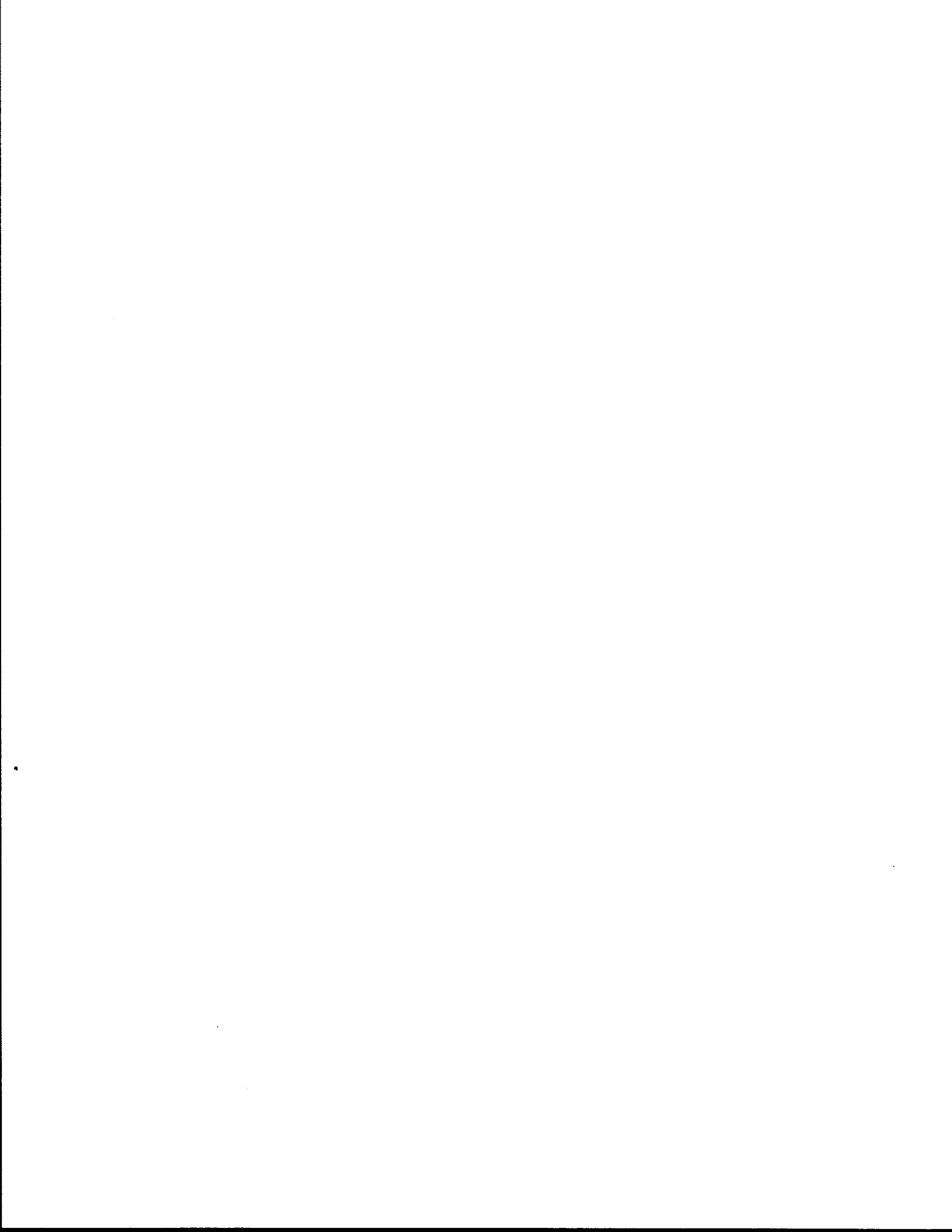


1. Report No.	2. Government Accession No.	3. Recipient's Catalog No.	
4. Title and Subtitle ELASTIC MODULI DETERMINATION FOR SIMPLE TWO-LAYER PAVEMENT STRUCTURES BASED ON SURFACE DEFLECTIONS		5. Report Date August, 1973	
		6. Performing Organization Code	
7. Author(s) William M. Moore		8. Performing Organization Report No. Research Report No. 136-5	
9. Performing Organization Name and Address Texas Transportation Institute Texas A&M University College Station, Texas 77843		10. Work Unit No.	
		11. Contract or Grant No. Research Study 2-8-69-136	
		13. Type of Report and Period Covered Interim - September 1, 1968 August, 1973	
12. Sponsoring Agency Name and Address Texas Highway Department 11th & Brazos Austin, Texas 78701		14. Sponsoring Agency Code	
15. Supplementary Notes Research performed in cooperation with DOT, FHWA. Research Study Title: Design and Evaluation of Flexible Pavements.			
16. Abstract <p style="margin-left: 40px;">This report gives the theoretical background and a description of a new computer program, which is capable of converting routine Dynaflect deflection measurements obtained on the surface of a two-layer highway pavement system, to give the elastic moduli of the pavement and subgrade layers.</p> <p style="margin-left: 40px;">A description of the program and several solutions to example problems are included with the report. The program has been designed to operate at less cost and to eliminate fitting problems encountered in similar existing programs.</p>			
17. Key Words Deflection, Pavement Evaluation, Elastic Modulus, Non-destructive testing.		18. Distribution Statement	
19. Security Classif. (of this report) Unclassified	20. Security Classif. (of this page) Unclassified	21. No. of Pages 74	22. Price



ELASTIC MODULI DETERMINATION
for
SIMPLE TWO-LAYER PAVEMENT STRUCTURES
BASED ON SURFACE DEFLECTIONS

by

William M. Moore

Research Report Number 136-5
Design and Evaluation of Flexible Pavements
Research Study 2-8-69-136

Sponsored by

The Texas Highway Department
In Cooperation with the
U. S. Department of Transportation
Federal Highway Administration

August, 1973

TEXAS TRANSPORTATION INSTITUTE
Texas A&M University
College Station, Texas

Preface

This is the fifth report issued under Research Study 2-8-69-136, "Design and Evaluation of Flexible Pavements," being conducted at the Texas Transportation Institute as part of the cooperative research program with the Texas Highway Department and the Department of Transportation, Federal Highway Administration.

Previous reports from this study are as follows:

- (1) "Seasonal Variations of Pavement Deflections in Texas," by Rudell Poehl and Frank H. Scrivner, Research Report 136-1, Texas Transportation Institute, January, 1971.
- (2) "A Technique for Measuring the Displacement Vector throughout the Body of a Pavement Structure Subjected to Cyclic Loading," by William M. Moore and Gilbert Swift, Research Report 136-2, Texas Transportation Institute, August, 1971.
- (3) "A Graphical Technique for Determining the Elastic Moduli of a Two-Layered Structure from Measured Surface Deflections," by Gilbert Swift, Research Report 136-3, Texas Transportation Institute, November, 1972.
- (4) "An Empirical Equation for Calculating Deflections on the Surface of a Two-Layered Elastic System," by Gilbert Swift, Research Report 136-4, Texas Transportation Institute, November, 1972.

The author wishes to thank the many members of the Institute who contributed to this research. Special appreciation is expressed to Mr. Gerald Turman, who wrote the computer program and description in Appendix A, Mr. Danny Y. Lu, who wrote Subroutine FIBO, and Messrs. F. H. Scrivner, Gilbert Swift and C. H. Michalak who provided valuable advice and assistance in many phases of the research.

The support given by the Texas Highway Department is also appreciated, particularly that of Messrs. James L. Brown and L. J. Buttler who suggested the subject of this report.

The contents of this report reflect the views of the author who is responsible for the facts and the accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of the Federal Highway Administration. This report does not constitute a standard, specification or regulation.

Abstract

This report gives the theoretical background and a description of a new computer program, which is capable of converting routine Dynaflect deflection measurements obtained on the surface of a two-layer highway pavement system, to give the elastic moduli of the pavement and subgrade layers.

A description of the program and several solutions to example problems are included with the report. The program has been designed to operate at less cost and to eliminate fitting problems encountered in similar existing programs.

Key Words: Deflection, Pavement Evaluation, Elastic Modulus, Non-destructive testing.

Summary

A technique is described for determining the elastic moduli for two-layer highway pavement structures from field deflection measurements. This technique is based upon the "best fit" of the entire measured deflection basin; therefore, the moduli are believed to be more representative of in-situ material properties than those obtained by other existing techniques.

Through an illustrative example, it is shown that the five deflection measurements conventionally made with the Dynaflect are not sufficient to determine a unique set of elastic moduli for some typical highway pavement structures. It is also shown that this ambiguity can be eliminated by taking one additional deflection measurement closer to the load wheels.

A computer program designed to compute moduli from routine deflection measurements is given in Appendix A.

Implementation Statement

A new computer program has been written to permit rapid inexpensive calculation of the elastic moduli of two-layer pavement structures from routine field-measured pavement deflections. These in-situ elastic modulus values are significant for pavement evaluation purposes and are expected to be required in future pavement design systems.

It is recommended that an observation be added to routine field deflection measurements in order to eliminate ambiguities found in the evaluation of some typical highway pavements.

Table of Contents

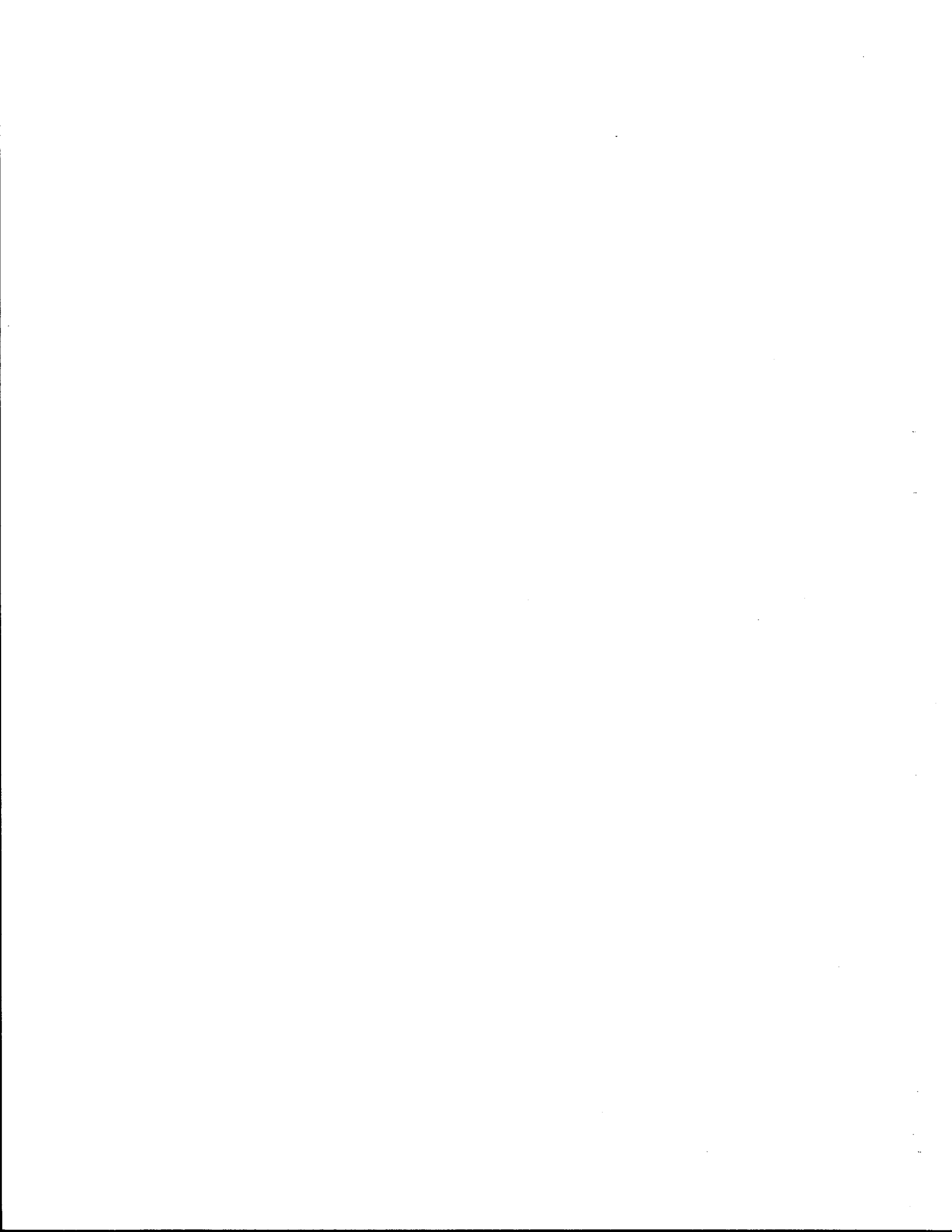
	Page
1. Introduction	1
2. Method of Approach	2
3. Example Solutions	8
4. Implication of Results	19
5. Conclusions and Recommendations	22
6. References	23
Appendix A	24

List of Figures

Figure		Page
1	B_1 versus RMSE for Typical Pavement having Alternate Sets of Elastic Moduli	7
2	Relative Position of Dynaflect Loads and Sensors	20

List of Tables

Table		Page
1	Two Cases for Predicting Deflections	5
2a-2g	Computer Print-outs for Flexible Pavements	9-15
3	Comparison between Elastic Modulus I, Elastic Modulus II and the New Program on Flexible Pavements	16
4	Calculated Moduli for Rigid Pavements	17
5	Comparison between Elastic Modulus II and the New Program on Rigid Pavements	18



1. Introduction

This report presents a technique for determining the elastic modulus for each layer in a simple two-layered pavement structure. The thickness of the top layer is known (or measured) and the thickness of the lower layer is assumed to be infinite. The basic concept is to determine the set of values E_1 and E_2 (elastic modulus of pavement subgrade, respectively) which will best predict a measured surface deflection basin in accordance with layered elastic theory.

The technique is somewhat similar to that developed previously by Scrivner, Michalak and Moore (1,2), the chief differences being that the present technique is more rapid and uses the "best fit" of the entire measured deflection basin rather than two arbitrarily selected points of the basin. It is more rapid because it employs the simple empirical equation developed by Swift (3) instead of a conventional, rigorous, mathematical technique for two elastic layers like that developed by Scrivner, et al. The two techniques are similar in that they both assume a point load on a two-layer elastic pavement structure for which the thickness of the top layer is known. Both determine the elastic moduli for the two layers and assume that the layers have a Poisson's ratio of 0.5.

2. Method of Approach

Deflection predictions are based upon the empirical equation given below which was developed by Swift (3).

$$\hat{w} = \frac{3P}{4\pi E_1} \left[\frac{1}{r} + \left(\frac{E_1}{E_2} - 1 \right) \left(\frac{1}{x} + \frac{a^2}{2x^3} + \frac{3a^4}{2x^5} \right) \right] \quad (1)$$

in which $x = \sqrt{r^2 + a^2}$

$$a = 2h \sqrt[3]{\frac{1}{3} (2 + E_1/E_2)}$$

and $P =$ magnitude of point load

$r =$ horizontal distance from loading point

$h =$ thickness of upper layer

$E_1, E_2 =$ elastic modulus of upper and lower layers respectively

$\hat{w} =$ predicted surface deflection at r

Swift found this equation to closely approximate surface deflections computed using rigorous elastic theory with a Poisson's ratio of 0.5. In this equation deflection is expressed as a function of the following five independent variables: $P, r, h, E_1,$ and E_2 . When deflections of a simple pavement structure of known thickness are measured with the Dynaflect, the first three independent variables are known and the last two are unknown. Thus, if one finds the set of values of E_1 and E_2 that best predicts the measured deflections, w_i , these values can be assumed to represent the elastic moduli for the two layers. The criterion selected for determination of the "best fit" is that the root mean square error, RMSE, be minimized, i.e.,

$$RMSE = \sqrt{\frac{1}{n} \sum_{i=1}^n (w_i - \hat{w}_i)^2} \quad \text{is minimum value.}$$

Equation 1 can be written in the following generalized form.

$$\hat{w} = B_o \cdot f(r, h, B_1) + \epsilon \quad (2)$$

where $B_o = \frac{3P}{4\pi E_1}$

$$B_1 = E_1/E_2 \quad \text{and}$$

ϵ is a prediction error ($w - \hat{w}$)

The RMSE is minimized by use of the following step-by-step procedure.

1. A trial value of B_1 is selected.
2. Five values of the function, f , are computed, one for each of the five standard values of r ($r = 10.0''$, $15.6''$, $26.0''$, $37.4''$ and $49.0''$).
3. B_o is computed using the following equation to obtain the least RMSE for the trial value of B_1 .

$$B_o = \frac{\sum_{i=1}^5 w_i f_i}{\sum_{i=1}^5 f_i^2}$$

4. The RMSE is computed, using the value of B_o computed in step 3.

$$RMSE = \sqrt{\frac{1}{5} \sum_{i=1}^5 (w_i - \hat{w}_i)^2}$$

5. Steps 1 through 4 are repeated using the search process described below until the values of B_o and B_1 are found which result in minimizing the RMSE.
6. The elastic moduli for the individual layers are then computed using the following equations.

$$E_1 = \frac{3000}{4\pi B_o}$$

$$E_2 = \frac{3000}{4\pi B_o B_1}$$

The search process consists of calculating the values of RMSE for each of 21 logarithmically spaced trial values of B_1 , which cover the entire range of reasonable values of the ratio, E_1/E_2 . These values sufficiently define the RMSE versus B_1 curve to determine the one or two ranges for B_1 within which minima of RMSE occur. The location of the minimum within a range is found to an accuracy of 0.5 percent employing a Fibonacci search technique (4).

In fitting two-layer elastic systems to normal Dynaflect measurements, it is often difficult to distinguish between two alternate sets of elastic moduli which result in similar deflection basins. This problem occurs because there are many cases where two entirely different pairs of elastic moduli will provide nearly equal values of deflections in the range of the standard measurements (r values between 10 and 49 inches). In such cases both alternate sets are determined. A typical example of such a difficult distinction is shown in Table 1. In this table, measured deflection values and sets of computed deflection values for two different pairs of elastic moduli are shown. Both computed sets reasonably predict the measured deflections and are almost alike in the normal measuring range ($r = 10$ to 49 inches). Figure 1 contains a log-log plot of RMSE versus the trial values of B_1 obtained as described in steps 1 through 4 above. Two distinct minimums are apparent which represent the cases compared in Table 1.

Based upon the step-by-step procedure described previously, a new computer program was developed to determine the "best fit" set of values for the pavement and subgrade moduli. In cases like the example the two

Table 1: Comparison of two cases for predicting measured deflections

<u>r</u>	<u>w</u>	<u>Case 1</u>		<u>Case 2</u>	
		<u>\hat{w}</u>	<u>$w - \hat{w}$</u>	<u>\hat{w}</u>	<u>$w - \hat{w}$</u>
10.0	1.86	1.83	0.03	1.90	-0.04
15.6	1.35	1.43	-0.08	1.28	0.07
26.0	0.90	0.90	0.00	0.91	-0.01
37.4	0.63	0.60	0.03	0.66	-0.03
49.0	0.50	0.44	0.06	0.51	-0.01
RMSE		0.051		0.038	
E_1		195,600 psi		3,600 psi	
E_2		11,700 psi		9,400 psi	
h		7.5 in		7.5 in	

Note: r = horizontal distance in inches

w = measured Dynaflect deflection in 0.001 inches

\hat{w} = predicted deflection in 0.001 inches (from Equation 1)

alternate "best fit" sets of moduli are determined. The new program is somewhat similar to several other existing programs used to compute pavement layer stiffness parameters from routine Dynaflect data. Three such existing programs, (1) The Texas Highway Department stiffness coefficient program, (2) Elastic Modulus I and (3) Elastic Modulus II ^(1,2,5), evaluate the layer stiffness parameters required to precisely fit two points on the measured deflection basin. As might be expected, the calculated basins which result from those programs often have rather large prediction errors at locations removed from the fitted points. The "best fit" technique employed in the new program, "Two-Layer Elastic Moduli for Five Deflections," eliminates this problem and thus is believed to more nearly represent the true material properties within an existing pavement structure insofar as elasticity theory applies to such structures. In addition, the new program has been found to be about ten times faster than the Elastic Modulus programs. Appendix A contains a description and a computer listing of this program.

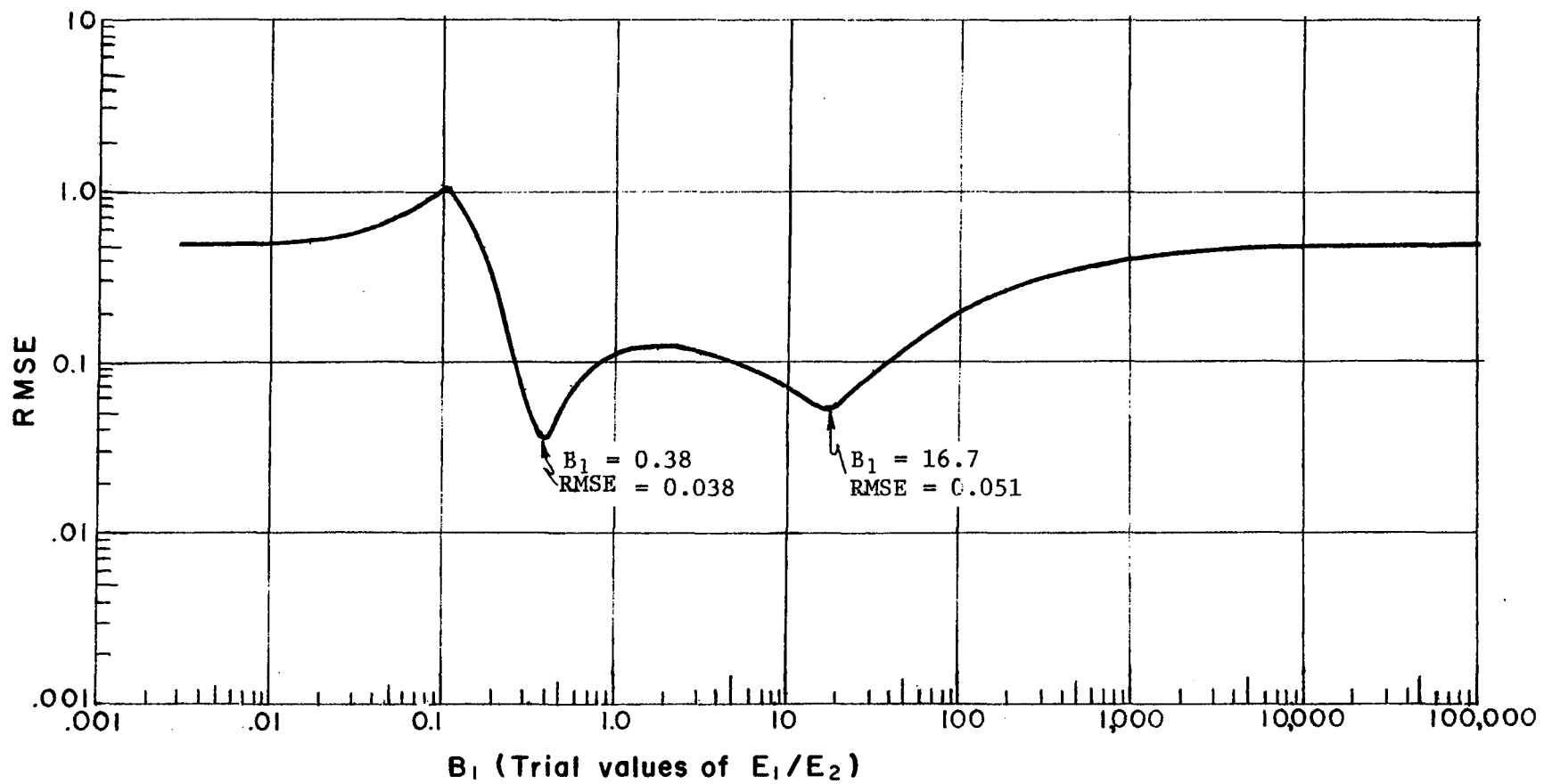


Figure 1: B_1 versus RMSE for typical pavement having alternate possible sets of elastic moduli.

3. Example Solutions

Tables 2a through 2g are computer print-outs based upon the same data used in References 1 and 2. These tables can be compared directly with Tables 6a through 6g in Reference 1 and Tables 5a through 5g in Reference 2. Such a comparison is made in Table 3. Note in this table that six of the seven comparisons appear to have two possible "best fit" solutions based upon the new program.

Table 4 contains solutions based upon the new program for eleven test points taken on rigid pavements at the Houston Intercontinental Airport. These solutions are based upon the same data reported in Reference 2. A direct comparison with the results of Reference 2 is made in Table 5.

TEXAS HIGHWAY DEPARTMENT

DISTRICT 17 - DESIGN SECTION

DYNAFLECT DEFLECTIONS AND CALCULATED ELASTIC MODULI

THIS PROGRAM WAS RUN - 08/28/73

DIST. COUNTY
17 BRAZOS

CONT. SECT. JOB HIGHWAY DATE DYNAFLECT
1560 1 1 FM 1687 5-21-68 1

PAV. THICK. = 12.50 INCHES

SEAL COAT 0.50 RED SANDY GRAVEL 12.00

GREY & BRWN SAND SUB 0.0

STATION	W1	W2	W3	W4	W5	SCI	** ES	** ** EP	** * RMSE *
1 - A	1.170	0.770	0.520	0.310	0.219	0.400	20000.	28000.	0.0252
1 - B	1.140	0.770	0.510	0.310	0.213	0.370	20400.	32400.	0.0221
2 - A	1.290	0.840	0.490	0.300	0.204	0.450	18700.	18400.	0.0334
2 - B	1.200	0.840	0.490	0.300	0.201	0.360	19500.	27700.	0.0311
3 - A	1.140	0.770	0.470	0.300	0.195	0.370	20700.	27100.	0.0214
3 - B	1.110	0.770	0.460	0.300	0.201	0.340	21000.	30800.	0.0176
4 - A	1.470	0.960	0.490	0.320	0.222	0.510	16500.	14100.	0.0546
4 - B	1.380	0.900	0.470	0.310	0.213	0.480	17600.	15200.	0.0458
5 - A	1.290	0.870	0.500	0.340	0.231	0.420	18300.	21900.	0.0202
5 - B	1.260	0.800	0.460	0.310	0.219	0.460	19200.	16900.	0.0198

AVERAGES

W'S, SCI 1.245 0.829 0.486 0.310 0.212 0.416
POINTS 10 10 10 10 10 10

STIFF ON TOP SOLUTIONS
POINTS

19983. 27983.
6 6

SOFT ON TOP SOLUTIONS
POINTS

18000. 16150.
4 4

- W1 DEFLECTION AT GEOPHONE 1
- W2 DEFLECTION AT GEOPHONE 2
- W3 DEFLECTION AT GEOPHONE 3
- W4 DEFLECTION AT GEOPHONE 4
- W5 DEFLECTION AT GEOPHONE 5
- SCI SURFACE CURVATURE INDEX (W1 MINUS W2)
- ES ELASTIC MODULUS OF THE SUBGRADE FROM W1, W2, W3, W4, & W5
- EP ELASTIC MODULUS OF THE PAVEMENT FROM W1, W2, W3, W4, & W5

Table 2a: Computer print-out for Section 3.

TEXAS HIGHWAY DEPARTMENT

DISTRICT 17 - DESIGN SECTION

DYNAFLECT DEFLECTIONS AND CALCULATED ELASTIC MODULI

THIS PROGRAM WAS RUN - 08/28/73

DIST. COUNTY
17 BRAZOS

CONT. SECT. JOB HIGHWAY DATE DYNAFLECT
2824 2 1 FM 2776 5-21-68 1

PAV. THICK. = 8.00 INCHES

SEAL COAT 0.50 ASPHALT STAB. GRAVEL 7.50

GREY SANDY CLAY SUBG 0.0

STATION	W1	W2	W3	W4	W5	SCI	** ES	** ** EP	** * RMSE *
1 - A	1.650	1.200	0.870	0.660	0.500	0.450	12000.	293100.	0.0702
	ALTERNATE SOLUTION						9200.	2800.	0.0409
1 - B	1.560	1.110	0.810	0.610	0.490	0.450	12900.	299900.	0.0761
	ALTERNATE SOLUTION						9900.	3000.	0.0297
2 - A	2.310	1.470	0.930	0.710	0.530	0.840	9200.	5100.	0.0161
2 - B	2.310	1.410	0.900	0.670	0.510	0.900	9700.	6300.	0.0270
3 - A	2.430	1.500	0.930	0.670	0.490	0.930	9700.	8100.	0.0213
3 - B	2.490	1.530	0.930	0.670	0.500	0.960	9600.	9000.	0.0254
4 - A	2.490	1.470	0.900	0.640	0.480	1.020	9900.	10200.	0.0497
4 - B	2.430	1.410	0.840	0.610	0.470	1.020	10300.	11100.	0.0625
5 - A	2.340	1.440	0.870	0.620	0.450	0.900	10400.	10900.	0.0250
5 - B	2.430	1.470	0.930	0.650	0.470	0.960	9800.	8800.	0.0331

AVERAGES

W'S, SCI	2.244	1.401	0.891	0.651	0.489	0.843			
POINTS	10	10	10	10	10	10			
STIFF ON TOP SOLUTIONS							11100.	125040.	
POINTS							5	5	
SOFT ON TOP SOLUTIONS							9586.	6157.	
POINTS							7	7	

W1 DEFLECTION AT GEOPHONE 1
W2 DEFLECTION AT GEOPHONE 2
W3 DEFLECTION AT GEOPHONE 3
W4 DEFLECTION AT GEOPHONE 4
W5 DEFLECTION AT GEOPHONE 5
SCI SURFACE CURVATURE INDEX (W1 MINUS W2)
ES ELASTIC MODULUS OF THE SUBGRADE FROM W1, W2, W3, W4, & W5
EP ELASTIC MODULUS OF THE PAVEMENT FROM W1, W2, W3, W4, & W5

Table 2b: Computer print-out for Section 4.

TEXAS HIGHWAY DEPARTMENT

DISTRICT 17 - DESIGN SECTION

DYNAFLECT DEFLECTIONS AND CALCULATED ELASTIC MODULI

THIS PROGRAM WAS RUN - 08/28/73

DIST. 17 COUNTY BURLESON

CONT. 1399 SECT. 1 JOB 1 HIGHWAY FM 1361 DATE 5-21-68 DYNAFLECT 1

PAV. THICK. = 12.00 INCHES

SEAL COAT 0.50 LIME STAB. SANDSTONE 11.50

TAN SANDY CLAY SUBGR 0.0

STATION	W1	W2	W3	W4	W5	SCI	** ES	** ** EP	** * RMSE *
1 - A	1.500	1.110	0.710	0.470	0.330	0.390	14600.	44600.	0.0096
1 - B	1.560	1.230	0.780	0.480	0.330	0.330	13500.	50800.	0.0315
2 - A	1.650	1.200	0.670	0.400	0.243	0.450	14100.	22800.	0.0641
2 - B	1.440	1.050	0.640	0.380	0.246	0.390	15900.	32000.	0.0387
3 - A	1.500	1.050	0.600	0.370	0.267	0.450	15700.	22600.	0.0345
3 - B	1.440	0.990	0.580	0.370	0.261	0.450	16400.	22900.	0.0242
4 - A	1.500	1.050	0.560	0.340	0.216	0.450	15900.	19900.	0.0607
4 - B	1.380	0.990	0.540	0.330	0.213	0.390	17000.	25000.	0.0501
5 - A	1.920	1.260	0.650	0.400	0.280	0.660	12700.	11500.	0.0787
5 - B	1.800	1.140	0.630	0.420	0.310	0.660	13400.	11300.	0.0390

AVERAGES

W'S, SCI 1.569 1.107 0.636 0.396 0.270 0.462
POINTS 10 10 10 10 10 10

STIFF ON TOP SOLUTIONS 15388. 30075.
POINTS 8 8
SOFT ON TOP SOLUTIONS 13050. 11400.
POINTS 2 2

- W1 DEFLECTION AT GEOPHONE 1
- W2 DEFLECTION AT GEOPHONE 2
- W3 DEFLECTION AT GEOPHONE 3
- W4 DEFLECTION AT GEOPHONE 4
- W5 DEFLECTION AT GEOPHONE 5
- SCI SURFACE CURVATURE INDEX (W1 MINUS W2)
- ES ELASTIC MODULUS OF THE SUBGRADE FROM W1,W2,W3,W4,& W5
- EP ELASTIC MODULUS OF THE PAVEMENT FROM W1,W2,W3,W4,& W5

Table 2c: Computer print-out for Section 5.

TEXAS HIGHWAY DEPARTMENT
DISTRICT 17 - DESIGN SECTION
DYNAFLECT DEFLECTIONS AND CALCULATED ELASTIC MODULI
THIS PROGRAM WAS RUN - 08/28/73

DIST. COUNTY
17 WASHINGTON
CONT. SECT. JOB HIGHWAY DATE DYNAFLECT
186 5 1 SH 36 5-21-68 1

PAV. THICK. = 19.90 INCHES

HOT MIX ASPH. CONC. 3.75 SANDSTONE 16.15
BLACK CLAY SUBGRADE 0.0

STATION	W1	W2	W3	W4	W5	SCI	** ES	** ** FP	** * RMSE *
1 - A	1.680	1.020	0.610	0.420	0.300	0.660	15000.	12800.	0.0112
1 - B	1.830	1.080	0.610	0.420	0.310	0.750	14400.	10600.	0.0073
2 - A	1.740	1.080	0.670	0.470	0.360	0.660	13900.	13600.	0.0135
2 - B	1.950	1.170	0.690	0.490	0.370	0.780	13000.	10900.	0.0104
3 - A	1.680	1.080	0.680	0.500	0.380	0.600	13700.	15800.	0.0184
3 - B	1.710	1.080	0.670	0.480	0.370	0.630	13800.	14500.	0.0143
4 - A	1.680	1.110	0.750	0.570	0.460	0.570	12600.	19500.	0.0416
4 - B	1.560	1.080	0.730	0.550	0.440	0.480	13000.	23100.	0.0300
5 - A	1.500	0.960	0.590	0.440	0.330	0.540	15600.	16900.	0.0151
5 - B	1.590	0.990	0.600	0.430	0.330	0.600	15200.	14700.	0.0107

AVERAGES

W'S, SCI 1.692 1.065 0.660 0.477 0.365 0.627
POINTS 10 10 10 10 10 10
STIFF ON TOP SOLUTIONS
POINTS 13740. 17960.
SOFT ON TOP SOLUTIONS
POINTS 14300. 12520.
5 5

- W1 DEFLECTION AT GEOPHONE 1
- W2 DEFLECTION AT GEOPHONE 2
- W3 DEFLECTION AT GEOPHONE 3
- W4 DEFLECTION AT GEOPHONE 4
- W5 DEFLECTION AT GEOPHONE 5
- SCI SURFACE CURVATURE INDEX (W1 MINUS W2)
- ES ELASTIC MODULUS OF THE SUBGRADE FROM W1, W2, W3, W4, & W5
- EP ELASTIC MODULUS OF THE PAVEMENT FROM W1, W2, W3, W4, & W5

Table 2d: Computer print-out for Section 12.

TEXAS HIGHWAY DEPARTMENT
DISTRICT 17 - DESIGN SECTION
DYNAFLECT DEFLECTIONS AND CALCULATED ELASTIC MODULI

THIS PROGRAM WAS RUN - 08/28/73

DIST. COUNTY
17 ROBERTSON

CONT. SECT. JOB HIGHWAY DATE DYNAFLECT
49 8 1 US 190 5-21-68 1

PAV. THICK. = 15.20 INCHES

HOT MIX ASPH. CONC. 1.25 CEM. STAB. LIMESTONE 13.95

REC SANDY CLAY SUBGR 0.0

STATION	W1	W2	W3	W4	W5	SCI	** ES	** ** EP	** * RMSE *
1 - A	0.680	0.590	0.490	0.390	0.310	0.090	18600.	347700.	0.0096
1 - B	0.680	0.600	0.490	0.390	0.310	0.080	18600.	341600.	0.0070
2 - A	0.720	0.630	0.510	0.390	0.310	0.090	18500.	280900.	0.0060
2 - B	0.700	0.620	0.490	0.390	0.310	0.080	18700.	302300.	0.0081
3 - A	0.750	0.650	0.520	0.390	0.300	0.100	18700.	234300.	0.0044
3 - B	0.760	0.650	0.510	0.390	0.300	0.110	18900.	222000.	0.0072
4 - A	0.600	0.540	0.450	0.350	0.280	0.060	20300.	411100.	0.0030
4 - B	0.580	0.520	0.430	0.330	0.264	0.060	21600.	397000.	0.0033
5 - A	0.620	0.550	0.450	0.350	0.273	0.070	20800.	351900.	0.0030
5 - B	0.650	0.570	0.470	0.360	0.280	0.080	20100.	325400.	0.0047

AVERAGES

W'S, SCI 0.674 0.592 0.481 0.373 0.294 0.082

POINTS 10 10 10 10 10 10

STIFF ON TOP SOLUTIONS

19480. 321420.

POINTS

10 10

NO SOFT ON TOP SOLUTIONS

- W1 DEFLECTION AT GEOPHONE 1
- W2 DEFLECTION AT GEOPHONE 2
- W3 DEFLECTION AT GEOPHONE 3
- W4 DEFLECTION AT GEOPHONE 4
- W5 DEFLECTION AT GEOPHONE 5
- SCI SURFACE CURVATURE INDEX (W1 MINUS W2)
- ES ELASTIC MODULUS OF THE SUBGRADE FROM W1, W2, W3, W4, & W5
- EP ELASTIC MODULUS OF THE PAVEMENT FROM W1, W2, W3, W4, & W5

Table 2e: Computer print-out for Section 15.

TEXAS HIGHWAY DEPARTMENT

DISTRICT 17 - DESIGN SECTION

DYNAFLECT DEFLECTIONS AND CALCULATED ELASTIC MODULI

THIS PROGRAM WAS RUN - 08/28/73

DIST. COUNTY
17 BRAZOS

CONT. SECT. JOB HIGHWAY DATE DYNAFLECT
1560 1 1 FM 1687 5-21-68 1

PAV. THICK. = 7.50 INCHES

ASPHALT SURFACING 1.00 ASPH EMUL STAB GRAVL 6.50
BRCWN CLAY SUBGRADE 0.0

STATION	W1	W2	W3	W4	W5	SCI	** ES	** ** EP	* RMSE *
1 - A	2.160	1.500	0.960	0.660	0.520	0.660	11000.	104400.	0.0497
	ALTERNATE SOLUTION						9100.	4400.	0.0328
1 - B	2.130	1.530	0.960	0.650	0.510	0.600	10900.	117900.	0.0337
2 - A	1.920	1.410	0.930	0.640	0.490	0.510	11400.	181400.	0.0394
	ALTERNATE SOLUTION						9300.	3700.	0.0490
2 - B	1.860	1.350	0.900	0.630	0.500	0.510	11700.	195600.	0.0507
	ALTERNATE SOLUTION						9400.	3600.	0.0379
3 - A	2.040	1.470	0.930	0.630	0.490	0.570	11300.	128200.	0.0330
	ALTERNATE SOLUTION						9400.	4500.	0.0503
3 - B	2.070	1.500	0.960	0.650	0.500	0.570	11000.	137300.	0.0342
	ALTERNATE SOLUTION						9100.	4100.	0.0528
4 - A	2.220	1.620	1.020	0.670	0.490	0.600	10400.	116000.	0.0202
4 - B	2.220	1.590	1.020	0.650	0.490	0.630	10500.	108500.	0.0292
5 - A	1.980	1.380	0.900	0.610	0.470	0.600	11800.	122200.	0.0454
	ALTERNATE SOLUTION						9800.	4600.	0.0329
5 - B	1.980	1.440	0.930	0.610	0.460	0.540	11500.	141400.	0.0274

AVERAGES

W'S, SCI	2.058	1.479	0.951	0.640	0.492	0.579			
POINTS	10	10	10	10	10	10			
STIFF ON TOP SOLUTIONS							11150.	135290.	
POINTS							10	10	
SOFT ON TOP SOLUTIONS							9350.	4150.	
POINTS							6	6	

- W1 DEFLECTION AT GEOPHONE 1
- W2 DEFLECTION AT GEOPHONE 2
- W3 DEFLECTION AT GEOPHONE 3
- W4 DEFLECTION AT GEOPHONE 4
- W5 DEFLECTION AT GEOPHONE 5
- SCI SURFACE CURVATURE INDEX (W1 MINUS W2)
- ES ELASTIC MODULUS OF THE SUBGRADE FROM W1, W2, W3, W4, & W5
- EP ELASTIC MODULUS OF THE PAVEMENT FROM W1, W2, W3, W4, & W5
- ***** IN CASES WITH ALTERNATES, RMSES ARE NOT SIGNIFICANTLY DIFFERENT

Table 2f: Computer print-out for Section 16.

TEXAS HIGHWAY DEPARTMENT
DISTRICT 17 - DESIGN SECTION
DYNAFLECT DEFLECTIONS AND CALCULATED ELASTIC MODULI

THIS PROGRAM WAS RUN - 08/28/73

DIST. COUNTY
17 BRAZOS

CONT. SECT. JOB HIGHWAY DATE DYNAFLECT
540 3 1 FM 974 5-21-68 1

PAV. THICK. = 8.30 INCHES

SEAL COAT 0.50 IRON ORE GRAVEL 7.80
GREY SANDY CLAY SUBG 0.0

STATION	W1	W2	W3	W4	W5	SCI	** ES	** ** EP	** * RMSE *
1 - A	2.400	1.530	0.960	0.680	0.500	0.870	9400.	6700.	0.0093
1 - B	2.250	1.440	0.900	0.630	0.480	0.810	10000.	7200.	0.0063
2 - A	1.770	1.170	0.820	0.600	0.480	0.600	13100.	114900.	0.0832
	ALTERNATE SOLUTION						10100.	3800.	0.0199
2 - B	1.800	1.200	0.820	0.620	0.490	0.600	12800.	112800.	0.0831
	ALTERNATE SOLUTION						9900.	3700.	0.0242
3 - A	1.650	1.170	0.840	0.640	0.510	0.480	12300.	237500.	0.0796
	ALTERNATE SOLUTION						9500.	2900.	0.0463
3 - B	1.590	1.170	0.840	0.610	0.510	0.420	12400.	270900.	0.0657
	ALTERNATE SOLUTION						9600.	2900.	0.0618
4 - A	2.250	1.470	0.990	0.750	0.600	0.780	10600.	67800.	0.1032
	ALTERNATE SOLUTION						8300.	3300.	0.0222
4 - B	2.340	1.590	1.050	0.790	0.630	0.750	9900.	82800.	0.0939
	ALTERNATE SOLUTION						7800.	3100.	0.0435
5 - A	2.220	1.470	0.990	0.710	0.550	0.750	8600.	3700.	0.0269
5 - B	2.100	1.410	0.960	0.680	0.530	0.690	11200.	83700.	0.0763
	ALTERNATE SOLUTION						8900.	3600.	0.0337

AVERAGES

W'S, SCI 2.037 1.362 0.917 0.671 0.528 0.675
POINTS 10 10 10 10 10 10

STIFF ON TOP SOLUTIONS 11757. 138629.
POINTS 7 7
SOFT ON TOP SOLUTIONS 9210. 4090.
POINTS 10 10

- W1 DEFLECTION AT GEOPHONE 1
- W2 DEFLECTION AT GEOPHONE 2
- W3 DEFLECTION AT GEOPHONE 3
- W4 DEFLECTION AT GEOPHONE 4
- W5 DEFLECTION AT GEOPHONE 5
- SCI SURFACE CURVATURE INDEX (W1 MINUS W2)
- ES ELASTIC MODULUS OF THE SUBGRADE FROM W1, W2, W3, W4, & W5
- EP ELASTIC MODULUS OF THE PAVEMENT FROM W1, W2, W3, W4, & W5
- ***** IN CASES WITH ALTERNATES, RMSES OF THE SOFT AND STIFF ON TOP SOLUTIONS ARE DIFFERENT AT A 10 PERCENT LEVEL OF SIGNIFICANCE

Table 2g: Computer print-out for Section 17.

Table 3: Comparison between Elastic Modulus I, Elastic Modulus II and the new program on flexible pavements.

Test Sect	Surfacing	Base	Pvmt Thick H	Computed Moduli Values					
				Elastic Mod I*		Elastic Mod II*		New Program	
				E ₁	E ₂	E ₁	E ₂	E ₁	E ₂
3	0.5" ST	12.0" Red sandy grav	15.2	24720	18970	23660	18980	27983	19983
4	0.5" ST	7.5" Asph stab Ls	8.0	78900	14900	110475	11800	16150	18000**
5	0.5" ST	11.5" Lime stab Ss	12.0	32340	14480	23760	14840	6157	9586**
12	3.7" HMAC	16.2" Crushed Ss	19.9	13900	14420	14920	14010	30075	15388
15	1.2" HMAC	14.0" Cement stab Ls	15.2	283180	19990	314100	19120	11400	13050**
16	1.0" HMAC	6.5" Asph stab grav	7.5	73910	11740	109330	11110	12520	14300**
17	0.5" ST	7.8" Iron ore grav	8.3	36600	12700	81910	11400	321420	19480
								No Alternate	
								135290	11150
								4150	9350**
								138629	11757
								4090	9210**

*See Table 6 Reference 2.

**Alternate Solutions.

Table 4: Dynaflect deflections and calculated elastic moduli for test points on rigid pavements at the Houston Intercontinental Airport.

Test Point	Pvmt Thick H	Deflections					Calculated Moduli Values		
		W1	W2	W3	W4	W5	E2	E1	RMSE
6	12.0	0.400	0.400	0.370	0.340	0.310	15,600	4,928,400	0.0035
10	12.0	0.500	0.470	0.440	0.380	0.330	16,300	2,389,400	0.0046
13	12.0	0.520	0.510	0.470	0.400	0.350	15,300	2,251,200	0.0053
25	12.0	0.400	0.390	0.360	0.330	0.290	17,400	4,052,300	0.0017
28	12.0	0.430	0.410	0.390	0.360	0.330	14,500	4,895,000	0.0041
32	12.0	0.410	0.390	0.360	0.320	0.280	18,900	3,247,900	0.0026
34	12.0	0.400	0.390	0.370	0.340	0.310	15,300	5,253,100	0.0011
49	12.0	0.410	0.400	0.370	0.350	0.350	12,200	8,191,900	0.0104
56	14.0	0.330	0.330	0.310	0.290	0.270	16,500	5,046,300	0.0025
63	12.0	0.390	0.380	0.350	0.320	0.290	17,300	4,451,100	0.0032
69	12.0	0.237	0.234	0.216	0.207	0.198	21,900	12,731,400	0.0039
AVERAGES							16,470	5,221,600	0.0039

Note: Deflection data from Figures 10a through 10k Reference 2.

Table 5: Comparison between Elastic Modulus II and the new program on rigid pavements.

Point	Pvmt Thick H	General Location	Subbase	Subg	Calculated Moduli Values			
					Elastic Mod II		New Program	
					E2	E1	E2	E1
6	12.0	Runway 14-32	6" Sand Shell	4' Compact	13,000	7,494,800	15,600	4,928,400
10	12.0	Runway 14-32	6" Sand Shell	4' Compact	14,300	3,066,500	16,300	2,389,400
13	12.0	Runway 14-32	6" Sand Shell	4' Compact	11,700	4,137,400	15,300	2,251,200
24	12.0	Taxiway A	6" Sand Shell	4' Compact	15,600	5,085,500	17,400	4,052,300
28	12.0	Taxiway A	6" Sand Shell	4' Compact	14,000	5,154,900	14,500	4,895,000
32	12.0	Taxiway A	6" Sand Shell	4' Compact	17,600	3,674,300	18,900	3,247,900
34	12.0	Taxiway A	6" Sand Shell	4' Compact	13,000	7,494,800	15,300	5,253,100
49	12.0	Taxiway B	6" Sand Shell	4' Compact	15,100	5,099,500	12,200	8,191,900
56	14.0	Taxiway K	9" Soil Cement	4' Compact	13,600	7,831,000	16,500	5,046,300
63	12.0	Taxiway K	9" Soil Cement	4' Compact	16,400	4,952,600	17,300	4,451,100
69	12.0	North Apron	12" Soil Cement	6' Compact	23,000	10,975,400	21,900	12,731,400

4. Implication of Results

As pointed out previously, there are many instances where two entirely different sets of elastic moduli provide nearly equal values of deflections at the locations of the normal set of Dynaflect measurements (r values between 10 and 49 inches). Thus, two alternate sets of elastic moduli may appear to be equivalent solutions in a particular pavement evaluation problem. This phenomena does not imply that point load, two-layer, elastic deflection basins are not unique. In fact, Swift's "Two-Layer Elastic Deflection Chart" (6) clearly demonstrates that each possible two-layer elastic case has its own unique characteristic deflection basin. However, the phenomena does indicate that two alternate cases can become confused when the set of measurement points is not extensive enough.

The distinction between alternate cases could be greatly improved by extending the range of observations to include measurements at values of r that are less than 10 inches and/or greater than 49 inches. For example, at a radius of 5 inches from a point load, the calculated deflections for the two different cases, illustrated in Table 1, would be 2.22 and 6.01, respectively. Thus, a measured deflection at a 5 inch radius from a point load would clearly distinguish between the two possible cases.

With the current configuration of the Dynaflect, 10 inches is the smallest radius that can be used on the symmetry axis (See Figure 2). However, it is possible to obtain measurements closer than 10 inches by employing the principle of superposition. For example, a deflection measured at location number 6, Figure 2, would be the sum of the deflection due to one 500-pound load at a 5 inch radius and another deflection due to a 500 pound load at

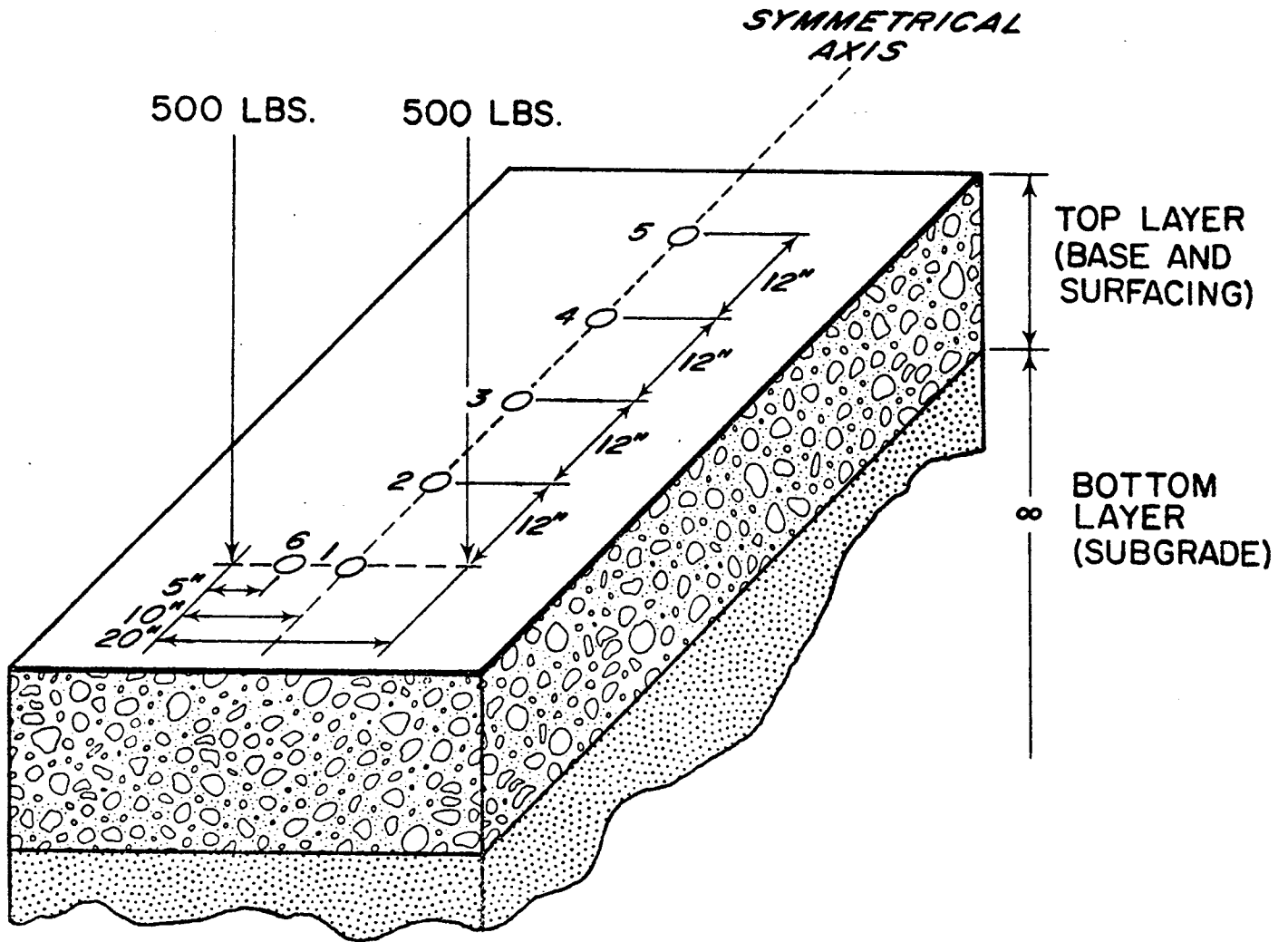


Figure 2: Relative position of Dynaflect loads and sensors. Vertical arrows represent load wheels. Points numbered 1 through 5 indicate location of sensors for standard test. Point 6 indicates the location of a desired additional measurement.

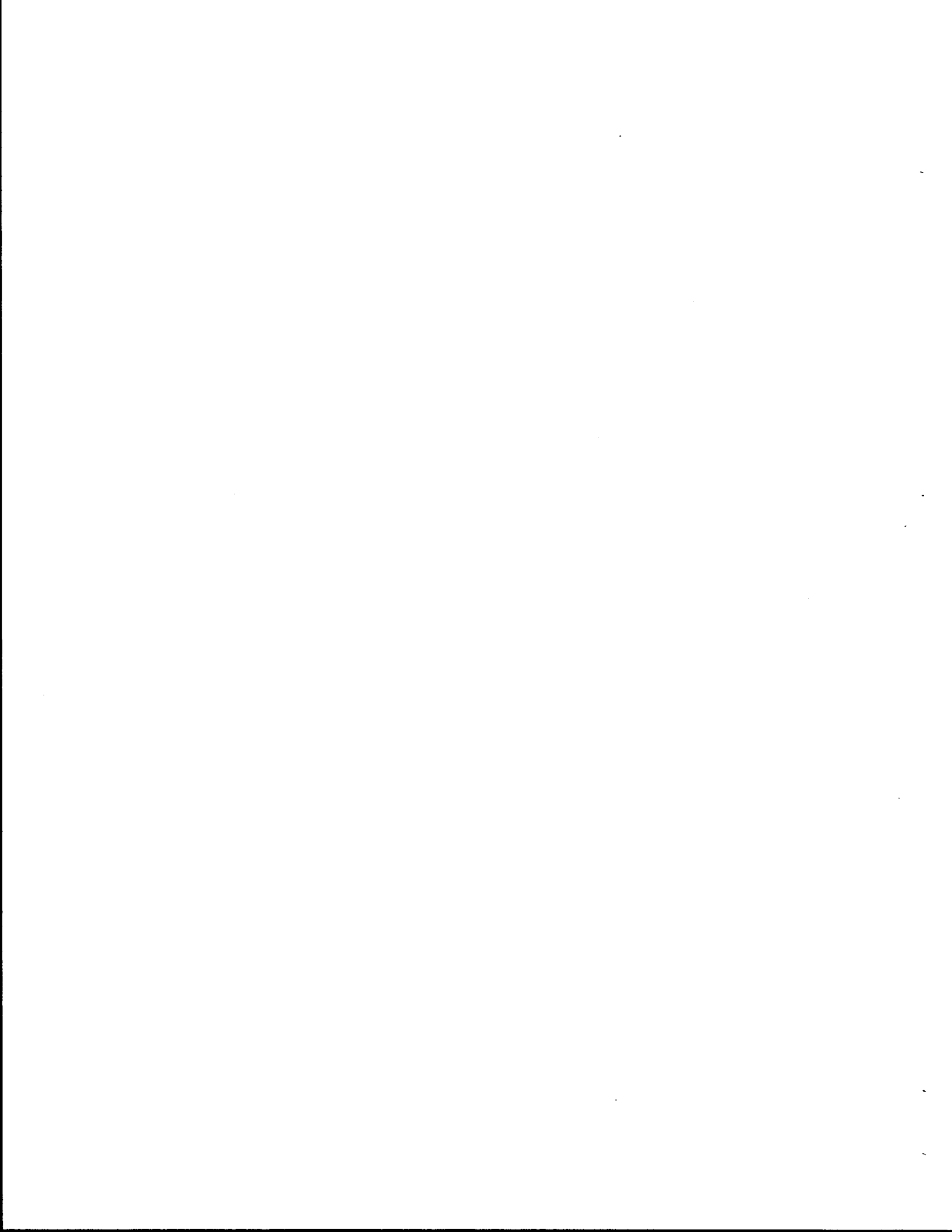
a 15-inch radius. The calculated value of deflection at this point for each of the two cases which were compared above, would be 1.85 and 3.66, respectively. Although this distinction is not as great as the previous comparison for r equal to 5 inches, it is significant enough to clearly distinguish between the two possible cases.

5. Conclusions & Recommendations

1. Because the presented technique for determining elastic moduli for simple two-layer pavement structures fits the entire measured deflection basin, it is believed to be more representative of the true material properties, insofar as elasticity theory applies to such structures, than any other technique known to the author.
2. The five Dynaflect deflection measurements normally made in field testing are not sufficient to determine a unique set of elastic moduli for some two-layer highway pavements.
3. The apparent two alternate solutions for many existing flexible pavement structures could be resolved by making an additional deflection measurement closer to the loading point. It is recommended that the mechanics of accomplishing such a measurement be given immediate consideration for use in future deflection based pavement evaluations.

5. References

1. Scrivner, F. H.; Michalak, C. H.; and Moore, W. M. "Calculation of the Elastic Moduli of a Two Layer Pavement System from Measured Surface Deflections," Research Report No. 123-6, Texas Transportation Institute, Texas A&M University, College Station, Texas, March 1971.
2. Scrivner, F. H.; Michalak, C. H.; and Moore, W. M. "Calculation of the Elastic Moduli of a Two Layer Pavement System from Measured Surface Deflections, Part II," Research Report No. 123-6A, Texas Transportation Institute, Texas A&M University, College Station, Texas, December, 1971.
3. Swift, Gilbert. "An Empirical Equation for Calculating Deflections on the Surface of a Two-Layered Elastic System," Research Report No. 136-4, Texas Transportation Institute, November, 1972.
4. Pierre, Donald A., Optimization Theory with Application's, John Wiley and Sons, Inc., New York, March 1969, pp. 280-283.
5. "Texas Highway Department Pavement Design System, Part I, Flexible Pavement Designer's Manual," Highway Design Division, Texas Highway Department, Austin, Texas, 1970.
6. Swift, Gilbert. "A Graphical Technique for Determining the Elastic Moduli of a Two-Layered Structure from Measured Surface Deflections," Research Report No. 136-3, Texas Transportation Institute, November, 1972.



APPENDIX A

This Appendix contains a description of a computer program, "Two-Layer Elastic Moduli for Five Deflections," which determines the pavement and subgrade moduli for simple two-layer pavement structures based on surface deflections.

TABLE OF CONTENTS

<u>SUBJECT</u>	<u>PAGE</u>
Description of Main Program	A-1
Main Program Variables	A-4
Main Program Flow Chart	A-8
Description of EMPI Subroutine	A-19
Subroutine EMPI Variables	A-21
Subroutine EMPI Flow Chart	A-23
Subroutine VARI	A-25
Subroutine SIGNIF	A-26
Subroutine FIBO	A-27
Program Listing	A-28

DESCRIPTION OF THE MAIN PROGRAM

The data input format for the main computer program is the same as that used by several previously written computer programs that compute pavement strength properties from Dynaflect data, namely the Texas Highway Department stiffness coefficient program, ELASTIC MODULUS I, and ELASTIC MODULUS II. Each input data card is read into a storage area and the subroutine CORE is used to select the read statement and data format to read each data card. Subroutine CORE allows a FORTRAN program to read under format control from a storage area which contains alphabetic character codes of a card image. Each data card has a code punched in the first three columns that designate the card type.

100 - Card that indicates the beginning of data cards for each job and contains control information about the job, location, date and total pavement thickness.

200 - Card contains word descriptions and thicknesses of the first three layers of the pavement.

300 - Card contains word descriptions and thicknesses of layers 4, 5 and 6 (if present).

400 or blank - Card contains station number and geophone deflection headings and multipliers for each observation. Two digit numbers in columns 75 and 76 of this card denotes end of data.

The deflections at each radial distance are calculated from the geophone deflection readings and multipliers on each 400 or blank card. The Surface Curvature Index (SCI) is also calculated, $SCI=W1-W2$. If any W (deflection) is equal to zero, or if any W is greater than its preceding W, the cases are flagged to denote data errors and are not

used for further calculations. If the W's are valid observations they are converted to inches and passed to subroutine EMPI along with the total pavement thickness for the elastic modulus and RMSE calculations.

EMPI returns to the main program two alternate solutions for pavement and subgrade elastic moduli with their corresponding RMSE's. In cases where only one solution exists, the variables for its alternate solution contain the flag number 99999. The pavement and subgrade moduli are then rounded to the nearest 100 psi. The counter N (the number of valid sets of observations) is incremented and the program reads the next data card to continue the process until all stations in a section are read. When all the data cards for a section have been read, the program prints output headings and initializes the variables used in calculating section averages.

A loop is set up to print the station numbers, deflections, SCI's, subgrade moduli, pavement moduli and RMSE's of all valid data observations. Messages for the following situations are printed for which a data observation is not used in the calculations:

1. Data observation computes a negative SCI in which case the message 'NEGATIVE SCI OTHER CALCULATIONS OMITTED' is printed.
2. Data observation where any W is equal to zero. 'ERROR IN DATA' is printed.
3. Data observation where both alternate pavement and subgrade solutions are "soft on top" (pavement modulus is less than subgrade modulus) or where both solutions are "stiff on top" (pavement modulus is greater than subgrade modulus). The message printed for this occurrence is 'NO SOLUTION'.

For all data observations other than the three mentioned above, elastic moduli for the soft and stiff on top solutions are summed separately. For all observations which have two alternate solutions for pavement and subgrade elastic moduli, the RMSE's of the stiff and soft for each solution are stored in separate arrays for a variance analysis calculation to denote significant differences.

After all data and any error messages for a section are printed, the average deflections, SCI's, subgrade moduli, pavement moduli and RMSE are calculated. If more than two observations in a section have alternate solutions, the program calls Subroutine VARI to calculate an analysis of variance between the RMSE's of the stiff on top solutions and the soft on top solutions to determine if these values are significantly different at the 10% level.

The averages are then printed along with the number of points used in calculating each average. These averages are divided into two groups: the average and points of the stiff on top solutions and the average and points of the soft on top solutions.

Definitions of the heading abbreviations are given next in footnote form. An additional footnote occurs when the variance analysis has been run, denoting whether or not the RMSE's of the alternate pavement and subgrade modulus are significantly different.

The program then returns to its beginning to read data for another section or terminates execution normally when all data have been read.

MAIN PROGRAM VARIABLES

A - Dummy array used with subroutine core to select the correct input format for each card read.

AAP2 - Sum of pavement moduli

AAS2 - Sum of subgrade moduli

AAP2V - Average pavement modulus

AAS2V - Average subgrade modulus

ALRMS - Stiff on top alternate solution RMSE array

AP2 - Elastic modulus of the pavement rounded to nearest 100 psi

AS2 - Elastic modulus of the subgrade rounded to nearest 100 psi

ASCI - Sum of (W1 - W2), W1 - W2 = surface curvature index

ASCIV - Average surface curvature index

AW1 - Sum of geophone 1 deflections

AW2 - Sum of geophone 2 deflections

AW3 - Sum of geophone 3 deflections

AW4 - Sum of geophone 4 deflections

AW5 - Sum of geophone 5 deflections

AW1V - Average geophone 1 deflections

AW2V - Average geophone 2 deflections

AW3V - Average geophone 3 deflections

AW4V - Average geophone 4 deflections

AW5V - Average geophone 5 deflections

BLRMS - Soft on top alternate solution RMSE array

CNT - Number of soft on top solutions

CORE - Subroutine to re-read a card under format control

CO1, CO2, CO3, CO4 - County name

D1 - Geophone 1 reading
D2 - Geophone 1 multiplier
D3 - Geophone 2 reading
D4 - Geophone 2 multiplier
D5 - Geophone 3 reading
D6 - Geophone 3 multiplier
D7 - Geophone 4 reading
D8 - Geophone 4 multiplier
D9 - Geophone 5 reading
D10 - Geophone 5 multiplier
DAP - Pavement elastic modulus (unrounded) as calculated in subroutine EMPI
DAS - Subgrade elastic modulus (unrounded) as calculated in subroutine EMPI
DATE - An IBM subroutine that returns the current month, day, and year
DP - Total pavement thickness
E11 - Alternate pavement elastic modulus rounded to nearest 100 psi
E21 - Alternate subgrade elastic modulus rounded to nearest 100 psi
EMPI - Subroutine to calculate pavement and subgrade moduli, RMSE, and alternate
if it exists.
HWY1, HWY2 - Highway name and number
I - Pointer for data read into storage
ICK - Switch to indicate last data card
ICONT - Contract number for the highway
IDAY - Day the deflections were taken
IDIST - District number
IDYNA - Dynaflect number
IJOB - THD job number
ISECT - THD section number for the highway
ISW - Switch to indicate whether the two RMSE arrays are significant and to
control the footnotes to be printed.

IXDATE - Return arguments from subroutine date (month, day, year)

IYEAR - Year the deflections were taken

JNT, LNT - Number of RMSE's in the two arrays to be tested for
significance

KNT - Number of stiff on top solutions

LA1 - Description of materials in layer 1

LA2 - Description of materials in layer 2

LA3 - Description of materials in layer 3

LA4 - Description of materials in layer 4

LA5 - Description of materials in layer 5

LA6 - Description of materials in layer 6

LO - Number of both data errors and no solutions

M - Month the deflections were taken

MNT - Number of solutions printed

N - Counter for number of error free data cards read

NCARD - Denotes card type

- 100 - Project identification card
- 200 - Existing pavement description card (layers 1, 2, & 3)
- 300 - Existing pavement description card (layers 4, 5, & 6)
- 400 - Data card (geophone readings and multipliers)

RATIO - Ratio of AP2/AS2

RATIO1 - Ratio of E11/E21

ROUND - Statement function to round a given value of E1 or E2 to the
nearest 100 psi

SCI - Surface curvature index, W1-W2 in mils

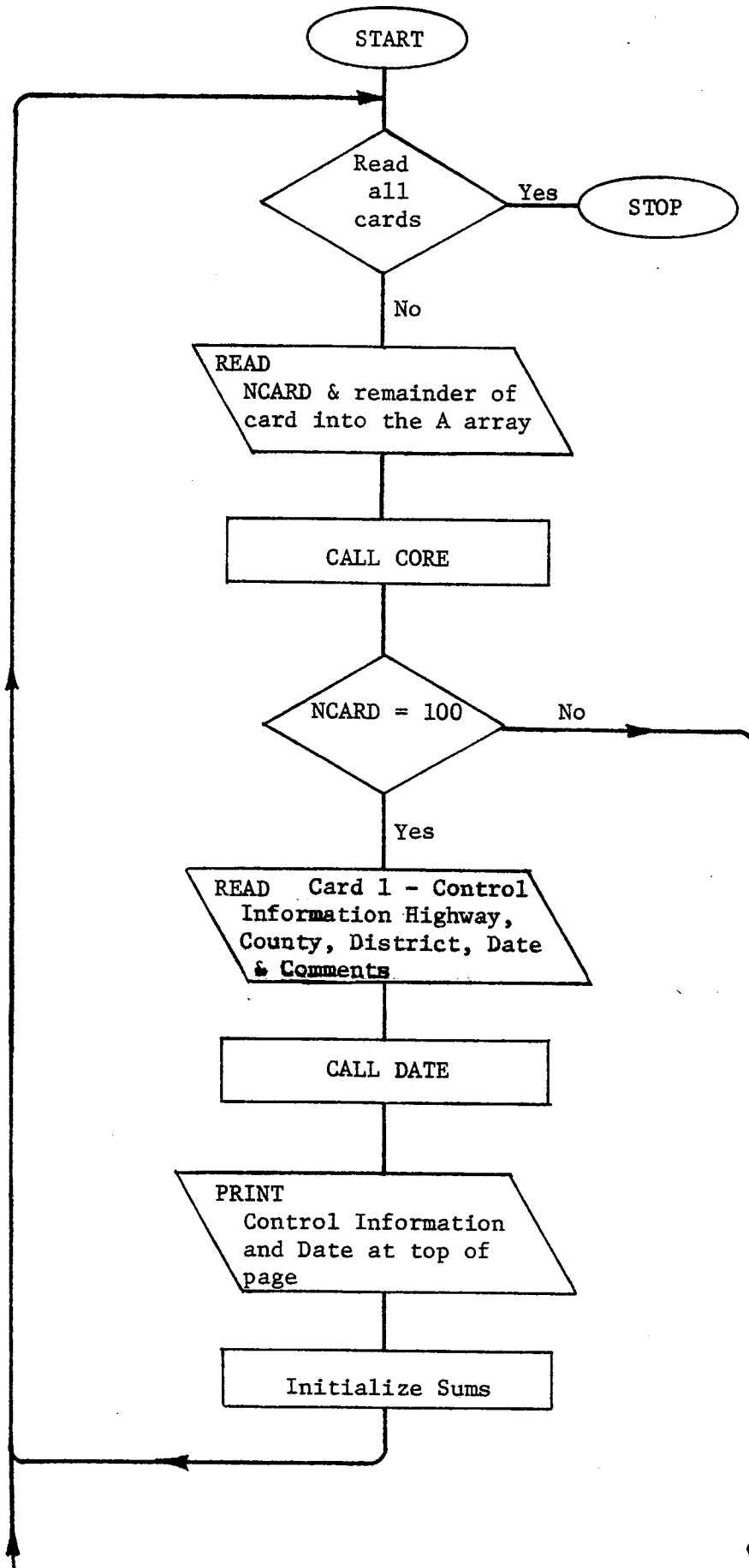
STA - Station number

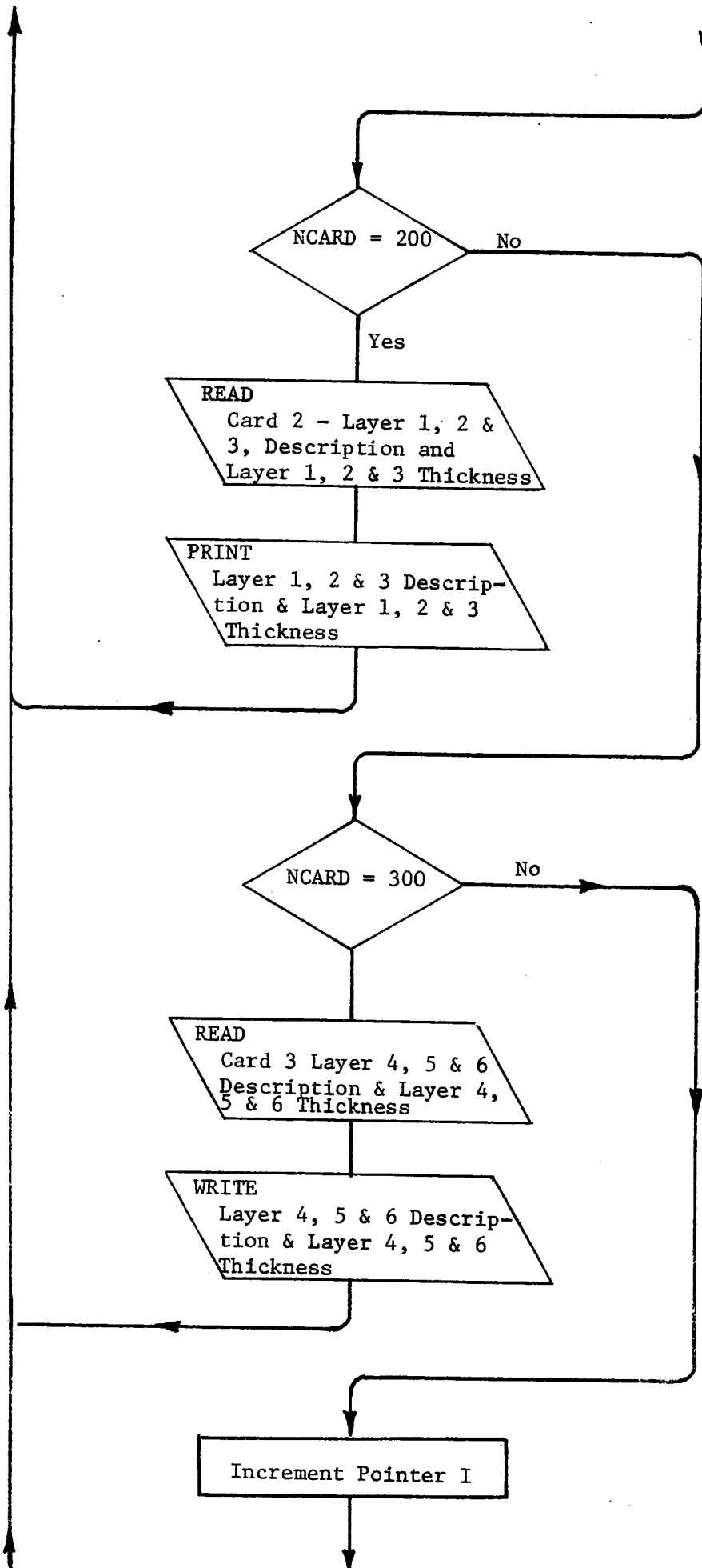
T1 - Layer 1 thickness

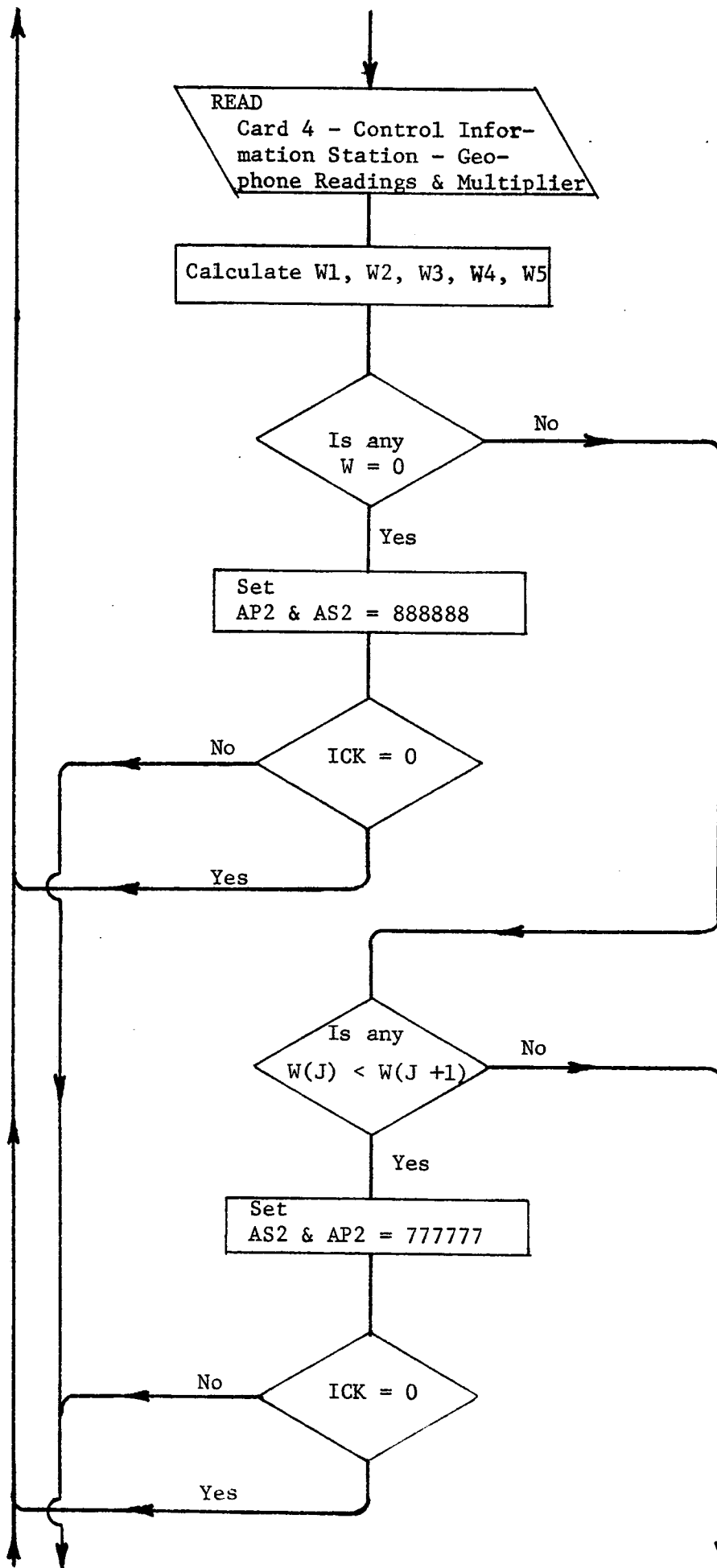
T2 - Layer 2 thickness

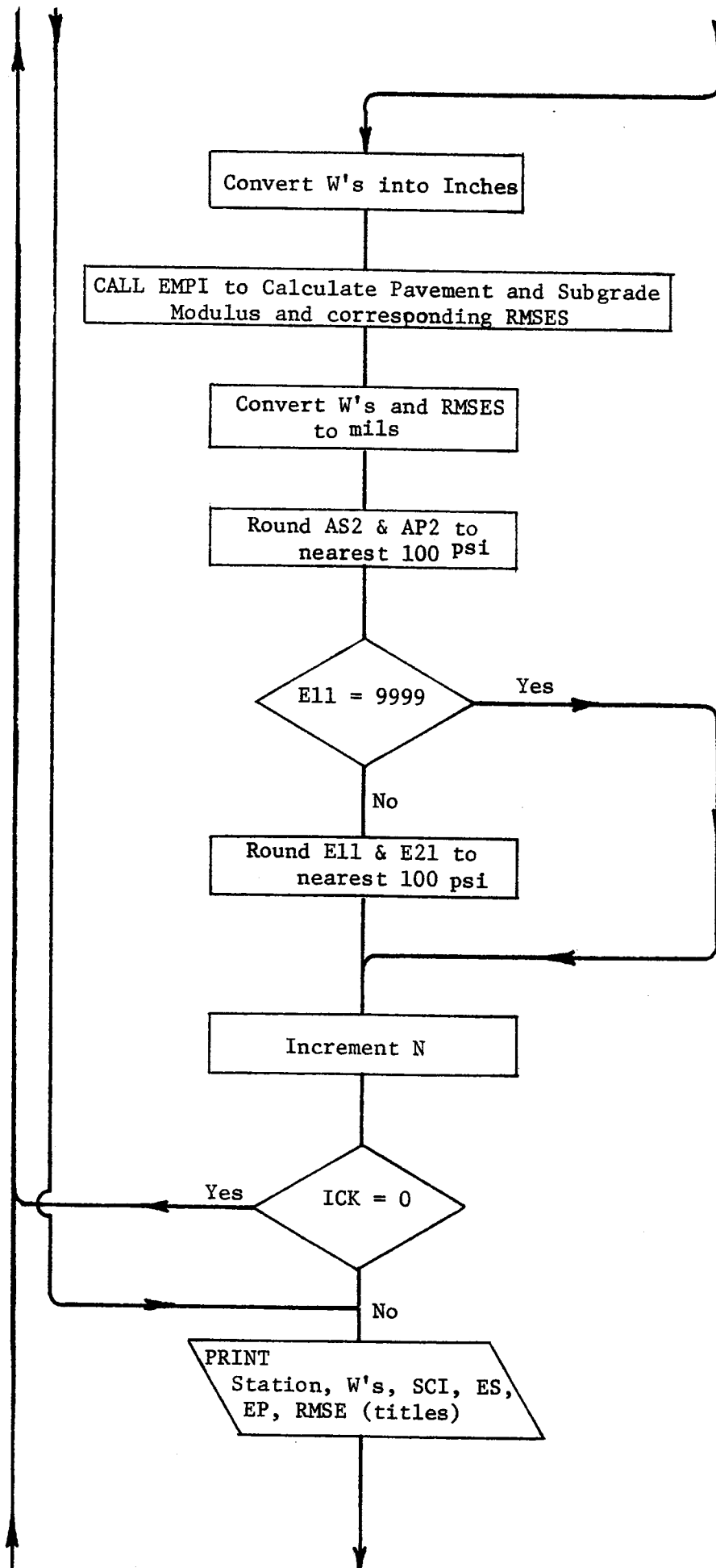
T3 - Layer 3 thickness
T4 - Layer 4 thickness
T5 - Layer 5 thickness
T6 - Layer 6 thickness
W1 - Deflection at geophone number 1
W2 - Deflection at geophone number 2
W3 - Deflection at geophone number 3
W4 - Deflection at geophone number 4
W5 - Deflection at geophone number 5
XLANE - Traffic lane and direction

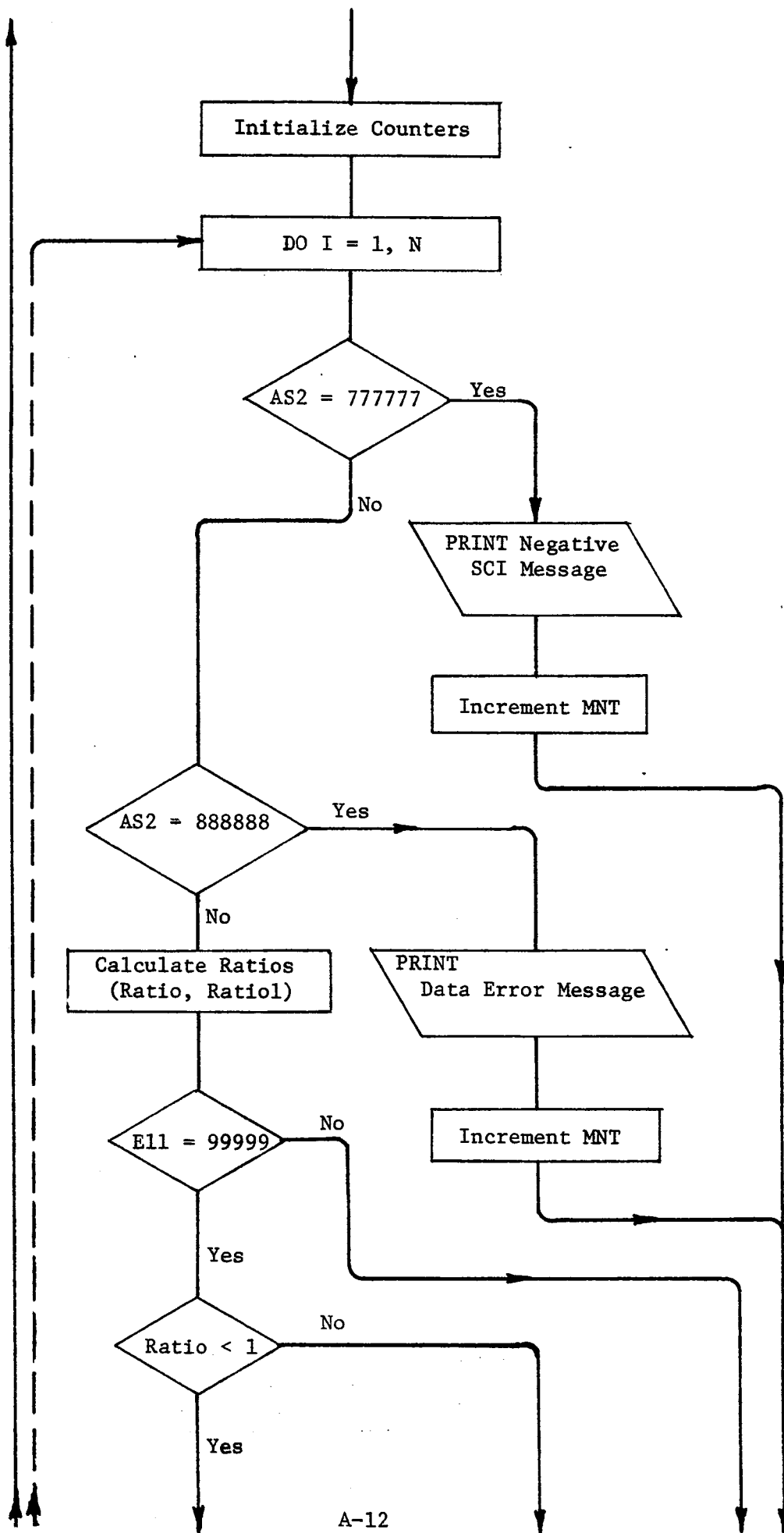
MAIN PROGRAM FLOW CHART

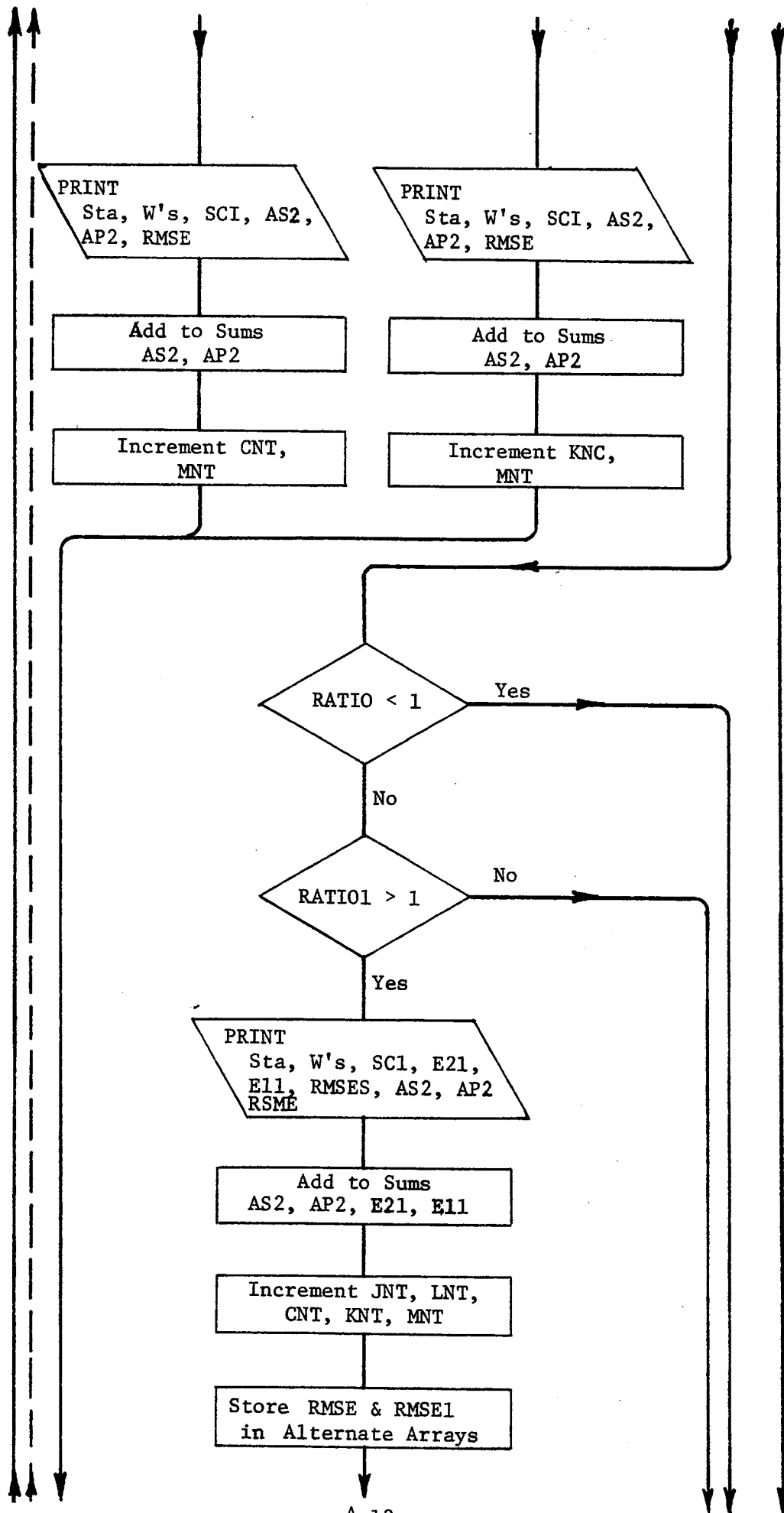


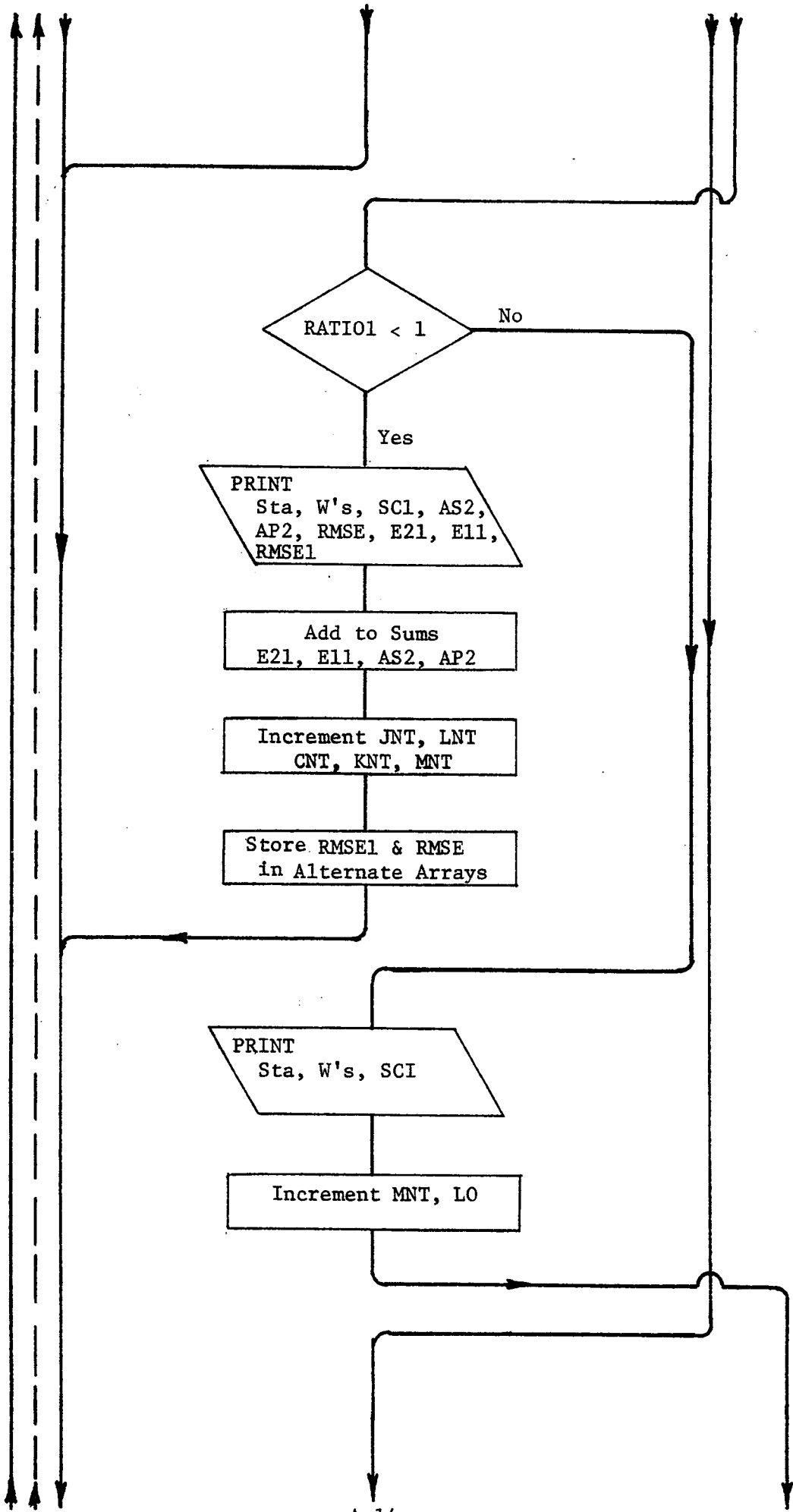


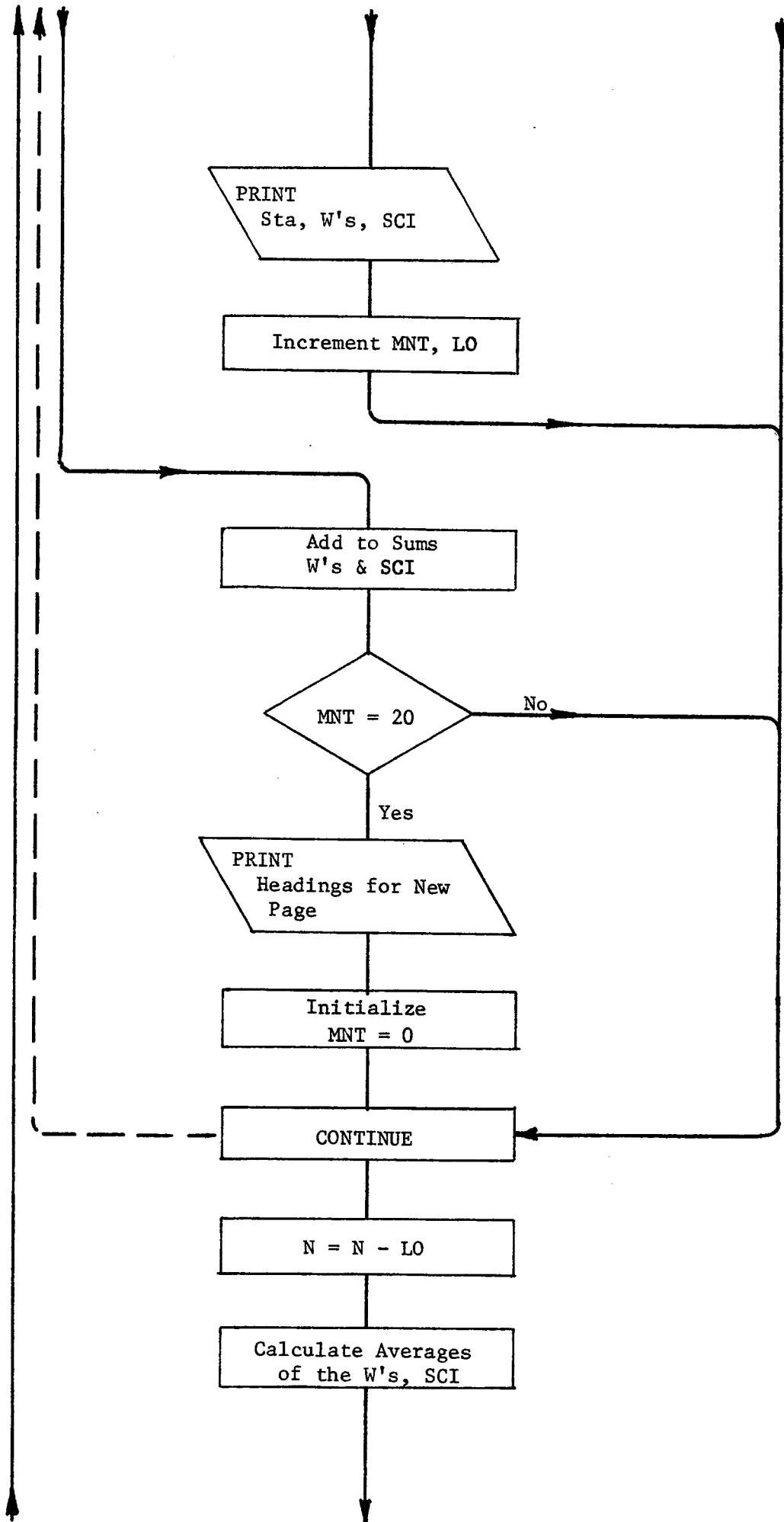


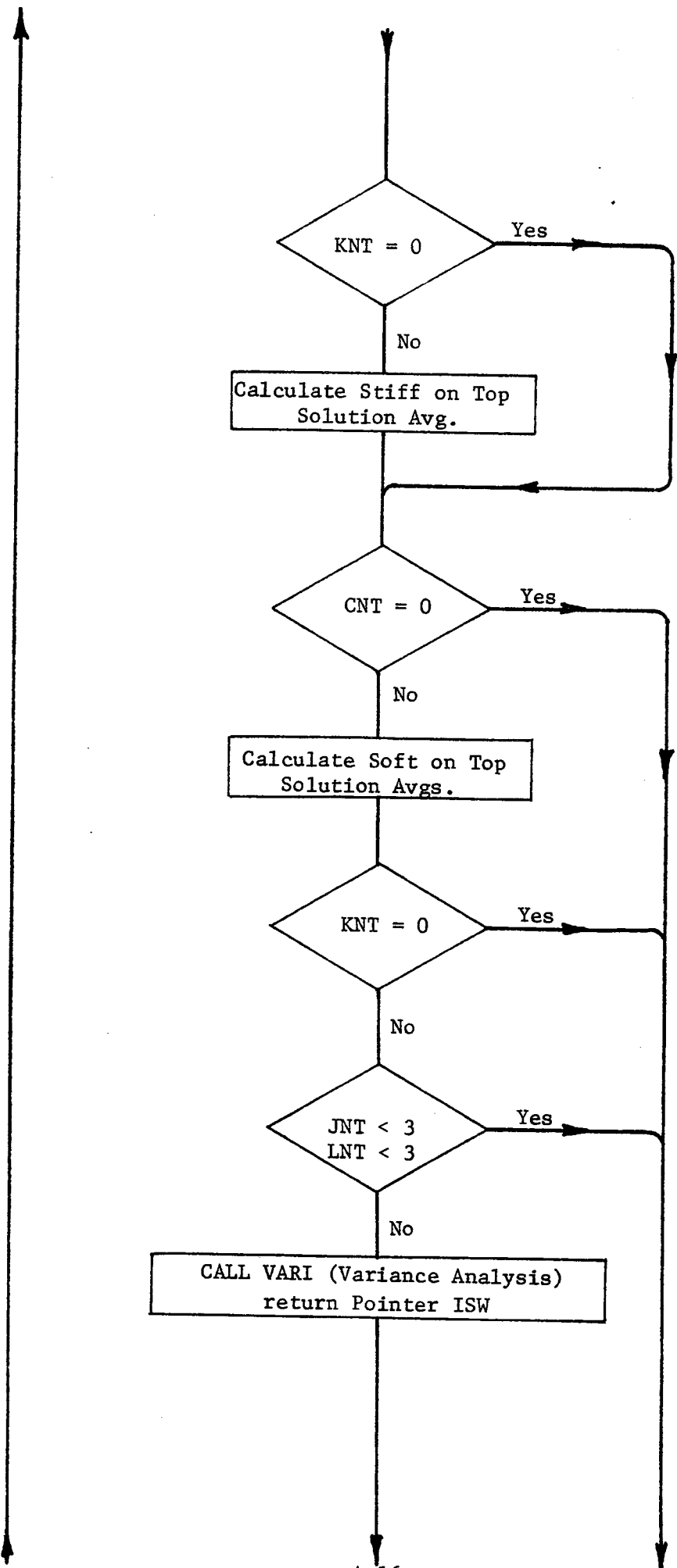


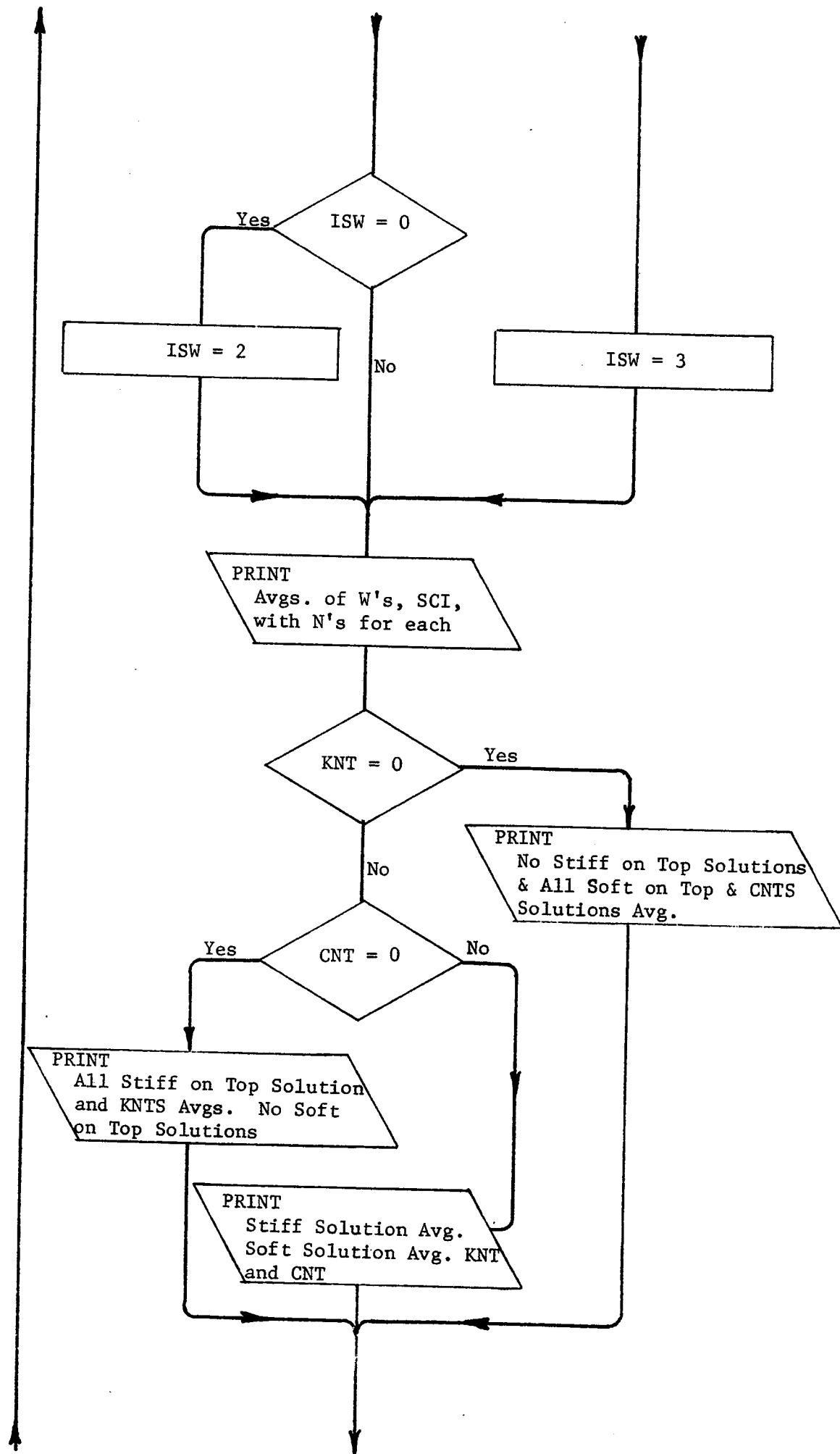


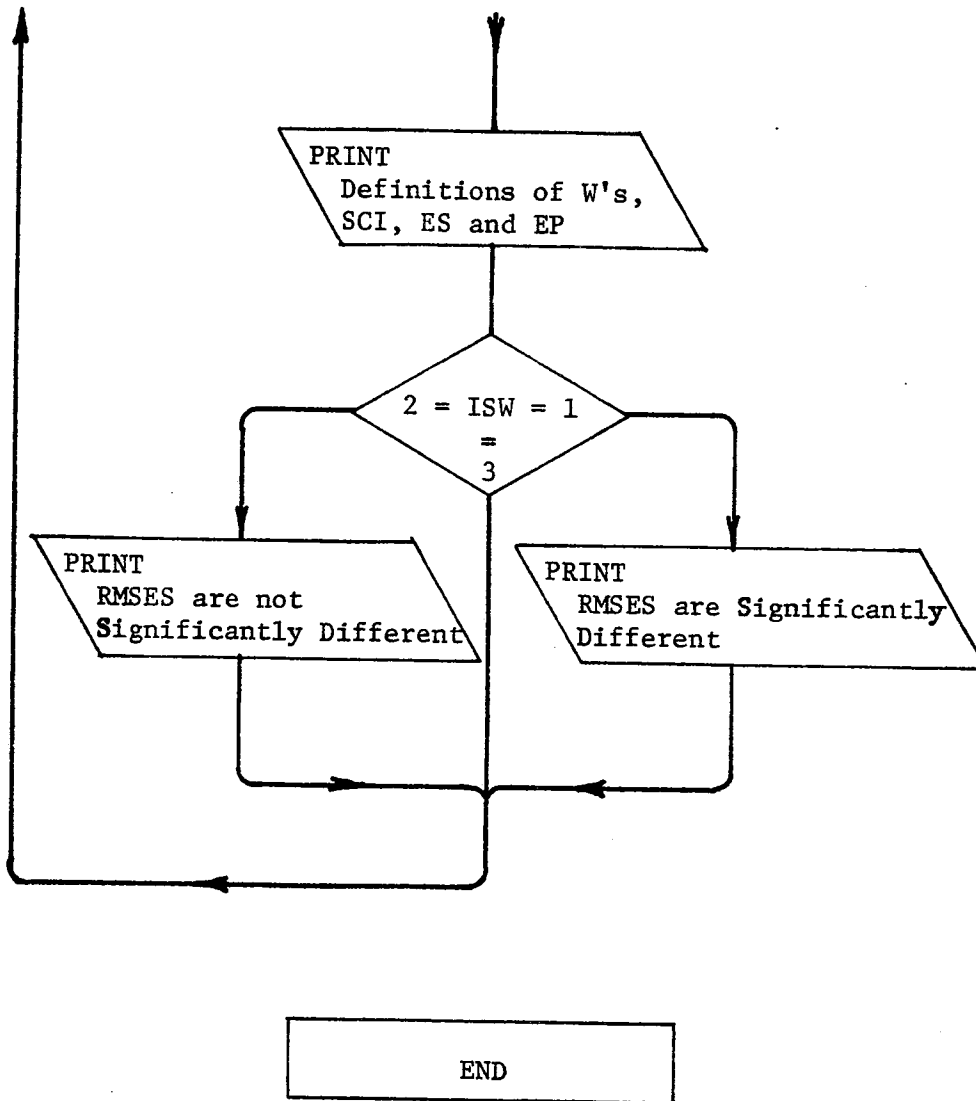












DESCRIPTION OF EMPI SUBROUTINE

This subroutine uses the five deflections and the pavement thickness and calculates pavement and subgrade moduli solutions from the empirical equation along with the corresponding RMSE (Root Mean Squares of the Errors). Due to the nature of data, two solutions for pavement and subgrade moduli sometimes exist.

EMPI calculates both solutions according to the following procedure.

1. An array of \log_{10} numbers from -2.5 to 7.0 with 0.5 intervals is built. The anti-log of each number is then used as the ratio of E_1/E_2 to calculate an array of RMSE's.

2. This array of RMSE's is searched and the three lowest values are found along with their corresponding locations in the array.

	<u>RMSE</u>	<u>Location</u>
lowest	TEMP1	ISUB1
2nd lowest	TEMP2	ISUB2
3rd lowest	TEMP3	ISUB3

3. The locations of these three RMSE's are then checked to determine whether or not there are one or two significant minimums.

If only one significant minimum exists all three locations of RMSE will be consecutive.

If two significant minimums exist one of the locations of RMSE will be separated from the other two.

4. The vicinity of each distinct minimum is searched using a Fibonacci search subroutine. This search will find the minimum value of a unimodal function between two points on its curve. It should be noted that the RMSE versus E_1/E_2 curve is not a unimodal function in its

entirety; however, it has been found to be unimodal between any three consecutive points in the stored array in the vicinity of a minimum. The subroutine returns to the program the minimum RMSE value and the corresponding value of the ratio E_1/E_2 .

5. Next the ratio (E_1E_2) that corresponds to the minimum RMSE is sent to a subroutine ANA. ANS calculates the pavement (E1) and subgrade (E2) moduli using this value for the ratio (E_1E_2).

6. Two sets of pavement and subgrade moduli along with their respective RMSE's are always sent back to the main program in the following form.

1st Solution E1 - Pavement modulus

E2 - Subgrade modulus

RMSE - Root mean square of the errors

*2nd Solution E11 - Pavement modulus

E21 - Subgrade modulus

RMSE1 - Root mean square of the errors

*If only one distinct minimum exists, values for E11, E21 and RMSE1 will be 99999.

SUBROUTINE EMPI VARIABLES

B - Sum of the XW's divided by sum of the X's squared (See equation)

DLTA - Intervals on \log_{10} scale

E1 - Pavement elastic modulus

E2 - Subgrade elastic modulus

E11 - Alternate pavement elastic modulus

E21 - Alternate subgrade elastic modulus

E(I) - Errors between recorded deflections and calculated deflections

EX - X (See equation)

EXSQ - X^2 (See equation)

H - Total pavement thickness

ISUB1 - Location of lowest RMSE in array

ISUB2 - Location of second lowest RMSE in array

ISUB3 - Location of third lowest RMSE in array

K - Number of RMSE's in array to find three lowest

MSE - Mean square of the errors

N - Number of deflections in each case

NOI - Number of points to be tested in Fibonacci search.

RATIO - Ratio of E1/E2

R(I) - Distances from the point at which load is applied

RLDG - Array of \log_{10} numbers to be searched

RMSE - Root mean square of the errors

SMESQ - Sum of the errors squared

SMXSQ - Sum of the X's squared

SMXW - Sum of the X's times the W's

TEMP - Minimum RMSE of the Fibonacci search.

TEMP1 - Lowest RMSE in array

TEMP2 - Second lowest RMSE in array

TEMP3 - Third lowest RMSE in array

TEMPO - Minimum RMSE of the alternate solution of the Fibonacci search

TX1, TX2, TX3 - Location of RMSE that is the left boundary in the
Fibonacci search

TX11, TX21, TX31 - Location of RMSE that is the right boundary in the
Fibonacci search

W(I) - Vertical deflections

$X(I) - \frac{1}{r} + (\text{RATIO} - 1) * \text{XTWO}$ (See equation)

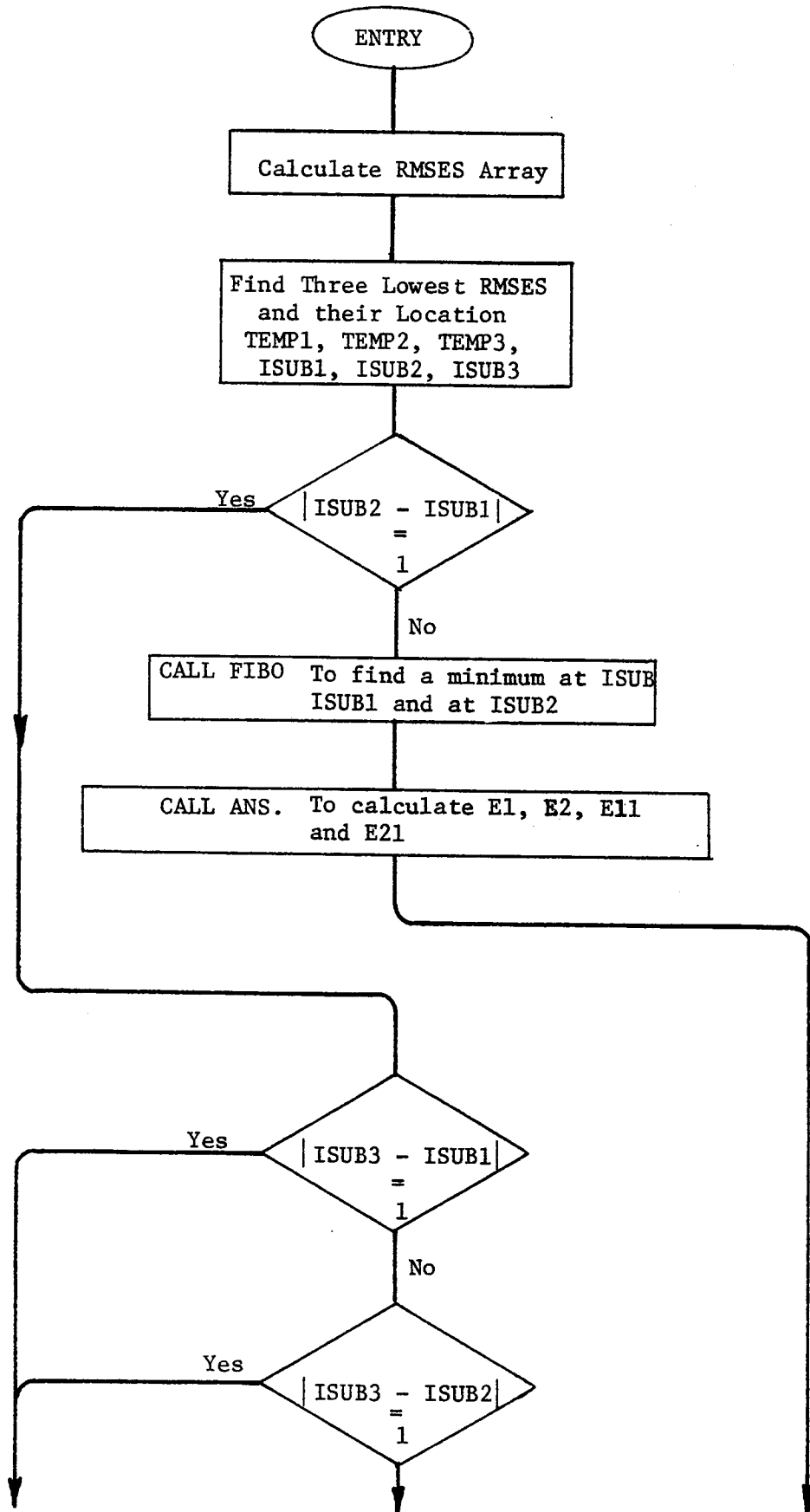
XL - L (See equation)

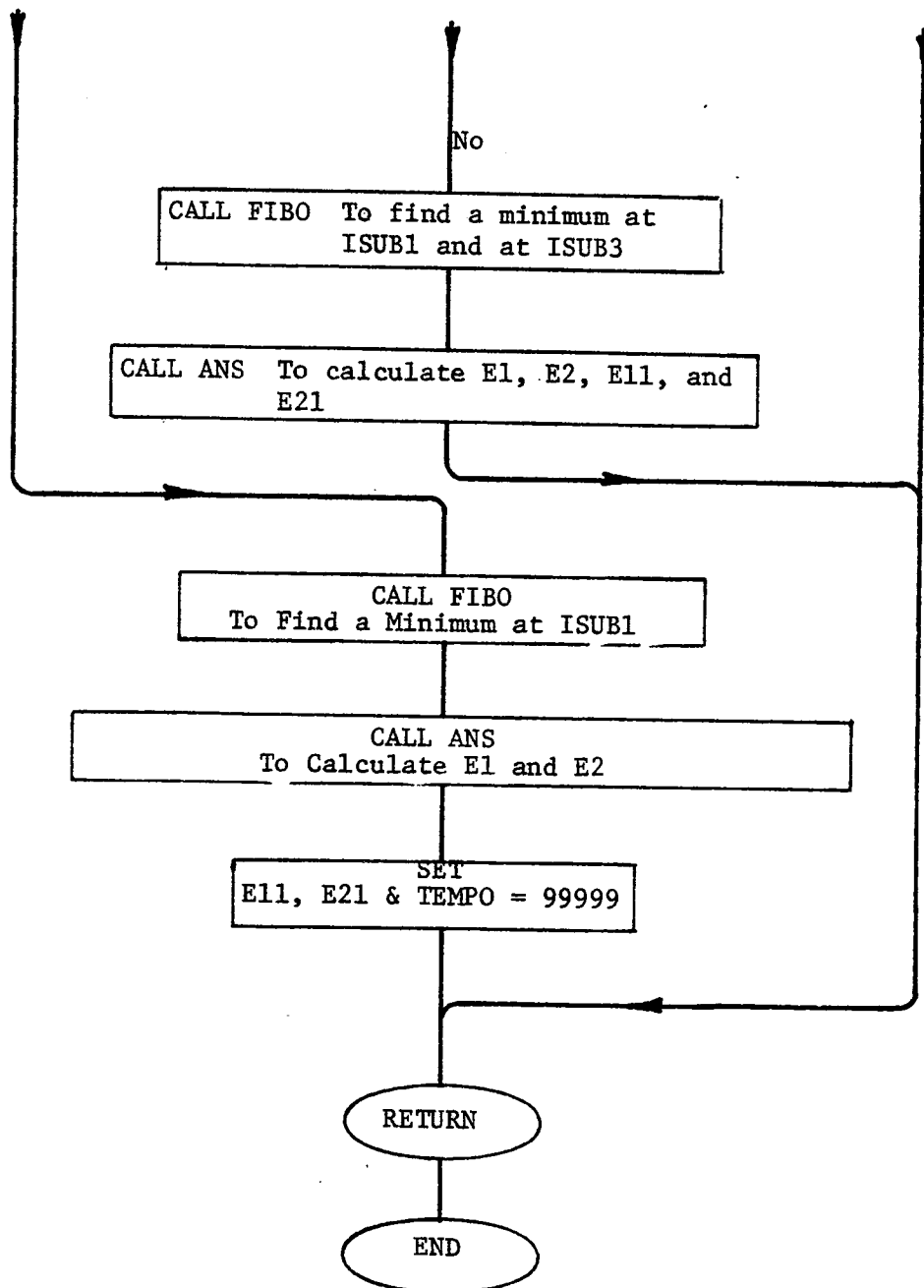
XL3 - L^3 (See equation)

XL5 - L^5 (See equation)

$\text{XTWO} - \frac{1}{L} + \frac{X^2}{2L^3} + \frac{3X^4}{2L^5}$ (See equation)

SUBROUTINE EMPI FLOW CHART





SUBROUTINE VARI

VARI is a variance analysis subroutine to determine whether or not there is a significant difference between the array of soft on top alternate RMSE's and the array of stiff on top alternate RMSE's at a 10% level of significance.

SUBROUTINE VARI VARIABLES

AMSBS - Mean square between sets
AMSWs - Mean square within sets
CNT - Number of soft on top solutions
F - Ratio of AMSBS over AMSWS
IDFBS - Degrees of freedom between sets
IDFWS - Degrees of freedom within sets
KNT - Number of stiff on top solutions
N - Total number of RMSE's tested
RMSE - Array of stiff on top alternate RMSE's
RMSEL - Array of soft on top alternate RMSE's
SSBS - Sums of squares between sets
SSQ(1) - Sum of all stiff on top alternate RMSE's squared
SSQ(2) - Sum of all soft on top alternate RMSE's squared
SSWS - Sums of squares within sets
SUM(1) - Sum of all stiff on top alternate RMSE's
SUM(2) - Sum of all soft on top alternate RMSE's
TSSQ - Total sum of alternate RMSE's squared
TSUM - Total sum of alternate RMSE's

SUBROUTINE SIGNIF

This is an F-distribution table for a 10% level of significance with the numerator at 1 since there will always be only two sets.

SUBROUTINE SIGNIF VARIABLES

F - Ratio of AMSBS over AMSWS (See VARI variables)

IDFWS - Degrees of freedom within sets (Denominator)

ISW - Pointer as to significance

ISW = 0 - Not significant at 10%.

ISW = 1 - Is significant at 10%.

SUBROUTINE FIBO

This subroutine is a Fibonacci search which is used to determine the minimal value of a unimodal function. A subroutine `FUNC (X,Y)` is called to obtain the Y value of the unimodal function, $Y = F(X)$.

SUBROUTINE FIBO VARIABLES

N - Number of search desired, Max = 20

X1 - Lower limit of X value

X2 - Upper limit of X value

X - Location of optimal Y

Y - Optimal Y value

C	TWO LAYER ELASTIC MODULI FOR FIVE DEFLECTIONS	TTI	0
	DIMENSION STA(200),W(200,5),D(10),AW(5),AWV(5),RATIO(200),	TTI	10
	* AP2(200),LA1(5),LA2(5),LA3(5),LA4(5),LA5(5),LA6(5),	TTI	20
	*RATIO1(200),AS2(200),A(20),SCI(200),	TTI	30
	*E11(200), E21(200),RMSE1(200),	TTI	40
	* IXDATE(3),COMM(7),REM(4),RMSE(200),BLRMS(200),ALRMS(200)	TTI	50
	INTEGER CNT	TTI	60
	REAL * 8 STA,DAS,DAP,DBLE,E11,E21,RMSE1,RMSE,BLRMS,ALRMS	TTI	70
C		TTI	80
CCC	STATEMENT FUNCTION TO ROUND 'X' TO NEAREST 'EVEN'	TTI	90
C		TTI	100
	ROUND(X, EVEN) = AINT((X + EVEN * .5) / EVEN)	TTI	110
	** EVEN	TTI	120
C		TTI	130
	10 CONTINUE	TTI	140
	READ(5,1,END=1000) NCARD, (A(I), I = 1 , 20)	TTI	150
	1 FORMAT(13, 19A4, A1)	TTI	160
	CALL CORE (A, 80)	TTI	170
	IF(NCARD.EQ.100) GO TO 11	TTI	180
	IF(NCARD.EQ.200) GO TO 12	TTI	190
	IF(NCARD.EQ.300) GO TO 13	TTI	200
	14 I=N+1	TTI	210
	READ(5,6) ICONT,ISECT,M,IDAY,IYEAR,STA(I),(D(K),K=1,10),	TTI	220
	*(REM(J),J=1,4),ICK	TTI	230
	6 FORMAT(I4,4I2,A7,3X, 5(F2.1,F3.2),8X,4A4,I2)	TTI	240
C	PRINT OUTPUT COLUMN HEADINGS	TTI	250
C		TTI	260
C	CALCULATE DEFLECTIONS & SCI (DEFLECTIONS IN MILS)	TTI	270
	L=1	TTI	280
	DO 4 J=1,5	TTI	290
	W(I,J) = D(L) * D(L + 1)	TTI	300
	L = L + 2	TTI	310
	4 CONTINUE	TTI	320
	SCI(I) = W(I,1) - W(I,2)	TTI	330
C		TTI	340
C	TEST FOR W1 OR W2 = 0, AND W1 LESS THAN W2	TTI	350
C		TTI	360
	DO 5 J=1,5	TTI	370
	IF(W(I,J) .EQ. 0.0) GO TO 21	TTI	380
	5 CONTINUE	TTI	390
	DO 7 J=1,4	TTI	400
	IF(W(I,J) .LT. W(I,J+1)) GO TO 22	TTI	410
	7 CONTINUE	TTI	420
	DO 8 J=1,5	TTI	430
	W(I,J) = W(I,J) / 1000.	TTI	440
	8 CONTINUE	TTI	450
C		TTI	460
C	PASS THE W'S & TOTAL PAVEMENT THICKNESS TO EMPI,	TTI	470
C	EMPI RETURNS UNROUNDED VALUES OF PAVEMENT & SUBGRADE AND RMSE	TTI	480
C	MODULI AS DAP & DAS & RMSE(I) AND E11(I),E21(I),& RMSE1(I)	TTI	490

C	CALL EMPI(DBLE(W(I,1)),DBLE(W(I,2)),DBLE(W(I,3)),DBLE(W(I,4)),	TTI 500
	*DBLE(W(I,5)),DBLE(DP),DAP,DAS, RMSE(I),E11(I),E21(I),RMSE1(I))	TTI 510
C	CONVERT THE W'S TO MILS	TTI 520
	DO 9 J=1,5	TTI 530
	W(I,J) = W(I,J) * 1000.	TTI 540
	9 CONTINUE	TTI 550
	RMSE(I) = RMSF(I) * 1000.	TTI 560
	RMSE1(I) = RMSE1(I) * 1000.	TTI 570
	DAS = ROUND(DAS, 100.)	TTI 580
	DAP = ROUND(DAP, 100.)	TTI 590
	AS2(I) = DAS	TTI 600
	AP2(I) = DAP	TTI 610
	IF(E11(I) .EQ. 99999) GO TO 23	TTI 620
	E11(I) = ROUND(E11(I),100.)	TTI 630
	E21(I) = ROUND(E21(I),100.)	TTI 640
	23 CONTINUE	TTI 650
	N = N + 1	TTI 660
	IF(ICK .EQ. 0) GO TO 10	TTI 670
	GO TO 80	TTI 680
	11 READ(5,2) IDIST,CO1,CO2,CO3,CO4,ICONT,ISECT,IJOB,HWY1,	TTI 690
	* HWY2, XLANE, DP,M,IDAY,IYEAR,IDYNA,(COMM(I),I=1,7)	TTI 700
	2 FORMAT(I2,3A4,A2,I4,2I2,A4,A3,A3,F5.2,4I2,7A4)	TTI 710
	PRINT 51	TTI 720
	51 FORMAT('1')	TTI 730
	PRINT 52	TTI 740
	52 FORMAT(35X,'TEXAS HIGHWAY DEPARTMENT' /)	TTI 750
	PRINT 53,IDIST	TTI 760
	53 FORMAT(33X,'DISTRICT ',I2,' - DESIGN SECTION' /)	TTI 770
	PRINT 54	TTI 780
	54 FORMAT(21X,'DYNAFLECT DEFLECTIONS AND CALCULATED ',	TTI 790
	* 'ELASTIC MODULI ' /)	TTI 800
	CALL DATE (IXDATE(1), IXDATE(2), IXDATE(3))	TTI 810
	PRINT 55,IXDATE	TTI 820
	55 FORMAT(32X,'THIS PROGRAM WAS RUN - ', 2A3,A2 /)	TTI 830
	PRINT 56, IDIST, CO1, CO2, CO3, CO4	TTI 840
	PRINT 57, ICONT, ISECT,IJOB,HWY1,HWY2,M,IDAY,IYEAR,IDYNA	TTI 850
	PRINT 58,(COMM(I),I=1,7),DP	TTI 860
	58 FORMAT(10X,7A4,2X,'PAV. THICK. = ',F5.2,' INCHES' /)	TTI 870
	N=0	TTI 880
	DO 15 J=1,5	TTI 890
	AW(J) = 0.0	TTI 900
	15 CONTINUE	TTI 910
	ASCI=0.	TTI 920
	AAS2=0.	TTI 930
	AAP2=0.	TTI 940
	BAS2 = 0.	TTI 950
	BAP2 = 0.	TTI 960
	GO TO 10	TTI 970
C	READ & PRINT INFORMATION ON DATA CARD 2	TTI 980
		TTI 990

12	READ(5,3) (LA1(I),I=1,5),T1,(LA2(I),I=1,5),T2, * (LA3(I),I=1,5), T3	TTI 1000
3	FORMAT(5A4,F4.2,5A4,F4.2,5A4,F4.2) PRINT 59,(LA1(I),I=1,5),T1,(LA2(I),I=1,5),T2 PRINT 59, (LA3(I), I=1,5), T3	TTI 1010 TTI 1020 TTI 1030
59	FORMAT(16X, 5A4, 1X, F5.2, 5X, 5A4, 1X, F5.2//) GO TO 10	TTI 1040 TTI 1050
C	READ & PRINT INFORMATION ON DATA CARD 3, IF PRESENT	TTI 1060
13	READ(5,3) (LA4(I),I=1,5),T4,(LA5(I),I=1,5),T5, * (LA6(I),I=1,5), T6 PRINT 59,(LA4(I),I=1,5),T4,(LA5(I),I=1,5),T5 PRINT 59, (LA6(I), I=1,5), T6 GO TO 10	TTI 1070 TTI 1080 TTI 1090 TTI 1100 TTI 1110 TTI 1120
22	CONTINUE AS2(I) = 7777777 AP2(I) = 7777777 IF(ICK .EQ. 0) GO TO 10 GO TO 80	TTI 1130 TTI 1140 TTI 1150 TTI 1160
21	CONTINUE AS2(I) = 8888888 AP2(I) = 8888888 IF(ICK .EQ. 0) GO TO 10 GO TO 80	TTI 1170 TTI 1180 TTI 1190 TTI 1200
80	CONTINUE PRINT 61	TTI 1210 TTI 1220 TTI 1230
61	FORMAT(/ 7X,'STATION W1 W2 W3 W4 W5', = ' SCI ** ES ** ** EP ** * RMSE *//) CNT = 0 KNT = 0 MNT = 0 JNT = 0 LNT = 0 LO = 0 DO 50 I=1,N IF(AS2(I) .EQ. 7777777) GO TO 24 IF(AS2(I) .EQ. 8888888) GO TO 25 RATIO(I) = AP2(I)/AS2(I) IF(E11(I) .EQ. 99999) GO TO 26 RATIO1(I) = E11(I)/E21(I) IF(RATIO(I) .LT. 1.0) GO TO 27 IF(RATIO1(I) .LT. 1.0) GO TO 28	TTI 1240 TTI 1250 TTI 1260 TTI 1270 TTI 1280 TTI 1290 TTI 1300 TTI 1310 TTI 1320 TTI 1330 TTI 1340 TTI 1350 TTI 1360 TTI 1370 TTI 1380 TTI 1390
C	BOTH ALTERNATES HARD ON TOP PRINT 62,STA(I),(W(I,J),J=1,5),SCI(I)	TTI 1400 TTI 1410
62	FORMAT(7X,A7,1X,6(F6.3),10X,'NO SOLUTION') MNT = MNT + 1 LO = LO + 1 GO TO 50	TTI 1420 TTI 1430 TTI 1440 TTI 1450
27	IF(RATIO1(I) .GT. 1.0) GO TO 29	TTI 1460
C	BOTH ALTERNATES ARE SOFT ON TOP PRINT 62,STA(I),(W(I,J),J=1,6),SCI(I)	TTI 1470 TTI 1480 TTI 1490

MNT = MNT + 1	TTI 1500
LD = LD + 1	TTI 1510
GO TO 50	TTI 1520
28 PRINT 63,STA(I),(W(I,J),J=1,5),SCI(I),AS2(I),AP2(I),RMSE(I)	TTI 1530
63 FORMAT(7X,A7,1X,6(F6.3),2F10.0,2X,F8.4)	TTI 1540
PRINT 64,E21(I),E11(I),RMSE1(I)	TTI 1550
64 FORMAT(T17,'ALTERNATE SOLUTION',T52,2F10.0,2X,F8.4)	TTI 1560
BAS2 = BAS2 + E21(I)	TTI 1570
BAP2 = BAP2 + E11(I)	TTI 1580
AAS2 = AAS2 + AS2(I)	TTI 1590
AAP2 = AAP2 + AP2(I)	TTI 1600
JNT = JNT + 1	TTI 1610
BLRMS(JNT) = RMSE1(I)	TTI 1620
LNT = LNT + 1	TTI 1630
ALRMS(LNT) = RMSE(I)	TTI 1640
CNT = CNT + 1	TTI 1650
KNT = KNT + 1	TTI 1660
MNT = MNT + 2	TTI 1670
GO TO 40	TTI 1680
29 PRINT 63,STA(I),(W(I,J),J=1,5),SCI(I),E21(I),E11(I),RMSE1(I)	TTI 1690
PRINT 64,AS2(I),AP2(I),RMSE(I)	TTI 1700
BAS2 = BAS2 + AS2(I)	TTI 1710
BAP2= BAP2 + AP2(I)	TTI 1720
AAS2 = AAS2 + E21(I)	TTI 1730
AAP2 = AAP2 + E11(I)	TTI 1740
JNT = JNT + 1	TTI 1750
BLRMS(JNT) = RMSE(I)	TTI 1760
LNT = LNT + 1	TTI 1770
ALRMS(LNT) = RMSE1(I)	TTI 1780
CNT = CNT + 1	TTI 1790
KNT = KNT + 1	TTI 1800
MNT = MNT + 2	TTI 1810
GO TO 40	TTI 1820
26 CONTINUE	TTI 1830
IF(RATIO(I) .LT. 1.0) GO TO 30	TTI 1840
PRINT 63 ,STA(I),(W(I,J),J=1,5),SCI(I),AS2(I),AP2(I),RMSE(I)	TTI 1850
AAS2 = AAS2 + AS2(I)	TTI 1860
AAP2 = AAP2 + AP2(I)	TTI 1870
KNT = KNT + 1	TTI 1880
MNT = MNT + 1	TTI 1890
GO TO 40	TTI 1900
30 PRINT 63,STA(I),(W(I,J),J=1,5),SCI(I),AS2(I),AP2(I),RMSE(I)	TTI 1910
BAS2 = BAS2 + AS2(I)	TTI 1920
BAP2 = BAP2 + AP2(I)	TTI 1930
CNT = CNT + 1	TTI 1940
MNT = MNT + 1	TTI 1950
40 CONTINUE	TTI 1960
DO 16 M=1,5	TTI 1970
16 AW(M) = AW(M) + W(I,M)	TTI 1980
ASCI = ASCI + SCI(I)	TTI 1990

IF(MNT .EQ. 20) GO TO 31	TTI 2000
GO TO 50	TTI 2010
31 PRINT 51	TTI 2020
PRINT 56, IDIST, CO1, CO2, CO3, CO4	TTI 2030
56 FORMAT(T35,'DIST. COUNTY'/ T36, I2,9X, 3A4,A2 /)	TTI 2040
PRINT 57, ICONT, ISECT, IJOB, HWY1, HWY2, M, IDAY, IYEAR, IDYNA	TTI 2050
57 FORMAT(T19, 'CONT. SECT. JOB HIGHWAY DATE',	TTI 2060
" ' DYNAFLECT' / T19, I4, 2I7, 4X, A4, A3, I4, 2('-', I2), I9 /)	TTI 2070
PRINT 61	TTI 2080
MNT = 0	TTI 2090
GO TO 50	TTI 2100
24 PRINT 65, STA(I)	TTI 2110
65 FORMAT(7X, A7, 3X, 'NEGATIVE SCI OTHER CALCULATIONS OMMITTED')	TTI 2120
MNT = MNT + 1	TTI 2130
GO TO 50	TTI 2140
25 PRINT 66, STA(I)	TTI 2150
66 FORMAT(7X, A7, 3X, 'ERROR IN DATA')	TTI 2160
MNT = MNT + 1	TTI 2170
50 CONTINUE	TTI 2180
C CALCULATE AVERAGES	TTI 2190
N = N - LO	TTI 2200
DO 17 M=1,5	TTI 2210
17 AWV(M) = AW(M)/N	TTI 2220
ASCIV = ASCI/N	TTI 2230
IF(KNT .EQ. 0) GO TO 32	TTI 2240
AAS2V = AAS2/KNT	TTI 2250
AAP2V = AAP2/KNT	TTI 2260
32 CONTINUE	TTI 2270
IF(CNT .EQ. 0) GO TO 33	TTI 2280
BAS2V = BAS2/CNT	TTI 2290
BAP2V = BAP2/CNT	TTI 2300
IF(KNT .EQ. 0) GO TO 33	TTI 2310
IF(JNT .LT. 3 .AND. LNT .LT. 3) GO TO 33	TTI 2320
CALL VARI(ALRMS, BLRMS, JNT, LNT, ISW)	TTI 2330
IF(ISW .EQ. 0) ISW = 2	TTI 2340
GO TO 34	TTI 2350
33 CONTINUE	TTI 2360
ISW = 3	TTI 2370
34 CONTINUE	TTI 2380
PRINT 81, (AWV(J), J=1,5), ASCIV	TTI 2390
81 FORMAT(/7X, 'AVERAGES', /, 8X, 7HW'S, SCI, 6(F6.3))	TTI 2400
PRINT 82, N, N, N, N, N, N	TTI 2410
82 FORMAT(8X, 'POINTS', T19, I3, 3X, I3, 3X, I3, 3X, I3, 3X, I3, 3X, I3)	TTI 2420
IF(KNT .EQ. 0) GO TO 35	TTI 2430
IF(CNT .NE. 0) GO TO 36	TTI 2440
PRINT 83, AAS2V, AAP2V	TTI 2450
83 FORMAT(8X, 'STIFF ON TOP SOLUTIONS', T52, 2F10.0)	TTI 2460
PRINT 84, KNT, KNT	TTI 2470
84 FORMAT(8X, 'POINTS', T58, I3, T68, I3)	TTI 2480
PRINT 85	TTI 2490

85	FORMAT(8X,'NO SOFT ON TOP SOLUTIONS')	TTI 2500
	GO TO 90	TTI 2510
36	CONTINUE	TTI 2520
	PRINT 83,AAS2V,AAP2V	TTI 2530
	PRINT 84,KNT,KNT	TTI 2540
	PRINT 86,BAS2V,BAP2V	TTI 2550
86	FORMAT(8X,'SOFT ON TOP SOLUTIONS',T52,2F10.0)	TTI 2560
	PRINT 87,CNT,CNT	TTI 2570
87	FORMAT(8X,'POINTS',T58,I3,T68,I3)	TTI 2580
	GO TO 90	TTI 2590
35	CONTINUE	TTI 2600
	PRINT 88	TTI 2610
88	FORMAT(8X,'NO STIFF ON TOP SOLUTIONS')	TTI 2620
	PRINT 86,BAS2V,BAP2V	TTI 2630
	PRINT 87,CNT,CNT	TTI 2640
90	CONTINUE	TTI 2650
	PRINT 91	TTI 2660
91	FORMAT(/10X,'W1 DEFLECTION AT GEOPHONE 1')	TTI 2670
	PRINT 92	TTI 2680
92	FORMAT(10X,'W2 DEFLECTION AT GEOPHONE 2')	TTI 2690
	PRINT 93	TTI 2700
93	FORMAT(10X,'W3 DEFLECTION AT GEOPHONE 3')	TTI 2710
	PRINT 94	TTI 2720
94	FORMAT(10X,'W4 DEFLECTION AT GEOPHONE 4')	TTI 2730
	PRINT 95	TTI 2740
95	FORMAT(10X,'W5 DEFLECTION AT GEOPHONE 5')	TTI 2750
	PRINT 96	TTI 2760
96	FORMAT(10X,'SCI SURFACE CURVATURE INDEX (W1 MIN',	TTI 2770
	* 'US W2)')	TTI 2780
	PRINT 97	TTI 2790
97	FORMAT(10X,'ES ELASTIC MODULUS OF THE SUBGRADE FRO',	TTI 2800
	* 'M W1,W2,W3,W4,& W5')	TTI 2810
	PRINT 98	TTI 2820
98	FORMAT(10X,'EP ELASTIC MODULUS OF THE PAVEMENT FRO',	TTI 2830
	* 'M W1,W2,W3,W4,& W5')	TTI 2840
	GO TO (100,200,300) , ISW	TTI 2850
100	PRINT 99	TTI 2860
99	FORMAT(10X,'***** IN CASES WITH ALTERNATES, RMSES OF THE SOFT AND	TTI 2870
	*STIFF ON TOP',/,16X,'SOLUTIONS ARE DIFFERENT AT A 10 PERCENT LEVEL	TTI 2880
	* OF SIGNIFICANCE')	TTI 2890
	GO TO 300	TTI 2900
200	PRINT 101	TTI 2910
101	FORMAT(10X,'***** IN CASES WITH ALTERNATES,RMSES ARE NOT SIGNIFICATTI	TTI 2920
	*NTLY DIFFERENT')	TTI 2930
300	CONTINUE	TTI 2940
	GO TO 10	TTI 2950
1000	CONTINUE	TTI 2960
	END	TTI 2970

```

SUBROUTINE EMPI(W1,W2,W3,W4,W5,H,E1,E2,TEMP,E11,E21,TEMPO)
IMPLICIT REAL*8(A-H,O-Z)
DIMENSION RLOG(30),RATIO(30),X(20),RMSE(30),E(20)
DIMENSION RAT1(30),RAT2(30),RAT3(30)
DIMENSION R(5),W(5)
COMMON /A/ R,W
REAL*8 MSE,LRMSE
N=5
R(1) = 10.0
R(2) = 15.620499
R(3) = 26.0
R(4) = 37.363083
R(5) = 49.030603
W(1)=W1
W(2)=W2
W(3)=W3
W(4)=W4
W(5)=W5
DLTA = .5
RLOG(1) = -2.5
DO 1 K=2,21
1 RLOG(K) = RLOG(K-1) + DLTA
DO 2 J=1,21
RATIO(J) = 10**(RLOG(J))
RATIO1 = RATIO(J)
DO 3 I=1,N
EX=2.*H*((2.+RATIO1)/3.)**0.33333333
EXSQ=EX*EX
XL=DSQRT(R(I)*R(I)+EXSQ)
XL3=XL*XL*XL
XL5=XL*XL*XL*XL*XL
XTWO=(1./XL)+(EXSQ/(2.*XL3))+((3.*EXSQ*EXSQ)/(2.*XL5))
3 X(I)=(1./R(I))+(RATIO1-1.)*XTWO
SMXW=0.0
SMXSQ=0.0
SMESQ=0.0
DO 4 I=1,N
SMXW=SMXW+X(I)*W(I)
4 SMXSQ=SMXSQ+X(I)*X(I)
B=SMXW/SMXSQ
DO 5 I=1,N
E(I)=W(I)-(B*X(I))
5 SMESQ=SMESQ+E(I)*E(I)
MSE=SMESQ/N
RMSE(J)=DSQRT(MSE)
LRMSE = DLOG10(RMSE(J))
2 CONTINUE
K=20
TEMP1 = RMSE(1)
TEMP2 = RMSE(1)

```

EMPI 10
EMPI 20
EMPI 30
EMPI 40
EMPI 50
EMPI 60
EMPI 70
EMPI 80
EMPI 90
EMPI 100
EMPI 110
EMPI 120
EMPI 130
EMPI 140
EMPI 150
EMPI 160
EMPI 170
EMPI 180
EMPI 190
EMPI 200
EMPI 210
EMPI 220
EMPI 230
EMPI 240
EMPI 250
EMPI 260
EMPI 270
EMPI 280
EMPI 290
EMPI 300
EMPI 310
EMPI 320
EMPI 330
EMPI 340
EMPI 350
EMPI 360
EMPI 370
EMPI 380
EMPI 390
EMPI 400
EMPI 410
EMPI 420
EMPI 430
EMPI 440
EMPI 450
EMPI 460
EMPI 470
EMPI 480
EMPI 490
EMPI 500

TEMP3 = RMSE(1)	EMPI 510
ISUB1 = 1	EMPI 520
ISUB2 = 1	EMPI 530
ISUB3 = 1	EMPI 540
DO 8 L=2,K	EMPI 550
IF(RMSE(L) .GT. TEMP1) GO TO 6	EMPI 560
TEMP3 = TEMP2	EMPI 570
ISUB3 = ISUB2	EMPI 580
TEMP2 = TEMP1	EMPI 590
ISUB2 = ISUB1	EMPI 600
TEMP1 = RMSE(L)	EMPI 610
ISUB1 = L	EMPI 620
GO TO 8	EMPI 630
6 CONTINUE	EMPI 640
IF(RMSE(L) .GT. TEMP2) GO TO 7	EMPI 650
TEMP3 = TEMP2	EMPI 660
ISUB3 = ISUB2	EMPI 670
TEMP2 = RMSE(L)	EMPI 680
ISUB2 = L	EMPI 690
GO TO 8	EMPI 700
7 CONTINUE	EMPI 710
IF (RMSE(L) .GT. TEMP3) GO TO 8	EMPI 720
TEMP3 = RMSE(L)	EMPI 730
ISUB3 = L	EMPI 740
8 CONTINUE	EMPI 750
TX1 = RLOG(ISUB1 - 1)	EMPI 760
TX2 = RLOG(ISUB2 - 1)	EMPI 770
TX3=RLOG(ISUB3-1)	EMPI 780
TX11 = TX1 + 1.0	EMPI 790
TX21 = TX2 + 1.0	EMPI 800
TX31 = TX3 + 1.0	EMPI 810
C TEST FOR MINIMUM AT TX2	EMPI 820
JR1 = ISUB2 - ISUB1	EMPI 830
JR = IABS(JR1)	EMPI 840
IF(JR .EQ. 1) GO TO 99	EMPI 850
C FIND MINIMUMS FOR TX1 & TX2	EMPI 860
NOI = 11	EMPI 870
CALL FIBO(NOI,TX1,TX11,RLOG1,RMLOG,H)	EMPI 880
NOI = 11	EMPI 890
CALL FIBO(NOI,TX2,TX21,RLOGO,RMLOGO,H)	EMPI 900
TEMP = 10**RMLOG	EMPI 910
TEMPO = 10**RMLOGO	EMPI 920
CALL ANS(RLOG1,E1,E2,H)	EMPI 930
CALL ANS(RLOGO,E11,E21,H)	EMPI 940
GO TO 70	EMPI 950
99 CONTINUE	EMPI 960
C NO MINIMUM AT ISUB2	EMPI 970
C TEST FOR MINIMUM AT ISUB3	EMPI 980
JW1 = ISUB3 - ISUB1	EMPI 990
JW = IABS(JW1)	EMPI1000

-4

```
JWT1 = ISUB3 - ISUB2
JWT = IABS(JWT1)
IF(JW .EQ. 1) GO TO 98
IF(JWT .EQ. 1) GO TO 98
C   MINIMUM AT ISUB1 AND ISUB3
    NOI = 11
    CALL FIBO(NOI, TX1, TX11, RLOG1, RMLOG, H)
    NOI = 11
    CALL FIBO(NOI, TX3, TX31, RLOGO, RMLOGO, H)
    TEMP = 10**RMLOG
    TEMPO = 10**RMLOGO
    CALL ANS(RLOG1, E1, E2, H)
    CALL ANS(RLOGO, E11, E21, H)
    GO TO 70
98  CONTINUE
C   O LY ONE DISTINCT MINIMUM
    NOI = 11
    CALL FIBO(NOI, TX1, TX11, RLOG1, RMLOG, H)
    TEMP = 10**RMLOG
    CALL ANS(RLOG1, E1, E2, H)
    E11 = 99999
    E21 = 99999
    TEMPO = 99999
70  CONTINUE
    RETURN
    END
```

```
EMPI1010
EMPI1020
EMPI1030
EMPI1040
EMPI1050
EMPI1060
EMPI1070
EMPI1080
EMPI1090
EMPI1100
EMPI1110
EMPI1120
EMPI1130
EMPI1140
EMPI1150
EMPI1160
EMPI1170
EMPI1180
EMPI1190
EMPI1200
EMPI1210
EMPI1220
EMPI1230
EMPI1240
EMPI1250
EMPI1260
```



```
SUBROUTINE FIBO(N,X1,X2,X,Y,H)
IMPLICIT REAL*8(A-H,O-Z)
DIMENSION FIB(20)
DATA FIB/1.000,2.000,3.000,5.000,8.000,13.000,21.000,34.000,
*55.000,89.000,144.000,233.000,377.000,610.000,987.000,1597.000,
*2584.000,4181.000,6765.000,10946.000/
DX=(X2-X1)/FIB(N)
XL=X1
XR=X2
N=N-1
X=XL+FIB(N)*DX
CALL FUNC(X,VR,H)
1 N=N-1
X=XL+FIB(N)*DX
CALL FUNC(X,VL,H)
2 IF(N.EQ.1) GO TO 4
IF(VL.GT.VR) GO TO 3
XR=XR-FIB(N)*DX
VR=VL
GO TO 1
3 XL=XL+FIB(N)*DX
VL=VR
N=N-1
X=XR-FIB(N)*DX
CALL FUNC(X,VR,H)
GO TO 2
4 IF(VL.GT.VR) GO TO 7
IF(XL.EQ.X1) GO TO 6
5 X=XL+DX
Y=VL
RETURN
6 CALL FUNC(X1,V,H)
IF(V.GT.VL) GO TO 5
X=X1
Y=V
RETURN
7 IF(XR.EQ.X2) GO TO 9
8 X=XR-DX
Y=VR
RETURN
9 CALL FUNC(X2,V,H)
IF(V.GT.VR) GO TO 8
X=X2
Y=V
RETURN
END
```

FIBO 10
FIBO 20
FIBO 30
FIBO 40
FIBO 50
FIBO 60
FIBO 70
FIBO 80
FIBO 90
FIBO 100
FIBO 110
FIBO 120
FIBO 130
FIBO 140
FIBO 150
FIBO 160
FIBO 170
FIBO 180
FIBO 190
FIBO 200
FIBO 210
FIBO 220
FIBO 230
FIBO 240
FIBO 250
FIBO 260
FIBO 270
FIBO 280
FIBO 290
FIBO 300
FIBO 310
FIBO 320
FIBO 330
FIBO 340
FIBO 350
FIBO 360
FIBO 370
FIBO 380
FIBO 390
FIBO 400
FIBO 410
FIBO 420
FIBO 430
FIBO 440
FIBO 450
FIBO 460

```
SUBROUTINE FUNC(P LOG1,RMLOG,H)
IMPLICIT REAL*8(A-H,O-Z)
REAL*8 MSE
DIMENSION WHAT(20),E(20),X(20),R(5),W(5)
COMMON /A/ R,W
N = 5
RATIO1 = 10** (RLOG1)
DO 3 I=1,N
EX=2.*H*((2.+RATIO1)/3.)*0.33333333)
EXSQ=EX*EX
XL=DSQRT(R(I)*R(I)+EXSQ)
XL3=XL*XL*XL
XL5=XL*XL*XL*XL*XL
XTWO=(1./XL)+(EXSQ/(2.*XL3))+((3.*EXSQ*EXSQ)/(2.*XL5))
3 X(I)=(1./R(I))+(RATIO1-1.)*XTWO
SMXW=0.0
SMXSQ=0.0
SMESQ=0.0
DO 4 I=1,N
SMXW=SMXW+X(I)*W(I)
4 SMXSQ=SMXSQ+X(I)*X(I)
B=SMXW/SMXSQ
DO 5 I=1,N
E(I)=W(I)-(B*X(I))
5 SMESQ=SMESQ+E(I)*E(I)
MSE=SMESQ/N
RMSE = DSQRT(MSE)
RMLOG = DLOG10(RMSE)
RETURN
END
```

	FUNC	10
	FUNC	20
	FUNC	30
	FUNC	40
	FUNC	50
	FUNC	60
	FUNC	70
	FUNC	80
	FUNC	90
	FUNC	100
	FUNC	110
	FUNC	120
	FUNC	130
	FUNC	140
	FUNC	150
	FUNC	160
	FUNC	170
	FUNC	180
	FUNC	190
	FUNC	200
	FUNC	210
	FUNC	220
	FUNC	230
	FUNC	240
	FUNC	250
	FUNC	260
	FUNC	270
	FUNC	280
	FUNC	290
	FUNC	300

SUBROUTINE ANS(RLOG1,E1,E2,H)	ANS(10
IMPLICIT REAL*8(A-H,O-Z)	ANS(20
DIMENSION RAT1(30),WHAT(20),E(20),X(20)	ANS(30
DIMENSION R(5),W(5)	ANS(40
COMMON /A/ R,W	ANS(50
N = 5	ANS(60
DO 27 I=1,N	ANS(70
RATIO1 = 10**RLOG1	ANS(80
EX=2.*H*((12.+RATIO1)/3.)*0.3333333	ANS(90
EXSQ = EX*EX	ANS(100
XL = DSQRT(R(I)*R(I)+EXSQ)	ANS(110
XL3=XL*XL*XL	ANS(120
XL5=XL*XL*XL*XL*XL	ANS(130
XTWO=(1./XL)+(EXSQ/(2.*XL3))+(3.*EXSQ*EXSQ)/(2.*XL5)	ANS(140
27 X(I)=(1./R(I))+(RATIO1 -1.)*XTWO	ANS(150
SMXW=0.0	ANS(160
SMXSQ=0.0	ANS(170
SMESQ=0.0	ANS(180
DO 28 I=1,N	ANS(190
SMXW=SMXW+X(I)*W(I)	ANS(200
28 SMXSQ=SMXSQ+X(I)*X(I)	ANS(210
B=SMXW/SMXSQ	ANS(220
DO 29 I=1,N	ANS(230
WHAT(I)= B*X(I)	ANS(240
29 E(I)=W(I)-WHAT(I)	ANS(250
E1=238.73241 *(1./B)	ANS(260
E2 = E1/RATIO1	ANS(270
RETURN	ANS(280
END	ANS(290

SUBROUTINE VARI(RMSE,RMSE1,CNT,KNT,ISW)	VARI 10
IMPLICIT REAL*8(A-H,O-Z)	VARI 20
DIMENSION SUM(50),SSQ(50),RMSE(200),RMSE1(200)	VARI 30
INTEGER CNT	VARI 40
DO 1 L=1,50	VARI 50
SSQ(L) = 0.0	VARI 60
1 SUM(L) = 0.0	VARI 70
KSET = 1	VARI 80
DO 11 I=1,KNT	VARI 90
SUM(KSET) = SUM(KSET) + RMSE(I)	VARI 100
SSQ(KSET) = SSQ(KSET) + (RMSE(I)**2)	VARI 110
11 CONTINUE	VARI 120
KSET = KSET + 1	VARI 130
DO 2 I=1,CNT	VARI 140
SUM(KSET) = SUM(KSET) + RMSE1(I)	VARI 150
SSQ(KSET) = SSQ(KSET) + (RMSE1(I)**2)	VARI 160
2 CONTINUE	VARI 170
TSUM = SUM(1) + SUM(2)	VARI 180
TSSQ = SSQ(1) + SSQ(2)	VARI 190
N = KNT + CNT	VARI 200
TSS = TSSQ - ((TSUM**2.)/N)	VARI 210
SSQSET = ((SUM(1)**2.)/KNT) + ((SUM(2)**2.)/CNT)	VARI 220
SSBS = SSQSET - ((TSUM**2.)/N)	VARI 230
SSWS = TSS - SSBS	VARI 240
IDFWS = N - KSET	VARI 250
IDFBS = KSET - 1	VARI 260
AMSBS = SSBS/IDFBS	VARI 270
AMWS = SSWS/IDFWS	VARI 280
F = AMSBS/AMWS	VARI 290
CALL SIGNIF(F,IDFWS,ISW)	VARI 300
RETURN	VARI 310
END	VARI 320

```
SUBROUTINE SIGNIF(F, IDFWS, ISW)                                SIGN 10
  IMPLICIT REAL*8(A-H, O-Z)                                    SIGN 20
  DIMENSION FDIST(30)                                          SIGN 30
  DATA FDIST/39.864D0, 8.5263D0, 5.5383D0, 4.5448D0, 4.0604D0,  SIGN 40
  *3.7760D0, 3.5894D0, 3.4579D0, 3.3603D0, 3.2850D0, 3.2252D0, 3.1765D0,  SIGN 50
  *3.1362D0, 3.1022D0, 3.0732D0, 3.0481D0, 3.0262D0, 3.007D0, 2.9899D0,  SIGN 60
  *2.9747D0, 2.9609D0, 2.9486D0, 2.9374D0, 2.9271D0, 2.9177D0, 2.9091D0,  SIGN 70
  *2.9012D0, 2.8939D0, 2.8871D0, 2.8807D0/                    SIGN 80
  ISW = 0                                                       SIGN 90
  IF(IDFWS .GE. 31) GO TO 2                                     SIGN 100
  IF(F .GE. FDIST(IDFWS)) ISW = 1                               SIGN 110
  RETURN                                                         SIGN 120
2 IF(IDFWS .GE. 40) GO TO 3                                     SIGN 130
  IF(F .GE. 2.8807) ISW = 1                                     SIGN 140
  RETURN                                                         SIGN 150
3 IF(IDFWS .GE. 60) GO TO 4                                     SIGN 160
  IF(F .GE. 2.8354) ISW = 1                                     SIGN 170
  RETURN                                                         SIGN 180
4 IF(IDFWS .GE. 120) GO TO 5                                   SIGN 190
  IF(F .GE. 2.7914) ISW = 1                                     SIGN 200
  RETURN                                                         SIGN 210
5 IF(F .GE. 2.7478) ISW = 1                                     SIGN 220
  RETURN                                                         SIGN 230
  END                                                            SIGN 240
```

