8.1
LD.(KPA)		488	677	934	1351
DEF: 1	(U)	1217	1437	1853	2910
DEF: 2	(U)	667	834	1108	1573
DEF: 3	(U)	380	501	682	967
DEF: 4	(U)	129	179	254	363
DEF: 5	(U)	50	70	103	151
DEF: 6	(U)	34	47	68	102
DEF: 7	(U)	25	36	52	72

DISTRICT:	17	COUNTY:	BRAZO	SECTION:	FM1179 4.0
LD.(KPA)		519	707	979	1374
DEF: 1	(U)	846	1085	1521	2168
DEF: 2	(U)	569	759	1088	1562
DEF: 3	(U)	406	556	805	1161
DEF: 4	(U)	187	265	386	547
DEF: 5	(U)	80	113	163	234
DEF: 6	(U)	46	69	99	146
DEF: 7	(U)	27	44	67	105

DISTRICT: 17	COUNTY: BRAZO	SECTION: FM1179 4.1
LD.(KPA)	524 715	974 1356
DEF: 1 (U)	756 984	1354 1932
DEF: 2 (U)	524 698	971 1394
DEF: 3 (U)	370 508	724 1044
DEF: 4 (U)	163 233	339 511
DEF: 5 (U)	71 97	139 208
DEF: 6 (U)	41 59	83 127
DEF: 7 (U)	33 44	59 88

DISTRICT: 17	COUNTY: BRAZO	SECTION: FM1687 2.0
LD.(KPA)	534 721	981 1397
DEF: 1 (U)	714 968	1391 1919
DEF: 2 (U)	509 692	1002 1487
DEF: 3 (U)	394 541	778 1175
DEF: 4 (U)	194 269	390 589
DEF: 5 (U)	87 120	171 248
DEF: 6 (U)	53 76	105 154
DEF: 7 (U)	41 56	81 113

DISTRICT: 17	COUNTY: BRAZO	SECTION: FM2038 8.0
LD.(KPA)	581 725	1019 1410
DEF: 1 (U)	497 633	864 1212
DEF: 2 (U)	380 496	691 973
DEF: 3 (U)	306 418	588 825
DEF: 4 (U)	170 234	328 463
DEF: 5 (U)	58 83	121 173
DEF: 6 (U)	41 60	83 120
DEF: 7 (U)	33 50	64 92

DISTRICT: 17	COUNTY: BRAZO	SECTION: FM2038 8.1
LD.(KPA)	527 703	992 1385
DEF: 1 (U)	690 848	1137 1574
DEF: 2 (U)	512 664	915 1311
DEF: 3 (U)	413 541	757 1083
DEF: 4 (U)	187 261	369 530
DEF: 5 (U)	65 93	132 188
DEF: 6 (U)	46 64	91 127
DEF: 7 (U)	39 50	73 104

DISTRICT: 17	COUNTY: BRAZO	SECTION: FM2038 10.0
LD.(KPA)	539 701	991 1389
DEF: 1 (U)	931 1064	1388 1902
DEF: 2 (U)	636 745	988 1369
DEF: 3 (U)	452 539	756 1035
DEF: 4 (U)	206 257	347 485
DEF: 5 (U)	73 100	130 181
DEF: 6 (U)	40 59	78 114
DEF: 7 (U)	32 48	61 84

DISTRICT: 17	COUNTY: BRAZO	SECTION: FM2038	10.1
LD.(KPA)	525	690	968 1370
DEF: 1 (U)	1129	1290	1693 2263
DEF: 2 (U)	674	804	1078 1535
DEF: 3 (U)	454	555	735 1045
DEF: 4 (U)	193	245	340 478
DEF: 5 (U)	69	92	129 182
DEF: 6 (U)	40	55	77 111
DEF: 7 (U)	30	39	51 79

DISTRICT: 17	COUNTY: BRAZO	SECTION: FM2776	0.0
LD.(KPA)	509	693	946 1325
DEF: 1 (U)	894	1183	1708 1961
DEF: 2 (U)	572	796	1174 1779
DEF: 3 (U)	367	530	757 1044
DEF: 4 (U)	171	248	366 542
DEF: 5 (U)	78	109	157 224
DEF: 6 (U)	48	66	96 133
DEF: 7 (U)	34	49	64 91

DISTRICT: 17	COUNTY: BRAZO	SECTION: FM2776	0.1
LD.(KPA)	485	666	918 1284
DEF: 1 (U)	1378	1704	1877 2991
DEF: 2 (U)	783	1038	1468 2152
DEF: 3 (U)	464	655	951 1404
DEF: 4 (U)	197	286	419 623
DEF: 5 (U)	80	112	155 230
DEF: 6 (U)	49	67	98 137
DEF: 7 (U)	34	51	68 97

DISTRICT: 17	COUNTY: BRAZO	SECTION: FM2776	2.0
LD.(KPA)	491	669	927 1328
DEF: 1 (U)	1009	1301	1680 3105
DEF: 2 (U)	582	781	1147 1722
DEF: 3 (U)	357	497	710 1091
DEF: 4 (U)	121	169	239 352
DEF: 5 (U)	55	75	102 145
DEF: 6 (U)	38	53	71 101
DEF: 7 (U)	29	39	52 78

DISTRICT: 17	COUNTY: BRAZO	SECTION: FM2776	2.1
LD.(KPA)	497	665	920 1314
DEF: 1 (U)	801	1049	1489 2180
DEF: 2 (U)	491	667	974 1504
DEF: 3 (U)	317	449	656 1024
DEF: 4 (U)	115	167	243 363
DEF: 5 (U)	50	71	97 141
DEF: 6 (U)	35	50	59 96
DEF: 7 (U)	26	38	49 74

DISTRICT: 17	COUNTY: BRAZO	SECTION: FM1687 2.1		
LD.(KPA)	530	714	983	1387
DEF: 1 (U)	678	936	1374	2147
DEF: 2 (U)	467	654	953	1484
DEF: 3 (U)	366	513	750	1113
DEF: 4 (U)	179	258	386	584
DEF: 5 (U)	93	130	184	273
DEF: 6 (U)	54	77	102	158
DEF: 7 (U)	38	54	78	120

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 2.0		
LD.(KPA)	516	694	961	1381
DEF: 1 (U)	953	1166	1580	1961
DEF: 2 (U)	616	779	1078	1538
DEF: 3 (U)	402	532	695	1047
DEF: 4 (U)	167	232	333	476
DEF: 5 (U)	77	108	157	222
DEF: 6 (U)	50	67	97	158
DEF: 7 (U)	37	51	74	108

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 2.1		
LD.(KPA)	506	685	952	1362
DEF: 1 (U)	1175	1428	1903	3100
DEF: 2 (U)	748	944	1276	1816
DEF: 3 (U)	490	648	890	1275
DEF: 4 (U)	197	270	386	562
DEF: 5 (U)	84	111	164	247
DEF: 6 (U)	57	68	102	163
DEF: 7 (U)	42	49	77	120

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 4.0		
LD.(KPA)	524	714	983	1380
DEF: 1 (U)	1241	1451	1861	2354
DEF: 2 (U)	839	1033	1352	1825
DEF: 3 (U)	586	749	1004	1373
DEF: 4 (U)	248	335	466	652
DEF: 5 (U)	98	136	194	280
DEF: 6 (U)	59	85	120	177
DEF: 7 (U)	317	332	377	609

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 4.1		
LD.(KPA)	528	712	978	1367
DEF: 1 (U)	1174	1360	1735	2216
DEF: 2 (U)	779	950	1230	1654
DEF: 3 (U)	527	674	894	1219
DEF: 4 (U)	199	273	382	525
DEF: 5 (U)	80	112	161	235
DEF: 6 (U)	52	74	106	156
DEF: 7 (U)	52	54	80	115

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 12.0
LD.(KPA)	566	731 1015 1438
DEF: 1 (U)	536	703 990 1455
DEF: 2 (U)	406	549 751 1109
DEF: 3 (U)	317	437 635 980
DEF: 4 (U)	166	233 344 507
DEF: 5 (U)	83	117 169 246
DEF: 6 (U)	42	65 96 137
DEF: 7 (U)	38	52 74 107

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 12.0L
LD.(KPA)	568	732 1029 1434
DEF: 1 (U)	527	677 945 1376
DEF: 2 (U)	373	507 681 1028
DEF: 3 (U)	288	397 550 760
DEF: 4 (U)	155	212 318 485
DEF: 5 (U)	78	109 159 229
DEF: 6 (U)	50	69 83 116
DEF: 7 (U)	34	50 71 107

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 12.0R
LD.(KPA)	537	711 1007 1402
DEF: 1 (U)	795	1000 1372 1483
DEF: 2 (U)	550	732 1006 1452
DEF: 3 (U)	393	551 796 1164
DEF: 4 (U)	200	278 413 612
DEF: 5 (U)	92	130 187 271
DEF: 6 (U)	50	69 100 159
DEF: 7 (U)	39	56 82 119

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 12.1
LD.(KPA)	574	743 1029 1433
DEF: 1 (U)	495	663 943 1345
DEF: 2 (U)	377	511 676 1054
DEF: 3 (U)	292	398 586 880
DEF: 4 (U)	185	252 359 510
DEF: 5 (U)	81	119 169 243
DEF: 6 (U)	38	70 93 141
DEF: 7 (U)	37	51 75 104

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 12.2
LD.(KPA)	556	722 1014 1408
DEF: 1 (U)	511	675 955 1420
DEF: 2 (U)	362	494 680 988
DEF: 3 (U)	292	407 583 875
DEF: 4 (U)	225	257 323 468
DEF: 5 (U)	89	116 166 233
DEF: 6 (U)	43	61 89 138
DEF: 7 (U)	44	55 75 90

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 14.0
LD.(KPA)	586 757	1020 1495
DEF: 1 (U)	397 523	742 1128
DEF: 2 (U)	308 419	604 842
DEF: 3 (U)	241 332	483 737
DEF: 4 (U)	161 222	323 502
DEF: 5 (U)	84 118	170 250
DEF: 6 (U)	47 66	100 146
DEF: 7 (U)	37 54	79 115

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 14.1
LD.(KPA)	579 765	1027 1444
DEF: 1 (U)	443 583	854 1275
DEF: 2 (U)	309 442	621 862
DEF: 3 (U)	247 344	495 736
DEF: 4 (U)	163 223	324 474
DEF: 5 (U)	82 116	167 247
DEF: 6 (U)	67 79	97 143
DEF: 7 (U)	43 51	77 112

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 14.2
LD.(KPA)	598 754	1015 1469
DEF: 1 (U)	796 559	787 1157
DEF: 2 (U)	278 430	598 844
DEF: 3 (U)	250 335	532 901
DEF: 4 (U)	155 290	323 461
DEF: 5 (U)	81 120	169 250
DEF: 6 (U)	27 141	97 145
DEF: 7 (U)	40 55	77 111

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 16.0
LD.(KPA)	594 752	1034 1480
DEF: 1 (U)	383 507	727 1067
DEF: 2 (U)	304 411	582 874
DEF: 3 (U)	240 331	479 709
DEF: 4 (U)	139 215	299 452
DEF: 5 (U)	71 100	143 207
DEF: 6 (U)	37 53	78 122
DEF: 7 (U)	30 42	61 86

DISTRICT: 17	COUNTY: BURLESON	SECTION: FM50 16.1
LD.(KPA)	582 744	1011 1448
DEF: 1 (U)	406 537	760 1112
DEF: 2 (U)	329 443	616 965
DEF: 3 (U)	246 343	512 717
DEF: 4 (U)	160 218	302 485
DEF: 5 (U)	66 92	134 195
DEF: 6 (U)	53 85	96 169
DEF: 7 (U)	33 46	78 96

DISTRICT: 17		COUNTY: BURLESON		SECTION: FM50 16.2	
LD.(KPA)		566	728	994	1464
DEF: 1	(U)	385	506	718	1071
DEF: 2	(U)	289	394	561	822
DEF: 3	(U)	237	324	467	701
DEF: 4	(U)	144	200	286	421
DEF: 5	(U)	70	96	140	205
DEF: 6	(U)	40	59	84	121
DEF: 7	(U)	29	40	58	85

Dynaflect Readings

TEXAS TRANSPORTATION INSTITUTE
 DYNAFLECT
 DATA OF TEST DONE IN MARCH 1983

NOTATIONS : W1 W2 W3 W4 W5 REFERS TO DEFLECTIONS
 @ SENSORS MEASURED IN MILS

DISTRICT: 17 COUNTY: BURLESON ROAD NAME: FM1361

SECTION	W1	W2	W3	W4	W5
6.0	2.64	1.26	.6	.42	.3
6.1	2.13	1.11	.54	.38	.3
6.2	2.28	1.11	.55	.38	.28
8.0	2.31	1.23	.52	.32	.2
8.1	2.61	1.44	.56	.31	.18
8.2	2.07	1.05	.39	.23	.15
10.0	3.1	1.74	.96	.64	.44
10.1	3.2	1.74	.99	.66	.46
10.2	2.61	1.44	.78	.57	.44

DISTRICT: 17 COUNTY: BURLESON ROAD NAME: FM3058

SECTION	W1	W2	W3	W4	W5
2.0	1.44	1.08	.81	.56	.39
2.1	1.41	1.08	.74	.56	.38
2.2	1.68	1.23	.81	.6	.42
4.0	1.02	.6	.35	.26	.11
4.1	.96	.56	.33	.24	.17
4.2	.93	.56	.32	.24	.16
6.0	1.62	1.08	.61	.41	.29
6.1	1.29	1.02	.6	.41	.27
6.2	1.38	1.02	.63	.44	.27
8.0	1.14	.84	.46	.33	.21
8.1	1.11	.66	.42	.29	.22
8.2	1.17	.78	.42	.29	.3
10.1	2.52	1.74	.96	.55	.34
10.2	3.4	1.83	.93	.51	.34

DISTRICT: 17 COUNTY: BURLESON ROAD NAME: FM908

SECTION	W1	W2	W3	W4	W5
10.0	3.8	.8	.55	.46	.38
10.1	1.44	1.02	.69	.6	.39
10.2	1.44	.96	.58	.48	.39

DISTRICT: 17 COUNTY: BURLESON ROAD NAME: FM2000					
SECTION	W1	W2	W3	W4	W5
8.0	2.13	1.53	1.14	.87	.6
8.1	1.89	1.5	1.14	.86	.66
10.0	1.41	1.08	.74	.55	.39
10.1	1.47	1.14	.74	.53	.35
12.0	3.3	1.14	.78	.54	.22
12.1	1.4	1	.7	.5	.4

DISTRICT: 17 COUNTY: BURLESON ROAD NAME: FM2155					
SECTION	W1	W2	W3	W4	W5
2.0	1.2	.7	.38	.28	.25
2.1	1.26	.93	.53	.38	.29
2.2	1.44	.73	.35	.24	.15
4.0	1.35	.96	.53	.38	.29
4.1	1.26	.93	.51	.36	.28
4.2	1.52	1.05	.55	.38	.28

DISTRICT: 17 COUNTY: BURLESON ROAD NAME: FM50					
SECTION	W1	W2	W3	W4	W5
2.0	1.71	1.14	.67	.57	.38
2.1	1.98	1.29	.76	.53	.4
4.0	1.65	1.11	.67	.52	.36
4.1	1.68	1.11	.64	.45	.36
12.0	1.53	1.16	.75	.53	.38
12.1	1.44	1.14	.71	.51	.37
12.2	1.44	1.14	.72	.52	.38
14.0	1.29	1.08	.74	.58	.44
14.1	1.35	1.08	.75	.58	.44
14.2	1.38	1.08	.75	.58	.44
16.0	1.29	1.05	.7	.49	.35
16.1	1.38	1.08	.7	.5	.31
16.2	1.29	1.05	.7	.5	.35

DISTRICT: 17 COUNTY: BRAZOS ROAD NAME: OSR					
SECTION	W1	W2	W3	W4	W5
2.0	1.74	1.35	.87	.56	.37
2.1	1.77	1.41	.96	.64	.44
4.0	1.08	.93	.68	.53	.41
4.1	1.11	.96	.68	.54	.4

DISTRICT: 17 COUNTY: BRAZOS ROAD NAME: FM974					
SECTION	W1	W2	W3	W4	W5
8.0	1.56	.99	.58	.52	.33
8.1	1.47	.96	.58	.68	.34

DISTRICT: 17 COUNTY: BRAZOS ROAD NAME: FM1179					
SECTION	W1	W2	W3	W4	W5
4.0	1.77	1.17	.73	.52	.37
4.1	1.68	1.14	.65	.46	.35

DISTRICT: 17 COUNTY: BRAZOS ROAD NAME: FM1687					
SECTION	W1	W2	W3	W4	W5
2.0	2.04	1.32	1.38	.79	.56
2.1	1.95	1.38	.79	.56	.42

DISTRICT: 17 COUNTY: BRAZOS ROAD NAME: FM2038					
SECTION	W1	W2	W3	W4	W5
8.0	1.56	1.11	.67	.48	.36
8.1	1.83	1.26	.73	.5	.33
10.0	1.44	1.15	.61	.43	.33
10.1	1.59	1.25	.57	.4	.27

DISTRICT: 17 COUNTY: BRAZOS ROAD NAME: FM2776					
SECTION	W1	W2	W3	W4	W5
0.0	2.13	1.44	.84	.59	.41
0.1	2.01	1.41	.86	.58	.42
2.0	1.74	1.05	.54	.44	.38
2.1	1.53	.99	.53	.38	.3

APPENDIX B - DATA USED TO COMPUTE THE MULTIPLIER

DISTRICT: 20 COUNTY: HARDIN ROAD: FM418
 RECORDED RUT(INS): 0.75 LOAD(LBS): 9000. PASSES: 0.6240D 06.

DATE: 7/21/1976 DYNAFLECT

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)				
		W1	W2	W3	W4	W5
2031- 1	13.50	1.20	0.93	0.72	0.60	0.51
2031- 2	13.50	1.20	1.11	0.71	0.58	0.49
2031- 3	13.50	1.53	1.08	0.64	0.46	0.36
2031- 4	13.50	1.50	1.05	0.64	0.48	0.38
2031- 5	13.50	1.62	1.02	0.58	0.42	0.35
2031- 6	13.50	1.74	1.14	0.64	0.46	0.38
2031- 7	13.50	1.38	0.64	0.22	0.11	0.08
2031- 8	13.50	1.53	0.72	0.21	0.10	0.07
2031- 9	13.50	2.34	1.50	0.81	0.54	0.42
2031-10	13.50	1.92	1.26	0.80	0.56	0.42
2031-11	13.50	1.14	0.58	0.34	0.24	0.19
2031-12	13.50	1.08	0.61	0.36	0.25	0.19
2031-13	13.50	0.96	0.58	0.33	0.23	0.17
2031-14	13.50	1.02	0.53	0.32	0.22	0.17

DISTRICT: 20 COUNTY: JEFFERSON ROAD: FM 365
 RECORDED RUT(INS): 0.75 LOAD(LBS): 9000. PASSES: 0.2087D 06

DATE: 7/21/1976 DYNAFLECT

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)				
		W1	W2	W3	W4	W5
2057- 3	7.00	2.34	1.56	1.08	0.84	0.68
2057- 4	7.00	2.46	1.68	1.11	0.87	0.69
2057- 5	7.00	1.38	1.02	0.77	0.66	0.55
2057- 6	7.00	1.77	1.08	0.81	0.69	0.57
2057- 7	7.00	1.50	1.14	0.82	0.66	0.51
2057- 8	7.00	1.74	1.26	0.80	0.65	0.50
2057- 9	7.00	1.44	1.02	0.61	0.44	0.35
2057-10	7.00	1.59	1.14	0.68	0.48	0.36
2057-11	7.00	1.14	0.84	0.57	0.44	0.35
2057-12	7.00	1.20	0.87	0.58	0.44	0.34
2057-13	7.00	0.96	0.68	0.45	0.35	0.27
2057-14	7.00	1.20	0.82	0.54	0.38	0.29

DISTRICT: 16 COUNTY: NUECES ROAD: FM 665
 RECORDED RUT (INS): 0.75 LOAD (LBS): 9000. PASSES: 0.1860D 06

DATE: 8/25/1976 DYNAFLECT

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)				
		W1	W2	W3	W4	W5
1705- 1	12.00	2.50	1.71	1.26	0.84	0.59
1705- 2	12.00	2.70	1.68	1.14	0.78	0.56
1705- 3	12.00	2.10	1.20	0.78	0.48	0.34
1705- 4	12.00	2.10	1.23	0.81	0.52	0.36
1705- 5	12.00	2.34	1.47	0.96	0.66	0.44
1705- 6	12.00	2.40	1.50	0.96	0.66	0.44
1705- 7	12.00	2.88	1.86	1.05	0.66	0.40
1705- 8	12.00	2.90	1.71	1.02	0.66	0.39
1705- 9	12.00	1.83	1.26	0.84	0.56	0.42
1705-10	12.00	2.07	1.32	0.87	0.60	0.42
1705-11	12.00	2.25	1.44	0.90	0.60	0.43
1705-12	12.00	2.28	1.41	0.90	0.60	0.42
1705-13	12.00	1.80	1.11	0.75	0.49	0.36
1705-14	12.00	1.74	1.02	0.72	0.46	0.34

DISTRICT: 8 COUNTY: BORDEN ROAD: FM 612
 RECORDED RUT (INS): 0.75 LOAD (LBS): 9000. PASSES: 0.2528D 05

DATE: 11/20/1975 DYNAFLECT

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)				
		W1	W2	W3	W4	W5
835- 1	5.00	2.70	1.38	0.54	0.30	0.20
835- 2	5.00	2.16	1.38	0.72	0.42	0.26
835- 3	5.00	1.62	1.02	0.44	0.25	0.15
835- 4	5.00	1.50	0.83	0.36	0.23	0.14
835- 5	5.00	1.44	0.90	0.44	0.28	0.17
835- 6	5.00	1.38	0.85	0.38	0.22	0.15
835- 7	5.00	1.44	0.77	0.33	0.21	0.14
835- 8	5.00	1.26	0.58	0.26	0.16	0.12
835- 9	5.00	1.32	0.70	0.30	0.18	0.12
835-10	5.00	1.14	0.58	0.40	0.16	0.11

DISTRICT: 13 COUNTY:FAYETTE ROAD:FM 1381
RECORDED RUT(INS): 0.75 LOAD(LBS): 9000. PASSES:0.1385D 06

DATE: 7/29/1976 DYNAFLECT

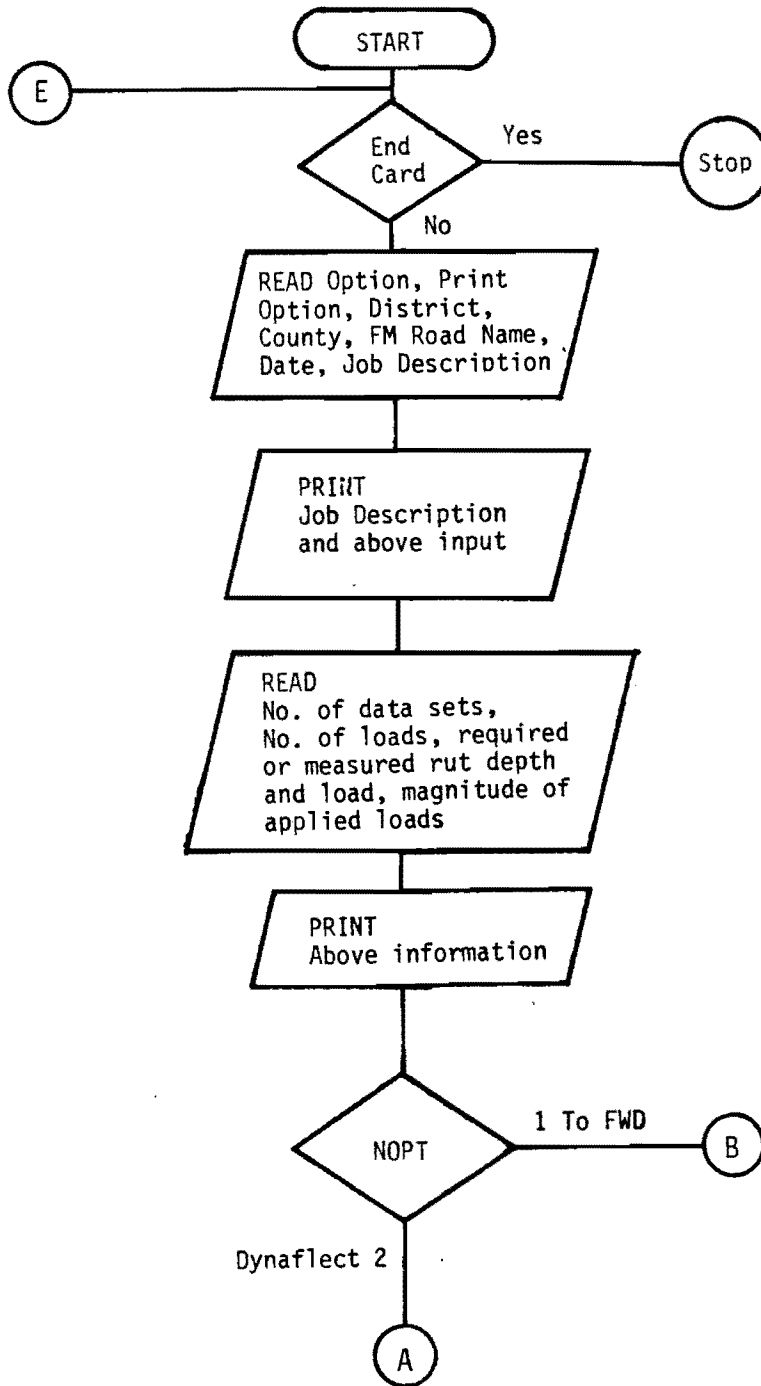
SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)				
		W1	W2	W3	W4	W5
1361- 1	9.00	3.20	1.50	0.66	0.33	0.20
1361- 2	9.00	3.50	1.68	0.75	0.36	0.20
1361- 3	9.00	2.71	1.59	0.90	0.46	0.32
1361- 4	9.00	3.20	1.80	0.96	0.49	0.31
1361- 5	9.00	1.32	0.90	0.50	0.32	0.22
1361- 6	9.00	1.32	0.84	0.48	0.32	0.22
1361- 7	9.00	2.19	1.41	0.84	0.53	0.36
1361- 8	9.00	2.34	1.50	0.93	0.56	0.37
1361- 9	9.00	2.10	1.20	0.74	0.50	0.37
1361-10	9.00	2.34	1.26	0.74	0.50	0.38
1361-11	9.00	2.46	1.47	0.74	0.44	0.33
1361-12	9.00	2.70	1.65	0.75	0.46	0.34
1361-13	9.00	2.01	1.08	0.56	0.36	0.00
1361-14	9.00	1.92	1.05	0.53	0.34	0.25

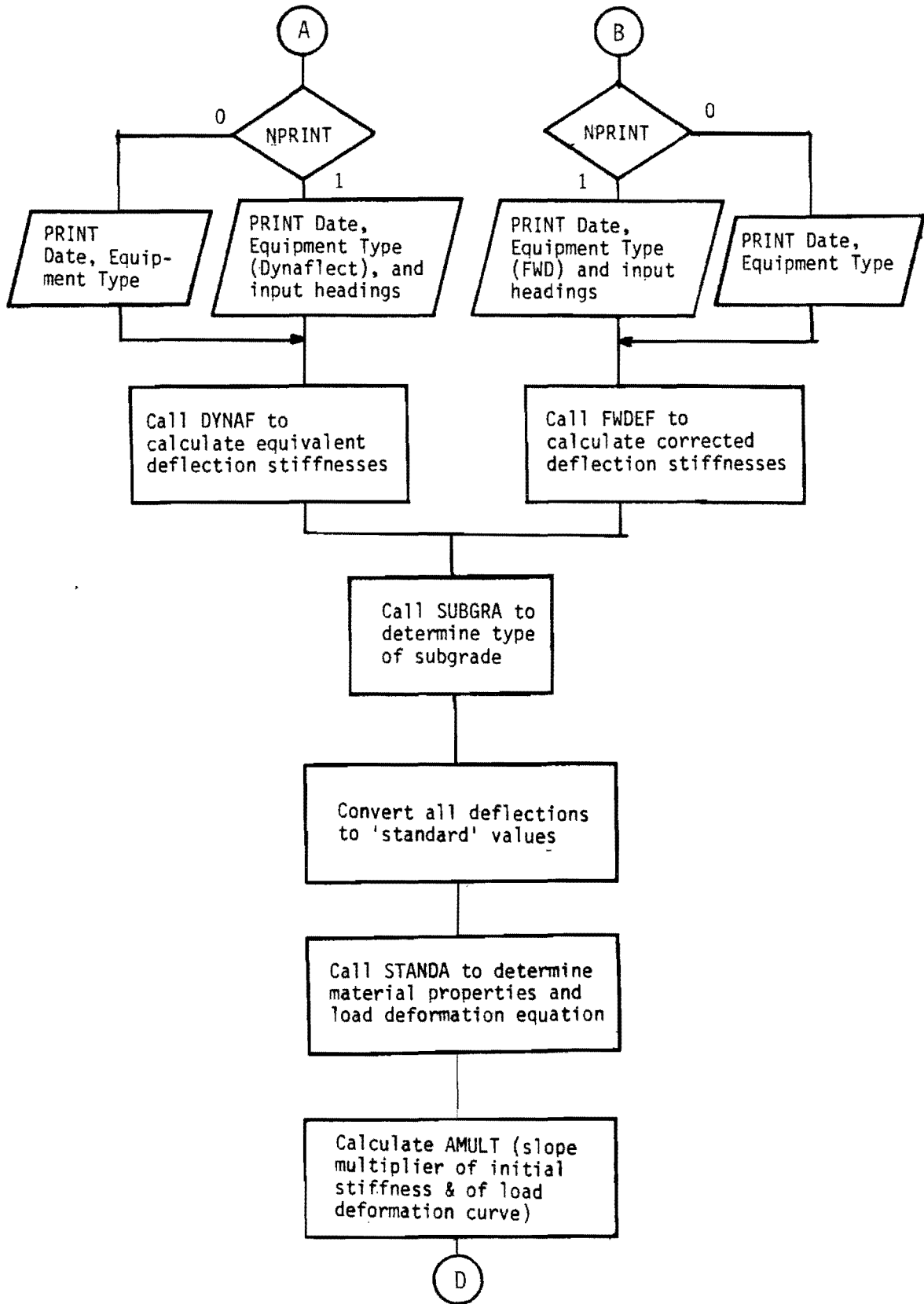


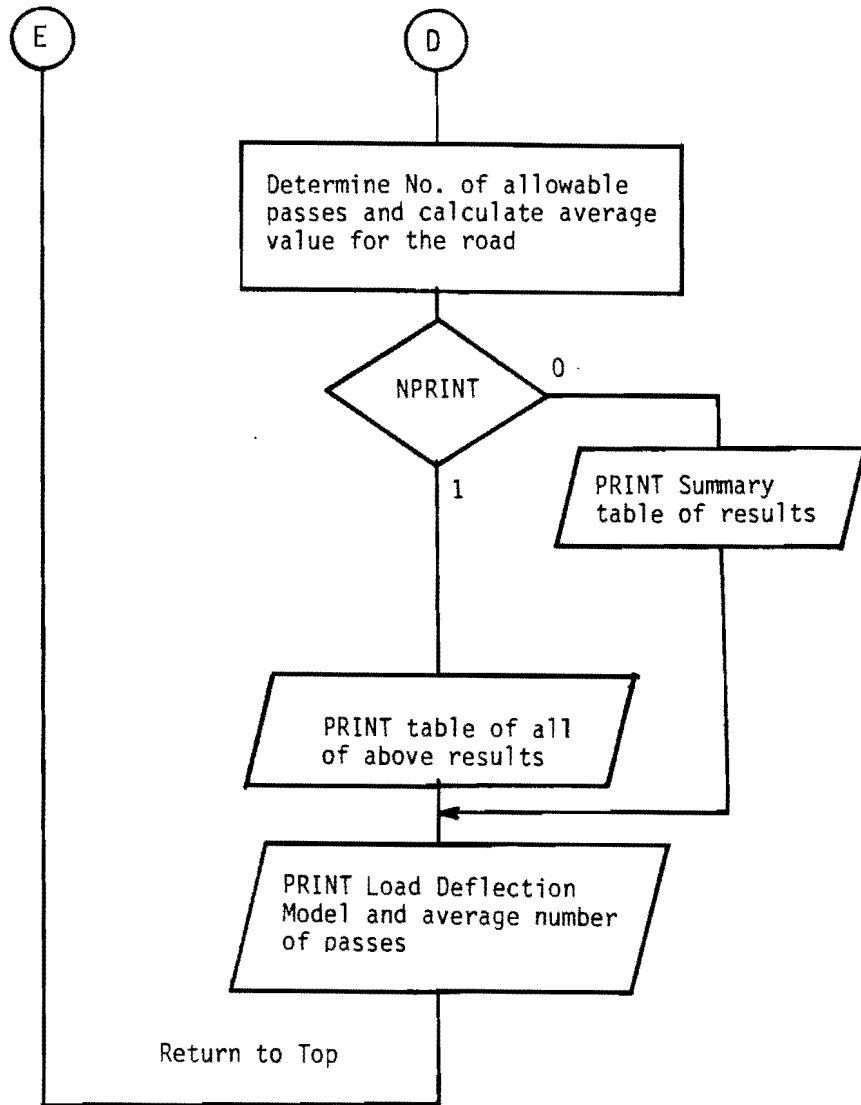
APPENDIX C - COMPUTER PROGRAM

Flow-Charts

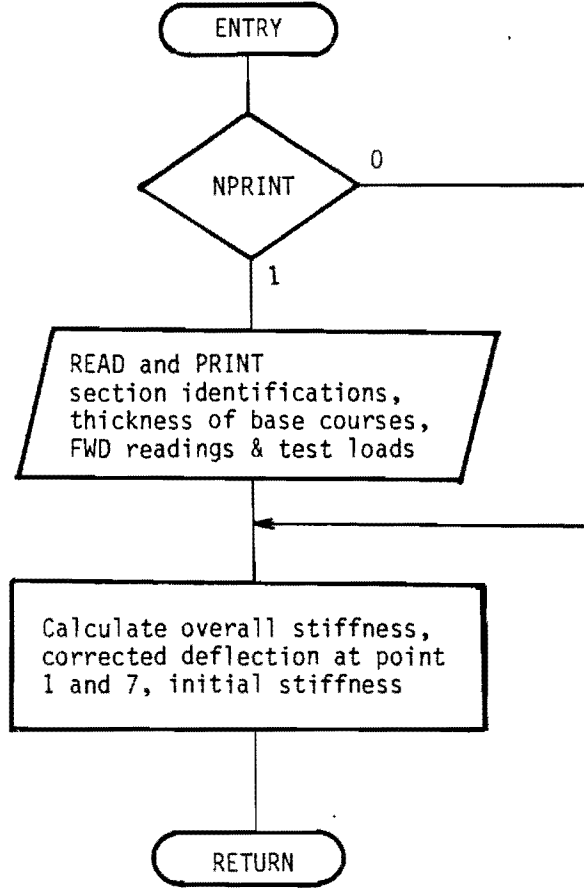
MAIN PROGRAM FLOW CHART



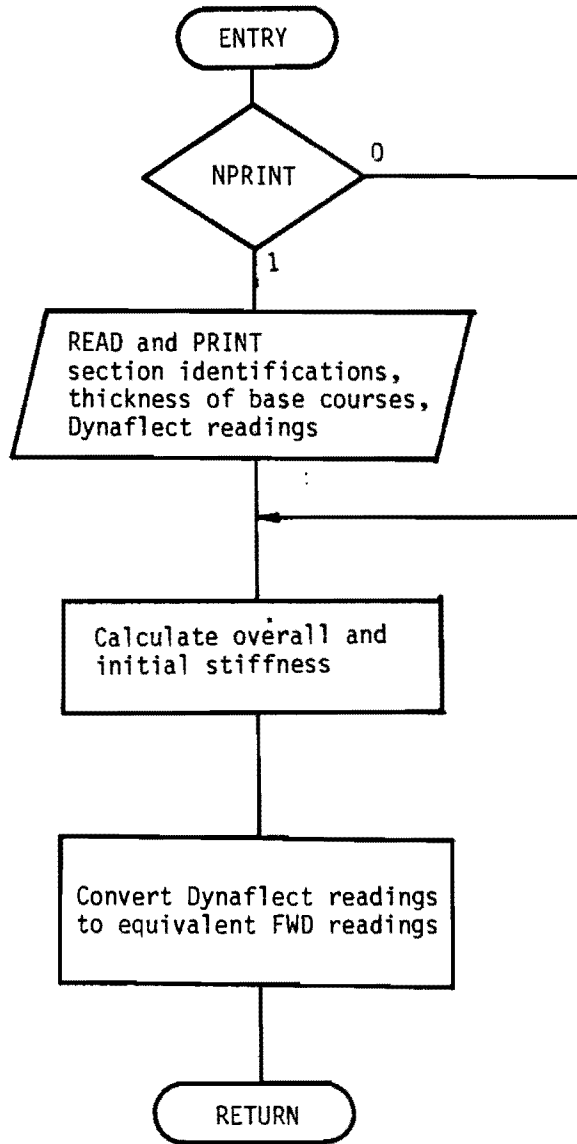




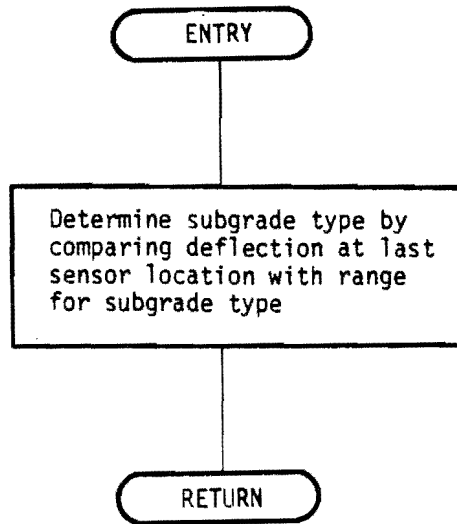
SUBROUTINE FWDEF FLOW CHART



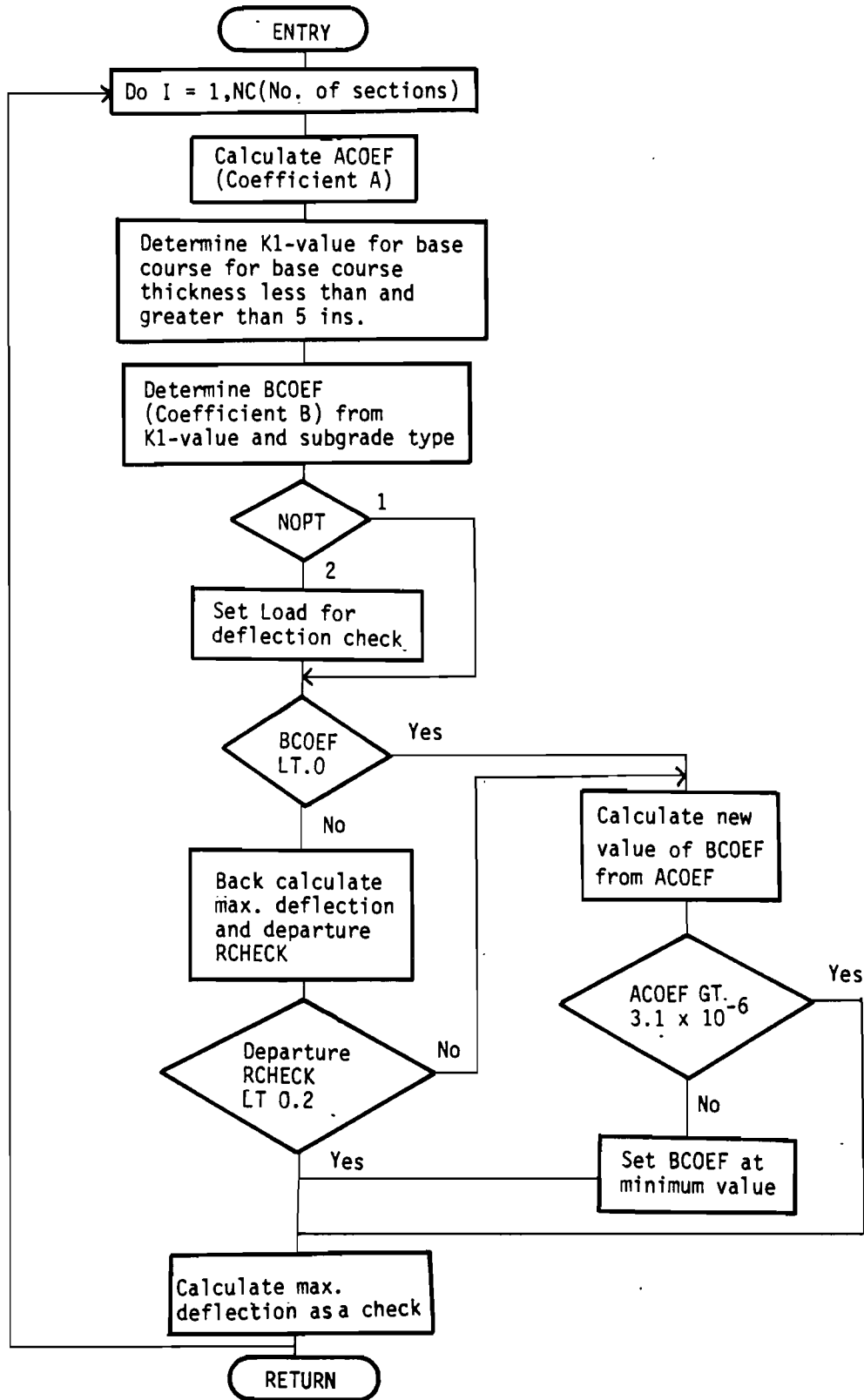
SUBROUTINE DYNAP FLOW CHART



SUBROUTINE SUBGRA FLOW CHART



SUBROUTINE STAND A FLOW CHART



Input Instructions, Listing and Sample Input


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C
C LOADRATE PROGRAM : VERSION DATED 12 NOVEMBER 1983 K.M.CHUA
C
C*****
C*          TEXAS TRANSPORTATION INSTITUTE          *
C*          LOAD RATING OF LIGHT PAVEMENT          *
C*          PROJECT NO 2284                        *
C*          FOR                                     *
C*          TEXAS HIGHWAY DEPARTMENT              *
C*          1983                                   *
C*****
C
C PROGRAM DESCRIPTION : _____
C
C THIS PROGRAM DETERMINES THE NUMBER OF PASSES FOR A SPECIFIC LOAD THAT
C WILL CAUSE A CRITICAL LEVEL OF RUT DEPTH IN FARM-TO-MARKET ROADS.
C
C A HYPERBOLIC LOAD-DEFLECTION MODEL IS USED. THE RESULTS ARE BASED
C ON AN EXTENSIVE STUDY OF LOAD-DEFLECTION CHARACTERISTICS OF LIGHT
C PAVEMENT STRUCTURES USING THE FALLING WEIGHT DEFLECTOMETER AND THE
C DYNAFLECT.
C
C WHEN USING A FALLING WEIGHT DEFLECTOMETER, A LOADING OF ABOUT 100 PSI
C SHOULD BE USED AND IS ASSUMED AS SUCH IN THE ANALYSIS.
C
C A 1 INCH THICK SURFACE/WEARING COURSE OF MODULUS OF ELASTICITY OF
C 30000 PSI (WHICH IS COMMONLY ENCOUNTERED) IS ASSUMED.
C
C BASE COURSE MODEL : MODULUS = K1 * (FIRST STRESS INVARIANT)**0.33
C
C SUBGRADE MODEL : REFER TRB852 P.44 TABLE 2.
C
C _____
C
C DEFINITIONS OF INPUT VARIABLES : _____
C
C NOPT      : OPTION FOR TYPE OF INPUT FROM FIELD MEASUREMENTS
C            1 = DYNATEST 8000 FALLING WEIGHT DEFLECTOMETER
C            2 = DYNAFLECT
C NPRINT    : OPTION FOR AMOUNT OF OUTPUT
C            0 = SUMMARIZED          1 = DETAILED
C IDIST     : DISTRICT NUMBER
C CTY       : COUNTY NAME
C FM        : ROAD NAME
C JJOB      : JOB DESCRIPTION (ONE LINE)
C NC        : NUMBER OF SETS OF READINGS
C NX        : NUMBER OF AXLES FOR THE VEHICLE
C RUTX      : MAXIMUM ALLOWABLE RUT DEPTH (INS)
C RUTM      : MEASURED RUT DEPTH (INS) -OPTIONAL-
C ALOADM    : LOAD CORRESPONDING TO RUTM -OPTIONAL-
C PASSM     : NUMBER OF PASSES OF ALOADM -OPTIONAL-
C WLOAD(I)  : LOADING(LBS) FROM EACH WHEEL OF THE VEHICLE IN ONE PASS
C NSECT1    : SECTION IDENTIFICATION (4-DIGITS)
C NSECT2    : SECTION IDENTIFICATION (2-DIGITS)
C NDATE1    : MONTH
C NDATE2    : DAY
C NDATE3    : YEAR
C BASEH     : THICKNESS OF BASE AND SUB-BASE (IF ANY) IN INCHES.
C FWD(I)    : DEFLECTIONS FROM FALLING WEIGHT DEFLECTOMETER IN MILS.
C DYN(I)    : DEFLECTIONS FROM DYNAFLECT IN MILS.
C PFWD(I)   : CORRESPONDING TEST LOAD (LBS)
C            THIS LOAD SHOULD BE ABOUT 10956 LBS OR 100PSI ON THE FWD
C            LOADING PLATE.
C
C _____
C
C DATA INPUT : _____
C
C CARD 1      : FORMAT (I1,I1,I2,3A4,4X,4A4,I2,I2,I4)
C VARIABLES READ : NOPT,NPRINT,IDIST,COUNTY,FMNAME,NDATE1,NDATE2,NDATE3
C
C CARD 2      : FORMAT (17A4)
C VARIABLE READ : JOB
C
C CARD 3      : FORMAT (I2,I2,1X,F5.2,11X,F5.2,F6.0,E10.4)
C VARIABLES READ : NC,NX,RUTX,RUTM,ALOADM,PASSM
C
C CARD 4 TO NX : FORMAT (F10.0)

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C VARIABLE READ : WLOAD(I)                                008000
C                                                         008100
C FOR NOPT = 1 (DT8000FWD)                                008200
C CARD 5 TO NC : FFORMAT (I4,I2,13X,F5.2,7F5.2,1X,6.0)    008300
C VARIABLES READ : NSECT1,NSECT2,BASEH,FWD(1) TO FWD(7),PFWD(1) 008400
C                                                         008500
C FOR NOPT = 2 (DYNAFLECT)                                008600
C CARD 5 TO NC : FFORMAT (I4,I2,13X,F5.2,5F5.3)           008700
C VARIABLES READ : NSECT1,NSECT2,BASEH,DYN(1) TO DYN(5)    008800
C                                                         008900
C                                                         009000
C                                                         009100
C _____ 009200
C                                                         009300
C                                                         009400
C MAIN PROGRAM ***** 009500
C                                                         009600
C     IMPLICIT REAL*B(A-H,O-Z)                             009700
C     DIMENSION FWD(20,7),DYN(20,5),DEF(20,7),STD(20,7),ISUB(20),STFI
* (20),ACDEF(20),BCDEF(20),H(20),K1(20),NSECT1(20),NSECT2(20) 009800
* ,BASEH(20),PASS(20),CTY(3),FM(4),STFD(20),PMA(20),STIFF(20) 009900
* ,AMULT(20),W(20),PFWD(20),WLOAD(20),JJOB(17)            010000
C                                                         010100
C                                                         010200
C DATA INPUT / PRINT TITLE / SELECT OPTION                010300
C                                                         010400
C     90 READ(5,100,END=5000)NOPT,NPRINT,IDIST,(CTY(L),L=1,3),(FM(M),M=1,4)
* ,NDATE1,NDATE2,NDATE3 010500
100 FORMAT(I1,I1,I2,3A4,4X,4A4,I2,I2,I4)                   010600
    WRITE (6,110)                                           010700
110 FORMAT(1H1,/,5X,'TEXAS HIGHWAY DEPARTMENT')           010800
    WRITE (6,120)                                           010900
120 FORMAT(5X,'LOAD RATING OF LIGHT PAVEMENT')            011000
    READ(5,122) (JJOB(I),I=1,17)                            011100
122 FORMAT(17A4)                                           011200
    WRITE(6,125) (JJOB(I),I=1,17)                            011300
125 FORMAT(5X,'JOB:',17A4,/)                               011400
    WRITE (6,130) IDIST,(CTY(L),L=1,3),(FM(M),M=1,4)       011500
130 FORMAT(5X,'DISTRICT:',I3,5X,'COUNTY:',3A4,4X,'ROAD:',4A4) 011600
    READ (5,140) NC,NX,RUTX,RUTM,ALOADM,PASSM              011700
140 FORMAT (I2,I2,1X,F5.2,11X,F5.2,F6.0,E10.4)            011800
    DD 134 II=1,NX                                          011900
    READ (5,135) WLOAD(II)                                   012000
135 FORMAT(F10.0)                                          012100
134 CONTINUE                                              012200
    WRITE (6,150) RUTX                                       012300
150 FORMAT(5X,'ALLOWABLE RUT(INS):',F5.2)                 012400
C                                                         012500
C TO PRINT VEHICLE DESCRIPTION                             012600
C                                                         012700
C     WRITE(6,154)                                          012800
154 FORMAT(5X,'AXLE NUMBER',3X,'WHEEL LOAD(LBS)')         012900
    DD 156 II=1,NX                                          013000
    WRITE(6,155) II,WLOAD(II)                               013100
155 FFORMAT(10X,I2,8X,F10.0)                               013200
156 CONTINUE                                              013300
    WRITE(6,160) RUTM,ALOADM,PASSM                          013400
160 FORMAT(/5X,'RECORDED RUT(INS):',F5.2,3X,'LOAD(LBS):',F6.0,3X,'PASS
*ES:',E10.4/)                                             013500
C                                                         013600
C TO SELECT TYPES OF EXECUTION                             013700
C                                                         013800
C     IF (NOPT.EQ.1) GO TO 162                             013900
C     IF (NOPT.EQ.2) GO TO 164                             014000
C                                                         014100
C ***** FALLING WEIGHT DEFLECTOMETER ***** 014200
C                                                         014300
C     162 WRITE (6,1020) NDATE1,NDATE2,NDATE3             014400
1020 FORMAT (5X,'DATE: ',I2,'/',I2,'/',I4,2X,'FALLING WEIGHT DEFLECTOME
*TER'//) 014500
C TO PRINT CARD IMAGE 014600
C                                                         014700
C     IF (NPRINT.EQ.0) GO TO 195                          014800
C                                                         014900
C     WRITE(6,170)                                         015000
170 FORMAT(5X,'SECTION',4X,'BASE',21X,'DEFLECTIONS')       015100
    WRITE(6,180)                                           015200
180 FORMAT(5X,' NO.',4X,'THICKNESS',20X,'(MILS)',29X,'LOAD') 015300
    WRITE(6,190)                                           015400
190 FORMAT(5X,11X,'(INS)',5X,'W1      W2      W3      W4      W5      W6
* W7      (LBS)'//) 015500
    015600
    015700
    015800
    015900

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195 CONTINUE
C
C TO READ IN RESULTS
C
      CALL FWDEF (NC,NSECT1,NSECT2,BASEH,FWD,STFI,DEF,H,STFO,PLOAD,PFWD,
      *NPRINT)
      GO TO 1500
C
C ***** DYNAFLECT *****
C
164 WRITE(6,2020) NDATE1,NDATE2,NDATE3
2020 FORMAT(5X,'DATE: ',I2,'/',I2,'/',I4,2X,'DYNAFLECT'///)
C
      IF (NPRINT.EQ.0) GO TO 295
C TO PRINT CARD IMAGE FOR DYNAFLECT
      WRITE(6,270)
270 FORMAT(5X,'SECTION',4X,'BASE',16X,'DEFLECTIONS')
      WRITE(6,280)
280 FORMAT(5X,' NO.',4X,'THICKNESS',16X,'(MILS)')
      WRITE(6,290)
290 FORMAT(5X,11X,'(INS)',5X,'W1      W2      W3      W4      W5'//)
295 CONTINUE
C
C TO READ IN RESULTS
C
      CALL DYNAF (NC,NSECT1,NSECT2,BASEH,DYN,STFI,DEF,H,STFO,PLOAD,
      *NPRINT)
      GO TO 1500
C
C
C
C TO DETERMINE TYPE OF SUBGRADE
1500 CONTINUE
      CALL SUBGRA (NC,DEF,ISUB)
C
C TO CONVERT READINGS TO STANDARD DEFLECTIONS
C
      DO 800 I=1,NC
      IF (ISUB(I).EQ.1) GO TO 700
      IF (ISUB(I).EQ.2) GO TO 710
      IF (ISUB(I).EQ.3) GO TO 720
      IF (ISUB(I).EQ.4) GO TO 730
700 STD(I,1)=DEF(I,1)
      GO TO 790
710 STD(I,1)=DEF(I,1)/(0.887257-2.70152D-03*DEF(I,1))
      GO TO 790
720 STD(I,1)=DEF(I,1)/(0.733096-6.83744D-03*DEF(I,1))
      GO TO 790
730 STD(I,1)=DEF(I,1)/(0.619104-8.39107D-03*DEF(I,1))
      GO TO 790
790 CONTINUE
800 CONTINUE
C
C TO DETERMINE MATERIAL PROPERTIES AND
C LOAD DEFORMATION EQUATION.
C
      CALL STANDA (NC,STFI,ISUB,H,ACDEF,BCDEF,STD,K1,W,PLOAD,PFWD,NOPT,F
      *WD,DEF,DYN,P)
C
C TO DETERMINE THE MULTIPLIER FOR 1/ACDEF
C
      APASSN=0
      DO 1300 I=1,NC
      IF (BCDEF(I).LE.0) GO TO 888
      AMULT(I)=-0.893347 * BCDEF(I) /10.D-05 + 1.00006
      GO TO 999
888 IF (BCDEF(I).LT.-0.4D-04) GO TO 887
      AMULT(I) = 1.00045 - 0.899468 * BCDEF(I)/10.D-05
      GO TO 999
887 AMULT(I) = 1.00025 - 0.899989 * BCDEF(I) / 10.D-05
999 IF (RUTM.LE.0) GO TO 1090
      DEFM=RUTM/PASSM
      AMULT(I)=ACDEF(I)*ALOADM/(ACDEF(I)*ALOADM/(1-BCDEF(I)*ALOADM)-DEFM
      *)
1090 CONTINUE
C
C TO DETERMINE THE NUMBER OF PASSES ALLOWED
C
      DSUM=0
      DO 1113 K=1,NX

```

DEFN=WLOAD(K)*ACDEF(I)/(1-WLOAD(K)*BCOEF(I))	024000
DSUM=DSUM+(DEFN-WLOAD(K)*ACDEF(I))/AMULT(I))	024100
1113 CONTINUE	024200
1111 PASSN=RUTX/DSUM	024300
PASS(I)=DABS(PASSN)	024400
C	024500
C TO CALCULATE AVERAGE NUMBER OF PASSES ALLOWED	024600
C	024700
APASSN=PASS(I) + APASSN	024800
1200 CONTINUE	024900
1300 CONTINUE	025000
C	025100
GO TO 4000	025200
C	025300
C MAIN PROGRAM CONTINUES	025400
C	025500
4000 CONTINUE	025600
C	025700
IF (NPRINT.EQ.0) GO TO 4500	025800
WRITE(6,200)	025900
200 FORMAT(//,5X,'SECTION',3X,'LAYER PROPERTIES',6X,'LOAD DEFORMATION'	026000
*,6X,'NO. OF')	026100
WRITE(6,210)	026200
210 FORMAT(5X,2X,'NO.',4X,'BASE/SUBB',1X,'SUBGRADE',6X,'CHARACTERISTIC	026300
*S',5X,'ALLOWABLE')	026400
IF (NOPT.NE.1) GO TO 227	026500
WRITE(6,220)	026600
220 FORMAT(5X,10X,'K1-VALUE',2X,'TYPE',4X,'STIFF(LB/IN)',2X,'BCOEF'	026700
*,7X,'PASSES',4X,'AMULT',5X,'W1CHECK'/)	026800
GO TO 229	026900
227 WRITE(6,228)	027000
228 FORMAT(5X,10X,'K1-VALUE',2X,'TYPE',4X,'STIFF(LB/IN)',2X,'BCOEF'	027100
*,7X,'PASSES',4X,'AMULT'/)	027200
229 CONTINUE	027300
C	027400
DO 4200 I=1,NC	027500
STIFF(I)=1/ACDEF(I)	027600
IF(ISUB(I).EQ.2) GO TO 4120	027700
IF(ISUB(I).EQ.3) GO TO 4140	027800
IF(ISUB(I).EQ.4) GO TO 4160	027900
IF (NOPT.NE.1) GO TO 4001	028000
WRITE(6,230) NSECT1(I),NSECT2(I),K1(I),STIFF(I),BCOEF(I),PASS(I)	028100
*,AMULT(I),W(I)	028200
230 FORMAT(5X,I4,'-',I2,2X,I8,1X,'VERY SOFT ',E10.3,1X,E12.5,1X,E10.3,	028300
*1X,F10.8,1X,F6.2)	028400
GO TO 4200	028500
4001 WRITE(6,231) NSECT1(I),NSECT2(I),K1(I),STIFF(I),BCOEF(I),PASS(I)	028600
*,AMULT(I)	028700
231 FORMAT(5X,I4,'-',I2,2X,I8,1X,'VERY SOFT ',E10.3,1X,E12.5,1X,E10.3,	028800
*1X,F10.8)	028900
GO TO 4200	029000
4120 IF (NOPT.NE.1) GO TO 4002	029100
WRITE(6,240) NSECT1(I),NSECT2(I),K1(I),STIFF(I),BCOEF(I),PASS(I)	029200
*,AMULT(I),W(I)	029300
240 FORMAT(5X,I4,'-',I2,2X,I8,2X,' SOFT ',E10.3,1X,E12.5,1X,E10.3,	029400
*1X,F10.8,1X,F6.2)	029500
GO TO 4200	029600
4002 WRITE(6,241) NSECT1(I),NSECT2(I),K1(I),STIFF(I),BCOEF(I),PASS(I)	029700
*,AMULT(I)	029800
241 FORMAT(5X,I4,'-',I2,2X,I8,2X,' SOFT ',E10.3,1X,E12.5,1X,E10.3,	029900
*1X,F10.8)	030000
GO TO 4200	030100
4140 IF (NOPT.NE.1) GO TO 4003	030200
WRITE(6,250) NSECT1(I),NSECT2(I),K1(I),STIFF(I),BCOEF(I),PASS(I)	030300
*,AMULT(I),W(I)	030400
250 FORMAT(5X,I4,'-',I2,2X,I8,2X,' MEDIUM ',E10.3,1X,E12.5,1X,E10.3,	030500
*1X,F10.8,1X,F6.2)	030600
GO TO 4200	030700
4003 WRITE(6,251) NSECT1(I),NSECT2(I),K1(I),STIFF(I),BCOEF(I),PASS(I)	030800
*,AMULT(I)	030900
251 FORMAT(5X,I4,'-',I2,2X,I8,2X,' MEDIUM ',E10.3,1X,E12.5,1X,E10.3,	031000
*1X,F10.8)	031100
GO TO 4200	031200
4160 IF (NOPT.NE.1) GO TO 4004	031300
WRITE(6,260) NSECT1(I),NSECT2(I),K1(I),STIFF(I),BCOEF(I),PASS(I)	031400
*,AMULT(I),W(I)	031500
260 FORMAT(5X,I4,'-',I2,2X,I8,2X,' STIFF ',E10.3,1X,E12.5,1X,E10.3,	031600
*1X,F10.8,1X,F6.2)	031700
GO TO 4200	031800
4004 WRITE(6,261) NSECT1(I),NSECT2(I),K1(I),STIFF(I),BCOEF(I),PASS(I)	031900

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      *,AMULT(I)
261 FORMAT(5X,I4,'-',I2,2X,I8,2X,' STIFF ',E10.3,1X,E12.5,1X,E10.3,
      *1X,F10.8)
4200 CONTINUE
C
      WRITE(6,300)
300 FORMAT(////,5X,'LOAD DEFLECTION MODEL : '/')
      WRITE(6,310)
310 FORMAT(5X,'LOAD = DEFLECTION / (BCOEF*DEFLECTION + 1/STIFFNESS)')
      APASSN=APASSN/NC
      WRITE(6,320) APASSN
320 FORMAT(5X,'AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : ',
      *E10.4/)
      GO TO 4600
C
4500 CONTINUE
C
      IF(NOPT.EQ.1) GO TO 4400
C TO PRINT CARD IMAGE FOR DYNAFLECT
      WRITE(6,370)
370 FORMAT(5X,'SECTION',4X,'BASE',16X,'DEFLECTIONS',15X,'NO. OF')
      WRITE(6,380)
380 FORMAT(5X,' NO.',4X,'THICKNESS',16X,'(MILS)',16X,'ALLOWABLE')
      WRITE(6,390)
390 FORMAT(5X,11X,'(INS)',5X,'W1      W2      W3      W4      W5',6X,
      * 'PASSES'/)
      DO 206 I=1,NC
      WRITE(6,205) NSECT1(I),NSECT2(I),BASEH(I),(DYN(I,J),J=1,5),PASS(I)
205 FORMAT(5X,I4,'-',I2,3X,F5.2,2X,5(2X,F5.2),3X,E10.3)
206 CONTINUE
      GO TO 4550
C
4400 CONTINUE
C TO PRINT CARD IMAGE FOR FALLING WEIGHT DEFLECTOMETER
C
      WRITE(6,17)
17 FORMAT(5X,'SECTION',4X,'BASE',21X,'DEFLECTIONS',30X,'NO. OF')
      WRITE(6,18)
18 FORMAT(5X,' NO.',4X,'THICKNESS',20X,'(MILS)',24X,'LOAD',3X,
      * ' ALLOWABLE')
      WRITE(6,19)
19 FORMAT(5X,11X,'(INS)',5X,'W1      W2      W3      W4      W5      W6
      * W7 (LBS)  PASSES'/)
      DO 36 I=1,NC
      WRITE(6,105) NSECT1(I),NSECT2(I),BASEH(I),(FWD(I,J),J=1,7),PFWD(I)
      *,PASS(I)
105 FORMAT (5X,I4,'-',I2,3X,F5.2,2X,7(2X,F5.2),2X,F6.0,1X,E10.3)
36 CONTINUE
C
4550 CONTINUE
C
      APASSN=APASSN/NC
      WRITE(6,32) APASSN
32 FORMAT(///5X,'AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : ',
      *E10.4/)
4600 CONTINUE
C TO RETURN TO FIRST STEP
      GO TO 90
C
C ENDING THE PROGRAM
C
5000 CONTINUE
      WRITE (6,810)
810 FORMAT(1H1,///,5X,'END OF JOB')
      STOP
      END
C
      SUBROUTINE FWDEF (NC,NSECT1,NSECT2,BASEH,FWD,STFI,DEF,H,STFO,PLOAD
      *,PFWD,NPRINT)
      IMPLICIT REAL*8(A-H,O-Z)
      DIMENSION NSECT1(20),NSECT2(20),BASEH(20),FWD(20,7),STFI(20),DEF(2
      *0,7),H(20),STFO(20),PFWD(20)
      DO 1015 I=1,NC
C
      READ (5,1010) NSECT1(I),NSECT2(I),BASEH(I),(FWD(I,J),J=1,7),PFWD(I
      *)
1010 FORMAT (I4,I2,12X,F5.2,7(1X,F5.2),1X,F6.0)
      IF(NPRINT.EQ.0) GO TO 1006
      WRITE(6,1005) NSECT1(I),NSECT2(I),BASEH(I),(FWD(I,J),J=1,7),PFWD(I
      *)

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1005 FORMAT (5X,I4,'-',I2,3X,F5.2,2X,7(2X,F5.2),6X,F6.0) 040000
1006 CONTINUE 040100
      PLOAD=10956.3 040200
      STFO(I)=PFWD(I)/FWD(I,1) 040300
      DEF(I,1)=PLOAD/STFO(I) 040400
      STFI(I)= -109.663 + 1.31393 * STFO(I) 040500
      DEF(I,7) = FWD(I,7)*PLOAD/PFWD(I) 040600
      H(I)=BASEH(I) 040700
1015 CONTINUE 040800
      RETURN 040900
      END 041000
C 041100
      SUBROUTINE DYNAF (NC,NSECT1,NSECT2,BASEH,DYN,STFI,DEF,H,STFO,PLOAD 041200
*,NPRINT) 041300
      IMPLICIT REAL*8(A-H,O-Z) 041400
      DIMENSION NSECT1(20),NSECT2(20),BASEH(20),DYN(20,5),STFI(20),H(20) 041500
*,DEF(20,7),STFO(20) 041600
      DO 2015 I=1,NC 041700
C 041800
C COMPUTATION WITH DYNAFLECT DATA ***** 041900
C 042000
      READ(5,2010) NSECT1(I),NSECT2(I),BASEH(I),(DYN(I,J),J=1,5) 042100
2010 FORMAT(I4,I2,12X,F5.2,5(1X,F5.2)) 042200
      IF (NPRINT.EQ.O) GO TO 2006 042300
C 042400
      WRITE(6,2005) NSECT1(I),NSECT2(I),BASEH(I),(DYN(I,J),J=1,5) 042500
2005 FORMAT(5X,I4,'-',I2,3X,F5.2,2X,5(2X,F5.2)) 042600
2006 CONTINUE 042700
      PLOAD=1000.0 042800
C TO CONVERT DYNAFLECT READINGS TO DEF( ) 042900
      DEF(I,7)=3.38075*DYN(I,5)**0.639462 043000
      DEF(I,1)= -7.24474 + (29.6906 * DYN(I,1)) 043100
C TO OBTAIN STIFFNESSES 043200
      STFO(I)= 86.0122*DEXP(1.87211D-03*PLOAD/DYN(I,1)) 043300
      STFI(I)= -109.663 + 1.31393 * STFO(I) 043400
      H(I)=BASEH(I) 043500
2015 CONTINUE 043600
      RETURN 043700
      END 043800
C 043900
      SUBROUTINE SUBGRA (NC,DEF,ISUB) 044000
      IMPLICIT REAL*8(A-H,O-Z) 044100
      DIMENSION ISUB(20),DEF(20,7) 044200
      DO 595 I=1,NC 044300
C TO DETERMINE TYPE OF SUBGRADE 044400
      IF (DEF(I,7).GE.1.75) GO TO 510 044500
      IF (DEF(I,7).GE.1.4.AND.DEF(I,7).LT.1.75) GO TO 520 044600
      IF (DEF(I,7).GE.0.80.AND.DEF(I,7).LT.1.4) GO TO 530 044700
      IF (DEF(I,7).LT.0.80) GO TO 540 044800
510 ISUB(I)=1 044900
      GO TO 590 045000
520 ISUB(I)=2 045100
      GO TO 590 045200
530 ISUB(I)=3 045300
      GO TO 590 045400
540 ISUB(I)=4 045500
      GO TO 590 045600
590 CONTINUE 045700
595 CONTINUE 045800
      RETURN 045900
      END 046000
C 046100
      SUBROUTINE STANDA (NC,STFI,ISUB,H,ACDEF,BCDEF,STD,K1,W,PLOAD,PFWD, 046200
*NOPT,FWD,DEF,DYN,P) 046300
      IMPLICIT REAL*8(A-H,O-Z) 046400
      DIMENSION H(20),STFI(20),ACDEF(20),K1(20),STD(20,7),BCDEF(20), 046500
*ISUB(20),W(20),PFWD(20),FWD(20,7),DEF(20,7),DYN(20,5) 046600
      DO 695 I=1,NC 046700
C TO DETERMINE INITIAL STIFFNESS AND COEFFICIENT 'A' 046800
C 046900
      ACDEF(I) = 1 / (STFI(I) * 1000) 047000
C 047100
C TO OBTAIN K1-VALUE FOR BASE COURSE 047200
C MODEL : E = K1 * BULK STRESS**0.33 047300
C 047400
      IF (H(I).GT.5.0) GO TO 1100 047500
C 047600
      FOR BASE THICKNESS OF LESS THAN 5 INS. 047700
      CC = 10** (16.1791*H(I)**(-0.349993)) 047800
      CD = -4.94876 * H(I)**(-0.39432) 047900
      GO TO 1111

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C
C FOR BASE THICKNESS OF 5 INS AND ABOVE
1100 CC = 10**(12.8778*H(I)**(-0.18345))
      CD = -2.95407 * DEXP(-0.0175321*H(I))
C
C
1111 AK1 = CC * STD(I,1)**CD
      K1(I) = AK1
C
C TO DETERMINE COEFFICIENT 'B'
C
2222 CONTINUE
      CE = 1.36543D-06*AK1**0.185895
      CF = 3.15679D-06+3.24823D-11*AK1 -1.05093D-16*AK1**2
      CG = -1.74866D-07 -1.00162D-11*AK1 +2.3941D-17*AK1**2
C
      BCDEF(I) = CE + CF*H(I) + CG*H(I)**2
      IF (ISUB(I).EQ.1) GO TO 690
      IF (ISUB(I).EQ.2) GO TO 620
      IF (ISUB(I).EQ.3) GO TO 630
      IF (ISUB(I).EQ.4) GO TO 640
620 BCDEF(I)=BCDEF(I)-0.5D-05
      GO TO 690
630 BCDEF(I)=BCDEF(I)-0.75D-05
      GO TO 690
640 BCDEF(I)=BCDEF(I)-0.9D-05
      GO TO 690
690 CONTINUE
      IF (NOPT.EQ.1) GO TO 5555
      PFWD(I)=PLOAD
5555 CONTINUE
      IF (BCDEF(I).LT.0) GO TO 5560
      WCHECK=10956.3*ACDEF(I)/(1-10956.3*BCDEF(I))*1000.0
      RCHECK =DABS((WCHECK-DEF(I,1))/DEF(I,1))
      IF (RCHECK.LT.0.20) GO TO 5556
5560 BCDEF(I)=-(-29.9362D-06 + 9.66528 * ACDEF(I))
      IF (ACDEF(I).GT.3.1D-06) GO TO 5556
      BCDEF(I)= -0.001D-10
5556 W(I)=PFWD(I)*ACDEF(I)/(1-PFWD(I)*BCDEF(I))*1000.0
695 CONTINUE
      RETURN
      END
C
C THE FOLLOWING IS THE SAMPLE DATA INPUT -----
C
1 17BURLESON          FM2000          12201982
SAMPLE PROBLEM 1
6 4 0.75
  9000.0
 15000.
 20000.
 25000.
000800          7.000 54.88 32.91 21.18 10.31 5.039 2.874 2.165 11108.
000801          7.000 55.98 32.83 21.69 11.18 5.591 3.228 2.047 10981.
001000          7.000 51.93 32.83 22.44 11.65 5.000 2.441 1.200 11616.
001001          7.000 53.74 34.09 23.27 12.36 5.551 2.953 1.700 11696.
001200          6.000 31.53 19.29 13.54 7.205 4.016 2.598 1.300 11759.
001201          6.000 32.24 18.74 12.83 7.007 3.898 2.441 1.200 11791.
2 17BURLESON          FM2000          03011983
SAMPLE PROBLEM 2
6 4 0.75
  9000.0
 15000.
 20000.
 25000.
000800          7.000 2.130 1.530 1.140 0.870 0.600
000801          7.000 1.890 1.500 1.140 0.860 0.660
001000          7.000 1.410 1.080 0.740 0.550 0.390
001001          7.000 1.470 1.140 0.740 0.530 0.350
001200          6.000 3.300 1.140 0.780 0.540 0.220
001201          6.000 1.470 1.080 0.780 0.540 0.400
1117BURLESON          FM2000          12201982
SAMPLE PROBLEM 3
6 1 0.75
  9000.0
000800          7.000 54.88 32.91 21.18 10.31 5.039 2.874 2.165 11108.
000801          7.000 55.98 32.83 21.69 11.18 5.591 3.228 2.047 10981.
001000          7.000 51.93 32.83 22.44 11.65 5.000 2.441 1.200 11616.
001001          7.000 53.74 34.09 23.27 12.36 5.551 2.953 1.700 11696.
001200          6.000 31.53 19.29 13.54 7.205 4.016 2.598 1.300 11759.

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001201	6.000	32.24	18.74	12.83	7.007	3.898	2.441	1.200	11791.
1117BURLESON	FM2000			12201982					
SAMPLE PROBLEM 4									
6 1 0.75									
15000.									
000800	7.000	54.88	32.91	21.18	10.31	5.039	2.874	2.165	11108.
000801	7.000	55.98	32.83	21.69	11.18	5.591	3.228	2.047	10981.
001000	7.000	51.93	32.83	22.44	11.65	5.000	2.441	1.200	11616.
001001	7.000	53.74	34.09	23.27	12.36	5.551	2.953	1.700	11696.
001200	6.000	31.53	19.29	13.54	7.205	4.016	2.598	1.300	11759.
001201	6.000	32.24	18.74	12.83	7.007	3.898	2.441	1.200	11791.
1117BURLESON	FM2000			12201982					
SAMPLE PROBLEM 5									
6 1 0.75									
20000.									
000800	7.000	54.88	32.91	21.18	10.31	5.039	2.874	2.165	11108.
000801	7.000	55.98	32.83	21.69	11.18	5.591	3.228	2.047	10981.
001000	7.000	51.93	32.83	22.44	11.65	5.000	2.441	1.200	11616.
001001	7.000	53.74	34.09	23.27	12.36	5.551	2.953	1.700	11696.
001200	6.000	31.53	19.29	13.54	7.205	4.016	2.598	1.300	11759.
001201	6.000	32.24	18.74	12.83	7.007	3.898	2.441	1.200	11791.
1117BURLESON	FM2000			12201982					
SAMPLE PROBLEM 6									
6 1 0.75									
25000.									
000800	7.000	54.88	32.91	21.18	10.31	5.039	2.874	2.165	11108.
000801	7.000	55.98	32.83	21.69	11.18	5.591	3.228	2.047	10981.
001000	7.000	51.93	32.83	22.44	11.65	5.000	2.441	1.200	11616.
001001	7.000	53.74	34.09	23.27	12.36	5.551	2.953	1.700	11696.
001200	6.000	31.53	19.29	13.54	7.205	4.016	2.598	1.300	11759.
001201	6.000	32.24	18.74	12.83	7.007	3.898	2.441	1.200	11791.
2117BURLESON	FM2000			03011983					
SAMPLE PROBLEM 7									
6 1 0.75									
9000.0									
000800	7.000	2.130	1.530	1.140	0.870	0.600			
000801	7.000	1.890	1.500	1.140	0.860	0.660			
001000	7.000	1.410	1.080	0.740	0.550	0.390			
001001	7.000	1.470	1.140	0.740	0.530	0.350			
001200	6.000	3.300	1.140	0.780	0.540	0.220			
001201	6.000	1.470	1.080	0.780	0.540	0.400			
2117BURLESON	FM2000			03011983					
SAMPLE PROBLEM 8									
6 1 0.75									
15000.									
000800	7.000	2.130	1.530	1.140	0.870	0.600			
000801	7.000	1.890	1.500	1.140	0.860	0.660			
001000	7.000	1.410	1.080	0.740	0.550	0.390			
001001	7.000	1.470	1.140	0.740	0.530	0.350			
001200	6.000	3.300	1.140	0.780	0.540	0.220			
001201	6.000	1.470	1.080	0.780	0.540	0.400			
2117BURLESON	FM2000			03011983					
SAMPLE PROBLEM 9									
6 1 0.75									
20000.									
000800	7.000	2.130	1.530	1.140	0.870	0.600			
000801	7.000	1.890	1.500	1.140	0.860	0.660			
001000	7.000	1.410	1.080	0.740	0.550	0.390			
001001	7.000	1.470	1.140	0.740	0.530	0.350			
001200	6.000	3.300	1.140	0.780	0.540	0.220			
001201	6.000	1.470	1.080	0.780	0.540	0.400			
2117BURLESON	FM2000			03011983					
SAMPLE PROBLEM 10									
6 1 0.75									
25000.									
000800	7.000	2.130	1.530	1.140	0.870	0.600			
000801	7.000	1.890	1.500	1.140	0.860	0.660			
001000	7.000	1.410	1.080	0.740	0.550	0.390			
001001	7.000	1.470	1.140	0.740	0.530	0.350			
001200	6.000	3.300	1.140	0.780	0.540	0.220			
001201	6.000	1.470	1.080	0.780	0.540	0.400			
21 8BORDEN	FM 612			11201975					
SAMPLE RUN --FOR BACKCALCULATING THE SLOPE MULTIPLIER									
10 1 1.00	0.75	9000.0		25279.0					
9000.0									
83501	5.000	2.700	1.380	0.540	0.300	0.200			
83502	5.000	2.160	1.380	0.720	0.420	0.260			
83503	5.000	1.620	1.020	0.440	0.250	0.150			
83504	5.000	1.500	0.830	0.360	0.230	0.140			
83505	5.000	1.440	0.900	0.440	0.280	0.170			

83506	5.000	1.380	0.850	0.380	0.220	0.150
83507	5.000	1.440	0.770	0.330	0.210	0.140
83508	5.000	1.260	0.580	0.260	0.160	0.120
83509	5.000	1.320	0.700	0.300	0.180	0.120
83510	5.000	1.140	0.580	0.400	0.160	0.110

Sample Output

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE PROBLEM 1

DISTRICT: 17 COUNTY: BURLESON ROAD: FM2000

ALLOWABLE RUT(INS): 0.75

AXLE NUMBER WHEEL LOAD(LBS)

1	9000.
2	15000.
3	20000.
4	25000.

RECORDED RUT(INS): 0.00 LOAD(LBS): 0. PASSES: 0.00000 00

DATE: 12/20/1982 FALLING WEIGHT DEFLECTOMETER

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)							LOAD (LBS)	NO. OF ALLOWABLE PASSES
		W1	W2	W3	W4	W5	W6	W7		
8- 0	7.00	54.88	32.91	21.18	10.31	5.04	2.87	2.17	11108.	0.113D 02
8- 1	7.00	55.98	32.83	21.69	11.18	5.59	3.23	2.05	10981.	0.104D 02
10- 0	7.00	51.93	32.83	22.44	11.65	5.00	2.44	1.20	11616.	0.156D 02
10- 1	7.00	53.74	34.09	23.27	12.36	5.55	2.95	1.70	11696.	0.142D 02
12- 0	6.00	31.53	19.29	13.54	7.21	4.02	2.60	1.30	11759.	0.361D 02
12- 1	6.00	32.24	18.74	12.83	7.01	3.90	2.44	1.20	11791.	0.336D 02

AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.2020D 02

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE PROBLEM 2

DISTRICT: 17 COUNTY: BURLESON ROAD: FM2000
ALLOWABLE RUT(INS): 0.75
AXLE NUMBER WHEEL LOAD(LBS)
1 9000.
2 15000.
3 20000.
4 25000.

RECORDED RUT(INS): 0.00 LOAD(LBS): 0. PASSES: 0.0000D 00

DATE: 3/ 1/1983 DYNAFLECT

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)					NO. OF ALLOWABLE PASSES
		W1	W2	W3	W4	W5	
8- 0	7.00	2.13	1.53	1.14	0.87	0.60	0.121D 02
8- 1	7.00	1.89	1.50	1.14	0.86	0.66	0.176D 02
10- 0	7.00	1.41	1.08	0.74	0.55	0.39	0.838D 02
10- 1	7.00	1.47	1.14	0.74	0.53	0.35	0.327D 02
12- 0	6.00	3.30	1.14	0.78	0.54	0.22	0.558D 01
12- 1	6.00	1.47	1.08	0.78	0.54	0.40	0.345D 02

AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.3105D 02

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE PROBLEM 3

DISTRICT: 17 COUNTY: BURLESON ROAD: FM2000
ALLOWABLE RUT(INS): 0.75
AXLE NUMBER WHEEL LOAD(LBS)
1 9000.

RECORDED RUT(INS): 0.00 LOAD(LBS): 0. PASSES: 0.00000 00

DATE: 12/20/1982 FALLING WEIGHT DEFLECTOMETER

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)							LOAD (LBS)
		W1	W2	W3	W4	W5	W6	W7	
8- 0	7.00	54.88	32.91	21.18	10.31	5.04	2.87	2.17	11108.
8- 1	7.00	55.98	32.83	21.69	11.18	5.59	3.23	2.05	10981.
10- 0	7.00	51.93	32.83	22.44	11.65	5.00	2.44	1.20	11616.
10- 1	7.00	53.74	34.09	23.27	12.36	5.55	2.95	1.70	11696.
12- 0	6.00	31.53	19.29	13.54	7.21	4.02	2.60	1.30	11759.
12- 1	6.00	32.24	18.74	12.83	7.01	3.90	2.44	1.20	11791.

SECTION NO.	LAYER PROPERTIES		LOAD DEFORMATION CHARACTERISTICS		NO. OF ALLOWABLE PASSES			AMULT	W1CHECK
	BASE/SUBB K1-VALUE	SUBGRADE TYPE	STIFF(LB/IN)	BCDEF	PASSES				
8- 0	30369	VERY SOFT	0.156D 06	-0.31908D-04	0.770D 05	1	28745631	52.48	
8- 1	27980	VERY SOFT	0.148D 06	-0.35336D-04	0.818D 05	1	31828598	53.43	
10- 0	3555	MEDIUM	0.184D 06	-0.22523D-04	0.673D 05	1	20303594	49.97	
10- 1	17387	SOFT	0.176D 06	-0.24886D-04	0.693D 05	1	22429461	51.38	
12- 0	43477	MEDIUM	0.380D 06	0.83217D-05	0.442D 05	0	92571837	34.27	
12- 1	40461	MEDIUM	0.371D 06	0.86308D-05	0.415D 05	0	92295711	35.39	

LOAD DEFLECTION MODEL :

LOAD = DEFLECTION / (BCDEF*DEFLECTION + 1/STIFFNESS)
AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.6350D 05

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE PROBLEM 4

DISTRICT: 17 COUNTY: BURLESON ROAD: FM2000
ALLOWABLE RUT(INS): 0.75
AXLE NUMBER WHEEL LOAD(LBS)
1 15000.

RECORDED RUT(INS): 0.00 LOAD(LBS): 0. PASSES: 0.00000 00

DATE: 12/20/1982 FALLING WEIGHT DEFLECTOMETER

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)							LOAD (LBS)
		W1	W2	W3	W4	W5	W6	W7	
8- 0	7.00	54.88	32.91	21.18	10.31	5.04	2.87	2.17	11108.
8- 1	7.00	55.98	32.83	21.69	11.18	5.59	3.23	2.05	10981.
10- 0	7.00	51.93	32.83	22.44	11.65	5.00	2.44	1.20	11616.
10- 1	7.00	53.74	34.09	23.27	12.36	5.55	2.95	1.70	11696.
12- 0	6.00	31.53	19.29	13.54	7.21	4.02	2.60	1.30	11759.
12- 1	6.00	32.24	18.74	12.83	7.01	3.90	2.44	1.20	11791.

SECTION NO.	LAYER PROPERTIES		LOAD DEFORMATION CHARACTERISTICS		NO. OF ALLOWABLE PASSES	AMULT	W1CHECK
	BASE/SUBB K1-VALUE	SUBGRADE TYPE	STIFF(LB/IN)	BCOEF			
8- 0	30369	VERY SOFT	0.156D 06	-0.31908D-04	0.778D 02	1.28745631	52.48
8- 1	27980	VERY SOFT	0.148D 06	-0.35336D-04	0.705D 02	1.31828598	53.43
10- 0	3555	MEDIUM	0.184D 06	-0.22523D-04	0.110D 03	1.20303594	49.97
10- 1	17387	SOFT	0.176D 06	-0.24886D-04	0.995D 02	1.22429461	51.38
12- 0	43477	MEDIUM	0.380D 06	0.83217D-05	0.305D 03	0.92571837	34.27
12- 1	40461	MEDIUM	0.371D 06	0.86308D-05	0.284D 03	0.92295711	35.39

LOAD DEFLECTION MODEL :

LOAD = DEFLECTION / (BCOEF*DEFLECTION + 1/STIFFNESS)
AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.1578D 03

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE PROBLEM 5

DISTRICT: 17 COUNTY: BURLESON ROAD: FM2000
ALLOWABLE RUT(INS): 0.75
AXLE NUMBER WHEEL LOAD(LBS)
1 20000.

RECORDED RUT(INS): 0.00 LOAD(LBS): 0. PASSES: 0.00000 00

DATE: 12/20/1982 FALLING WEIGHT DEFLECTOMETER

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)							LOAD (LBS)
		W1	W2	W3	W4	W5	W6	W7	
8- 0	7.00	54.88	32.91	21.18	10.31	5.04	2.67	2.17	11108.
8- 1	7.00	55.98	32.83	21.69	11.18	5.59	3.23	2.05	10981.
10- 0	7.00	51.93	32.83	22.44	11.65	5.00	2.44	1.20	11616.
10- 1	7.00	53.74	34.09	23.27	12.36	5.55	2.95	1.70	11696.
12- 0	6.00	31.53	19.29	13.54	7.21	4.02	2.60	1.30	11759.
12- 1	6.00	32.24	18.74	12.83	7.01	3.90	2.44	1.20	11791.

SECTION NO.	LAYER PROPERTIES		LOAD DEFORMATION CHARACTERISTICS		NO. OF ALLOWABLE PASSES	AMULT	W1CHECK
	BASE/SUBB K1-VALUE	SUBGRADE TYPE	STIFF(LB/IN)	BCOEF			
8- 0	30369	VERY SOFT	0.156D 06	-0.31908D-04	0.352D 02	1.28745631	52.48
8- 1	27980	VERY SOFT	0.148D 06	-0.35336D-04	0.322D 02	1.31828598	53.43
10- 0	3555	MEDIUM	0.184D 06	-0.22523D-04	0.487D 02	1.20303594	49.97
10- 1	17387	SOFT	0.176D 06	-0.24886D-04	0.443D 02	1.22429461	51.38
12- 0	43477	MEDIUM	0.380D 06	0.83217D-05	0.119D 03	0.92571837	34.27
12- 1	40461	MEDIUM	0.371D 06	0.86308D-05	0.111D 03	0.92295711	35.39

LOAD DEFLECTION MODEL :

LDAD = DEFLECTION / (BCOEF*DEFLECTION + 1/STIFFNESS)
AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.6517D 02

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE PROBLEM 6

DISTRICT: 17 COUNTY: BURLESON ROAD: FM2000
ALLOWABLE RUT(INS): 0.75
AXLE NUMBER WHEEL LOAD(LBS)
1 25000.

RECORDED RUT(INS): 0.00 LOAD(LBS): 0. PASSES: 0.00000 00

DATE: 12/20/1982 FALLING WEIGHT DEFLECTOMETER

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)							LOAD (LBS)
		W1	W2	W3	W4	W5	W6	W7	
8- 0	7.00	54.88	32.91	21.18	10.31	5.04	2.87	2.17	11108.
8- 1	7.00	55.98	32.83	21.69	11.18	5.59	3.23	2.05	10981.
10- 0	7.00	51.93	32.83	22.44	11.65	5.00	2.44	1.20	11616.
10- 1	7.00	53.74	34.09	23.27	12.36	5.55	2.95	1.70	11696.
12- 0	6.00	31.53	19.29	13.54	7.21	4.02	2.60	1.30	11759.
12- 1	6.00	32.24	18.74	12.83	7.01	3.90	2.44	1.20	11791.

SECTION NO.	LAYER PROPERTIES		LOAD DEFORMATION CHARACTERISTICS		ND. OF ALLOWABLE PASSES	AMULT	W1CHECK
	BASE/SUBB K1-VALUE	SUBGRADE TYPE	STIFF(LB/IN)	BCOEF			
8- 0	30369	VERY SOFT	0.156D 06	-0.31908D-04	0.213D 02	1.28745631	52.48
8- 1	27980	VERY SOFT	0.148D 06	-0.35336D-04	0.195D 02	1.31828598	53.43
10- 0	3555	MEDIUM	0.184D 06	-0.22523D-04	0.289D 02	1.20303594	49.97
10- 1	17387	SOFT	0.176D 06	-0.24886D-04	0.264D 02	1.22429461	51.38
12- 0	43477	MEDIUM	0.380D 06	0.83217D-05	0.625D 02	0.92571837	34.27
12- 1	40461	MEDIUM	0.371D 06	0.86308D-05	0.581D 02	0.92295711	35.39

LOAD DEFLECTION MODEL :

LOAD = DEFLECTION / (BCOEF*DEFLECTION + 1/STIFFNESS)
AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.3611D 02

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE PROBLEM 7

DISTRICT: 17 COUNTY: BURLESON ROAD: FM2000
ALLOWABLE RUT(INS): 0.75
AXLE NUMBER WHEEL LOAD(LBS)
1 9000.

RECORDED RUT(INS): 0.00 LOAD(LBS): 0. PASSES: 0.00000 00

DATE: 3/ 1/1983 DYNAFLECT

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)				
		W1	W2	W3	W4	W5
8- 0	7.00	2.13	1.53	1.14	0.87	0.60
8- 1	7.00	1.89	1.50	1.14	0.86	0.66
10- 0	7.00	1.41	1.08	0.74	0.55	0.39
10- 1	7.00	1.47	1.14	0.74	0.53	0.35
12- 0	6.00	3.30	1.14	0.78	0.54	0.22
12- 1	6.00	1.47	1.08	0.78	0.54	0.40

SECTION NO.	LAYER PROPERTIES		LOAD DEFORMATION CHARACTERISTICS		NO. DF ALLOWABLE PASSES	AMULT
	BASE/SUBB K1-VALUE	SUBGRADE TYPE	STIFF(LB/IN)	BCOEF		
8- 0	27795	VERY SOFT	0.163D 06	-0.29539D-04	0.741D 05	1.26614338
8- 1	39667	VERY SOFT	0.195D 06	-0.19719D-04	0.652D 05	1.17781897
10- 0	97648	VERY SOFT	0.317D 06	0.35344D-05	0.838D 05	0.96848570
10- 1	46098	SOFT	0.294D 06	0.73670D-05	0.389D 05	0.93424741
12- 0	34	MEDIUM	0.896D 05	-0.77889D-04	0.896D 05	1.70124384
12- 1	131541	VERY SOFT	0.294D 06	0.70676D-05	0.406D 05	0.93692154

LOAD DEFLECTION MODEL :

LOAD = DEFLECTION / (BCOEF*DEFLECTION + 1/STIFFNESS)
AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.6535D 05

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE PROBLEM 8

DISTRICT: 17 COUNTY: BURLESON ROAD: FM2000
ALLOWABLE RUT(INS): 0.75
AXLE NUMBER WHEEL LOAD(LBS)
1 15000.

RECORDED RUT(INS): 0.00 LOAD(LBS): 0. PASSES: 0.00000 00

DATE: 3/ 1/1983 DYNAFLECT

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)				
		W1	W2	W3	W4	W5
8- 0	7.00	2.13	1.53	1.14	0.87	0.60
8- 1	7.00	1.89	1.50	1.14	0.86	0.66
10- 0	7.00	1.41	1.08	0.74	0.55	0.39
10- 1	7.00	1.47	1.14	0.74	0.53	0.35
12- 0	6.00	3.30	1.14	0.78	0.54	0.22
12- 1	6.00	1.47	1.08	0.78	0.54	0.40

SECTION NO.	LAYER PROPERTIES		LOAD DEFORMATION CHARACTERISTICS		NO. OF ALLOWABLE PASSES	AMULT
	BASE/SUBB K1-VALUE	SUBGRADE TYPE	STIFF(LB/IN)	BCOEF		
8- 0	27795	VERY SDFT	0.163D 06	-0.29539D-04	0.839D 02	1.26614338
8- 1	39667	VERY SDFT	0.195D 06	-0.19719D-04	0.126D 03	1.17781897
10- 0	97648	VERY SDFT	0.317D 06	0.35344D-05	0.675D 03	0.96848570
10- 1	46098	SDFT	0.294D 06	0.73670D-05	0.273D 03	0.93424741
12- 0	34	MEDIUM	0.896D 05	-0.77889D-04	0.354D 02	1.70124384
12- 1	131541	VERY SDFT	0.294D 06	0.70676D-05	0.287D 03	0.93692154

LOAD DEFLECTION MODEL :

LOAD = DEFLECTION / (BCOEF*DEFLECTION + 1/STIFFNESS)
AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.2468D 03

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE PROBLEM 9

DISTRICT: 17 COUNTY: BURLESON ROAD: FM2000
ALLOWABLE RUT(INS): 0.75
AXLE NUMBER WHEEL LOAD(LBS)
1 20000.

RECORDED RUT(INS): 0.00 LOAD(LBS): 0. PASSES: 0.00000 00

DATE: 3/ 1/1983 DYNAFLECT

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)				
		W1	W2	W3	W4	W5
8- 0	7.00	2.13	1.53	1.14	0.87	0.60
8- 1	7.00	1.89	1.50	1.14	0.86	0.66
10- 0	7.00	1.41	1.08	0.74	0.55	0.39
10- 1	7.00	1.47	1.14	0.74	0.53	0.35
12- 0	6.00	3.30	1.14	0.78	0.54	0.22
12- 1	6.00	1.47	1.08	0.78	0.54	0.40

SECTION NO.	LAYER PROPERTIES		LOAD DEFORMATION CHARACTERISTICS		NO. OF ALLOWABLE PASSES		AMULT
	BASE/SUBB K1-VALUE	SUBGRADE TYPE	STIFF(LB/IN)	BCDEF	PASSES		
8- 0	27795	VERY SOFT	0.163D 06	-0.29539D-04	0.378D 02		1.26614338
8- 1	39667	VERY SOFT	0.195D 06	-0.19719D-04	0.554D 02		1.17781897
10- 0	97648	VERY SOFT	0.317D 06	0.35344D-05	0.273D 03		0.96848570
10- 1	46098	SOFT	0.294D 06	0.73670D-05	0.108D 03		0.93424741
12- 0	34	MEDIUM	0.896D 05	-0.77889D-04	0.171D 02		1.70124384
12- 1	131541	VERY SOFT	0.294D 06	0.70676D-05	0.113D 03		0.93692154

LOAD DEFLECTION MODEL :

LOAD = DEFLECTION / (BCDEF*DEFLECTION + 1/STIFFNESS)
AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.1007D 03

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE PROBLEM 10

DISTRICT: 17 COUNTY: BURLESON ROAD: FM2000
ALLOWABLE RUT(INS): 0.75
AXLE NUMBER WHEEL LOAD(LBS)
1 25000.

RECORDED RUT(INS): 0.00 LOAD(LBS): 0. PASSES: 0.00000 00

DATE: 3/ 1/1983 DYNAFLECT

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)				
		W1	W2	W3	W4	W5
8- 0	7.00	2.13	1.53	1.14	0.87	0.60
8- 1	7.00	1.89	1.50	1.14	0.86	0.66
10- 0	7.00	1.41	1.08	0.74	0.55	0.39
10- 1	7.00	1.47	1.14	0.74	0.53	0.35
12- 0	6.00	3.30	1.14	0.78	0.54	0.22
12- 1	6.00	1.47	1.08	0.78	0.54	0.40

SECTION NO.	LAYER PROPERTIES		LOAD DEFORMATION CHARACTERISTICS		NO. OF ALLOWABLE PASSES	AMULT
	BASE/SUBB K1-VALUE	SUBGRADE TYPE	STIFF(LB/IN)	BCOEF		
8- 0	27795	VERY SOFT	0.163D 06	-0.29539D-04	0.227D 02	1.26614338
8- 1	39667	VERY SOFT	0.195D 06	-0.19719D-04	0.326D 02	1.17781897
10- 0	97648	VERY SOFT	0.317D 06	0.35344D-05	0.148D 03	0.96848570
10- 1	46098	SOFT	0.294D 06	0.73670D-05	0.568D 02	0.93424741
12- 0	34	MEDIUM	0.896D 05	-0.77889D-04	0.108D 02	1.70124384
12- 1	131541	VERY SOFT	0.294D 06	0.70676D-05	0.599D 02	0.93692154

LOAD DEFLECTION MODEL :

LOAD = DEFLECTION / (BCOEF*DEFLECTION + 1/STIFFNESS)
AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.5507D 02

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE RUN --FOR BACKCALCULATING THE SLOPE MULTIPLIER

DISTRICT: 8 COUNTY: BORDEN ROAD: FM 612
ALLOWABLE RUT(INS): 1.00
AXLE NUMBER WHEEL LOAD(LBS)
1 9000.

RECORDED RUT(INS): 0.75 LOAD(LBS): 9000. PASSES: 0.2528D 05

DATE: 11/20/1975 DYNAFLECT

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)				
		W1	W2	W3	W4	W5
835- 1	5.00	2.70	1.38	0.54	0.30	0.20
835- 2	5.00	2.16	1.38	0.72	0.42	0.26
835- 3	5.00	1.62	1.02	0.44	0.25	0.15
835- 4	5.00	1.50	0.83	0.36	0.23	0.14
835- 5	5.00	1.44	0.90	0.44	0.28	0.17
835- 6	5.00	1.38	0.85	0.38	0.22	0.15
835- 7	5.00	1.44	0.77	0.33	0.21	0.14
835- 8	5.00	1.26	0.58	0.26	0.16	0.12
835- 9	5.00	1.32	0.70	0.30	0.18	0.12
835-10	5.00	1.14	0.58	0.40	0.16	0.11

SECTION NO.	LAYER PROPERTIES		LOAD DEFORMATION CHARACTERISTICS		NO. OF ALLOWABLE PASSES	AMULT
	BASE/SUBB K1-VALUE	SUBGRADE TYPE	STIFF(LB/IN)	BCOEF		
835- 1	691	MEDIUM	0.116D 06	-0.53087D-04	0.337D 05	1.47862390
835- 2	29654	SOFT	0.159D 06	-0.30773D-04	0.337D 05	1.27780952
835- 3	19797	MEDIUM	0.249D 06	-0.88382D-05	0.337D 05	1.08050215
835- 4	29195	MEDIUM	0.284D 06	0.10639D-04	0.337D 05	0.90501794
835- 5	35678	MEDIUM	0.305D 06	0.10450D-04	0.337D 05	0.90677925
835- 6	43826	MEDIUM	0.329D 06	0.10154D-04	0.337D 05	0.90951474
835- 7	35678	MEDIUM	0.305D 06	0.10450D-04	0.337D 05	0.90677925
835- 8	67388	MEDIUM	0.390D 06	0.91008D-05	0.337D 05	0.91917692
835- 9	54157	MEDIUM	0.357D 06	0.97180D-05	0.337D 05	0.91351896
835-10	107011	MEDIUM	0.474D 06	0.70874D-05	0.337D 05	0.93758519

LOAD DEFLECTIDN MODEL :

LOAD = DEFLECTION / (BCOEF*DEFLECTION + 1/STIFFNESS)
AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.3371D 05

TEXAS HIGHWAY DEPARTMENT
LOAD RATING OF LIGHT PAVEMENT

JOB: SAMPLE RUN --TO VERIFY THE BACKCALCULATION OF THE SLOPE MULTIPLIER

DISTRICT: 8 COUNTY: BORDEN ROAD: FM 612
ALLOWABLE RUT(INS): 1.00
AXLE NUMBER WHEEL LOAD(LBS)
1 9000.

RECORDED RUT(INS): 0.00 LOAD(LBS): 0. PASSES: 0.00000 00

DATE: 11/20/1975 DYNAFLECT

SECTION NO.	BASE THICKNESS (INS)	DEFLECTIONS (MILS)				
		W1	W2	W3	W4	W5
835- 1	5.00	2.70	1.38	0.54	0.30	0.20
835- 2	5.00	2.16	1.38	0.72	0.42	0.26
835- 3	5.00	1.62	1.02	0.44	0.25	0.15
835- 4	5.00	1.50	0.83	0.36	0.23	0.14
835- 5	5.00	1.44	0.90	0.44	0.28	0.17
835- 6	5.00	1.38	0.85	0.38	0.22	0.15
835- 7	5.00	1.44	0.77	0.33	0.21	0.14
835- 8	5.00	1.26	0.58	0.26	0.16	0.12
835- 9	5.00	1.32	0.70	0.30	0.18	0.12
835-10	5.00	1.14	0.58	0.40	0.16	0.11

SECTION NO.	LAYER PROPERTIES		LOAD DEFORMATION CHARACTERISTICS		NO. OF ALLOWABLE PASSES	AMULT
	BASE/SUBB K1-VALUE	SUBGRADE TYPE	STIFF(LB/IN)	BCOEF		
835- 1	691	MEDIUM	0.116D 06	-0.53087D-04	0.116D 06	1.47802935
835- 2	29654	SOFT	0.159D 06	-0.30773D-04	0.101D 06	1.27723957
835- 3	19797	MEDIUM	0.249D 06	-0.88382D-05	0.801D 05	1.07994676
835- 4	29195	MEDIUM	0.284D 06	0.10639D-04	0.336D 05	0.90501954
835- 5	35678	MEDIUM	0.305D 06	0.10450D-04	0.369D 05	0.90670844
835- 6	43826	MEDIUM	0.329D 06	0.10154D-04	0.411D 05	0.90935360
835- 7	35678	MEDIUM	0.305D 06	0.10450D-04	0.369D 05	0.90670844
835- 8	67388	MEDIUM	0.390D 06	0.91008D-05	0.549D 05	0.91875839
835- 9	54157	MEDIUM	0.357D 06	0.97180D-05	0.468D 05	0.91324430
835-10	107011	MEDIUM	0.474D 06	0.70874D-05	0.869D 05	0.93674456

LOAD DEFLECTION MODEL :

LOAD = DEFLECTION / (BCOEF*DEFLECTION + 1/STIFFNESS)
AVERAGE NUMBER OF PASSES TO CAUSE SPECIFIED RUT : 0.6336D 05

APPENDIX D - CODING FORMS



