PROCEDURES FOR ESTIMATING THE TOTAL LOAD EXPERIENCE OF A HIGHWAY AS CONTRIBUTED BY CARGO VEHICLES

by

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ABSTRACT

The primary purpose of this research study was to develop and test procedures for making accurate estimates of the total load in terms of 18-kip axle equivalents that a highway will experience from cargo vehicles over its design period. Such an endeavor involved an evaluation of vehicle weight and classification count data previously collected at existing loadometer and manual count stations located throughout the State of Texas.

Two procedures were used to make estimates of the actual total 18-kip axle equivalents generated by cargo vehicles weighed at each of the 21 conventional static weight loadometer stations during 1967 and 1964-68. One procedure used multiple regression models in which the "dummy" variables represent various characteristics of the vehicles weighed. The sets of variables entered into the models included vehicle type, body type, fuel type, time of weighing (night, day of week, summer and year) and load status. The other procedure used axle weight frequency distribution sets composed of one-kip (1000-pound) weight classes, 40 for single axles and 50 for tandem axles. The frequency sets developed were as follows: (1) Combined stations, (2) Combined stations by vehicle type, (3) Combined stations by fuel type, (4) Combined stations by load status, (5) Combined stations by highway system and vehicle type and, (6) Combined stations by highway system. Frequency Set *5* proved to be the most accurate. In fact, it was more accurate than the regression models.

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Another purpose of this research study was to test the adequacy of previously collected vehicle weight and count samples at the various loadometer stations. These samples were tested for representativeness of the vehicle traffic and reliableness of statistics generated therefrom. To determine the above, the weighing and counting schedules and sample sizes were evaluated. Also, conventional loadometer station data were compared with limited weigh-in-motion station data. The weight and count sample size requirements were established through the use of a statistical formula which utilizes sample averages and variances with 10 percent error and 95 percent probability level criteria.

It was found that a considerable amount of station to station variation in the sample statistics was due to differences in the weighing or counting schedules and sample sizes. Combining stations and/or years made the data more representative and increased the reliability of the sample statistics.

SUMMARY OF FINDINGS

This report presents procedures and findings which relate primarily to estimating the total load experience (measured in 18 kip axle equivalents) of an existing or future highway over its design life through the use of adequate cargo vehicle weight and annual average daily traffic (AADT) count data. Such estimates are needed as considerations in highway design. The most important findings of this research effort are summarized here.

An analysis of vehicle and axle weight distributions developed from previously collected loadometer data gave the following results:

- 1. Significant differences exist between most of the station and highway system averages within vehicle type. Even the grouping of stations according to highway system failed to produce homogeneous weight distributions. Various geographical groupings of stations also showed significant differences.
- 2. Much of the station to station or system to system variation is due to changes in the proportion of loaded and empty tandem axle vehicles. Such proportions change with vehicle and body types.

An analysis to determine the adequacy of cargo vehicle and axle weight samples taken at loadometer stations during the past few years gave the following results:

1. Part of the station to station variation in the averages of vehicle and axle weights is due to differences in the weighing schedule. Additional between station variation is

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due to small samples which are susceptible to greater chance differences. Therefore, samples from the 21 stations combined produced a more accurate estimate of the true population variance than samples from only one station. However, if continuous seven-day weighing periods for every season of the year were used, the number of stations might be reduced drastically.

- 2. The number of vehicles weighed in 1967 at all 21 stations combined was more than enough to produce feliable averages of vehicle 18-kip axle equivalents. The same was true for the combined stations of the interstate highway (IH) system, but the reverse was true for those of the other systems. Therefore, the number of weighings of certain vehicle types could be reduced, especially those at stations on the IH system.
- 3. Considerably more vehicles must be weighed to obtain accurate average vehicle weights in 18-kip axle equivalents than to obtain accurate average axle weights fn 18-kip axle equivalents.

An analysis to determine the adequacy of cargo vehicle manual classification count samples taken at.loadometer stations during the past few years gave the following results:

1. Considerable variation in the averages and variances of 24-hour volume counts for five-axle semitrailer vehicles occurred at individual stations. Contributing to this variation is the time of counting, the length of counting periods and the number of 24-hour volume counts.

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- 2. Considerable variation in the averages and variances of 24 hour volume counts for five-axle semitrailer vehicles occurred from station *to* station. Consequently, the number of counts necessary varied extensively between stations. However, only a few stations required a larger number of counts.
- 3. The number of 24-hour volume counts necessary to collect at each station varied widely between vehicle types.
- 4. Within-year and between-year fluctuations in the estimated base AADT count for each vehicle type are much less when based on three 24-hour counts per year for four years than when based on only one 24-hour count per year.
- 5. Of four methods used to estimate the AADT count of five-axle semitrailers at a station, those employing only 24-hour volume counts of this vehicle type in the calculations showed the least within-year and between-year fluctuations.

An analysis of loadometer data to develop and test procedures for use in estimating each loadometer station's total load experience measured in 18-kip axle equivalents produced the following results:

- 1. Of five sets of axle weight frequency distributions Set *5* {based on data classified according to highway system and vehicle type) produced the most accurate station estimates.
- 2. Of two multiple regression models, Model 2 {based on sets of "dunnny" variables) produced the more accurate station estimates.

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- 3. Of the two estimating procedures, the axle weight frequency distributions of Set 5 produced the more accurate station estimates.
- 4. Neither of the above estimating procedures produced station estimates which were within 10 percent of the actual value for every station.
- 5. The multi-year (1964-68) loadometer data produced more accurate estimates of total 18*kip axle equivalents at each station than did the one-year (1967) data, thus removing some of the differences due to sample size and weighing schedule.

These findings do not fully satisfy the requirements of all the objectives. For instance, more weigh-in-motion loadometer data need to be collected before Objective 2 (see list in introductory section) can be properly researched. Findings based on additional data from this source could affect the results presented here for the other three objectives.

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IMPLEMENTATION STATEMENT

The research findings reported here can be used immediately by the various government agencies responsible for designing and maintaining highways. They can be applied to the loadometer weight and manual classification count data to make more accurate estimates of a highway's total load experience, measured in 18-kip axle equivalents. A proposed new procedure using multiple regression models was evaluated and rejected in favor of a more accurate conventional procedure using axle weight frequency distribution sets. Among the frequency distribution sets developed, Set *5* is recommended for use in estimating a station's total 18-kip axle equivalents. This set was generated from multi-year (1964-68) loadometer data by classifying the 21 stations according to highway system and vehicle type. The applicable percentage frequencies for Set *5* are presented in Appendix A of the report.

Then, to arrive at an estimated annual average daily traffic (AADT) count of each vehicle type for the base year of a highway, it was concluded that at least several 24-hour volume counts per year for three or four years should be used. Of the methods used in making AADT count estimates for the cargo vehicle types, Method 2 is recommended.

Further research is recommended to determine true station to station differences in vehicle type weights and counts. The type of data which will probably aid most in this determination should be that collected at several weigh-in-motion stations on each highway system over continuous seven=day a week weighing periods during each season of

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the year. The resulting percentage axle weight frequency distributions and estimated based year vehicle type AADT's would probably be more representative of the stream of cargo vehicle traffic and generate more reliable weight and count statistics than have been generated in the past. The number of weighing stations needed also could be . determined more accurately.

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INTRODUCTION

In September 1967, the Texas Transportation Institute in cooperation with the Texas Highway Department and the U. S. Department of Transportation, began a study entitled "Studies of Truck Characteristics Relating to Highway Use and Taxation in Texas".

During the first year of the study, research efforts were concentrated on the first two of six objectives which dealt with determining whether Texas cargo vehicles of various types and weight classes were being equitably taxed (fuel imposts plus licenses and fees) in relation to their highway use. The findings of this research endeavor were published in May 1968, as Research Report 131-1, entitled "Fuel Tax Differentials of Texas Cargo Vehicles".

During the last two years of the study, research efforts have been concerned with the four remaining objectives which are as follows:

- 1. To determine the frequency distributions of axle weights by cargo vehicle classes on various highway systems, to compare these data with total loadometer data and to derive associated highway use and taxation inferences.
- 2. To analyze the potential of the weigh-in-motion station in Austin as a tool for simplifying data development.
- 3. To test the adequacy of samples at various count and loadometer stations.
- 4. To develop and test techniques for loadometer data reduction and analysis.

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A partial report of findings from these research efforts was submitted to the sponsors in an unpublished interim report entitled ''Procedures for Estimating Cargo Vehicle 18-Kip Axle Equivalents and Determining the Adequacy of Loadometer and Count Station Samples", dated August 21, 1969.

Problem Statement

Officials of the Highway Planning Survey Division of the Texas Highway Department (THD) are constantly striving to upgrade their data collection and analytical procedures so that they can furnish the other divisions and districts within that organization more accurate projections necessary for optimum highway engineering and highway economy. The four objectives mentioned previously indicate areas which currently need immediate attention. Other related problems can be explored at a later date,

Cargo vehicles make up only a small proportion of the total traffic stream, but they account for a very large percentage of the total load experience of the public roads of Texas. The collection of adequate vehicle weight and volume samples and identification of critical cargo vehicle characteristics are necessary for making accurate estimates of the actual load experience of a particular road in a given time period,

Underestimating the load experience of a proposed highway would result in an underdesigned facility having a shorter physical life than planned for. Thus, road replacement and repairs (resulting in additional costs) would be needed much sooner than expected,

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The objectives of this research effort do not require an analysis on,the basis of costs. Instead, they call for the development of a procedure which will provide reasonable assurances that estimates of the load experience are kept within the 10 percent margin of error requested by the THD. Acceptance of such a margin of error is admission that it is very difficult to make extremely accurate estimates of the actual load experience of a facility using historical data.

Scope of Study

The study is limited to an analysis of data collected at loadometer and count stations in recent years. It is not designed to determine the representativeness of loadometer and count stations in measuring the vehicle weight and number frequencies on the various highways in the State. Further, the study is limited to an analysis of the heavier cargo vehicles (excluding the 2-axle 4-tire vehicles) which greatly influence the weight bearing design of proposed highways. Last, the vehicle weight estimates in terms of 18-kip axle equivalents are applicable only to flexible pavement. However, the same techniques developed in this study can be used in making estimates that apply to rigid pavement.

Source of Data

The study is based on data collected by the Highway Planning Division of the THD at its loadometer and manual classification count stations located throughout Texas.

Cargo vehicles are weighed at the 21 conventional loadometer stations and one weigh-in-motion station shown in Figure 1. Nineteen of the

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conventional stations are located in rural areas and two in urban areas. On a highway system basis, the 19 conventional rural stations are located as follows: Nine along Interstate routes, 10 along U. S. Numbered routes and one along a State Numbered route. One of the urban stations is located along a U. S. Numbered route and the other along a Farm-to-Market route.

During March 1969, the THD began operating a new portable weigh-inmotion scale at a rural location along Interstate Highway 35 just south of Austin, and it is labeled as Station 35-2 in Figure 1. Thus far, weighings have been made at this station over one continuous seven (24 hour) day period. These weighings were limited to vehicles using the outside lane.

Since the initial weighings, the weighing device has had to undergo necessary changes to permit easier monitoring when it is in I operation. Therefore, it was not possible to obtain as much data as was expected for use in this study.

For several years, the THD has been taking vehicle classification counts at approximately 188 manual count stations, 21 of which are the permanent loadometer stations mentioned above.

The classification count stations are located primarily in rural areas along Farm-to-Market roads, State highways, U. s. Numbered highways and Interstate highways. About 55 percent of these stations are located at intersections of the above mentioned roads and highways, allowing separate counts to be made on each type of road involved. Thus, about 300 separate road counts can be taken rather than one for each of the 188 stations. About 37 percent of the 300 separate road counts are two directional.

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The special weigh-in-motion loadometer station (35-2') provided a continuous seven-day classification count of all vehicles by axle configuration during one week in March 1969.

Automatic recording stations are located at or near the loadometer and manual count stations to give accurate annual average daily traffic (AADT) count data. These data were used to a limited extent in the analyses in this study.

Division of Study

The study procedures and results of analyses are dealt with under four major headings as follows: (i) Estimating the total load experience of a highway; (2) Determining the adequacy of cargo vehicle weight and classifica $+$ tion count samples; (3) Reducing and analyzing loadometer data; and (4) Appendices.

ESTIMATING THE TOTAL LOAD EXPERIENCE OF A HIGHWAY

Procedures

Estimates of the total weight in 18-kip axle equivalents generated by cargo vehicles on a given day at a loadometer station can be accomplished by applying several different procedures. In this study, two procedures were used: (1) axle weight frequency distributions and (2) multiple regression models.

These two procedures are first summarized, then the results of the two procedures are presented and compared.

Axle Weight Frequency Distributions

The wheel weights obtained at loadometer stations are combined into single and tandem axle weishts by AASHO recommendations (1).

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The two groups of axles are divided into weight classes of 1,000 pound increments to obtain frequency distributions of axle weights for a station or group of stations. This frequency distribution, expressed in percentages, is used as an estimate of the mixed traffic load and is projected over the design life of a road section to obtain the total design load experience of a highway (2).

The percentage axle weight frequency distributions may be applied to road locations with only truck traffic estimates if one may assume that the percentage axle weight frequency distribution is similar to a particular known frequency distribution.

Each loadometer station has a unique frequency distribution so that some method of selection is necessary. If the nearest loadometer station is selected, an assumption is made of a geographical traffic characteristic. Stations with some common characteristic may be grouped. Three assumptions which were investigated by Heathington and Tutt (3) were as follows:

1. Grouping stations by percent of trucks

2. Grouping stations by highway system

3. Grouping stations by statewide area

Estimations of 18-kip axle equivalents at three selected locations yielded estimating errors from seven to fifty percent. Grouping stations by highway systems evidently gave some improvement over statewide averages, but no data were presented that nearness of geographical location improved prediction.

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At the present time, three years of data are used by the THD to help overcome sampling fluctuations in the preparation of data from each loadometer station as follows:

- 1. Weight data for the three most recent years are used.
- 2. The data are tabled by single axles and tandem sets, by vehicle type and weight group.
- 3. Average daily traffic (ADT) counts by vehicle type for the the three most recent years are used.
- 4. The number of single axles and tandem sets for each vehicle type is calculated.
- *5.* The table produced by the weight data {Step 2) is prorated by the counted data (Step 4).
- 6. All single axles are combined by weight group, and all tandem sets are combined by weight group.
- 7. The number of axles in each weight group is shown as a percent of the total.
- 8. This table of percentages is then used as the basic weight data.

The loadometer station axle weight frequencies are made one time each year as new data become available.

When a load experience estimate is requested for highway design purposes, the following steps are used in making this estimate:

- 1. The ADT and percent trucks for the highway section in question is developed from representative automatic traffic recorder and manual count stations.
- 2. The axle factor (converting number of trucks to axles) and percent single axles are developed,

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- 3. The most representative basic weight table is selected.
- 4. The percent of single axles of the highway section in question is used to prorate the percentages in the basic weight table.
- 5. The total number of axles for the highway section in question is multiplied by the percent of axles in each weight group and by the 18-kip axle equivalency factor for each weight group. The product is accumulated.
- 6. The total accumulation is multiplied by the number of days in the design period.

Step 3 (the selection of the basic weight table) is the most critical. *^A*poor selection can result in large errors in the estimation of 18 kip axle equivalents used in pavement design. Therefore, an attempt is made in the present study to explore several sets of single and tandem axle weight frequency distributions in order to determine which set would produce the most accurate estimates. The steps in the analytical process leading to this determination are as follows:

- 1. Decide which axle weight frequency distributions should be explored.
- 2. Generate frequency charts and averages, variances, standard deviations and standard errors for each of the selected axle weight frequency distributions.
- 3. Perform visual and statistical analyses to determine the extent of differences between various axle weight frequency distributions.
- 4. Select alternative sets of axle weight frequency distributions to transform into percentage frequency distributions for making estimates of total axle weights at a location.

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- 5. Compute estimates of each station's total axle weights in 18-kip axle equivalents.
- 6. Compute each station's actual total axle weights in 18-kip axle equivalents.
- 7. Determine which set of axle weight percentage frequency distributions produced the most accurate axle weight estimates for each station.

When using axle weight frequency distributions to estimate a location's total axle weight in 18-kip axle equivalents for a design period, two assumptions are made:

- 1. The axle weight distribution will remain constant over the design petiod.
- 2. The A&SHO Road Test equations for generation of equivalency factors are applicable to Texas. conditions over the design period.

Multiple Regression Models

An alternative to the above procedure is to develop from loadometer α data a multiple regression model capable of making estimates of total vehicle weights in 18-kip axle equivalents at a particular location.

The specific sequence in this research effort is as follows:

- 1. Generate 18-kip axle equivalents on a per vehicle basis for data to be used in developing model.
- 2. Generate frequency charts for visual inspection of the shape of the distribution of vehicle 18-kip axle equivalents.

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- 3. If needed, convert the 18-kip axle equivalents per vehicle to logarithms (log-kip axle equivalents).
- 4. Generate frequency charts for visual inspection of the distribution of vehicle log-kip axle equivalents.
- 5. Compute averages, variances, standard deviations and standard errors for selected distributions of 18-kip and log-kip axle equivalents.
- 6. Test for significant variation between the averages of selected distributions of log-kip axle equivalents.
- 7. Select vehicle characteristics to be considered as independent variables in the regression model.
- *B.* Measure the change in 18-kip axle equivalents between vehicles with the multiple regression technique.
- 9. Estimate the total 18-kip axle equivalents generated by cargo vehicles weighed at each loadometer station using the resultant coefficients of the regression model.
- 10. Compare the actual and estimated station totals to determine the level of accuracy achieved.

Concerning Step 1, it has already been noted that the THD applies a commonly used procedure to calculate total 18-kip axle equivalents which separates the single and tandem axles of all cargo vehicles and then makes a frequency distribution of the axles by one-kip weight groups which are multiplied by corresponding equivalency factors. This method has the advantage of simplicity. However, some accuracy

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may be sacrificed in obtaining the actual total 18-kip axle equivalents for a station. In contrast, the procedure used here calculates the 18-kip axle equivalents directly; for example, for an 8,200 pound axle (coded to the nearest 200 pounds at the weighing station) by using the AASHO Road Test equivalency equations (4). This is done for each axle on a vehicle, and the results are totaled to obtain the number of 18-kip axle equivalents per vehicle. This procedure allows the study of the $18-$ kip axle equivalents across vehicle types without having to adjust for differing numbers and types of axles per vehicle.

In regard to Step 3, it was anticipated that the frequency distributions of 18-kip axle equivalents would be highly skewed to the right. If so, a logarithmic transformation would be desirable for use in statistical testing and possibly model building. Therefore, the computer program was altered to generate both 18-kip and log-kip axle equivalents.

The variables selected for the multiple regression model use the numbers of weighed vehicles with specific characteristics; for example, a 3-82 axle configuration, tank body, user of diesel fuel, weighed at night and weighed on Thursday, A model employing only vehicle characteristics either presently available or obtainable at manual count stations is considered highly desirable. The model generates estimates (coefficients) for each vehicle characteristic obtained visually at the count stations.

The independent variables are of the discrete type, that is, not conventionally measured on a numerical scale. They are also called

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"dummy" variables. According to Suits, who has worked with dunnny variables, "the dummy variable is a simple and useful method of introducing into regression analysis information contained in variables that are not conventionally measured on a numerical scale, e.g., race, sex, region, occupation, etc." (5). In this respect, dummy variables are ideally suited for analyzing loadometer data. Recently, Kentucky researchers used dummy variables on loadometer data to determine traffic parameters for the prediction, projection and computation of equivalent wheel loads (6).

The model assumes a linear additive relationship between the number of 18-kip axle equivalents (dependent variable) and the numbers of vehicles with certain characteristics (independent variables). Actually, when using dummy independent variables, the above assumption is not needed. In fact, Mr. Suits concluded that "by partitioning the scale of a conventionally measured variable into intervals and defining a set of dummy variables on them, we obtain unbiased estimates since the regression coefficients of the dummy variables conform to any curvature that is present" (5). *A* similar conclusion was reached by Ferber (7). This is one reason why the number of 18-kip axle equivalents, instead of log-kip axle equivalents, was chosen for the dependent variable.

Using the resultant predictive model to estimate the total 18 kip axle equivalents that might be experienced at some location over a design period of say 20 years involves making additional assumptions which are as follows:

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- 1. AASHO Road Test equations for generation of equivalency factors are applicable to Texas conditions over the design period.
- 2. The average axle load of each type of vehicle identified in a model will remain constant over the design period;
- 3. The total 18-kip axle equivalents will change by the same percentage rate as the annual ADT predicted for cargo vehicles over the design petiod.
- 4. The 18-kip axle equivalents generated by automobiles and 2 axle 4-tire pickups and panel trucks may be predicted using passenger car ADT projections.
- 5. If the average axle loads of each type of vehicle do not remain constant, it is assumed that the total 18-kip axle equivalents generated by the cargo vehicles will remain in the same proportion to the predicted ADT of cargo vehicles. (This assumption means that if, for instance, the legal vehicle weight limit is raised, then the number of vehicles required to move the cargo would be reduced so that the total 18-kip axle equivalents would grow at the same rate as predicted.)

Results

The results obtained from the application of actual loadometer data to the above procedures are presented and discussed here. The most significant results deal with the comparison of estimates generated from the two alternative procedures.

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Axle Weight Frequency Distributions

For exploratory and testing purposes, charts of single and tandem axle weight frequency distributions in kips and the corresponding averages, variances, standard deviations and standard errors were generated from vehicle weighings at the 21 loadometer stations during the 1966-68 period. This period was selected because it contained the latest available data and a workable number of observations with which to. generate the many initial frequencies necessary for test purposes.

The principal group of frequency distributions generated and evaluated was tpat of the vehicle type frequencies for individual loadometer stations. Other groups generated and evaluated on a combined 21 station basis are as follows: By vehicle type; By axle location, overall and by vehicle type; By load characteristic; overall and by vehicle type; By year of weighing; By summer of weighing; and By urban or rural location. In addition to these distributions, three highway system frequency distributions were computed on the basis of vehicle type.

A visual study of all frequency charts revealed that single and tandem axle weight frequency distributions can be divided according to the following shapes:

1. One peak - empty single and tandem axles.

2. One peak and skewed to right - loaded single axles.

3. One peak and skewed to left - loaded tandem axles.

4. Two peaks - tandem axles (combined loaded and empty).

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Figure 2 shows the double peaked distribution of tandem axles (combined loaded and empty). The primary cause of a double peaked distribution is the presence of both empty and loaded tandem axles. Thus, to the extent that the proportion of loaded and empty tandem axle vehicles varies from station to station or system to system (as seen in Appendix A); one can expect a similar variation in the visual shapes and, hence, in the axle weight averages.

Appropriate statistical tests, such as Student's t and analysis of variance (ANOV), revealed that there is a significant difference between the averages of the following single and tandem axle weight frequency distributions:

- 1. Overall average versus individual averages of 21 stations (by ANOV).
- 2. Overall rural station average versus individual averages of all rural stations (by ANOV).
- 3. Overall Interstate Highway (IH) average versus individual averages of each IH station (by ANOV).
- 4. Overall rural station average versus urban station average (by $t-test$).
- 5. Overall average of all IH stations versus all other rural stations (by t-test).
- 6. Overall average of any one major vehicle type versus another. except for single axles of vehicle type 2-Sl-2 versus those of the 3-Sl-2 and for tandem axles of the vehicle type 3 axle single unit versus those of the 2-82 (by t-test).

Figure 2. Chart showing tandem axle weight frequency distribution in kips for cargo vehicles studied from 1966-68 weighings at 21 loadometer stations.

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- 7. Overall average of each major vehicle type of all IH stations versus the average of the same vehicle type of all other rural stations (by t-test).
- 8. Overall average of each major vehicle type of rural stations versus the average of the same vehicle type of urban stations, except the 3-axle single unit and the 2-S2 single axles (by t-test).
- 9. Overall average of one major vehicle type versus another.of each axle location (by t-test).
- 10. Overall average of one major vehicle type versus another for empty and loaded axles, except for the 2-Sl-2 versus 3-Sl-2 single empty axles (by t-test).

To summarize, the results of the above statistical tests indicate that, with few exceptions, the major vehicle type distributlons for single and tandem axles cannot be combined without giving up some accuracy in estimating the total axle weights in 18-kip equivalents at a particular station. Also, combining the stations by highway system produces unlike groups, but the vehicle type axle weight distributions are also heterogeneous between stations in each group. Stations grouped geographically yield essentially the same results.

In an attempt to determine just how accurate combined station weight frequency distribution sets would be in making weight estimates at individual stations, five diverse sets were chosen. These alternative sets of single and tandem axle weight frequency distributions were used in estimating total axle weights in 18-kip axle equivalents at individual stations. The number of individual frequency distributions and the

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number of stations required for each set are presented in Table 1. Set 1 requires only two frequency distributions, whereas Set 5 requires 63 separate frequency distributions. Whether Set 5's estimates are more accurate than Set l's will be shown shortly. Economically speaking, the less complex sets are more desirable, especially if very little accuracy in the estimates is sacrificed.

For estimating purposes, the trial axle weight frequency distributions of Sets 1-5, as identified in Table 1, were generated from 1967 loadometer data. The amount of data used to develop these distributions was reduced in order to save in computer costs. The single axle distributions are made up of 40 one-kip weight classes and those for tandem axles are composed of 50 one-kip classes. The midpoints of these classes are located at each full kip.

The above weight frequency distributions were transformed into the corresponding percentage frequency distributions. Such percentage frequency distributions were applied to the total number of single and tandem axle sets of each vehicle type weighed at a station in order to determine the number of axle sets in each weight class. Next, the total number of 18-kip axle equivalents were generated for each weight class by multiplying the flexible pavement 18-kip axle equivalency factor (for midpoint of weight class) by the number of axle sets in the weight class. Then the weight class totals were summed to obtain the estimated total number of 18-kip axle equivalents for each station. Also, the same procedure was used to calculate the actual total number of 18-kip axle equivalents for each station to determine how much accuracy was achieved.

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Alternative Sets of Single and Tandem Axle Weight Frequency Distributions for Use in Estimating Ax1e Weights in 18-Kip Axle Equivalents at Stations

lvehicle types as determined by the axle configurations.

2Two fuel types, diesel and other.

3Two load characteristics, loaded and empty.

Table 2 shows the absolute and percentage estimating errors produced by the frequency distribution sets in estimating each station's actual total 18-kip axle equivalents. It appears that Set *5* produced the most accurate estimates, as it had the lowest average absolute and percentage errors of the five sets. It also had the fewest stations with percentage errors over 10 percent. However, it is also the most complex set.

To determine how much historical data should be used in making station estimates, loadometer data collected during the 1964-68 period were combined to generate new percentage axle weight frequency distributions for not only Set *5* but also Sets 1 and 2. In addition, another set (called Set 6) was generated. This set is the same as Set 5, except it is not broken down according to vehicle type.

Table 3 shows the absolute and percentage estimating errors produced by each of the new frequency distribution sets. Again, Set *5* had the lowest average errors of the four sets. Also, when comparing the average errors of Tables 2 and 3, it can be seen that the frequency distributions generated by multi-year loadometer data produced more accurate station estimates than did those generated by one-year data.

Multiple Regression Models

As was done in the previous analysis, the initial multiple regression models were developed from the loadometer data of cargo vehicles weighed during 1967.

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Absolute and Percentage Estimating Errors of Five Different Axle Weight Frequency Distribution Sets Used to Estimate the Total Axle Weights in 18-Kip Axle Equivalents Generated by Texas Cargo Vehicles Weighed at Each Loadometer Station in 1967

lThese frequency distribution sets are those described in Table 1.

2Based on 1,000 pound (midpoint) groupings for application of the equivalency factors. 3The signs of the errors were ignored.

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Absolute and Percentage Estimating Errors of Four Different Axle Weight Frequency Distribution Sets Used to Estimate the Total Axle Weights in 18-Kip Axle Equivalents Generated by Texas Cargo Vehicles Weighed at Each Loadometer Station During 1964-68

1Based on 1000 pound (midpoint) groupings for application of equivalency factors.

 $^{\rm 2}$ Set 6 is composed of all data grouped according to highway system. The other sets are described in Table 1.

3The signs of the errors were ignored.

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The initial charts of the 18-kip axle equivalents per vehicle frequency distributions confirmed the hypothesis that such frequency distributions were skewed to the right. Thus, the original data were transformed to log-kip equivalents. As a result, the charts of log-kip axle equivalents (18-kip axle equivalents plotted on a logarithmic scale) frequency distributions had shapes approaching normality except for being bimodal. For example, the overall distribution is shown in Figure 3. As was the case with tandem axle (loaded and empty combined) weight frequency distributions, the bimodal characteristic of the above distribution is due to the presence of both loaded and empty vehicles in the same distribution. The loaded vehicles represent about 64 percent of the vehicles in the combined distribution.

Depending on the degree of load, frequency distributions of the log-kip axle equivalents per vehicle for the several vehicle and body types varied from having distinct double peaks to having weak single peaks. For instance, the single unit vehicle types showed what might be loosely defined as a single peaked distribution whereas the 3-82 tank type showed a distinct double peak. On the other hand, the combined van (excluding insulated van) and panel body types showed weak double peaks, regardless of vehicle type. Charts of some of these frequency distributions are presented in Appendix B.

Based on the above observations, it is evident that individual loadometer stations have varying shaped frequency distributions of log-kip axle equivalents depending on the proportion of loaded or partially loaded to empty vehicles weighed. This loaded to empty vehicle proportion varied widely from station to station, even within

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vehicle types, and is the source of much of the variation in log•kip axle equivalents between vehicles. This was verified by running an AmV test on the 3-82 vehicle type. Eveh when 3-82 vehicles were separated into loaded and empty groups, the tests still revealed statistically significant between station variation. An inspection of the averages of the station log-kip axle equivalents per vehicle for the other vehicle types indicated that about the same results would have been obtained for those vehicle types. Therefore, no further tests of significance of this kind were made.

Since neither a geographical nor a definite highway system pattern of variation in the per vehicle log-kip axle equivalents between stations could be identified; attention was directed toward using the multiple regression technique on the combined 21 station data to isolate and quantify significant sources of variation between the individually weighed vehicles. It was pointed out earlier that "dummy" variables provide an easy way of quantifying the many qualitative variables available for analyzing loadometer data.

The sets of ''dunmy" independent variables (characteristics of vehicles weighed) introduced into one or more of the linear muitiple regression models are shown in Table 4.

In order that each model be determinant, no single variable or combination of variables could include all the weighed vehicles introduced into the analysis. To accomodate this, the following · characteristics of each "dwmny" set named in Table 4 were not expressed as independent variables: Day, Other fuel, Miscellaneous body, Friday, Miscellaneous vehicle type and Empty vehicles. For example,

 $-26-$

Sets of Dummy Independent Variables Introduced Into the Linear Multiple Regression Models

 $^{\rm 1}$ The groupings of the THD classifications to form the above body type variables are given in Appendix A.

if a vehicle was weighed at night, it was coded a 1; whereas a day weighing was given a 0. All of the vehicles coded 0 are accounted for in the constant term (a) in the model equation. Also, vehicles of nonsignificant variables are averaged in the constant term. Therefore, all the "dummy" models have a constant term to give logical results. To obtain logical results from models without a constant term (where the line of regression passes through the origin), the vehicles coded 0 would have to be included in independent variables corresponding to their characteristics. However, nonsignificant variables (those with nonsignificant regression coefficients) could not be deleted because this would again make the results illogical.

The first 19 dummy variables in Table 4 were introduced into the Model 1 equation which is as follows:

 $Y = a + b_1X_1 + ...+b_{19}X_{19}$

where (Y) is the dependent variable measured in 18-kip axle equivalents per vehicle: (a) is the constant term; the (b's) are partial regression coefficients; and the $(X's)$ are the independent variables.

Also, since the load characteristic was found *to* be a major source of variation in log-kip axle equivalents between vehicles, a loaded vehicle dummy variable (X_{20}) was introduced with 18 of the above 19 variables into Model 2. Variable X_{10} was deleted to keep the total number of variables at 19, the capacity of the computer program. Model 2 is as follows:

 $Y = a + b_1X_1 + ... + b_9X_9 + b_{11}X_{11} + ... + b_{20}X_{20}$

$$
-28-
$$

The backward elimination method was used to determine which variables should remain in the above models (8). The method begins with all the variables introduced and then eliminates the variable which has the least significant partial regression coefficient (b) in terms of computed t-values (ratio of each b to its standard error) at the 95 percent confidence level. After each variable elimination, the remaining variables are reintroduced into the models to generate new partial regression coefficients. The method takes as many steps as necessary to delete all variables which do not have statistically significant partial regression coefficients. Also, at each step, the variation in the dependent variable explained by the independent variables remaining in the model is tested for statistical significance using the F-values generated from the ANOV technique. In addition, the R^2 (the proportion of the total variance in the dependent variable explained by the independent variables and δ_{u} (standard deviation of regression) are calculated at each level (7).

Table 5 shows the variables which have significant partial regression coefficients for the two models. Model 1 had 14 significant variables, and Model 2 had 15. All the nonsignificant variables of Model 1 were those of body type. On the other hand, three of the five nonsignificant variables of Model 2 were day of the week variables. So, the addition of the load characteristic variable caused a considerable change in the significance of the body type and day of week variables. Also, the majority of the signs of the significant regression coefficients are negative. This was not the case with those of Model 1.

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Dummy Variables with Significant Partial Regression Coefficients for Multiple Regression Models 1 and 2

lMeasuring 18-kip axle equivalents per vehicle, based on 1967 data.

2variables X10 and X20 were not introduced into model.

N.8•Nonsignificant variables (coefficients).

Table 6 shows the standard statistical measures used to evaluate and compare multiple regression models. As measured by the correlation coefficient (R), the extent of the correlation of the "dummy" variables with 18-kip axle equivalents per vehicle is fairly low. Consequently, the coefficient of determination (R^2) , amount of variation in 18-kip axle equivalents per vehicle explained by these variables, is also low. But a comparison of the statistics of the two models shows the superiority of Model 2 over Model 1. Of particular importance is the fact that R^2 more than tripled, reaching 32.8 percent ($R^2 \times 100$) of explained variation. Also, R almost doubled, reaching a more respectable 0. 571 out of a possible 1. 000. Not to be overlooked is the fact that the amount of variation about the line of regression as measured by σ_{u} was reduced considerably. Therefore, the introduction of the load characteristic variable seems to have been a step in the right direction. These results may suggest the need to develop a method that will be able to distinguish the loaded vehieles from the empty in count station data used to estimate 18-kip axle equivalents generated by a traffic stream at some location.

However, the really critical test of the validity of the two regression models is how well they perform in estimating each station's actual total 18-kip axle equivalents. Table 7 presents each model's abselute and percentage estimating errors resulting from the application of the regression coefficients to the 1967 loadometer weighings at each station.

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Correlation of Significant Dummy Variables with 18-Kip Axle Equivalents of a Texas Cargo Vehicle Weighed at Any of the 21 Loadometer Stations in 1967, According to the Multiple Regression Model Used¹

 $^{\mathbf{1}}$ Table $\mathbf 5$ shows the significant variables in each model.

***This F ratio is significant at the .001 probability level, indicating that the variance due to regression has less than a 1 in 1000 chance of being due to chance alone.

Absolute and Percentage Estimating Errors of Two Dummy Variable Multiple Regression Models Used to Estimate the Total 18-Kip Axle Equivalents Generated by Texas Cargo Vehicles Weighed at Each Loadometer Station in 1967

lBased on 200-pound groupings for application of equivalency factors.

 2 Based on ignoring signs of errors.

Table 7

Using the percentage estimating errors as a basis for evaluating the performance of each model, Table 7 shows that a majority of the percentage errors for both models were over plus. or minus 10 percent of the actual station totals. However, most of the large errors (over 10 percent) were overestimates. Also, the performance of both models in making estimates for the interstaee rural stations ismuch better than for the other stations.

Comparing the two models, Model 2's average percentage error for the 21 stations is somewhat smaller than Model 1's. Also, 12 of the percentage errors of Model 2 were less than those of Model 1. So, the addition of the load characteristic variable into Model 2 did allow it to make more accurate estimates (especially for certain stations) than Model 1. For example, Model 1's percentage error for Station 147 was a minus 16.7, whereas, Model 2's was a minus 5.3.

To determine how much historical data should be used in generating and/ or used in regression models, loadometer data collected during 1964-68 period were combined and applied to the 1967 partial regression coefficients of Models 1 and 2, as presented in Table·S. The resulting absolute and percentage estimating_errors are presented in Table 8. Model 2 still had the lowest average absolute and percentage errors of the two models. When comparing these new estimating errors with those of Table 7, it is found that both models made more accurate multi-year station estimates than single-year estimates. The average percentage errors were smaller for both models, but, Model 2 had one less station with a percentage error of over 10 percent. Also percentage errors of the

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Absolute and Percentage Estimating Errors of Two Dummy Variable Multiple Regression Models Used to Estimate the Total 18-Kip Axle Equivalents Generated by Texas Cargo Vehicles Weighed at Each Loadometer Station During 1964-68

lBased on 200 pound groupings for application of equivalency factors. 2The signs of the errors were ignored.

urban stations were considerably smaller for Model 2. Next, multi-year data were used in two separate analyses to generate new partial regression coefficients and the corresponding station estimates for other models.

 $\mathcal{R}_{\mathcal{A}}$

In the first case, 1964-68 data were transformed to log-kip equivalents and applied to all the variables of Model 1, except Variable 15 to keep the number of variables under 20. Also, two other variables were introduced, one for year of weighing and one for summer weighings. The resulting coefficient for the year variable was not significant, but the one for the summer variable was significant. When applying these new significant regression coefficients to 1964-68 loadometer weighings at each station, the overall average (in 18 -kip equivalents) percentage error. per station was 11.2. This average is considerably lower than the average error generated by Models 1 and 2 using 1967 regression coefficients on single-year (1967) data and also lower than the average percentage error generated by Model 1 using 1967 coefficients on multi-year (1964-68) data. However. Model 2's average was the lowest of all.

In the second case, the 1966-68 data were divided according to loaded and empty vehicle weighings. This model, with two equations, had all of the Model 1 variables introduced, except Variable 10. Again the summer variable was introduced and its resulting coefficient in the empty equation was significant. When applying the new coefficients from each equation to the 1966-68 loadometer weighings at each station, the overall percentage error per station was 12.7 which is larger than the average for the above multi-year $(1964-68)$ analyses but somewhat lower than the average for the single-year (1967) analysis.

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Estimates of Alternative Procedures Compared

A comparison can now be made of the estimating accuracy obtained from the two alternative procedures, namely axle weight frequency distribution sets versus multiple regression models. Table 9 shows the percentage estimating errors of each procedure for both one-year and multi-year data. Regardless of the amoung of data used, Frequency Set 5 yielded lower average percentage errors than did Regression Model 2. It also generated fewer stations with percentage errors over 10 percent.

The use of multi-year data helped to produce more accurate estimates for both procedures. For the regression method, the greatest improvement occurred in the urban station estimates. For the axle weight frequency distribution method, improvement occurred in the rural station estimates.

Both procedures had difficulty in producing estimating errors lower than 10 percent at five particular loadometer stations. Their numbers are 10-1, 7, 16, 81 and 88. Although these stations are all in the lower range of total 18-kip axle equivalents output, this doesnot seem to be the only explanation for the large estimating errors. There are other stations with even lower total 18-kip axle equivalents outputs that have very small estimating errors. Also, multi-year data failed to lower the errors of some of these stations. Nor is the cause necessarily geographical, these stations being located in more than one area of the State. Some of the five stations are very close to major metropolitan centers, and others are of a considerable distance from such places.

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Percentage Estimating Errors of Two Alternative Procedures Used to Estimate the Total 18-Kip Axle Equivalents Generated by Texas Cargo Vehicles Weighed at EacH Loadometer Station During One Year (1967) Versus Several Years (1964-68)

te signs of the errors were ignored.

As was indicated earlier, the estimates obtained from axle weight frequency distributions used 1,000 pound weight classes for applying the 18-kip axle equivalency factors. On the other hand, the estimates from the multiple regression models used 200 pound weight classes for the application of the equivalency factors. Therefore, it was necessary to determine how sensitive the total output of 18-kip axle equivalents would be to the size of the weight class. The results of such an analysis showed that it made very little difference which weight class was used. In fact, the difference between the multi-year totals was aegligible (See Tables 3 and 8). For the 1967 totals, in Tables 2 and 7, the 7.4 percent difference is not due to the size of the weight class. Instead, it is due to a less accurate interpolation program which was used to compute the 1967 totals for the 200 pound weight class. The more accurate program was used to generate the multiyear totals and is the one presented in Appendix C.

DETERMINING THE ADEQUACY OF CARGO VEHICLE WEIGHT AND CLASSIFICATION COUNT SAMPLES

Procedures

This section of the report is directed toward determining the adequacy of cargo vehicle weight and classification count samples collected at the previously mentioned stations to represent the unknown population of vehicles passing them, individually or as a group.

The problem requires a two directional approach. One task is to determine the adequacy of loadometer samples for use in establishing accurate base weight characteristics of the various vehicle types. The other task is to determine the adequacy of manual classification count

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samples for use in establishing accurate annual average daily traffic (AADT) counts of the various vehicle types.

The adequacy of a sample taken from a larger population is measured in terms of the representativeness of its individual observations and the reliableness of its statistics. In the case of this study, weight and classification count samples of cargo vehicles were evaluated as to their representativeness of the population of cargo vehicles passing the stations and as to their ability to produce reliable estimates of population parameters, such as the average axle or vehicle weight in kips or 18-kip axle equivalents and the AADT count by vehicle type. Representativeness of Samples

In theory, a collection system which gives every vehicle passing a station an equal chance to be counted, classified or weighed is one that obtains a representative or random sample. To determine whether the samples are reasonably representative, collections obtained according to the time of day, day of the week, week of the month, month of the year and year of the planning period should be studied.

With the above criterion in mind, the present and past weighing and counting schedules and the samples collected therefrom are reviewed and evaluated to determine the degree of representativeness obtained. Reliableness of Samples

If a representative sample has been collected *at* each station, then the initial foundation is laid for yielding reliable estimates of the populations parameters. But an additional prerequisite for

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generating reliable estimates is that of obtaining a sample large enough to overcome large chance sampling errors.

In this study, an attempt is made to determine how many vehicles to weigh at loadometer stations and how many 24-hour volume counts to make at manual count stations in order to overcome chance sampling errors of stated magnitude at a designated probability level. In other words, the sample size depends on the accuracy needed in the estimates, the extent of variation in the sample observations and the stated probability level.

The THD has indicated that the level of accuracy desired for estimating the average vehicle weights and counts of a population is an error of no more than 10 percent. The absolute size of this error is based on the averages of the sample data evaluated in this study. Also, the extent of the expected variation in the individual observations of a population is assumed to be the same as that reflected by sample data used in this study. Last, the 95 percent probability level for avoidance of large sampling errors in the estimates was considered acceptable.

. Since statistics of previously collected samples have to be used in estimating the required size of future samples, it is important that such base samples themselves be of adequate size. Therefore, multi-station and/or multi-year data were used to generate estimates of adequate sample size. However, when multi-year sample data were used, a trend adjustment was made before generating sample statistics. For example, to estimate the population variance in multi-year 24 hour volume counts, a trend adjustment was applied.

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Sample sizes may vary according to the statistic being estimated even when using the same sample base data for example, the sample size can be different for estimating average vehicle weight in 18-kip axle equivalents and for estimating average axle weight in 18-kip axle equivalents. Also, the sample sizes could vary according to how the base data are stratified, such as by vehicle type only or by additional stratifications like load characteristic and highway system. Therefore, these sample size variations ate demonstrated in this report.

The limited data collected from the weigh-in-motion station are used to give some indication of the variations in data collected over a continuous seven-day period versus that collected over a non-continuous five-day period.

Results

The results of analyzing previously collected cargo vehicle weight and classification counts for representativeness and reliableness are presented here, according to the type of sampling station.

Loadometer Station Weight Samples

Vehicle weight data collected at the 21 stations during 1964- 68 were used in the various analyses. The 1964-66 data were obtained from the 19 rural stations during 12 eight-hour periods (three per season) per year and from the two urban stations during three periods of the summer season. In 1967, the amount of data collected at the 19 rural stations was reduced by one-third because no weighings were made

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after August 31st. In 1968, the amount of data collected was reduced further at these stations, because weighings were made only during three eight-hour periods in the summer months. The number of weighing periods for the urban stations remained the same throughout the 1964-68 period.

The three eight-hour weighing periods used by the THD are as follows: 6:00 AM to 2:00 PM, 2:00 PM to 10:00 PM and 10:00 PM to 6:00 AM.

Representativeness. The THD has not been collecting weight samples with a system developed entirely on a theoretical basis to yield purely random sample data. This is partially due to scheduling difficulties that would have greatly increased the costs of collecting data at the 21 conventional stations. Too, the gradual reduction of the weighing operations to cover only the summer months instead of all months in the year has contributed to the nonrepresentativeness of the data. Although the weighings were made during every month of the year, the 19 rural stations were scheduled in the same sequence. However, the weighings on particular days of the week or eight-hour periods of the day were in a nonsequential order, somewhat random in nature.

The above mentioned eight-hour periods for 1964-66 were distributed evenly over the three eight-hour periods required to account for one 24-hour day each season of the year at each of the 19 rural stations. The same was true for summer weighings of 1968. But in 1967, only the spring and summer seasons had three of these eight-hour periods at each of the 19 rural stations.

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All of the weighings were done on weekdays at the 21 stations. Therefore, the data may not adequately represent that of a full seven day week.

All the five weekdays were not represented at all of the stations. Prior to 1967, only three stations, two of them urban, had missing days. In 1967, as many as two days were missing, and only three stations had all days represented. In 1968, nearly all the stations had three missing days.

None of the stations had exactly the same distribution of eighthour periods over the five weekdays, Some stations were heavy on Monday weighing periods. Others were heavy on Tuesday and so on. However, with all 21 of the stations combined the number of weighing periods were distributed about evenly across weekdays.

Not all of the above three eight-hour periods necessary to make one 24-hour day were represented on a particular weekday at a station. Some weekdays were heavy on 2:00 PM to 10:00 PM weighing periods. Others were heavy on one of the other weighing periods. Cancellations due to bad weather contributed to the above imbalance in numbers and types of eight-hour periods.

During each weighing period, two directional weighings were made. This was accomplished by weighing vehicles coming from one direction for four hours and then weighing vehicles coming from the opposite direction for four hours.

Aggregation of stations helps to even out the number and type of eight-hour weighing periods across days of the week, tending to make the combined station data more representative than individual station data. This is one argument in favor of using statistics developed from

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all stations grouped together rather than those from one station.

The analysis of the data used in this study indicates that the number of weighings of each vehicle type was generally proportional to the number of each vehicle type passing a station during any given eight-hour period or group of eight-hour periods. Table 10 shows this to be true for both annual (1966) and summer (1968) data collected at selected high, low and medium volume loadometer stations.

Part of the station to station variation in the percent of vehicles weighed and counted within vehicle types was caused by differences in the days and eight-hour periods in which these activities took place. In addition, seasonal and annual (trend) variations enter into the year to year differences.

The problem of not being able to obtain a 100 percent weight sample for a given time period can be overcome by using weigh-in-motion scales. Then it is possible to make continuous weighings for all time periods critical to obtaining a proportional or representative sample. Unfortunately, the only test data available represent weighings taken for one week during the month of March, 1969 at one location. Therefore, only a few comparisons can be made to establish how representative past data collections have been.

Table 11 shows the number and percentage distributions of vehicles weighed by vehicle type at the weigh-in-motion station in 1969 and the conventional stations in 1967. A comparison of the weigh-in-motion station's percentage distribution with that of the 21 conventional stations reveals only small differences between them for each vehicle type. They differ greatest among the lowest volume vehicle types. But when the distribution of one conventional station (35-1) is compared

-45-

Percentage Distributions of Vehicles Weighed and Counted by Vehicle Type at Selected Loadometer Stations During 1966 and 1968 $^{\mathrm{T}}$

I the counts and weighings were taken during the same eight-hour periods, but due to bad weather cancella-
tions, the number of eight-hour periods varied from station to station in the case of the 1966 data. During 1966, the number of periods were as follows: Station 147 had nine; Stations 10-1, 45-2, and 81 had eight; and Station 20-1 had six. During the summer of 1968, each station had three periods. "

I -97

Number and Percentage Distributions of Vehicles by Type Weighed at the Weigh-in-Motion and Conventional Loadometer Stations¹

 1 Data from Station 35-2 represents seven consecutive 24-hour days of vehicle weighings obtained during one week in March 1969. Data from the conventional stations represent various eight-hour period weekday weighings obtained during the first 10 months of 1967.

with that of the weigh-in-motion station, greater differences are revealed. The differences between these distributions are due to several factors. Chief among these are seasons, days and hours of weighings. In the case of the last two, the weigh-in-motion station reflects continuous weighings all seven days of the week, and the 21 conventional stations reflect variable eight-hour period weighings taken during a maximum of four days of the week at any one station. It was found that very few of the conventional stations had full 24-hour day weighings for each day of the week.

Table 12 shows the extent of the differences in the percentage distributions of 3-S2's and other vehicle types by day of weighing at both types of stations. Although wide differences show up between the distributions of the two data sources, they are minimized between the weigh-in-motion station and the combined 21 stations. Also, between vehicle types, both of these distributions show nearly the same proportions, regardless of day of weighing. In the case of Station 35-1 versus Station 35-2, the above is not true.

Further analysis of the weigh-in-motion data revealed that the frequency of weekend weighings average about one-half that of weekday weighings, regardless of vehicle type.

As a result of all of the above comparisons, one conclusion definitely can be made. When loadometer data from the existing and previous weighing schedules are used to generate vehicle weight statistics, the combined station data are more likely to be representative of the actual population of all vehicles passing a station than the data collected at one station, even when it is used to represent itself.

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Percentage Distribution of 3-S2's and Other Vehicle Types by Day of Weighing at the Weigh-in Motion Station and the Conventional Stational

1 Data from Station 35-2 represent seven consecutive 24-hour days of vehicle weighings obtained during one week in March 1969. Data from the conventional stations represent various eight-hour period week day weighings obtained during the first 10 months of 1967.

* No weighings.

Reliableness. The results of an analysis to determine the reliableness of the data may tend to confirm or reject the conclusion reached from analyzing the loadometer data for representativeness.

The analysis of individual station's average vehicle and axle weights according to vehicle types, load characteristic and highway system indicated that such averages vary significantly between stations (See Appendix A for tables of the averages). Much of the station to station variation between these averages is due to the nonrepresentativeness of the data on an individual station basis.

There are other station to station differences not caused directly by the weighing schedule. For instance, there are those due to chance, which become quite large in the case of very small samples. This is indicated by the fact that, in most cases, the number of vehicles (by type) weighed at individual stations in 1967 is too small compared to the number of vehicles required by the station's own statistics in order to overcome chance sampling errors of a given magnitude and stated probability level. The formula used in this evaluation is as follows:

$$
N = \frac{t^2 S^2}{E^2}
$$
, where

N is the number of vehicles necessary to weigh; t^2 is the square of the tabulated t-value for the degrees of freedom at the 95 percent probability level; S^2 is the variance of the characteristic in the sample; and E^2 is the square of 10 percent of the average generated from the sample data. The E actually stands for the standard error of the average (mean), and E^2 is the sampling variance. The magnitude

-50-

of the latter is inversely proportional to the sample size and directly proportional to the variance of the characteristic measured in the population (7). Since the variance of the population is not known, the variance (S^2) of the characteristic in the sample is used as its best estimate. Therefore, the variance generated by a small sample at one station may be suspect. So the samples of several stations combined should produce a better estimate of the population variance than only one. Thus, variations due to chance and the weighing schedule make it all the more important to use combined station data in generating sample statistics and sample size estimates.

No geographical pattern in the magnitude of the station averages was found to serve as a basis for grouping the stations. Even grouping the stations by highway system was not clearly justified by the analysis of the data. For within highway systems, many of the station averages and/or variances were significantly different. Hence, to compute statistics used in estimating sample sizes and to generate other estimates, all 21 stations were combined in most instances.

If the weight characteristic to be estimated for the population is the average vehicle weight in 18-kip axle equivalents, then Table 13 shows the number of vehicles by type that should have been weighed at a station to assure plus or minus 10 percent accuracy at the 95 percent probability level. These calculations are based on 1967 data from all stations combined and also stations grouped by highway system. On an all station basis, the number necessary to weigh is less than the number actually weighed for every vehicle type, except the 3-axle

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Number and Percent of Vehicles Actually Weighed and Necessary to Weigh of Each Major Vehicle Type to Obtain an Average Vehicle Weight in 18-Kip axle Equivalents Within 10 Percent of the True Population Average on an All Station and Highway System Basis¹

1 Based on 1967 loadometer data and using the standard formula shown in the text of the report. Station 37~1 was included in the Other Rural group of stations.

2 Only the sum of the vehicles in the above groups.

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V21
single-unit. The same is true for the IH rural system. But for the other rural system, the number necessary to weigh is greater than the number actually weighed for every vehicle type, except 3-S2's and 2-SZ's. For the urban system, the number necessary to weigh is consistently less than the number actually weighed.

On the basis of vehicle type, Table 13 shows that only the 2-S2's have about the same percent of the total number of vehicles weighed and necessary to weigh, regardless of highway system. For the 3-S2's, the percent of total vehicles necessary to weigh is considerably lower than the percent of total vehicles actually weighed, especially for the rural highway systems. In the case of the other vehicle types, the reverse is true.

The results presented in Table 13 suggest that sampling rates for each vehicle type could be set according to the percent of total vehicles necessary to weigh. In effect, the overall sampling rate for all groups, except the urban group, could be reduced by the above stated percentages.

Dividing the 21 station data according to load characteristic apparently changes the sample size requirements somewhat. For 3-S2's, the required number is 697 (221 loaded and 476 empty). Sample sized for the other vehicle types were not computed.

If the weight characteristic to be estimated for the population is the average axle weight in kips and 18-kip axle equivalents, then Tables 14 and 15 show the number of axles (and vehicles) by vehicle

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Table 14

Number of Single and Tandem Axles Actually Weighed and Necessary to Weigh of Each Major Vehicle Type to Obtain an Average Axle Weight in Kips Within 10 Percent of the True Population Average for all Stations Combined¹

1 Based on 1966-68 data from the 21 conventional loadometer stations and using the standard formula shown in the text of the report.

2 Controlled by the axle requiring the largest number of vehicles of that type.

* Not applicable

י
17
1

Number of Single and Tandem Axles Actually Weighed and Necessary to Weigh of Each Major Vehicle Type to Obtain an Average Axle Weight in 18-Kip Axle Equivalents Within 10 Percent of the Population Average for all Stations Combined¹

Table 15

1 Based on 1966-68 data from the 21 conventional loadometer stations and using the standard formula shown in the text of the report.

2 Controlled by the axle type requiring the largest number of vehicles of that type.

* Not applicable. ϵ .

type that should have been weighed at a station to obtain the level of reliableness specified. These calculations were based on 1966-68 data from all 21 stations combined. The results of Table 14 indicate that the number of axles necessary to weigh was very small compared to the number weighed. When put in terms of the number of vehicles of each type, the same was true. Thus, it is relatively easy to obtain reliable average axle weights in kips with the actual number weighed. On the other hand, Table 15 shows that considerably more axles or vehicles must be weighed to obtain reliable average axle weights in 18-kip axle equivalents than to obtain reliable average axle weights in kips. Even so, the number of axles actually weighed was in excess of the number required to be weighed, except for the 3-axle singleunit and miscellaneous vehicle types.

When analyzed on the basis of highway system, the number of single and tandem axles necessary to weigh, of each vehicle type, did not change significantly. At least, this was the case in obtaining reliable average axle weights in kips. It seems likely that the same relationship exists in the case of average axle weights in 18-kip axle equivalents.

Dividing the 21 station data according to load characteristic apparently increases the number of vehicles necessary to weigh in obtaining reliable average axle weight. This conclusion is based on the analysis of 3-S2's, where it was found that a total of 2,762 vehicles, 1,693 loaded and 1,069 empty, would have to be weighed to obtain reliable average axle weights in 18-kip axle equivalents.

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When comparing the results of Tables 13 and 15, it is discovered that more vehicles must be weighed to obtain accurate average vehicle weights in 18-kip axle equivalents than to obtain accurate average axle weights in 18-kip axle equivalents. This fact confirms the assumption that there is more variation in the units weighed as vehicles than as axles.

So long as multi-station data are used to generate sample size estimates, multi-year data may not be necessary. This is indicated by the fact that the averages and variances for each vehicle type have not changed significantly from year to year, even when not corrected for trend.

In an effort to cast some light on how many stations may be needed to collect adequate loadometer data, a limited comparison was made between data collected at the weigh-in-motion station and the conventional stations. First, a comparison of Figures 3 and 4 revealed a close similarity between frequency distributions of the two sets of data. Also, Table 16 shows that the standard deviations about the averages of vehicle weights in log-kip axle equivalents for the two sets of data are highly similar. The same is true even on a vehicle type basis. However, the standard deviations measured in 18-kip axle equivalents are quite different. Then too, the averages themselves are statistically different. It seems that the difference between the averages of the two sets of data is primarily due to the type of weighing device. The scale used at conventional stations record static weights, while the one used at the weigh-in-motion station records dynamic weights.

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 $-35 -$

Table 16

Numbers, Averages, and Standard Deviations Generated from all Vehicles Weighed at the Weigh-in-Motion Station and the 21 Conventional Loadometer Stations¹

 1 Data from Weigh=in-Motion Station 35-2 represent seven consecutive days of weighings obtained during March 1969, and data from the 21 conventional stations represent 48.66 nonconsecutive weekdays of weighings obtained during the first 10 months of 1967.

Carrying the above analysis a step further, Table 17 shows a comparison of the averages and standard deviations for the two sets of data according to days of weighing. Again, the standard deviations are highly similar for data collected on weekdays, but the averages are different. Since weekend weighings were not made at the conventional stations, no comparison can be made between the two sets of data on this basis. But weekend versus weekday comparisons were made using the weigh-in-motion data. For 3-S2's, the standard deviations are somewhat different, and averages are definitely different, However, the averages and standard deviations for the combined vehicle types are nearly identical for both sets of data.

Not enough weigh-in-motion data have been collected and analyzed to allow definite conclusions concerning the reliableness of data collections at only one location. However, the results of the above analysis indicate that data collected at only a few stations, perhaps two for each highway system, may be adequate to make weight estimates for highway design purposes. Also, there is enough difference between the weekday and weekend data for a major vehicle type to suggest ·the necessity of collecting data seven days of the week. The regression analysis has already indicated that there is a significant difference in the weight data between seasons.

Manual Classification Count Samples

Prior to 1970, four 24-hour weekday vehicle classification counts were taken annually at each manual count station. These counts were actually taken during eight-hour periods as follows: 12~00 AM to 8:00 AM,

 $-60-$

Numbers, Averages, and Standard Deviations Generated from all Vehicles Weighed on Weekdays Compared to Weekends at the Weigh-in-Motion Station and the 21 Conventional Loadometer Stational

1 Data from Weigh-in-Motion Station 35-2 represent seven consecutive days of weighings obtained during March 1969, and data from the 21 conventional stations represent 48.66 nonconsecutive weekday of weighings obtained during the first 10 months of 1967.

2 Monday through Friday

- 3 The difference between these averages is not statistically significant at the .01 probability level.
- 4 The difference between these averages is statistically significant at the .01 probability level.

* No data available.

Table 17

8:00 AM to 4:00 PM and 4:00 PM to 12:00 AM. All three of. these periods (to make one 24-hour day) were represented one time each season or one period per month. Two continuous 24-hour period weekend counts were taken during the summer at each station having a permanent automatic recording station in the same road section. In 1970, the weekday counting was reduced to 16 hours (8:00 AM to 12:00 AM) per season at each station.

At the loadometer stations, additional counts were made during every weighing period. Also proor to 1967, one continuous 24-hour count for each summer month was taken at the 19 rural stations.

The manual classification count samples taken at loadometer stations were used almost exclusively in determining the adequacy of such data to estimate the base AADT count of cargo vehtcles at any station. The weigh-in-motion station counts were used as supplemental data.

Representativeness. To some extent, the findings and conclusions reached concerning the representativeness of previously collected weight samples are applicable to manual count samples. But, generally, the latter samples have been more representative, especially since 1967. One major reason for this is that weekend counting has been done at some of the stations, whereas, no weekend weighings were done at the conventiona! loadometer stations. Also, the counting schedule includes **all** seasons, while the present loadometer schedule does not.

However, it is difficult to reflect actual seasonal changes with only one 24-hour count per season. Thus, aggregation of count data by years or stations may help establish adjustments for seasonal and day of week differences.

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Reliableness. Assuming that the manual counts taken at the loadometer stations were fairly representative of the traffic, the reliableness of such data for estimating the annual average daily traffic count by vehicle type at a station was evaluated. The criteria applied on the weight data were used here also.

The 1965-68 loadometer manual count samples were used to determine how many 24-hour day manual counts should be made to obtain reliable base AADT's at a station (9). Count data from loadometer stations were more numerous than the same data from other manual count stations. Also, more continuous 24-hour counts were collected from loadometer stations.

From the above data, averages of the 24-hour volume counts and the variances from these averages were utilized in determining the necessary sample sizes. These statistics were used in the same formula applied to the weight data to generate sample size estimates. Also, the error requirements and probability level were the same.

First, it is demonstrated how much the averages and variances changed depending on the time of counting and the number of counts utilized. Data for five-axle semitrailer vehicles taken at Loadometer Station 45-2 serve as an example. As shown in Table 18, there is a considerable difference between the lowest and highest variance. The averages differ to a lesser extent. The number of counts, time of counting and length of the counting periods (24-hour versus 8-hour) j I apparently contributed to these differences. Such differences influence the number of counts necessary to derive reliable base year AADT estimates.

-63-

Estimated Number of Counts Necessary to Make of Five-Axle Semitrailer Vehicles Based on the Averages and Variances Computed from Varying Numbers of Actual Counts Taken at Loadometer Station 45-2 During 1965-68

1 This is the variance (S^2) used in the sample size formula shown in the text.

2 The spring and fall counts are based on three eight-hour periods per season, but the summer counts are based on three 24-hour periods.

12,305

881

115

12

5

1

3

115

3 Three eight-hour counts per season, except none for summer of 1965.

1,150

1,153

4 Same as Footnote 1, except eight-hour periods for summer of 1967 and 1968.

1965-68

Summer

Spring, Summer and Fall 4
24 and 8-Hour Periods

8-Hour Periods

Table 18

Also, when the number of necessary counts are determined from a small number of actual counts, the t-values used in the sample size formula are quite large and cause the number of counts required to increase. Therefore, as many as 10 or 12 actual counts should be used in making estimates of the number of counts necessary to produce reliable AADT estimates. In fact, this number of actual counts indicates that five counts of five-axle semitrailers are necessary at Station 45-2.

Compining years of 24-hour volume counts seems to be a necessary procedure for making more accurate sample size estimates. Of course, trend adjustments may be required. In general, it seems acceptable to assume a constant trend in absolute terms over the years. If this is correct, the equation to measure or adjust for trend can be stated as follows:

$Y_c = a + bX$, where

 Y_c is the computed or trend value of the actual 24-hour volume count of a vehicle type for the year numbered X. The constant a is the value of Y_c when X equals zero, and the constant b is the slope of the trend line or change in Y_c per unit change in X. Data in Table 18 required no such trend adjustment.

Second, it is demonstrated how much the averages and variances changed for the 24-hour volume counts of five-axle semitrailer vehicles, depending on where these counts were made. Table 19 shows these differences, generated from trend adjusted counts. Stations with essentially the same averages had significantly different variances and vice versa. Again, such differences account for the varying number of counts necessary from

-65-

Estimated Number of Counts Necessary to Make of Five-Axle Semitrailer Vehicles Based on the Averages and Variances Computed from Varying Numbers of Actual Counts Taken at the 21 Loadometer Stations During 1965-68

Table 19

1 These counts are adjusted for trend, using the linear equation presented in the text of this report.

2 This is the variance used in the sample size formula shown in the text.

3 All stations have counts for spring, summer, and fall, except the urban stations which have only summer counts. Also, see Footnote 3 at bottom of Table 18.

station to station. However, the number of counts necessary was less than the number of actual counts for 15 of the 21 stations. If averaged by highway system, the numbers of counts necessary are as follows: 6 for interstate rural, 11 for other rural and 45 for urban. Therefore, only the urban system shows that more than 12 counts are necessary to produce a reliable AADT count estimate.

Third, it is demonstrated the extent of the differences in the sample size requirements for the major vehicle types. Again, data from Loadometer Station 45-2 are used. The results are presented in Table 20. Based on eight-hour period counts heavily. weighted in favor of the summer season, these findings show a wide difference in the number of counts necessary between vehicle types. The number of 24-hour volume counts necessary was greater than that of the actual counts for every vehicle type, except the five-axle semitrailers.

The above analysis demonstrates the need for using an adequate number of sample 24-hour volume counts in arriving at the base year AADT count of each vehicle type. However, the analysis needs to be taken a step further to show the effects of using varying numbers of 24-hour volume counts within years and/or across years in estimating a base year AADT count for each vehicle type. Also, the analysis should show the effects of using manual and automatic recorder counts of all vehicles in making AADT count estimates for cargo vehicles. The above effects are demonstrated by using four methods *to* estimate the AADT of fiveaxle semitrailers based on counts taken during the summers at Loadometer

 $-67-$.

Table 20

Estimated Number of Counts Necessary to Make of the Major Vehicle Types Based on the Averages and Variances Computed From Seven Actual Counts Taken at Loadometer Station $45-2$ During $1965-68¹$

- 1 Three eight-hour counts per season as follows: Spring and fall of 1965; Spring, summer and fall of 1966; and Summer of 1967 and 1968.
- 2 This is the variance used in the sample size formula shown in the text.
- 3 The actual counts were adjusted for trend for this vehicle type, using the linear equation presented in the text of this report.

Station 45-2. The four methods of arriving at the base year AADT count for this or any vehicle type are described here in terms of using only one 24-hour volume count per year as follows:

- 1. Use the latest year's count.
- 2. Use the latest year's trend line count (computed from at least three years of data).
- 3. Use the number Of vehicles derived from multiplying the latest year's automatic recorder AADT by the percent of the vehicle type (latest year's count of individual vehicle type divided by the corresponding count all vehicles).
- 4. Use the number of vehicles derived from multiplying the latest year's automatic recorder AADT by the percent of a vehicle type (sum of at least three yearly counts of an individual vehicle type divided by the corresponding sum of count of all vehicles).

Methods 1 and 2 use only the actual 24-hour volume counts of each vehicle type to arrive at the base year AADT. Whereas, Methods 3 and 4 applies the 24-hour volume counts of each vehicle type to the corresponding 24-hour volume count of all vehicles and the automatic recorder AADT count of all vehicles to arrive at the base year AADT for each vehicle type. Method 2 assumes a constant absolute change in the actual count of a vehicle type from year to year, and Method 4 assumes that the percent of trucks is constant over the time period used in the calculations.

In applying the four methods to three 24-hour volume counts per year, a weighted annual average number (Methods 1 and 2) or percent (Methods 3 and 4) was used to make the base year AADT count estimates.

Figure *5* shows the AADT count derived from the application of the above four methods using one versus three actual counts per year. Figure 6 shows the AADT manual count of all vehicles based on one versus three actual counts per year and the AADT automatic recorder count of all vehicles. As indicated on the graphs, it was only during the summers of 1965 and 1966 that three manual counts were collected. During 1967 and 1968, only one count was available.

The results of this analysis, as shown in Figure 5, indicate that the within and between-year fluctuations in the estimated AADT counts of five-axle semitrailers for 1965-1967 are very large when using only one count per year, especially in the case of Method 3. The use of three counts per year reduced these fluctuations considerably. The same was true for the AADT manual count of all vehicles shown in Figure 6. In other words, if more than one count per year is used, all four methods will yield more accurate estimates of the base year AADT count. When this analysis was applied to data from other loadometer stations, essentially the same results were obtained.

The results also indicate that the large differences between the estimated AADT counts for each year (especially for 1965 and 1966 are due to employing 24-hour manual counts and AADT automatic recorder counts of all vehicles in the calculations of Methods 3 and 4. Thus, it might be concluded that more accurate base year AADT counts for the cargo

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YEAR OF COUNT

Figure 5. Graph showing the AADT count of five-axle semitrailers derived from four methods with the use of one versus three manual counts per year.

YEAR OF COUNT

Figure 6. Graph showing the AADT count of all vehicles based on one versus three manual counts per year and the AADT automatic recorder count of all vehicles.

vehicle types can be obtained from Methods 1 and 2. Of these two, Method 2 gives the greatest accuracy, because it depends on the vehicle type counts for all years in the series rather than only one year. Then, to gain even greater accuracy using either method, at least three counts per year should be employed.

In conclusion, it is highly desirable to take at least three counts per year per manual count station in which to estimate the base AADT of each vehicle type. Also, the method of estimating the base AADT count for the cargo vehicle types is of considerable importance.

REDUCING AND ANALyz:tNG LOADOMETER DATA

Reducing and analyzing loadometer data can be formidable tasks if the uses and output requirements are not kept clearly in mind, if improper amounts of input data are collected and if the wrong techniques are used to generate the needed outputs. This statement can be supported by the fact that many studies, in the past, have dealt with such problems. In fact, this research study was conceived to deal with these probelms. Thus, an attempt is made in this section to summarize the requirements for a loadometer data reduction and analytical system which seem to be supported by the findings of this study. Stated more explicitly, the primary puppose of this section of the report is to suggest a methodology developed during the life of the study which will meet the present and foreseeable output needs of the THD as well as other state and federal agencies responsible for highway construction and maintenance.

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Uses of Loadometer Data

As a first step in fulfilling the above purpose, the primary uses of loadometer data were conceived through the help of officials in the Planning Survey Division of the THD and from the Federal Highway Administration's instructional memorandum referred to earlier in this report (1). These uses of loadometer data are summarized as follows:

- 1. To determine the design requirements of highway pavements.
- 2. To aid in the determination of the geometric design requirements of highways.
- 3. To help in allocating highway costs among the users.
- 4. To assist in allocating highway revenues among the various government agencies responsible for building and maintaining highways.
- 5. To assist in establishing vehicle size and weight limits.
- 6. To assist governments in the establishment of a sound transportation policy.
- 7. To furnish basic data for continuing research efforts.

Perhaps other uses could be added to the above list, but they likely would have the same output requirements. Also, those listed are considered to be, by far, the principal uses of loadometer data.

Outputs Required from Loadometer Data

Each of the above uses requires somewhat different outputs from loadometer data. Also, the output for one use may become the input to generate the output for another use. Additional data other than

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loadometer data are required to obtain certain outputs.

To satisfy the requirements of the above uses, the loadometer data reduction and analytical system must yield adequate outputs according to the vehicle characteristics of each station, each highway system and the combined 21 stations. The principal types of outputs are as follows:

1. Total number of vehicles weighed.

- 2. Total weight of all vehicles expressed in pounds, kips and 18-kip axle equivalents.
- 3. Number and percentage distributions of vehitles by weight classes in pounds, kips and 18-kip axle equivalents.
- 4. Averages, variances, standard deviations and standard errors for each vehicle weight frequency distribution.
- 5. Total number of axles (single and tandem) weighed.
- 6. Total weight of all axles by type, expressed in pounds, kips and 18-kip axle equivalents.
- 7. Number and percentage distributions of single and tandem axles by weight classes in pounds, kips and 18-kip axle equivalents.
- 8. Averages, variances, standard deviations and standard errors for each axle weight frequency distribution.
- 9, Estimated total load experience of an existing or proposed highway generated from some of the above outputs using a selected estimating procedure.

Quantity of Loadometer Data to Collect

The findings of this report indicate that as long as the present weighing schedule is followed, the system should combine data collections

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from all 21 of the conventional loadometer stations to obtain representative input data for load experience estimates. To assure reliable outputs, the minimum quantity of combined station data should be about that collected during one summer. However, if the data are broken down on a station or highway system basis, the quantity of data collections should be increased, especially in the case of the urban system. Much the same results could be accomplished by combining enough data collected during previous years or summers.

The findings tend to indicate that fewer stations could be used to obtain the necessary input data to produce reliable statistics or estimates of the population parameters. Thus, continuous seven-day weighing periods during each season of the year are recommended to be conducted at several stations. Perhaps two or three stations per highway system would be enough. However, a final decision should not be made until more data are generated with the weigh-in-motion scales at several station locations on each highway system. Then, it could be determined whether true station to station differences in vehicle or axle weights actually exist.

Future loadometer data collections should be periodically tested for adequacy, that is, tested for representativeness and reliableness. The procedures used in this study are recommended for such determinations. Also, the same tests should be performed on the manual count data. The continuous need for adequate data to support future research in this area should always be kept in mind.

Selecting an Estimating Technique

Since this study has concentrated on developing a technique to generate more accurate estimates of the total load experience of a

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highway (required for design purposes), the process of reducing and analyzing loadometer data is oriented toward achieving that goal. The outputs needed for other uses can or will be generated in the process of making the above estimates. Also, the THD has already developed computerized programs to obtain outputs needed for the other uses,

The findings of this study indicate that an axle weight frequency distribution set, such as Set *5,* should be used in estimating the total load experience of a highway. The loadometer data requirements and the steps in the analytical process leading to such determination have already been outlined in this report. As in the case of alternative estimating procedures, the axle weight frequency distributions should be updated with the most current loadometer data every two or three years.

Further research is recommended for the purpose of attempting to develop an estimating model which would be more accurate than those presented in this report. *A* comprehensive analysis of sufficient data collected by weigh-in-motion scales should yield more accurate estimating models for each highway system.

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SUPPORTING TABLES

Table 1

Texas Cargo Vehicles Defined According to Axle Combination and Corresponding Vehicle type Code

Appendix A

Table 2

Texas Highway Department Classifications of Body Types Included in the Body Type Variables as Introduced into the Analyses of Loadometer Data¹

 1 These variables were specifically combined for the dummy variable analyses.

Table 3

Percentage Distribution of Single Axles for Frequency Set 5 Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Nine Interstate Rural Loadometer Stations During 1964-68

Table 3 (Continued)

Percentage Distribution of Single Axles for Frequency Set 5 Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Nine Interstate Rural Loadometer Station During 1964-68

*Less than 0.05 percent.

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lThe total percent for each vehicle type's single and tandem axles added together should be 100.0 percent for each highway system. However, the values less than 0.05 percent were not added in the totals. Also, those percentages representing axles miscoded were left out of the totals.

Table 4

Percentage Distributions of Tandem Axles for Frequency Set 4 Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Nine Interstate Rural Loadometer Stations During 1964-68

Table 4 (Continued)

Percentage Distributions of Tandem Axles for Frequency Set 4 Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Nine Interstate Rural Loadometer Stations During 1964-68

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*Less than 0.05 percent.

lsee Footnote 1 at the bottom of Table 3_

Table *5*

Percentage Distributions of Single Axle for Frequency Set *5* Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Ten Other Rural Loadometer Stations During 1964-68

Table 5 (Continued)

Percentage Distributions of Single Axle for Frequency Set 5 Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Ten Other Rural Loadometer Stations During 1964-68

*Less than 0,05 percent.

1see Footnote 1 at the bottom of Table 3.

Table 6

Percentage Distributions of Tandem Axles for Frequency Set 5 Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Ten Other Rural Loadometer Stations During 1964-68

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Table 6 (Continued)

Percentage Distribution of Tandem Axles for Frequency Set *5* Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Ten Other Rural Loadometer Stations During 1964-68

*Less than 0.05 percent.

lsee Footnote 1 at bottom of Table 3.

Table 7

Percentage Distributions of Single Axles for Frequency Set *5* Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Two Urban Loadometer Stations During 1964-68

Table 7 (Continued)

Percentage Distributions of Single Axles for Frequency Set 5 Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Two Urban Loadometer Stations During 1964-68

*Less than 0.05 percent.

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lsee Footnote 1 at the bottom of Table 3.

Table 8

Percentage Distributions of Tandem Axles for Frequency Set 5 Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Two Urban Loadometer Stations During 1964-68

Table 8 (Continued)

Percentage Distributions of Tandem Axles for Frequency Set 5 Based on the Number of Axles in Each Kip Class as Generated by Each Type of Texas Cargo Vehicle Weighed at Two Urban Loadometer Stations During 1964-68

 $*$ Less than 0.05 percent.

lsee Footnote 1 at the bottom of Table 3.

 $^{\rm 1}$ Contains the following combinations: 2-1, 2-2, and 3-2 truck-trailer combinations; 3-Sl tractor-semitrailer combinations; and 2-81-2 and 3-Sl-2 tractor-semitrailer-trailer combinations.

f''

Table 10

Number and Percentage Distribution of Texas Cargo Vehicles Weighed at 21 Stations During 1964-68, 1966-68 and 1967 by Vehicle Type

 $\frac{1}{1}$ Contains 2-S3, 3-1, 2-3 and 3-3 multi-units.

Table Il

NUmber and Percentage Distribution of Texas Cargo Vehicles Weighed at 21 Loadometer Stations in 1967, by Body and Vebicte Type

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 $^{\rm 1}$ Contains the following vehicle types: 2-1, 2-2, and 3-2 truck-trailer combinations; 3-Sl tractor - semitrailer combination; and 2-Sl-2 and 3-Sl-2 tractor - semitrailer-trailer combinations.

 $\mathcal{A}=\{0,1,2,\ldots\}$

 $\mathcal{A}^{\mathcal{A}}$

 $\sim 10^{-1}$

 $\langle \xi \rangle$

 ϵ .

Table 12·

Number of Stations, Percentages of Loaded Vehicles, and Range of Station Percentages Involving Various Combinations of Wehicle and Body Types of Vehicles Weighed at Loadometer Stations, 1964-68

*No data or less than two vehicles for each station.

I *\0 \0* I

Table 13

Percentage Distribution of Loaded Vehicles With Various Combinations of Vehicle and Body Types Weighed at Each of 21 Loadometer Stations, 1964-68.

 1 This cell had only one loaded vehicle.

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Table 14

Number and Percentage Distribution of Texas Cargo Vehicles Weighe9 at. 21 Loadometer Stations in 1967, by Station and Across Body Type

Table 15

Number and Percentage Distribution of Texas Cargo Vehicles Weighed at 21 Loadometer Stations in 1967, by Station and Across Fuel Type and Time of DaY

Table 16

Number and Percentage Distribution of texas Cargo Vehicles Weighed at 21 Loadometer Stations in 1967, by Station and Across Weekdays

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Table 17

Number and Average 18-Kip Axle Equivalents Generated by Texas Cargo Vehicles Weighed at 21 Loadometer Stations in 1967, by Station and Body Type

Table 18

Average Vehicle 18-Kip and Log-Kip Axle Equivalents Generated by Texas Cargo Vehicles Weighed at 21 Loadometer Stations in 1967, by Vehicle Type

1The log kip equivalents for each vehicle are derived by taking the logarithm (to the base 10) fo the total 18 kip (axle) equivalents computed for that vehicle.

Table 19

Average and Standard Deviation from the Average of Log-Kip Axle Equivalents Per Vehicle for Cargo Vehicles.Weighed at Each of 21 Loadometer Stations in 1967 by Major Vehicle Type

 $\sim 10^7$

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l_{Includes} miscellaneous vehicle types.

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 \mathcal{L}

Table 20

Average and Standard Deviation from the Average of 18-Kip Axle Equivalents Per Vehicle for Cargo Vehicles Weighed at Each of 21 Loadometer Stations in 1967 by Major Vehicle Type

Table 21

Average Vehicle 18-Kip and Log-Kip Axle Equivalents Generated by Texas Cargp Vehicles Weighed at 21 Loadometer Stations in 1967, by Vehicle and Body Type

1 Contains the following vehicle types: 2-1, 2-2 and 3-2 truck-trailer combinations; 3-Sl tractor - semitrailer combinations; and 2-Sl-2 and 3-Sl-2 tractor - semitrailer-trailer combinations.

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 $\langle t \rangle$

 $\mathcal{A}=\mathcal{A}^{\mathrm{c}}$, where \mathcal{A}^{c}

 $\mathcal{R}^{\mathrm{max}}$

Table 22

Number and Average 18-Kip Axle Equivalents Generated by Texas Cargo Vehicles Weighed at

21 Loadometer Stations in 1967, by Station and Weekdays

Table 23

Number and Average 18-Kip Axle Equivalents Generated by Texas Cargo Vehicles Weighed at 21 Loadometer Stations
in 1967, by Station, Fuel Type, Time of Day, and Degree of Load

Table 24

Average and Standard Deviation from the Average of 18-Kip Axle Equivalents Per Tandem Axle for Cargo Vehicles Weighed at Each of 21 Loadometer Stations in 1966-68 by Major Vehicle Type

lIncludes miscellaneous vehicle types.

Table 25

Number, Average Weight, and Standard Deviation from Average Weight of Single Axles Weighed at 21 Loadometer Stations During 1966-68, According to Vehicle Type and Location of Station 1

 l only front axles are represented in this table for those axle types with no tandem axle.

2rnterstate Station 37-1 is included in this group.

3Interstate Stations 45-2 and 35-1 are the only stations in this group; thus the other group has 17 stations.

 4 Interstate Station 10-1 is in the other group made up of 12 stations.

Table 26

Number, Average Weight and Standard Deviation from Average Weight of Tandem Axles Weighed at 21 Loadometer Stations During 1966-68 According to Vehicle Type and Location of Station

1Interstate Station 37-1 is included in this group.

Table 27

Number, Average Weight, and Standard Deviation from Average Weight of Single and Tandem Axles Weighed at 21 Loadometer Stations During 1966-68 According to Vehicle Type and Load Characteristic

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Table 27

(Continued)

 $\label{eq:2.1} \frac{1}{\sqrt{2}}\int_{0}^{\infty}\frac{1}{\sqrt{2\pi}}\left(\frac{1}{\sqrt{2\pi}}\right)^{2}d\mu\left(\frac{1}{\sqrt{2\pi}}\right)\frac{d\mu}{\sqrt{2\pi}}\left(\frac{1}{\sqrt{2\pi}}\right)^{2}d\mu\left(\frac{1}{\sqrt{2\pi}}\right).$

 $\label{eq:2.1} \frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^{2} \left(\frac{1}{\sqrt{2}}\right)^{2} \left(\$

 $\label{eq:2.1} \frac{1}{\sqrt{2\pi}}\int_{0}^{\infty}\frac{d\mu}{\sqrt{2\pi}}\left(\frac{d\mu}{\mu}\right)^{\mu}d\mu\,d\mu\,.$ $\label{eq:2.1} \begin{split} \mathcal{L}_{\text{max}}(\mathbf{r}) & = \frac{1}{2} \sum_{i=1}^{N} \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r}) \\ & = \frac{1}{2} \sum_{i=1}^{N} \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf$

 $\frac{1}{\sqrt{2}}$

 $\label{eq:2} \frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1$

 $\label{eq:1} \frac{1}{2} \sum_{i=1}^n \frac{1}{2} \sum_{j=1}^n \frac{1}{$

 $\label{eq:2.1} \frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2\left(\frac{1}{\sqrt{2}}\right)^2\left(\frac{1}{\sqrt{2}}\right)^2\left(\frac{1}{\sqrt{2}}\right)^2.$

SUPPORTING CHARTS

 $-1117-$

Figure 2. Chart showing a frequency distribution of flexible pavement log-kip axle equivalents for all loaded cargo vehicles studied from the 1967 loadometer weighings.

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Figure 3. Chart showing a frequency distribution of flexible pavement log-kip axle equivalents for all single unit cargo vehicles studied from the 1967 loadometer weighings.

 $-119-$

Figure 4. Chart showing a frequency distribution of flexible pavement log-kip axle equivalents for all van (except insulated vans) and panel cargo vehicles studied from the 1967 loadometer weighings.

APPENDIX B

 $-120-$

Chart showing a frequency distribution of flexible pavement log-kip axle equivalents **Figure 5.** for all cargo vehicles studied from the 1967 weighings at loadometer stations on rural interstate roads.

Figure 6. Chart showing a frequency distribution of flexible pavement log-kip axle equivalents for all cargo vehicles studied from the 1967 weighings at loadometer stations on urban roads.

 $-122-$

 $-123-$

KIP AXLE EQUIVALENTS 18

Figure 8. Chart showing a frequency distribution of Flexible pavement log-kip axle equivalents for all cargo vehicles of the 2-S2 axle type from the 1967 loadometer weighings.

 $-124-$

Chart showing a frequency distribution of flexible pavement log-kip axle equivalents Figure 9. for all cargo vehicles of the 3-S2 axle type from the 1967 loadometer weighings.

 $-125 -$

KIP AXLE EQUIVALENTS 18

Figure 40. Chart showing a frequency distribution of flexible pavement log-kip axle equivalents for all cargo vehicles of the oil or platform body type studied from the 1967 loadometer weighings.

126-

$|8|$ KIP AXLE EQUIVALENTS

Figure 11. Chart showing a frequency distribution of flexible pavement log-kip axle equivalents for all cargo vehicles of the cattle or rack body type studied from the 1967 loadometer weighings.

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Figure 12. Chart showing a frequency distribution of flexible pavement log-kip axle equivalents for all cargo vehicles of the tank body type studied from the 1967 loadometer weighings.

 $-128-$

160 $\mathbf N$ $L E G E N D$ Ñ N N Equal To 4 N N_N Less Than 3 N_N Equal To 1 140 N.N **NNN NNN NNN** N N NNN 120 N NNN N NNN $-$ NNNNN-N **NNNNNNN** -N **NN NNNNNNN** 100 **NN NNNNNNN NN NNNNNNN** VEHICLES NN **NNNNNNN NN NNNNNNN NN NNNNNNN** 80 -N **NN NNNNNNN** NN.NNN **NNNNNNN NNNNNN NNNNNNN NNNNNN NNNNNNN NNNNNN** N NNNNNNNN, \overline{C} 60 NNNNNN . N-NNNNNNNNN NNNNNN-N **NNNNNNNNNNN** N **NNN NNNNN NNNNNNNNNNN** NNNNNNNN. NUMBER **NNNNNNNNNNN** N NNN NNNNNNNNN **NNNNNNNNNNN** 40 N NNNNN-NNNNNNNNN N N **NNNNNNNNNNN NNNNNNNNNNNNNNNNN** N N N **NNNNNNNNNNN** N NNNNNNNNNNNNNNNNN NN **N.NNNNNNNNNNNN** N N N -NNNNNNNNNNNNNNNNN **NN NNNNNNNNNNNNNNN** N N N N NNNNNNNNNNNNNNNNNNN-NN N N NNNNNNNNNNNNNN N IO N N N.WHOMMANNANNNNNNNN N. N **NN** N N N **NN** МИМИМИМИМИМИМИМИМИМИМИМИЗМ "МИМИМИМИМИМИМИМИМИМИМИМИ N . NNN O 7 $\mathbf{2}$ $\overline{3}$ 5 $\overline{7}$ $\overline{2}$ 3 45 \overline{O} $\dot{2}$ $\mathbf{\dot{3}}$ 45 $\overline{7}$ $\overline{2}$ 3 5 0.001 0.01 ιò 10.0

18 KIP AXLE EQUIVALENTS

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Figure M. Chart showing a frequency distribution of flexible pavement log-kip axle equivalents for all cargo vehicles of the auto transport type studied from the 1967 loadometer weighings.

-oan-

 $-131 -$

KIP AXLE EQUIVALENTS 18

18 KIP AXLE EQUIVALENTS

Figure 16. Chart showing a frequency distribution of flexible pavement log-kip axle equivalents for all cargo vehicles studied from the 1967 weighings at Loadometer Station 20-1.

Chart showing a frequency distribution of flexible pavement log-kip axle equivalents Figure 17. for all cargo vehicles studied from the 1967 conventional loadometer weighings at Station 35-1.

Figure 18. Chart showing a frequency distribution of flexible pavement log-kip axle equivalents for all cargo vehicles studied from the 1967 weighings at Loadometer Station 7.

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APPENDIX 6

KIP AXLE EQUIVALENTS 18

Chart showing a frequency distribution of flexible pavement log-kip axle equivalents Figure 19. for all cargo vehicles studied from the 1967 weighings at Loadometer Station 42.

18 KIP AXLE EQUIVALENTS

Figure 20. Chart showing a frequency distribution of flexible pavement log-kip axle equivalents for all cargo vehicles studied from the 1967 weighings at Loadometer Station 3.

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APPENDIX C $\sqrt{2}$ and $\sqrt{2}$ $\mathcal{F}^{\text{max}}_{\text{max}}$ COMPUTER PROGRAMS Stan. $\frac{1}{2} \frac{1}{2} \frac{1}{2}$ is in

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 $\label{eq:2.1} \mathcal{L}_{\mathbf{A}}(\mathbf{y}) = \mathcal{L}_{\mathbf{A}}(\mathbf{y}) = \mathcal{L}_{\mathbf{A}}(\mathbf{y}) = \mathcal{L}_{\mathbf{A}}(\mathbf{y}) = \mathcal{L}_{\mathbf{A}}(\mathbf{y})$ \sim $\mathcal{L}(\mathcal{L})$ 第二次提示 人名拉克 $\mathcal{L}_{\mathcal{A}}$ $\chi=1$. $\langle \sigma \rangle$ \mathbf{r} \mathcal{L}_{max} and \mathcal{L}_{max} $\mathcal{A}^{\text{max}}_{\text{max}}$ and $\mathcal{A}^{\text{max}}_{\text{max}}$

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APPENDIX 0

COMPUTER PROGRAMS

The following computer programs, written in FORTRAN IV subroutines which were run on an IBM 360/65, are included to define the 18-kip axle equivalents used in the regression models, either as the dependent variable or as an argument to the ALOGlO function which was used as the dependent variable.

SUBROUTINE AXLE (Figure 1) was used to group individual axle weights (from array AW) into single or tandem axle groups (into array LOAD) using the "Policy on Maximum Dimensions and Weights of Motor Vehicles to be operated OVer the Highways of the United States", officially adopted by AASHO in December of 1964 .¹ If axles are spaced less than 40 inches apart (spacing in array AD), their weights are combined and considered as a single axle load. Individual axle weights of a tandem axle group are added together, but the sign is changed to negative to flag the weight as a tandem axle load.

Functions FKIP (Figure 2) and RKIP (Figure 3) were used to calculate the flexible or rigid 18-kip equivalents respectively with the argument the axle loads which were returned in array LOAD from SUB-ROUTINE AXLE.

The functions were initialized using an extension of FORTRAN IV, the ENTRY statement. If this extension is unavailable, equivalent

 $10.$ S. Department of Transportation Instructional Memorandum 50-4-66(4), June 14, 1967, page 18.

coding may be programmed by initializing the constants in COMMON which adds considerably to the computational speed as compared with a method which uses structural number, initial serviceability and terminal serviceability on each function call.

Kip equivalents used for this study were calculated using the AASHO Road Test formulas² with SN=3.0, CO=4.2, and P=2.5 with flexible pavement and with $D=8.0$, $CO=4.5$, and $P=2.5$ with rigid pavement, but results using different values would be very similar unless extreme values are selected. The example coding below will calculate the logarithm of the 18-kip equivalents for a vehicle on flexible pavement and place in variable LKIP.

INTEGER AW, AD

DIMENSION AW (8), AD(7), LOAD(8)

 $DUMMY = FLEXIN (3.0, 4.2, 2.5)$

Read in data NA, AW, and AD

 $TKIP = 0.0$

CALL AXLE (NA, AW, AD, LOAD, NL, NTL, N3, NTSL, NE)

IF $(NE.NE. 0)$ GO TO 3

DO $2 I = 1$, NL

2 TKIP = TKIP + FKIP (FLOAT (LOAD (I))

 $LKIP = ALOG10$ (TKIP)

3 STOP END

²AASHO Road Test, Highway Research Board Special Report No. 73, pages 432-438.

Appendix C Figure 1

SUBROUTINE AXEE

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SUBPOUTINE AXLE (NA, AW, AD, LOAD, NL, NTL, N3, NTSL, NE)
A SUBROUTINE TO DETERMINE THE NUMBER AND WT. OF SINGLE AND TANDEM **AXLF** - 10 C. AXLE -20 C AXLES OF A VEHICLE USING AASHO DECISION RULES ADOPTED DECEMBER 7, AXLE 30 C 1964. AXLE 40 REFERENCE . $\mathbf c$ **AXLE** -50 U.S. DEPT. UF TRANS. INSTRUCTIONAL MEMORANDUM 50-4-66(4) OF JUNE $\mathsf c$ AXLF 60 Ċ 14, 1967, PGE 18. AXLE 70 C 80 AXLE NA = NUMBER OF AXLE WTS. $\mathbf C$ **AXLE** 90 AW = ORIGINAL ARRAY OF INDIVIDUAL AXLE WTS. C. **AXLE 100** $\mathsf C$ AD = OPIGINAL ARRAY OF DISTANCES BETWEEN AXLES IN INCHES **AXLE 110** AXLE 120 C C **AXLE 130** LCAD = ARRAY OF AXLE LOADS WHERE THE AXLE MAY BE A SINGLE AXLE $\mathbf c$ **AXLE 140** AND THE LOAD THE SAME AS AXLE WEIGHT OR OR IT MAY BE THE AXLE 150 $\mathbf c$ LOAD ON A TANDEM AXLEITHE COMBINED WEIGHT OF TWO AXLES) ϵ **AXLE 160** THE SIGN OF A TANDEM AXLE LOAD IS CHANGED TO NEGATIVE **AXIF 170** C C **AXLE 180** INTEGER AW(1), LCAD(1), AD(1) AXLE 190 \mathcal{C} **INITIALIZATION AXLE 200** $\mathbf C$ **AXLE 210** NUMBER OF AXLE LOADS **AXLE 220** $CML =$ $NL = 1$ **AXLE 230** NTSL = NUMBER OF TIMES CONSECUTIVE AXLE WTS COMBINED INTO A SINGLE C AXLE 240 \mathfrak{c} LOAD (AXLES CLOSER THAN 40 INCHES) **AXLE 250** $NTSL = 0$ **AXLE 260** C M = LOAD CCUNTER **AXLE 270** $M = 1$ **AXLE 280** C N = AXLE CCUNTER AXLE 290 $N = 1$ **AXLE** 300 C NTL = NUMBER OF TANDEM LOADS AXLE 310 $NTL = 0$ **AXLE 320** NUMBER OF 3 AXLE GROUPS - ONE TANDEM AND ONE SINGLE LOAD EACH C^{\prime} N3 = AXLE 330 $N3 = 2$ **AXLE 340** C NE = NUMERIC CODE SET NCNZFRO IF ERROR DETECTED **AXLE 350** $1 = WT$. FRROR C AXLE 360 C $2 = 01$ STANCE ERROR AXLE 370 3 OR GREATER = COMBINATION OF 1 & 2 OR A LOGIC ERROR C AXLE 380 $NF = C$ AXLE 390 c **AXLE 400** $LCD(M) = AW(N)$ **AXLE 410** IF (N .GE. NA) GO TO 2 **AXLE 420** \mathbf{l} IF (AD(N) .GE. 0) GO TO 111 **AXLE 430** $NE = NE + 2$ AXLE 440 GO TO 2 **AXLE 450** 111 $N = N + 1$ **AXLE 460** IF(AW(N) .GT. 0) GO TO 121 **AXLE 470 AXLE 480** $NE = NE + 1$ GO TO 2 **AXLE 490** AXLE 500 121 **CONTINUE**

Appendix C
Figure 1

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SUBROUTINE AXLE
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Appendix *C* Figure 2

FUNCTION FKIP

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