

A COMPUTER PROGRAM TO ANALYZE BEAM-COLUMNS UNDER MOVABLE LOADS

by

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Research Report Number 56-4

Development of Methods for Computer Simulation
of Beam-Columns and Grid-Beam and Slab Systems

Research Project 3-5-63-56

conducted for

The Texas Highway Department

in cooperation with the
U. S. Department of Transportation
Federal Highway Administration
Bureau of Public Roads

by the

CENTER FOR HIGHWAY RESEARCH
THE UNIVERSITY OF TEXAS AT AUSTIN

June 1968

The opinions, findings, and conclusions
expressed in this publication are those
of the authors and not necessarily those
of the Bureau of Public Roads.

PREFACE

This report describes a computer program developed to analyze beam-columns subjected to movable static loads. The method incorporates previously developed finite-element beam-column techniques.

The report is one of a series that describes work in Research Project No. 3-5-63-56, "Development of Methods for Computer Simulation of Beam-Columns and Grid-Beam and Slab Systems," and it may be desirable to review Report No. 56-1 (see List of Reports) for background.

The computer program presented here is written for the CDC 1604 and 6600 computers, but it is in FORTRAN language and only minor changes are required to make it compatible with IBM systems. Duplicate copies of the program deck and test data cards for the example problems in this report are available from the Center for Highway Research, The University of Texas at Austin.

The efforts of many people helped to produce the report. The assistance of the Texas Highway Department contact representative, Larry G. Walker, is greatly appreciated. The support of the U. S. Bureau of Public Roads is gratefully acknowledged. The excellent facilities of the Computation Center of The University of Texas and the cooperation of its staff have contributed significantly.

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LIST OF REPORTS

Report No. 56-1, "A Finite-Element Method of Solution for Linearly Elastic Beam-Columns" by Hudson Matlock and T. Allan Haliburton, presents a finite-element solution for beam-columns that is a basic tool in subsequent reports.

Report No. 56-2, "A Computer Program to Analyze Bending of Bent Caps" by Hudson Matlock and Wayne B. Ingram, describes the application of the beam-column solution to the particular problem of bent caps.

Report No. 56-3, "A Finite-Element Method of Solution for Structural Frames" by Hudson Matlock and Berry Ray Grubbs, describes a solution for frames with no sway.

Report No. 56-4, "A Computer Program to Analyze Beam-Columns under Movable Loads" by Hudson Matlock and Thomas P. Taylor, describes the application of the beam-column solution to problems with any configuration of movable non-dynamic loads.

Report No. 56-5, "A Finite-Element Method for Bending Analysis of Layered Structural Systems" by Wayne B. Ingram and Hudson Matlock, describes an alternating-direction iteration method for solving two-dimensional systems of layered grids-over-beams and plates-over-beams.

Report No. 56-6, "Discontinuous Orthotropic Plates and Pavement Slabs" by W. Ronald Hudson and Hudson Matlock, describes an alternating-direction iteration method for solving complex two-dimensional plate and slab problems with emphasis on pavement slabs.

Report No. 56-7, "A Finite-Element Analysis of Structural Frames" by T. Allan Haliburton and Hudson Matlock, describes a method of analysis for rectangular plane frames with three degrees of freedom at each joint.

Report No. 56-8, "A Finite-Element Method for Transverse Vibrations of Beams and Plates" by Harold Salani and Hudson Matlock, describes an implicit procedure for determining the transient and steady-state vibrations of beams and plates, including pavement slabs.

Report No. 56-9, "A Direct Computer Solution for Plates and Pavement Slabs" by C. Fred Stelzer, Jr., and W. Ronald Hudson, describes a direct method for solving complex two-dimensional plate and slab problems.

Report No. 56-10, "A Finite-Element Method of Analysis for Composite Beams" by Thomas P. Taylor and Hudson Matlock, describes a method of analysis for composite beams with any degree of horizontal shear interaction.

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ABSTRACT

A computer program, BMCOL 43, is presented which can efficiently analyze a beam-column subjected to movable static loads. The program is built around the computer simulation of a beam-column subjected to stationary loads which is described in Ref 1.

The methods used in the program are such that a beam-column can be analyzed for any pattern of transverse loads that move across the member. These load patterns may be any diverse system of loadings such as a highway truck, a series of trucks, or possibly a train on a railroad structure. In addition, the effects of fixed loads can be included in the analysis. Furthermore, the effects of various changes in the support history (such as settlement of a support) can be studied. The results of an analysis may include (1) solutions for the member under fixed loads, (2) influence diagrams for movable-load patterns, and (3) envelopes of maximum values of deflection, bending moment, shear, and support reaction that occur due to the loading and support history.

Five example problems that are typical of those encountered by highway structural designers illustrate the uses of the program and the options available.

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NOMENCLATURE

<u>Symbol</u>	<u>Typical Units</u>	<u>Definition</u>
a	-	Coefficient in stiffness matrix
b	-	Coefficient in stiffness matrix
c	-	Coefficient in stiffness matrix
d	-	Coefficient in stiffness matrix
e	-	Coefficient in stiffness matrix
E	lb/in. ²	Modulus of elasticity
f	-	Coefficient in load matrix
F	lb-in. ²	Flexural stiffness = EI
h	in.	Increment length
i	-	Station number
I	in. ⁴	Moment of inertia of the cross section
M	in.-lb	Bending moment
P	lb	Axial tension or compression
Q	lb	Concentrated applied transverse load
Q ^R	lb	Support reaction
Q ^{RR}	lb	Transverse force due to a rotational restraint
Q ^T	lb	Transverse force due to an applied couple
Q _f	lb	Fixed load
Q _m	lb	Movable load
R	in.-lb/rad	Concentrated rotational restraint
S	lb/in.	Concentrated transverse spring restraint
T	in.-lb	Concentrated applied couple

<u>Symbol</u>	<u>Typical Units</u>	<u>Definition</u>
V	lb	Shear
w	in.	Transverse deflection
θ	radians	The simple-difference slope of the individual bars
θ^J	radians	The average slope of adjacent bars at a joint

CHAPTER 1. INTRODUCTION

To design a structural member, the engineer needs the extreme values of moment, shear, deflection, and reaction that occur at key points due to any combination of prescribed design loadings. For a complete design, the extreme values at all points in the member must be known. These design maximums are of particular importance to the highway bridge designer who is primarily concerned with moving loads. Static loads that can cause these maximum values are of two types: fixed loads and movable loads. Straightforward methods are available for determining the maximum forces and moments in members that sustain only fixed loads, but approximations and engineering judgment are often necessary to expedite the design of members which sustain movable static loads. The computer program described in this report allows the user to efficiently determine the critical values of the design parameters at any location along the length of a beam-column subjected to fixed and movable loads. The computer program utilizes the finite-element beam-column solution presented in Ref 1.

Purposes and Uses of Program BMCOL 43

Computer Program BMCOL 43 is a finite-element simulation of a linearly elastic beam-column subjected to fixed and movable loads. The program is versatile and efficient for (1) solving beams under fixed loads, (2) computing influence diagrams, and (3) computing envelopes of maximum deflections, bending moments, shears, and reactions that may be accumulated under any desired sequence of fixed and movable loads.

Fixed-Load Solution. BMCOL 43 can be used to determine the deflection, slope, bending moment, and shear at each point of a beam-column loaded with fixed loads. Each support reaction can also be determined.

Envelopes of Maximums. The most important use of the program is the progressive selection and accumulation of the maximum positive and negative values of deflection, bending moment, shear, and reaction at each station along a beam-column for a given load and support history. The collection of

positive and negative maximums for each quantity (deflection, bending moment, etc.) results in an envelope of maximums. Included in the maximum values are the effects of fixed loads, movable loads, and changes in support conditions (such as settlement of a support). Envelopes of maximums provide the information usually needed in design.

Influence Diagrams. BMCOL 43 can be used to determine the influence of the position of a load pattern on the deflection, bending moment, shear, and support reaction at any point along a beam-column. The load pattern can be a unit load, which is conventionally used for computing influence diagrams. On the other hand, any desired pattern of loads may be used, possibly to represent a series of vehicles. The resulting diagrams give the integrated influence of the load pattern more directly and are therefore more convenient to use in actual design problems.

Review of Remaining Chapters

The following chapters describe the various aspects of the movable-load analysis. Chapter 2 presents a solution of a statically loaded beam-column which is very similar to the method presented in Ref 1. The uses of Program BMCOL 43 and the options available to the user are described in Chapter 3. In Chapter 4 a detailed description of the computer program is presented. Example problems are shown in Chapter 5, and Chapter 6 summarizes the uses and capabilities of the program.

CHAPTER 2. SOLUTION OF A STATICALLY LOADED BEAM-COLUMN

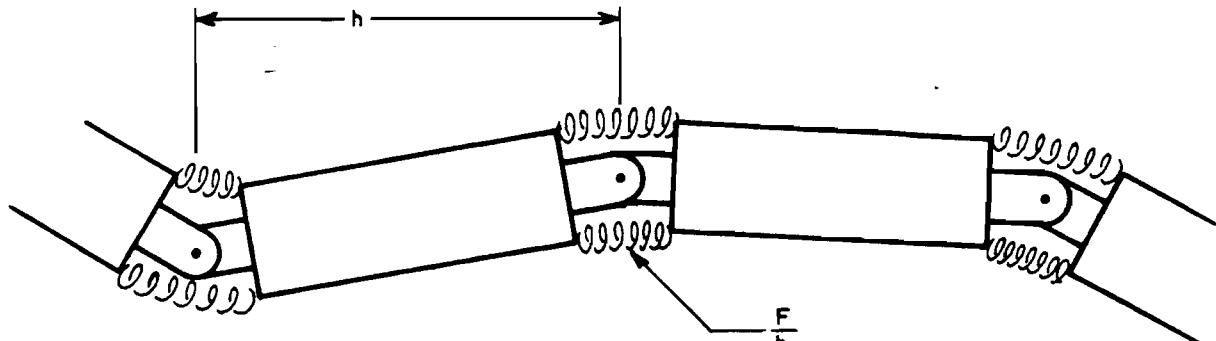
Program BMCOL 43 was developed for the analysis of beam-columns under movable static loads. No dynamic effects are included in the analysis. The program is based on the solution of a beam-column under a stationary load pattern, and, except for slight modifications, the procedures described in Ref 1 are used. The program makes one complete beam-column solution for each of many uniformly spaced locations of a load pattern that moves across the member. A brief review of the beam-column method is included in this chapter.

Computer Simulation of a Beam-Column

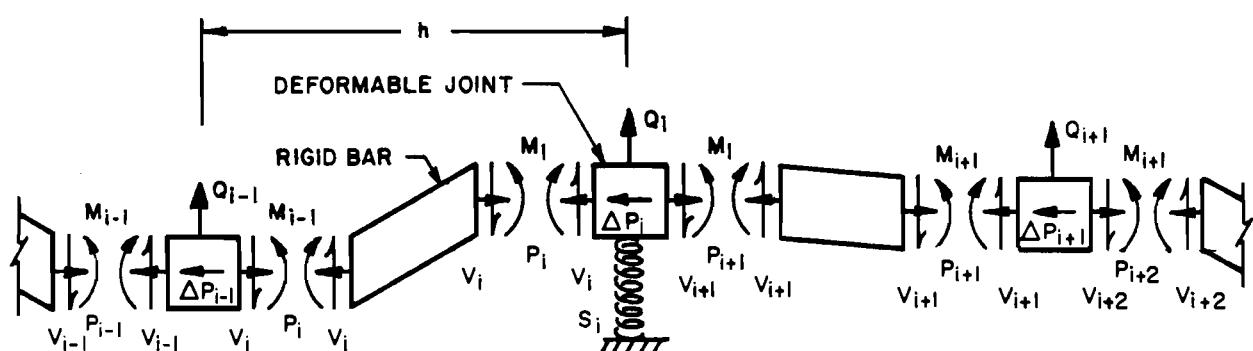
It has been shown in Ref 1 that a model composed of rigid bars and springs (Fig 1a) can be used to simulate a beam-column. In this model, all external loads and restraints are introduced at the joints between the bars. Spring-restrained hinges at the joints represent the flexural stiffness of the actual beam-column. The flexural stiffness is defined as the product EI and is represented by the letter F . The increment length is the distance between joints and is designated by h . All stiffness and load values may vary in a freely discontinuous manner along the model, and equations which describe the model are susceptible to a quick and efficient computer solution.

A representation of the bar-and-spring model with the internal and external forces and restraints acting on Joint i is shown in Fig 1b. The load normal to the axis of the beam-column is represented by Q and a linearly elastic spring support is represented by S . Axial forces applied to a beam-column will develop axial tension or compression P in the bars and joints. The moment M and the shear V are developed in the bars and joints. It should be noted that the shear and axial tension are constant throughout the length of each rigid bar.

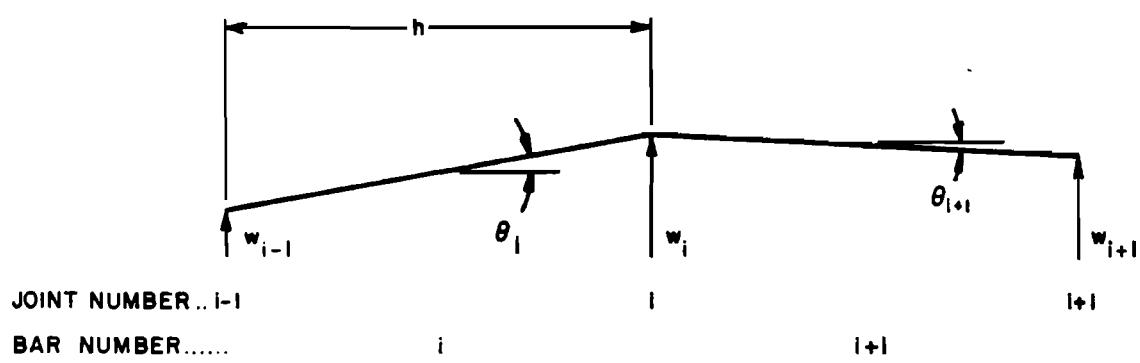
The joint-and-bar numbering system is shown in Fig 1c. Note that the bar to the left of each joint has the same number as that joint. The symbol θ measures the slope of the bars, and w represents the deflection which is positive upward.



(a)



(b)



(c)

Fig 1. Mechanical model of beam-column.

Equilibrium equations of Joint i and Bars i and $i+1$, combined with the flexure equation, result in the following equation for the deflected shape of a beam-column:

$$a_i w_{i-2} + b_i w_{i-1} + c_i w_i + d_i w_{i+1} + e_i w_{i+2} = f_i \quad (2.1)$$

where

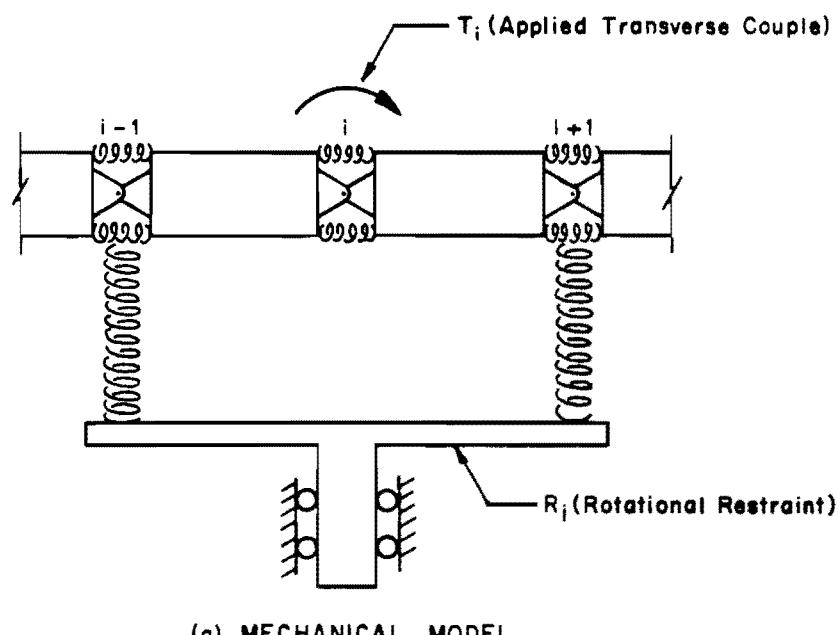
$$\begin{aligned} a_i &= F_{i-1} - 0.25hR_{i-1} \\ b_i &= -2(F_{i-1} + F_i) - h^2 P_i \\ c_i &= F_{i-1} + 4F_i + F_{i+1} + h^3 S_i + 0.25h(R_{i-1} + R_{i+1}) \\ &\quad + h^2(P_i + P_{i+1}) \\ d_i &= -2(F_i + F_{i+1}) - h^2 P_{i+1} \\ e_i &= F_{i+1} - 0.25hR_{i+1} \\ f_i &= h^3 Q_i - 0.5h^2(T_{i-1} - T_{i+1}) \end{aligned} \quad (2.2)$$

The R terms in the above equations represent the effect of a rotational restraint at Station i which is modeled as a differential spring system as shown in Fig 2a. These restraint springs provide a force couple that resists a change in slope of the beam-column, but does not restrain deflection at Station i .

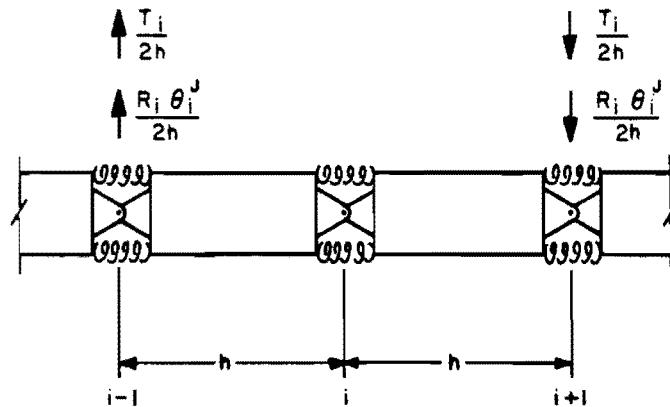
In the last equation above, the T terms represent the effects of externally applied couples at Station i which are felt as equal and opposite transverse loads at the adjacent stations as shown in Fig 2b.

A detailed derivation of Eqs 2.1 and 2.2 is included in Appendix 1.

When Eq 2.1 is written for every joint along the model, the resulting system of equations forms a diagonally banded matrix of stiffness terms. A two-pass, recursive technique described in Ref 1 is used to solve the system of equations for the deflection of every joint. Once the deflected shape of



(a) MECHANICAL MODEL



(b) EQUIVALENT FORCES

Fig 2. Rotational restraint R and applied couple T acting on the mechanical model.

a member is known, the slopes, bending moments, shears, and support reactions for the member can be determined by using the procedures described below.

Slope. Equation 2.3 is the simple-difference expression for slope of the individual Bar i in terms of the deflections of Joints $i-1$ and i and the increment length (see Fig 1c).

$$\theta_i = \frac{-w_{i-1} + w_i}{h} \quad (2.3)$$

The slope of the bar-and-spring model is not defined at the joint, but the slope of the actual beam represented by the model can be very closely approximated as the average of the slopes of the adjacent bars:

$$\theta_i^J = \frac{\theta_i + \theta_{i+1}}{2} = \frac{-w_{i-1} + w_{i+1}}{2h} \quad (2.4)$$

When the results are visualized in terms of the bar-and-spring model, it is more convenient to deal with the slope of the individual bars. It will be seen in results printed from BMCOL 43 that the slope is printed between stations; this slope is that of the bar between the indicated stations.

Bending Moment. In conventional beams, the bending moment is equal to the product of the flexural stiffness and the curvature. In the finite-element model the flexibility of the beam and the curvature are lumped at the station points. The corresponding relation for bending moment M in the model is

$$M_i = \frac{F_i}{h} \left(\frac{w_{i-1} - 2w_i + w_{i+1}}{h} \right) \quad (2.5)$$

Shear. Shear V_i in the model is found from the equation of moment equilibrium of Bar i (Fig 1). Thus,

$$V_i = \frac{-M_{i-1} + M_i}{h} - P_i \left(\frac{-w_{i-1} + w_i}{h} \right) \quad (2.6)$$

As seen in Fig 1b, V_i is the shear throughout the length of Bar i . Therefore, Program BMCOL 43 is written such that the shear computed in each bar is printed between the adjacent stations. In conventional analysis, the effects of rotational restraints and applied couples are considered as concentrated at a point. In BMCOL 43 they act on the beam as equal and opposite loads separated by two increments as shown in Fig 2b. Therefore, the shear for only the bars

adjacent to stations with applied couples or rotational restraints is affected by these loads and is not the same as conventional shear.

Support Reaction. As described in Ref 1, rotational restraints and applied couples are introduced into the beam-column model as shown in Fig 2. When rotational restraints R and applied couples T act at Joints $i-1$ and $i+1$, and a linear spring support S_i resists deflection at Station i , a free-body diagram of Joint i is as shown in Fig 3a. If instead of a spring support S_i , Joint i is supported on a non-yielding support, the free-body diagram of the joint is as shown in Fig 3b.

If Joint i is supported on a linear spring S_i , the reaction Q_i^R is

$$Q_i^R = -S_i w_i \quad (2.7)$$

If Joint i is supported on a non-yielding support (deflection w_i equal to zero), the reaction Q_i^R can be found from equilibrium of vertical forces on the joint shown in Fig 3b. Thus the reaction is

$$\begin{aligned} Q_i^R = & -v_i + v_{i+1} - Q_i - \frac{-T_{i-1} + T_{i+1}}{2h} \\ & - \frac{-R_{i-1}\theta_{i-1}^J + R_{i+1}\theta_{i+1}^J}{2h} \end{aligned} \quad (2.8)$$

If Eqs 2.3 and 2.6 are substituted into Eq 2.8, the support reaction is

$$\begin{aligned} Q_i^R = & \frac{M_{i-1} - 2M_i + M_{i+1}}{h} - Q_i + \frac{T_{i-1} - T_{i+1}}{2h} \\ & - \frac{R_{i-1}w_{i-2} - R_{i-1}w_i - R_{i+1}w_i + R_{i+1}w_{i+2}}{4h^2} \\ & - \frac{P_i w_{i-1} - P_i w_i - P_{i+1} w_i + P_{i+1} w_{i+1}}{h} \end{aligned} \quad (2.9)$$

Comparison of the Methods of BMCOL 34 and BMCOL 43

The reader who has used Program BMCOL 34 (Ref 1) will notice that the equations used in that program differ slightly from those developed above. In the current development, axial tension and transverse shear are held constant within each bar. In Program BMCOL 34, the changes in these two parameters take place within the bars. It should be re-emphasized, however, that

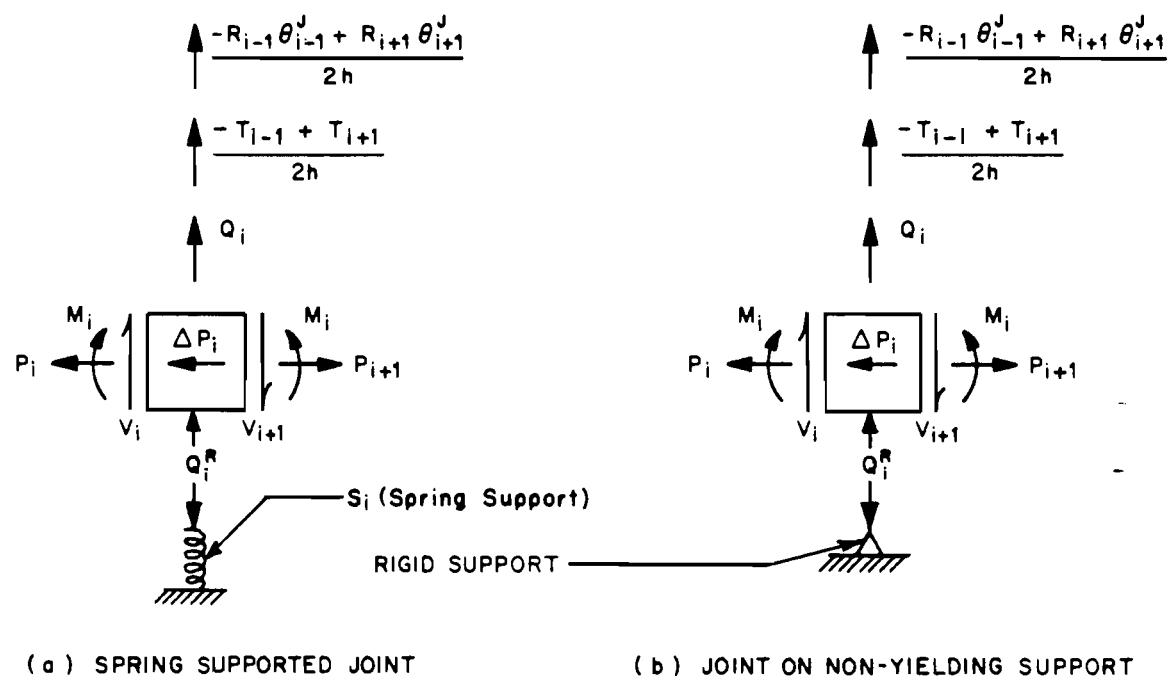


Fig 3. Support reaction at Joint i .

the basic methods and procedures used in formulating the equations in terms of a bar-and-spring model and the recursive technique to solve the system of equations are identical.

CHAPTER 3. FEATURES OF THE PROGRAM

Several different types of results are produced by BMCOL 43. The method of computation and uses of each are explained in this chapter. In addition, the important features of the program are discussed.

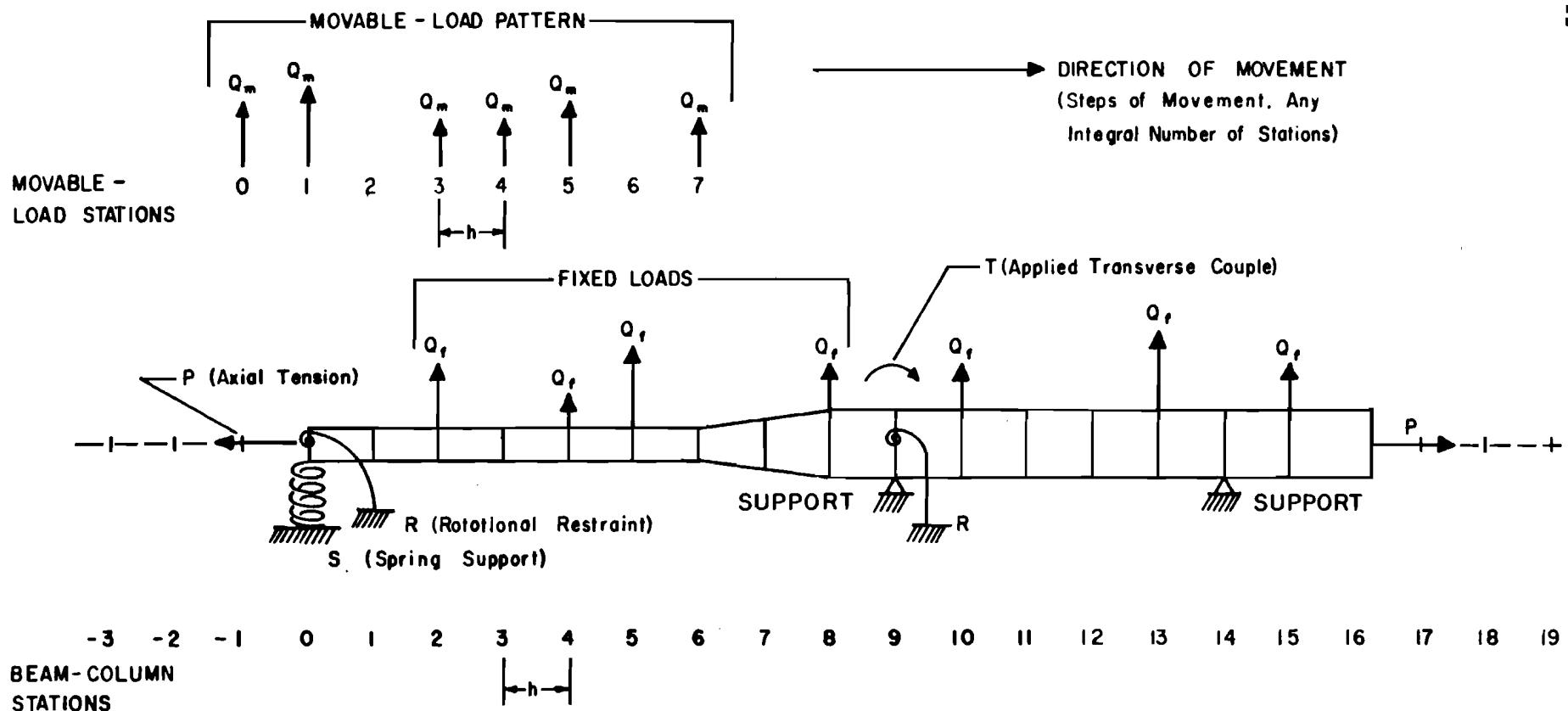
Fixed Loads

There are two types of transverse loads that may act on BMCOL 43: fixed loads and movable loads. For the fixed-load solution, only the fixed loads are applied and the beam-column is solved for deflections, slopes, bending moments, shears, and support reactions. The fixed-load solution is a quick and efficient means of solving a beam-column under a particular configuration of loads and restraints.

Movable Loads

The most important feature of BMCOL 43 is its ability to simulate the movement of a load pattern along a beam-column. As shown in Fig 4, the movable-load pattern may be a diverse system of loads. It is described according to its own system of station numbers. The movable-load station system is completely independent of the beam-column station system except that both must have the same increment length. Any desired range of movement may be specified by the user by giving initial and final positions of the movable-load pattern. All positions of the load pattern along the beam-column station system are given in terms of the position of the zero station of the movable-load system.

The movable-load analysis does not include dynamic effects; it is a series of static-load solutions. A load pattern is shifted in steps within the designated range of movement. The number of increments between positions of the movable load is specified by the user. For each position of the load pattern, the movable loads are added to the fixed loads and the beam-column is completely solved. After the beam-column has been solved, the load pattern is moved to the next position and the process is repeated. The results from the movable-load solutions are combined into envelopes of maximums..



LOADS AND FORCES ARE SHOWN
IN THEIR POSITIVE SENSE

Fig 4. A beam-column with a movable-load pattern.

Envelopes of Maximums

Envelopes of deflection, bending moment, shear, and support reaction are produced by a comparison process. The envelopes of maximums contain the extreme values, both positive and negative, which every station of the beam-column has experienced. Each beam-column solution is compared with the existing envelopes and the envelopes are expanded when a new maximum is encountered at a point.

Envelopes are automatically generated if the movable-load capability is exercised. In addition, an option is available which allows the envelopes to be retained and expanded through a sequence of separate but related independent problems. The option exercised is completely independent of the movable-load capability. This means that the envelopes may be held over a sequence which contains both fixed-load and moving-load problems.

The example problems of Chapter 5 illustrate the usefulness of the envelopes of maximums.

Influence Diagrams

The influence diagrams of BMCOL 43 may be used to study the behavior of a particular point in the beam-column as a load pattern is moved across the member. Influence diagrams may be obtained for deflection, bending moment, shear, or support reaction. Fixed loads are not included in the solutions for influence diagrams. Computation of the influence diagrams is based only on the movable-load. A load pattern is moved, step by step, between the two designated end stations, and the beam-column is solved for each position of the load. The values that are computed for the designated influence-line stations are stored, as well as the location of the movable load that caused each value.

It should be noted that the influence diagrams of BMCOL 43 are much more general than conventional influence diagrams because the beam-column may be solved under any type of support condition and any type of load, including the bending effect of axial loads. Also, any configuration of movable load may be used to generate them. The BMCOL 43 influence diagrams will be equivalent to conventional influence diagrams if the movable-load pattern consists of a concentrated unit load.

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CHAPTER 4. DESCRIPTION OF BMCOL 43

General

Program BMCOL 43 is written in FORTRAN 63 language for the Control Data Corporation 1604 and 6600 computers. With minor changes, the program would be compatible with IBM 7090 computers or with other systems. Four FORTRAN subroutines are included in the program. Compile time is approximately 3 minutes for a FORTRAN version of the program. The program storage requirement is 24,000 words. The definitions of symbols used in the program and a listing of the program are included in Appendix 4 and Appendix 5.

The beam-column must be divided into a number of equal increments which are designated by station numbers. The left end of the beam-column must be located at Station 0. Any number of increments up to 200 can be used. Computation time and the accuracy of the solution both increase with the number of increments that are used. Therefore, it is desirable to use the minimum number of increments that will yield the desired accuracy. Experience will enable the user to select the proper number of increments.

In the finite-element model, all loads and restraints are defined at the station points (joints). Therefore, distributed loads and restraints from one-half of the increment on each side of the station are "lumped" at the station. End stations receive half-values of distributed effects. The input of distributed effects is facilitated by SUBROUTINE INTERP3. This subroutine interpolates linearly between the extreme values of a distribution sequence and stores an interpolated value at each intermediate station. Concentrated loads that occur between stations should be split by the user to the two adjacent stations.

Any system of units may be used to describe the problem (for example, pounds and inches), but the system must be used consistently. The sign convention is shown in Fig 4. Upward deflections are positive.

Procedure for Data Input

A guide for data input is included in Appendix 2. The guide is designed so that additional copies may be furnished as separately bound extracts for

routine use. A parallel study of the guide will help the reader to understand the following discussion.

Any number of problems may be stacked and run together. The sequence of problems is preceded by two cards which describe the run. The first card of each problem contains the problem number and a brief description of the problem. The program continues working problems until a blank problem number or a data error is encountered; then the run is terminated.

Tables of Data Input

Table 1 is the data-control table. It consists of a single card which must be input for each problem. The number of cards in the remaining tables and the program options are specified in this table. The program options will be discussed later in this chapter.

Table 2 contains the following:

- (1) number of increments into which the beam-column is divided,
- (2) increment length,
- (3) number of increments in the movable-load pattern,
- (4) start position of the movable-load pattern,
- (5) stop position of the movable-load pattern, and
- (6) the size (number of increments) of each stepwise movement of the movable-load pattern.

The start position is the beam-column station where the zero station of the movable load is placed for the first movable-load solution. Any positive station may be designated as the start station. A negative start station is permissible if after one stepwise movement of the movable-load pattern, a portion of the pattern is on the beam. The stop station is the last beam-column station where the zero station of the movable-load pattern is placed. It may be any station on the beam-column or any station of no more than one step of movement past the right end of the beam-column. Each stepwise movement of the movable-load pattern may be any integral number of beam-column increments. Larger steps require less computation time. If the number of movable-load steps is specified to be zero, the problem is terminated immediately after the fixed-load solution.

For any station in the beam, a deflection, slope, or both may be specified in Table 3. A specified deflection is equivalent to a single support. A

fixed-end support can be simulated in the computer by the specification of a zero deflection and a zero slope at the same station. The ability to specify a deflection other than zero provides the user with a simple method of studying problems in which the supports settle. Due to the method of simulation (see Fig 2), the specified slope and deflections must conform to the following minimum spacing requirements:

- (1) A slope may not be specified closer than three increments from another specified slope.
- (2) A deflection may not be specified closer than two increments from a specified slope, except that both a slope and a deflection may be specified at the same station.

Slope and deflection conditions may be specified at no more than 20 stations. Each specification requires a separate card. The cards may be stacked in any order within the tables.

Fixed loads and restraints are described in Table 4. The input values are beam stiffness, fixed loads, applied couples, rotational restraints, support springs, and axial tensions. Couples and rotational restraints appear only as concentrated effects, while axial tensions are usually distributed. The remaining values may be either concentrated or distributed. The method used for the description of distributed data is illustrated in Appendix 2. All of the input values of Table 4 are accumulated algebraically in storage. Therefore there are no restrictions on the order of the cards, except that within a distribution sequence the stations must be in ascending order. Axial tensions must be described in the same manner as other values in this table because there is no provision in the program for automatically distributing the internal effects of an externally applied load. For example, an axial load applied to the ends of the beam-column must be specified as an axial tension at each interior station. The number of cards in Table 4 must not exceed 100.

Table 5 is used to describe the movable load. The description is based on a system of stations which is independent of the beam-column stations, - except that both systems must have the same increment length (see Fig 4). Data in this table are input the same as in Table 4. The load pattern is limited by the number of increments in the beam. The maximum allowable number of cards is 100.

Stations selected for influence diagrams are designated in Table 6. Diagrams may be specified for each parameter: deflection, bending moment, shear,

and support reaction. The number of cards in this table is always four (one or more diagrams desired) or zero (no diagrams desired). Because of a core storage limitation, no more than 5 stations may be specified on each parameter card. Each parameter may have a different set of specified stations. Each card contains the following:

- (1) number of influence diagram stations (1 to 5),
- (2) type of output (results may be plotted, tabulated, or both), and
- (3) the stations for which influence diagrams are desired.

The cards must be stacked in the same order as the previous list. If no influence lines are desired for one of the parameters, a blank card must be inserted for that parameter.

Program Options

The data-hold options, which are contained in Table 1, allow the user to retain any of the data tables from the preceding problem except Table 1. If Table 2, 3, or 6 is held, it may not be modified; the number of added cards specified for it must equal zero. The data in Tables 4 and 5 may be held and modified by the addition of new cards, but the total accumulated number of cards in each of these tables must be less than 100.

The hold options for the various tables are independent of each other, but care must be exercised in order to insure that the data in the various tables are compatible. For example, if a previous 40-increment problem had a distributed load from Station 0 to Station 40, then the user should not change the total number of increments in the new problem to some number less than 40 unless he erases the loads past the end of the new beam.

As explained in Chapter 3, the envelopes of maximums may be retained and modified through a series of problems. If the envelopes are not held, they are set equal to zero before the fixed-load solution of each problem. If the user so specifies, the envelopes of maximums will be automatically plotted in a 4-inch by 10-inch space.

The first three example problems of Chapter 5 are shown on typical coding forms in Fig 5. The user may desire to have specialized forms made for particular types of problems.

Fig 5. Coded input data for first three problems.

Error Messages

Checks for common types of data errors are included in the program. An error message which defines the error is printed if any of the following conditions occur:

- (1) the start or stop station for the movable load is improperly specified,
- (2) a slope or deflection is specified too close to another slope or deflection,
- (3) the allowable number of cards for an input table is exceeded,
- (4) data is input at stations beyond the end of the beam-column,
- (5) the station numbers in a distribution sequence are out of order, or
- (6) the allowable number of influence diagrams is exceeded.

In addition to the specific error messages, a general purpose error message is provided for a number of unlikely errors. If the message "UNDESIGNATED ERROR STOP" is printed, the user must investigate the program to determine what caused the error. Any error detected by the program will cause the entire run to be abandoned.

Description of Problem Results

The output of results is arranged so that the input quantities of Tables 1 through 6 are available for all problems and are printed with explanatory headings. Table 7 presents the fixed-load results prior to any movable-load solutions. Table 8 tabulates the envelopes of maximums for deflection, moment, shear, and reaction that have occurred due to the moving load acting on the beam-column in addition to any fixed loads. Beside each value in the envelopes of maximums, the movable-load location that caused the maximum is tabulated. A location equal to 999 indicates that the maximum was caused by fixed loads. A location indicator followed by an asterisk means that the maximum was created and held from a prior problem. Table 9 presents the scales for the plots of the envelopes of maximums. Tables 10A through 10D are a tabulation of any influence diagrams that have been specified. The influence diagrams are for the movable load only; the effect of fixed loads are not included. Scales for the influence diagrams are printed in Table 11.

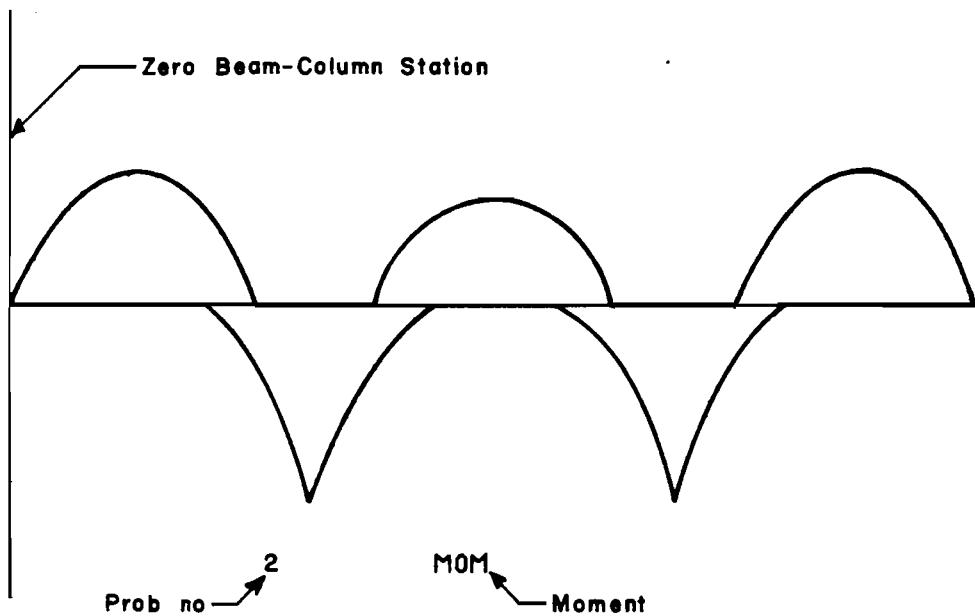
Plots of Results

Options are available which allow the user to obtain plots of the envelopes of maximums and the influence diagrams. Both types of plots fit in a 4-inch by 10-inch space. The optimum scales for the plots, both horizontal and vertical, are automatically selected by the program.

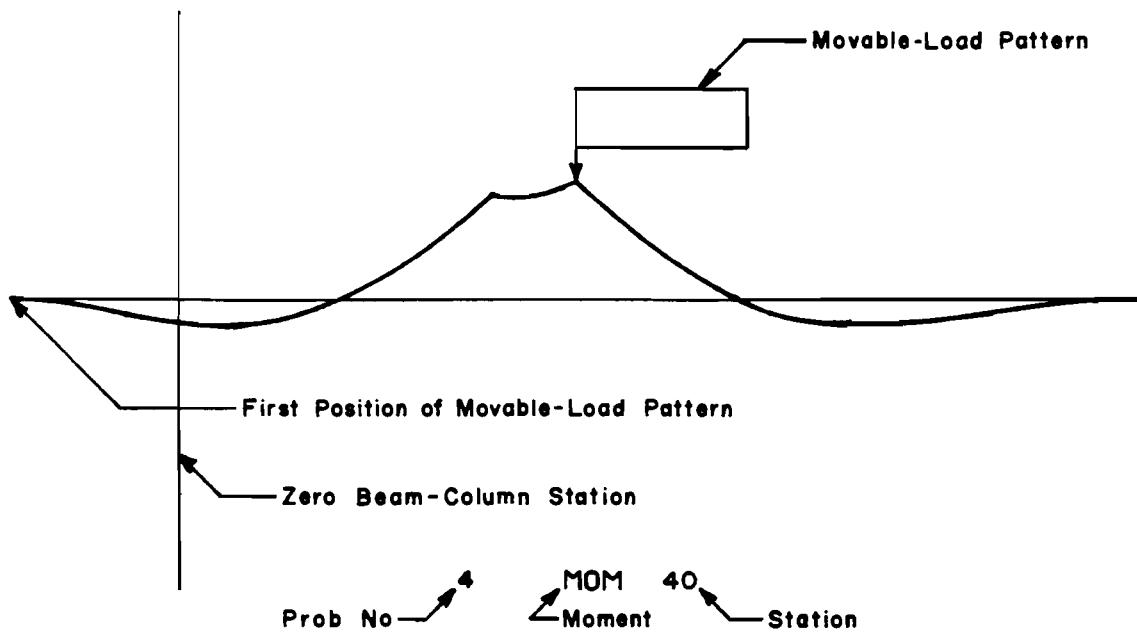
An example plot of an envelope of maximum moments for a 3-span structure subjected to a movable-load pattern is shown in Fig 6a. Beneath the plot, the problem number and an identification of the plot variable are printed. The plot is from Example Problem 2 described in Chapter 5.

A plot of an influence diagram for positive moment in the center span of the same structure is shown in Fig 6b. The left end of the beam-column, Station zero, is designated by the vertical axis. Points to the left of the vertical axis indicate that the zero station of the movable load is not on the beam-column. To the right of the vertical axis is the plot. The set on the left is the problem number. In the center, DEFL, MOM, SHEAR, or REACT appears, which indicates the plot variable (Deflection, Moment, Shear, Reaction). On the right is the station number for which the influence diagram is plotted. This plot is from Example Problem 1 of Chapter 5.

No special provision is made in the program for plotting results of the fixed-load solution. However, such plots may be obtained by plotting envelopes of maximums that contain nothing but the fixed-load solution.



(a) A TYPICAL ENVELOPE OF MAXIMUM MOMENTS



(b) A TYPICAL MOMENT INFLUENCE DIAGRAM

Fig 6. Typical plots.

CHAPTER 5. EXAMPLE PROBLEMS

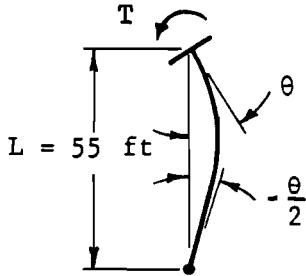
Some of the various uses of BMCOL 43 are illustrated by the following example problems. The same basic structure is used for the first four problems, with varying input data. The coded input data for the first three problems are shown in Fig 5. A listing of input data is included in Appendix 6, and the computer output listing is included in Appendix 7.

Example Problem 1. Three-Span Structure - Fixed Load

Figure 7 shows a three-span concrete slab structure, 42 ft wide, with haunches over the interior supports. Each interior support consists of three rigidly connected concrete columns of 30-inch diameter.

A computer model of a one-foot-wide longitudinal strip of the structure is divided into 80 twenty-four-inch increments. The stiffness of the haunched sections is assumed to vary linearly from 4.147×10^{10} lb-in² to 8.100×10^{10} lb-in². For a linear taper of beam depth this is not precisely correct for stations between the limits, but the error is negligible.

The estimate of the rotational restraint to the slab provided by the columns is computed as follows:



An assumption is made that the clay offers no side support and the column acts as if pinned at the sand stratum. Therefore, a rotation at the top induces one-half as much rotation at the bottom (see sketch).

The couple T from the conventional slope-deflection equation is

$$T = \frac{4EI}{L} \theta + \frac{2EI}{L} \left(-\frac{\theta}{2} \right) = \frac{3EI}{L} \theta \quad (5.1)$$

The rotational restraint R is equal to

$$R = \frac{T}{\theta} = \frac{3EI}{L} \quad (5.2)$$

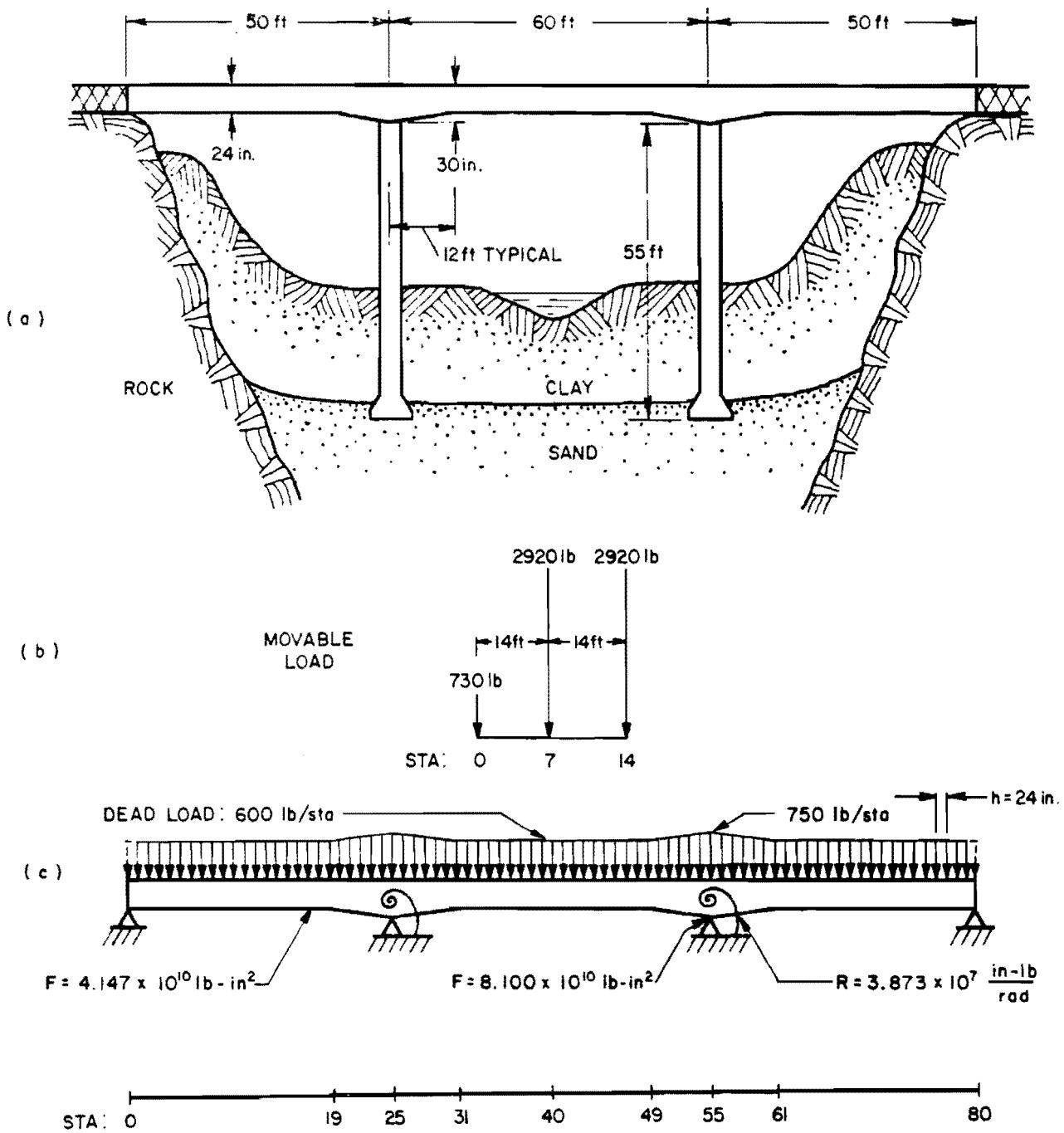


Fig 7. Example Problems 1, 2, 3, and 4.
Concrete slab structure.

$$R = \frac{3(3.0 \times 10^6) (3.976 \times 10^4)}{12(55)} = 5.422 \times 10^8 \frac{\text{in-lb}}{\text{radian}}$$

For a one-foot-wide longitudinal slab strip,

$$R = (5.422 \times 10^8) \frac{3 \text{ cols}}{42 \text{ ft}} = 3.873 \times 10^7 \frac{\text{in-lb}}{\text{radian}}$$

Note that if the column had been assumed fixed at the bottom, the restraint would be 4/3 of the computed R when pinned.

Values of shear that are computed in bars immediately adjacent to stations at interior supports should be disregarded because of the rotational restraints at those stations.

This first problem is intended to show the effect of fixed loads (i.e., dead loads) on the structure. Normally it would not be necessary to have a separate fixed-load run since fixed-load results are computed and tabulated in all problems. If a maximum occurs due to dead load only, that maximum is held at that point and the control is listed as 999, which indicates it was caused by fixed loads. This fixed-load solution will give identical results as those that could be obtained by the use of a BMCOL 34 solution (Ref 1) except for the listed values of slope, shear, and support reactions as explained in Chapter 2.

Example Problem 2. Three-Span Structure - Movable Load

This problem demonstrates the effect of a movable load on the structure. The data that describe the support conditions, fixed loads, and beam stiffness are retained from Problem 1 by holding these values in Tables 3 and 4. Table 2 was not retained because of the added movable load. The movable-load pattern is moved completely across the structure by specifying a start station equal to -14 and a stop station equal to 80.

The envelopes of maximums generated in this problem show the maximum positive or negative values experienced by each station along the beam-column. They can be directly used in design since the effect of fixed loads, axial loads (if they had been present), and a single movable load are included. Figure 8 shows the envelope of deflections for this problem.

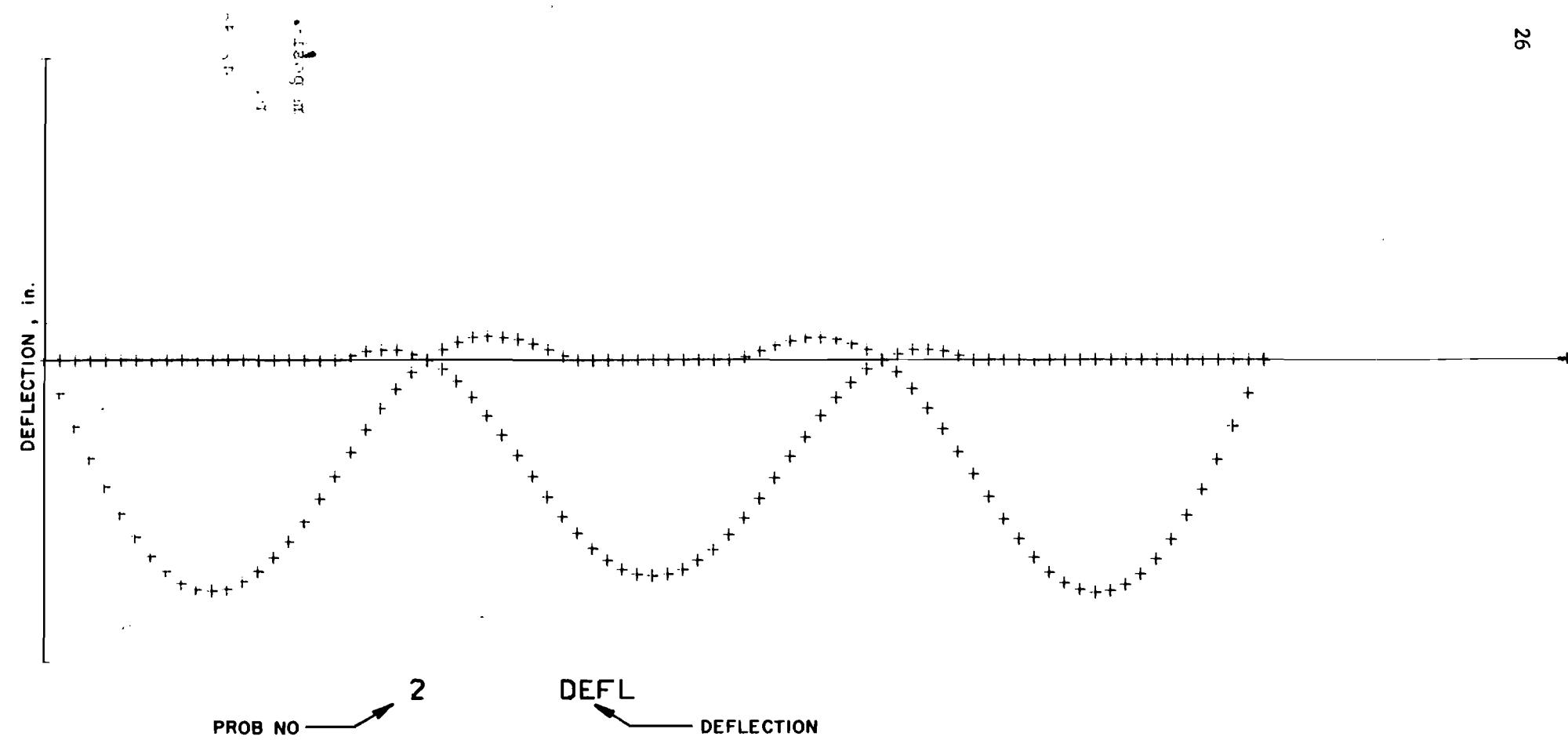


Fig 8. Example Problem 2. Envelope of deflections.

Example Problem 3. Three-Span Structure - Support Settlement

This problem illustrates how BMCOL 43 can be utilized to study settlement of supports. The same fixed loads, beam properties, and movable load are held from Problem 2. In addition, the envelopes of maximums are held so that any greater maximum positive or negative values induced by support settlement will be retained. This process could be extended through several problems so that accumulative envelopes of maximums representing all past loading and support history would be generated.

The first interior support at Station 25 is assumed to settle 1.25 inches and the second interior support at Station 55 settles 0.625 inch. The movable load is again moved completely across the structure. Comparison of the moments (Fig 9), shows that the support settlements with the same moving load caused a slight increase in the negative moment at the second interior support, and an increase in positive moments in the first two spans.

Example Problem 4. Three-Span Structure - Influence Diagrams

This problem demonstrates how the influence diagram feature of the program can be used to determine the number and location of a series of movable-load patterns that will cause the maximum condition at a desired location. From a study of the envelopes of maximums in Problem 2, it is decided to study moments at Stations 9, 25, and 40, and deflections at Stations 11 and 40. Had the location of these key points been previously determined, the influence diagrams could have been computed and plotted in Problem 2. Influence diagrams are generated for these stations by designating them in Table 6 of the input form.

The fixed loads and beam properties are again specified in this problem by using the same Table 7 data as used in Problem 2. The fixed-load results and envelopes of maximums are therefore seen to be the same as previously computed in Problem 2.

The only load placed on the beam-column for the influence diagram solutions is the movable-load pattern shown in Fig 7b. This load is moved entirely across the structure by specifying again as in Problem 2 a start station equal to -14 and a stop equal to 80. The incremental size of each stepwise movement of the pattern was chosen to be one station, although a larger incremental size could

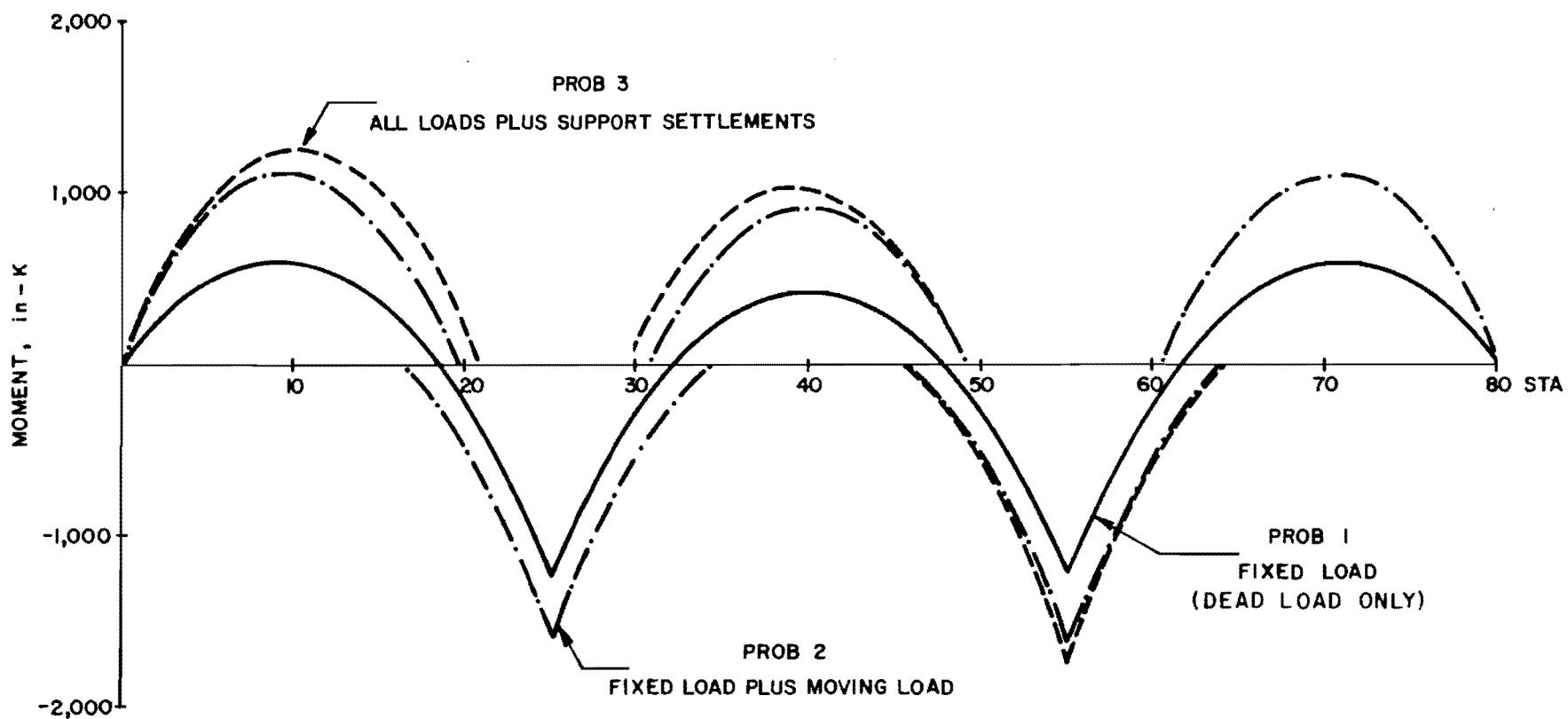


Fig 9. Envelopes of maximum moment.

be used to reduce computation time. To obtain conventional influence diagrams a unit concentrated movable load could be used.

Examination of the influence moment diagram at Station 9 (Fig 10) shows that for a single movable-load pattern, the maximum moment occurs with the zero station of the movable-load pattern at Station 2. This result is seen to agree with that in Problem 2. A worse condition could exist if there were two movable loads with the second one located at Station 57.

The moment influence diagram is loaded with these two movable loads (Fig 10). The zero station of each is the index which is used to place the load pattern on the influence diagram. From Table 10b of the computer results, a load pattern at Station 2 causes a moment of 514.0 in-kips, and a load at Station 57 causes a moment of 35.1 in-kips. The moment at Station 9 from the fixed-load solution in Problem 1 was 599.5 in-kips. The addition of these three moments gives the maximum possible moment at Station 9 of 1148.6 in-kips.

A similar analysis of the other maximums of deflection and moment could be made. Influence diagrams for shear and reaction at particular points could also have been computed if further investigation were necessary.

Example Problem 5. Two-Span Railroad Girder

This problem shows how BMCOL 43 can be used to solve a structure with many movable loads. A rigorous solution of the problem for necessary information for a complete design would require many repeated solutions with programs presently available, or tedious trial-and-error hand solutions.

Figure 11a represents one girder of a two-span continuous railroad structure 86 ft long. The girder, deck system, ballast, and rails place a total dead load of 1000 lb per linear ft on each girder. A "live" or movable load of a portion of one track loading is also carried by each girder. The girder is supported with no restraint at the center support, but the end supports are fixed against translation. Due to temperature expansion, an axial longitudinal force of 33,000 lb is applied to each girder.

The resulting envelopes of maximum deflection and moment are shown in Fig 11d and 11e.

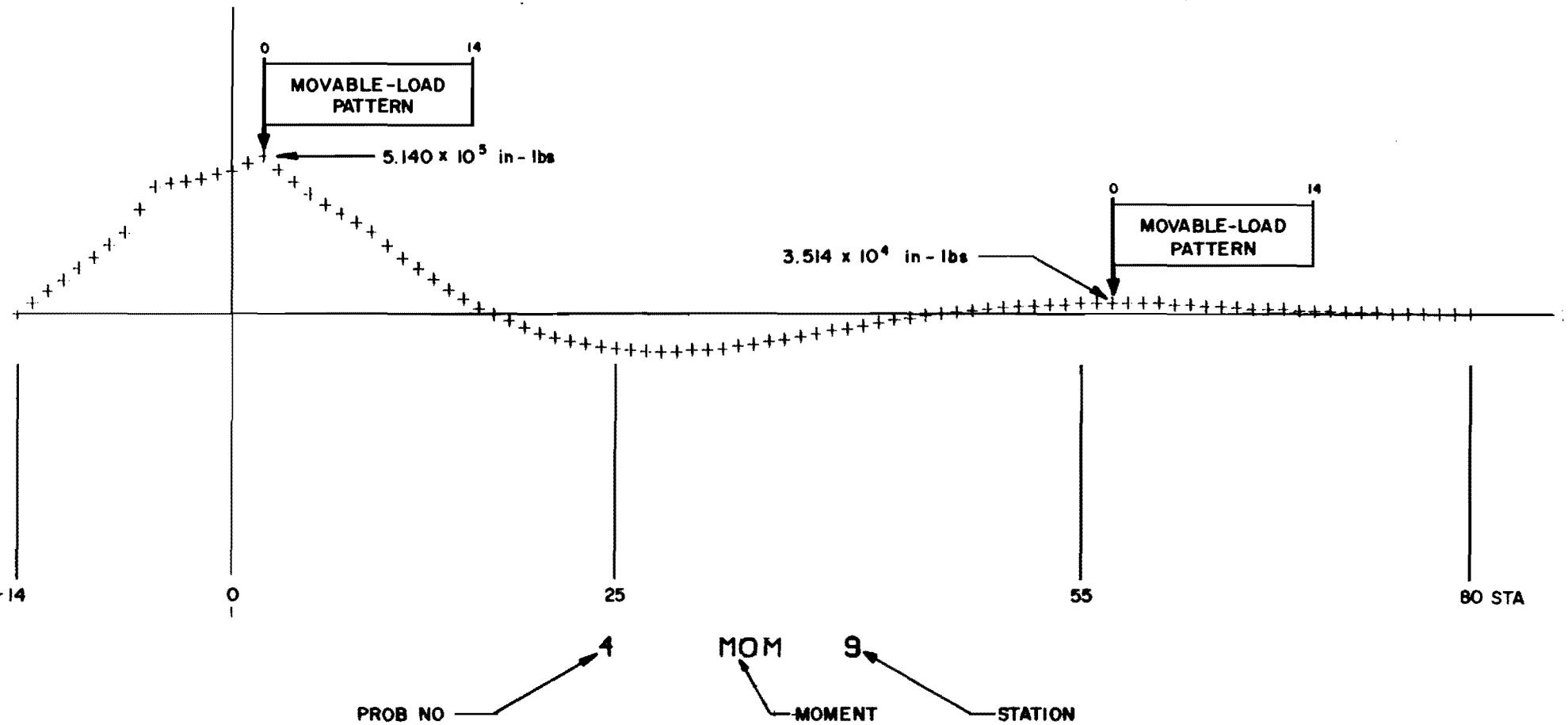


Fig 10. Example Problem 4. Influence diagram for moment at Station 9.

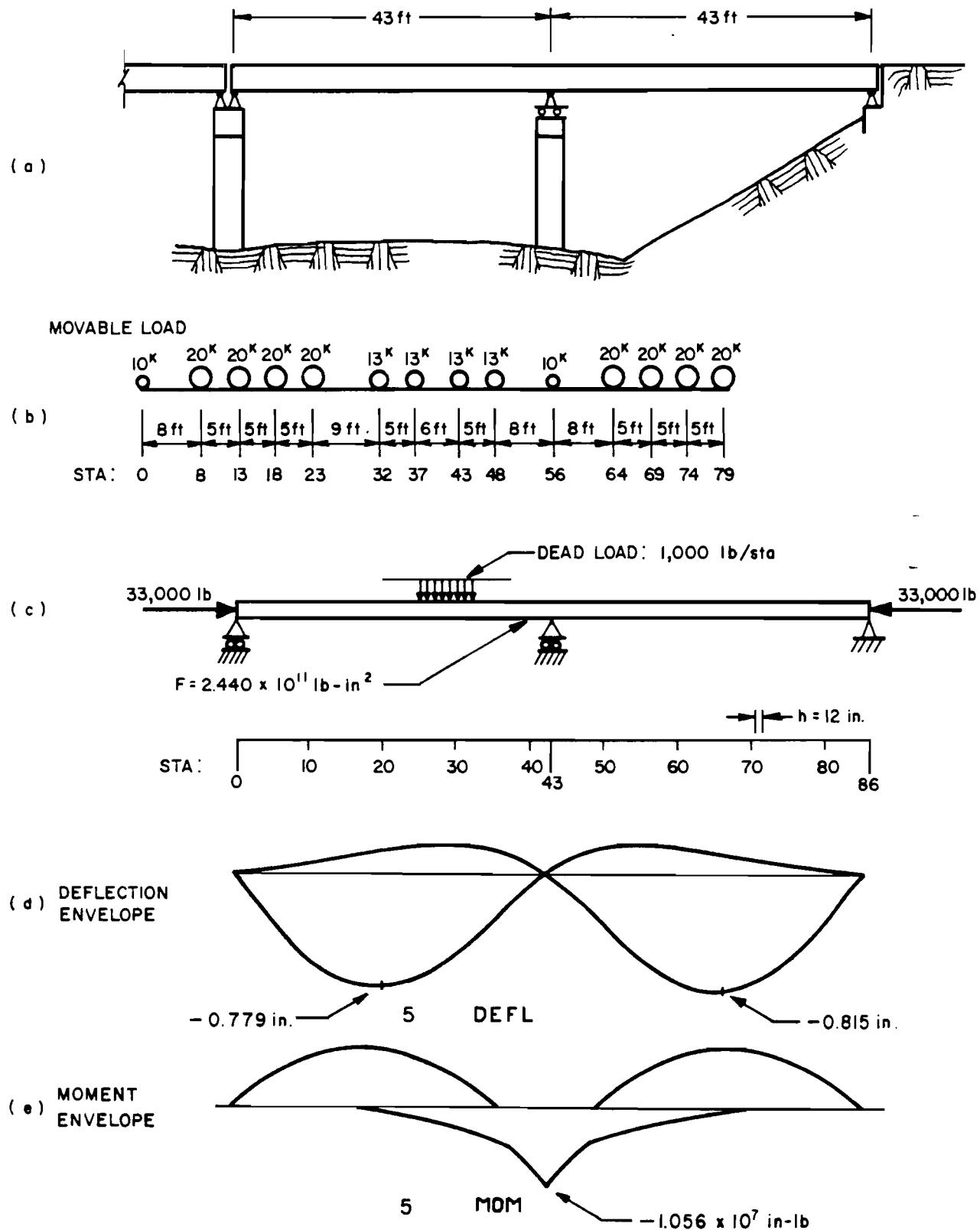


Fig 11. Example Problem 5. Two-span girder structure with railroad loading.

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CHAPTER 6. CONCLUSION

In this report, the finite-element beam-column solution of Ref 1 (BMCOL 34) has been modified and extended to cover the case of a movable load. The solution does not include any dynamic effects; it is a series of static solutions. A computer program, BMCOL 43, has been presented which allows the user to obtain a variety of results.

Problems in which only one loading condition is of interest may be solved by the fixed-load solution which is similar to the BMCOL 34 solution. All of the uses and capabilities of BMCOL 34 have been incorporated in the BMCOL 43 program.

A more thorough investigation of a problem is made possible by the movable-load solution in which envelopes of maximums are generated for deflection, bending moment, shear, and support reaction. The information usually needed in design is provided by the envelopes of maximums. A particularly useful application of BMCOL 43 is its ability to hold results in envelopes of maximums from problem to problem in order to simulate the past or anticipated loading history.

Influence diagrams for any of the parameters may be obtained for any point on the beam-column. The influence diagrams produced by BMCOL 43 may be for any diverse load system. They can be the same as conventional "unit load" influence diagrams, or they can represent "one movable load" influence diagrams.

This report is intended to serve as the basis for using BMCOL 43 as a tool in computer-aided design. Highway structural designers can use it to solve many problems they encounter that presently are solved by rough approximations or by tedious hand calculations.

Further application to particular design problems that are difficult to solve by conventional means can be undertaken. Some of the possible variations include analysis of a bridge structure as foundation supports settle, railroad loadings on continuous spans, the effect of axial loads induced from tractive forces or temperature changes, and approach slabs connected integrally with the bridge structure and supported on elastic foundations. A very important

use that could be made is a study of actual truck loadings to determine a possible alternate loading pattern for the presently used standard trucks.

REFERENCE

1. Matlock, Hudson, and T. Allan Haliburton, "A Finite-Element Method of Solution for Linearly Elastic Beam-Columns," Research Report No 56-1, Center for Highway Research, The University of Texas, Austin, September 1966.

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APPENDIX 1

**DERIVATION OF THE FINITE-DIFFERENCE EQUATION
FOR THE DEFLECTED SHAPE OF A BEAM-COLUMN**

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APPENDIX 1. DERIVATION OF THE FINITE-DIFFERENCE EQUATION
FOR THE DEFLECTED SHAPE OF A BEAM-COLUMN

The BMCOL 43 method of handling axial loads is slightly different from the method used by BMCOL 34. Therefore, the relationship between deflection and applied load is derived for a beam-column loaded and supported in a very general manner. A finite-element model is used in the derivation so that the resulting equation will allow freely discontinuous beam-column properties, loads, and restraints.

As is shown in Fig 2, a rotational restraint applied at Station i of a beam-column is introduced into the model as forces at Stations $i-1$ and $i+1$. These forces form a couple which has the magnitude $R_i \theta_i^J$. Thus the magnitude of the forces depends upon the increment length h and slope θ_i^J . For example, the magnitude of the transverse force acting at Station $i+1$ due to rotational restraint at Station i is

$$Q_{i+1}^{RR} = \frac{R_i \theta_i^J}{2h} \quad (A1.1)$$

The value of slope at Station i is approximated on a two-increment basis, i.e.,

$$\theta_i^J = \frac{-w_{i-1} + w_{i+1}}{2h} \quad (A1.2)$$

An applied couple also is introduced into the model as equal and opposite transverse forces (see Fig 2). The magnitude of the force at Station $i+1$ due to a couple at Station i is

$$Q_{i+1}^T = \frac{T_i}{2h} \quad (A1.3)$$

The effects of rotational restraints and applied couples are shown as transverse loads in the free-body diagram of a portion of the finite-element model shown in Fig A1. The equation of moment equilibrium of Bar i is

$$-M_{i-1} + M_i - V_i h - P_i (-w_{i-1} + w_i) = 0 \quad (A1.4)$$

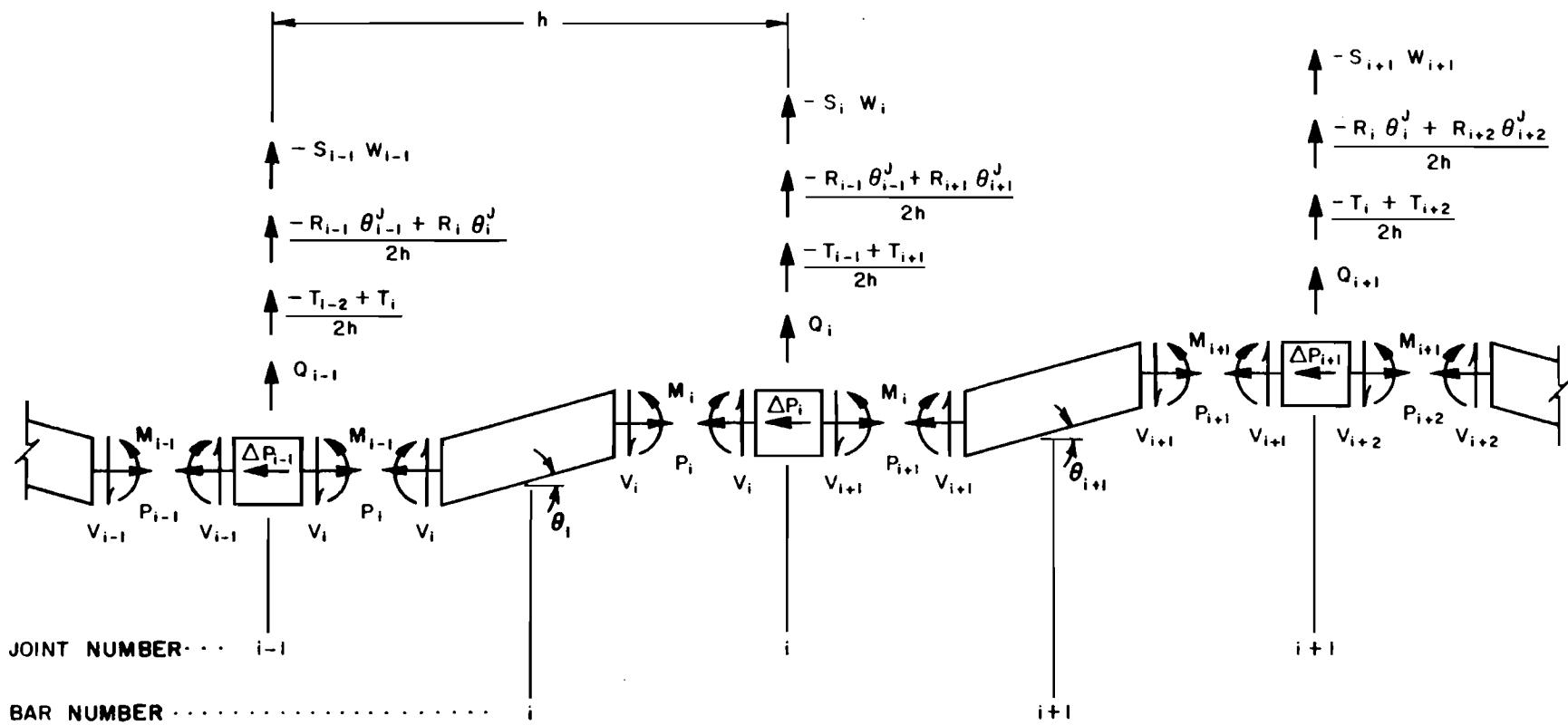


Fig A1. Free-body diagram of a portion of the finite-element model of a beam-column.

Similarly, the equilibrium equation of Bar $i+1$ is

$$-M_i + M_{i+1} - V_{i+1}h - P_{i+1}(-w_i + w_{i+1}) = 0 \quad (A1.5)$$

Subtracting Eq A1.5 from Eq A1.4 gives

$$\begin{aligned} -M_{i-1} + 2M_i - M_{i+1} &= h(V_i - V_{i+1}) - P_i w_{i-1} + (P_i + P_{i+1})w_i \\ &\quad - P_{i+1}w_{i+1} \end{aligned} \quad (A1.6)$$

Equilibrium of vertical forces acting on Joint i yields

$$\begin{aligned} V_i - V_{i+1} &= -Q_i + S_i w_i - \frac{1}{2h} (-T_{i-1} + T_{i+1}) \\ &\quad - \frac{1}{2h} (-R_{i-1}\theta_{i-1}^J + R_{i+1}\theta_{i+1}^J) \end{aligned} \quad (A1.7)$$

Substitution of Eq A1.7 into Eq A1.6 yields

$$\begin{aligned} -M_{i-1} + 2M_i - M_{i+1} &= -hQ_i + hS_i w_i - \frac{1}{2} (-T_{i-1} + T_{i+1}) \\ &\quad - \frac{1}{2} (-R_{i-1}\theta_{i-1}^J + R_{i+1}\theta_{i+1}^J) - P_i w_{i-1} \\ &\quad + (P_i + P_{i+1})w_i - P_{i+1}w_{i+1} \end{aligned} \quad (A1.8)$$

Substitution of Eq A1.2 into Eq A1.8 gives

$$\begin{aligned} M_{i-1} - 2M_i + M_{i+1} &= hQ_i - hS_i w_i + \frac{1}{2} (-T_{i-1} + T_{i+1}) + \frac{1}{4h} \\ &\quad [-R_{i-1}(-w_{i-2} + w_i) + R_{i+1}(-w_i + w_{i+2})] \\ &\quad + P_i w_{i-1} - (P_i + P_{i+1})w_i + P_{i+1}w_{i+1} \end{aligned} \quad (A1.9)$$

The bending moment M_i is equal to the product of the lumped bending stiffness and the angle change at Joint i .

$$M_i = \frac{F_i}{h} \left(\frac{w_{i-1} - 2w_i + w_{i+1}}{h} \right) \quad (A1.10)$$

When Eq A1.10 is introduced into Eq A1.9, the resulting equation is

$$a_i w_{i-2} + b_i w_{i-1} + c_i w_i + d_i w_{i+1} + e_i w_{i+2} = f_i \quad (2.1)$$

where

$$a_i = F_{i-1} - 0.25hR_{i-1}$$

$$b_i = -2(F_{i-1} + F_i) - h^2 P_i$$

$$c_i = F_{i-1} + 4F_i + F_{i+1} + h^3 S_i + 0.25h(R_{i-1} + R_{i+1})$$

$$+ h^2(P_i + P_{i+1}) \quad (2.2)$$

$$d_i = -2(F_i + F_{i+1}) - h^2 P_{i+1}$$

$$e_i = F_{i+1} - 0.25hR_{i+1}$$

$$f_i = h^3 Q_i - 0.5h^2(T_{i-1} - T_{i+1})$$

When Eq 2.1 is written at every station on a beam-column, a set of simultaneous equations results. The recursive solution described in Ref 1 is used in Program BMCOL 43 to solve the set of equations for the deflection at each station. Once the deflected shape is known, bending moment, shear, and support reaction can be determined using the methods described in Chapter 2.

APPENDIX 2

GUIDE FOR DATA INPUT FOR ECOL A3

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GUIDE FOR DATA INPUT FOR EMCOL 43

with Supplementary Notes

extract from

A COMPUTER PROGRAM TO ANALYZE
BEAM-COLUMNS UNDER MOVABLE LOADS

by

Hudson Matlock and Thomas P. Taylor

June 1968

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BMCOL 43 GUIDE FOR DATA INPUT -- Card forms

IDENTIFICATION OF PROGRAM AND RUN (2 alphanumeric cards per run)

I		80
I		80
I		80

IDENTIFICATION OF PROBLEM (one card each problem; program stops if PROB NUM is left blank)

PROB NUM

I	5	II	Description of problem (alphanumeric)	80
---	---	----	---------------------------------------	----

TABLE 1. PROGRAM CONTROL DATA (one card each problem)

ENTER "1" TO HOLD PRIOR						NUM CARDS ADDED FOR TABLE						ENTER "1" TO PLOT ENVELOPES FOR			
ENVELOPES	TABLE 2	3	4	5	6	2	3	4	5	6	DEFL	MOM	SHEAR	REACT	

10 15 20 25 30 35 40 45 50 55 60 65 70 75 80

TABLE 2. CONSTANTS AND MOVABLE-LOAD DATA (one card, or none if Table 2 of preceding problem is held)

NUM INCRS	INCR LENGTH	MOVABLE - LOAD - DATA					
		NUM IN PATTERN	INCRS	START STA	STOP STA	STEP SIZE	
				45	50	55	60

10 21 30 45 50 55 60

TABLE 3. SPECIFIED DEFLECTIONS AND SLOPES (number of cards according to Table 1; none if preceding Table 3 is held)

STATION	CASE	DEFLECTION	SLOPE	CASE = 1 for deflection only, 2 for slope only, 3 for both
10	20	30	40	

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TABLE 4. FIXED LOADS AND RESTRAINTS, (number of cards according to Table 1). Data added to storage as lumped quantities per increment length, linearly interpolated between values input at indicated end stations, with 1/2-values at each end station. Concentrated effects are established as full values at single stations by setting final station = initial station.

ENTER 1 IF CONT'D			F	Q_f	S	T	R	P
STA	TO STA	ON NEXT CARD	BENDING STIFFNESS	TRANSVERSE FORCE	SPRING SUPPORT	TRANSVERSE COUPLE	ROTATIONAL RESTRAINT	AXIAL TENSION OR COMPRESSION
6	10	15	20	30	40	50	60	70
								80

TABLE 5. MOVABLE LOADS, (number of cards according to Table 1). Data added to storage just as in Table 4.

ENTER 1 IF CONT'D			Q_m
STA	TO STA	ON NEXT CARD	TRANSVERSE FORCE
6	10	15	20
			31
			40

TABLE 6. SPECIFIED STATIONS FOR INFLUENCE DIAGRAMS (4 cards or none).

NUM OF DIAGRAMS	TYPE OF OUTPUT*	SPECIFIED STATIONS (max = 5 per variable) FOR:						DEFLECTION
6		21	25	30	35	40	45	
6		21	25	30	35	40	45	MOMENT
6		21	25	30	35	40	45	SHEAR
6		21	25	30	35	40	45	REACTION
6		21	25	30	35	40	45	

STOP CARD (one blank card at end of run)

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GENERAL PROGRAM NOTES

The data cards must be stacked in proper order for the program to run.

A consistent system of units must be used for all input data, for example, kips and inches.

All 5-space or less words are understood to be right justified integers or whole decimal numbers.

- 4 3

All 10-space words are right justified floating-point decimal numbers.

- 4 . 321 E + 0 3

TABLE 1. PROGRAM-CONTROL DATA

For each of Tables 2, 3, and 6, a choice must be made between holding all of the data from the preceding problem or entering entirely new data. If the hold option for any of these tables is set equal to 1, the number of cards input for that table must be zero. Envelopes may also be held if desired.

For Tables 4 and 5, the data is accumulated in storage by adding to previously stored data. The number of cards input is independent of the hold option, except that the cumulative total of cards can not exceed 100.

Card counts in Table 1 should be rechecked carefully after the coding of each problem is completed.

The plot option for each of the envelopes of maximums is independent of the others. No plots are drawn for those options that are blank or zero. For each plot option that is set equal to 1, a plot is drawn on 4 X 10 in. axes. Only envelopes of maximums are plotted. Fixed loads can be plotted indirectly as discussed in Table 2 below.

TABLE 2. CONSTANTS AND MOVABLE-LOAD DATA

The maximum number of increments into which the beam-column may be divided is 200. Typical units for the value of increment length are inches.

The number of increments in the movable-load pattern may not exceed the number of increments in the beam-column. The start station is the first position at which the zero station of the movable-load pattern is placed.

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Any positive start station for the movable load is permissible. A negative start station is permissible if one step of the load pattern will place some portion of the load pattern on the beam-column.

Any stop station on the beam is allowed. A stop station of no more than one step of movement past the right end of the beam-column is permissible.

The movable-load pattern may be moved across the beam in steps of as many increments as desired.

To plot envelopes of maximum for fixed-load solutions, enter zero increment of pattern length, zero start and stop stations, and 1 for step size. No Table 5 necessary.

TABLE 3. SPECIFIED DEFLECTIONS AND SLOPES

The maximum number of stations at which deflections and slopes may be specified is 20.

A slope may not be specified closer than 3 increments from another specified slope.

A deflection may not be specified closer than 2 increments from a specified slope, except that both a deflection and a slope may be specified at the same station.

TABLE 4. STIFFNESS AND FIXED-LOAD DATA

Typical units,

variables:

values per station:	F	Q	S	T	R	P
	1b × in ³	1b	1b/in	in × 1b	in × 1b rad	1b

Axial tension or compression values P must be stated at each station in the same manner as any other distributed data; there is no mechanism in the program to automatically distribute the internal effects of an externally applied axial force.

Data should not be entered in this table (nor held from the preceding problem) which would express effects at fictitious stations beyond the ends of the real beam-column.

The left end of the beam-column must be located at Station 0.

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For the interpolation and distribution process, there are four variations in the station numbering and the referencing for continuation to succeeding cards. These variations are explained and illustrated on page 6.

There are no restrictions on the order of cards in Table 4, except that within a distribution sequence the stations must be in regular order.

TABLE 5. MOVABLE-LOAD DATA

The data in Table 5 is governed by the same rules as Table 4.

TABLE 6. SPECIFIED STATIONS FOR INFLUENCE DIAGRAMS

The number of cards in Table 6 is either 4 or 0.

A maximum of 5 stations may be specified for each of the four variables: deflection, bending moment, shear, and support reaction.

The data cards must be stacked in the same order as the above list.

If no influence lines are desired for one variable, a blank card must be inserted for that variable.

Shear is computed one-half increment to the left of the designated station.

If 1 is specified for the type of output, the influence lines are plotted on 4 × 10 in. axes.

If 2 is specified for the type of output, the influence lines are tabulated in numerical form in Table 10.

If 3 is specified for the type of output, the influence lines are tabulated and plotted.

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Individual - card Input

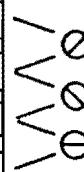
- Case a.1 Data concentrated at one sta
 Case a.2. Data uniformly distributed

FROM STA	TO STA	CONT'D TO NEXT CARD ?	F	Q	etc...
7	7	O=NO		3.0	
5	15	O=NO	2.0		
15	20	O=NO	4.0	1.0	
10	20	O=NO		2.0	

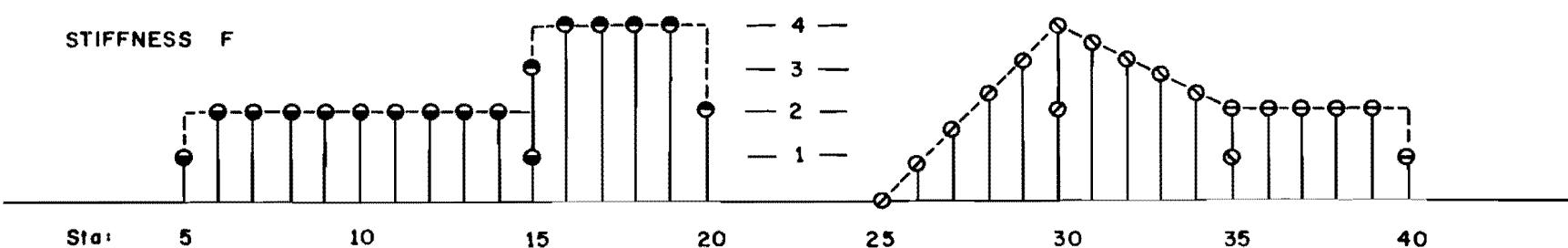
Multiple - card Sequence

- Case b. First - of - sequence
 Case c. Interior - of - sequence
 Case d. End - of - sequence

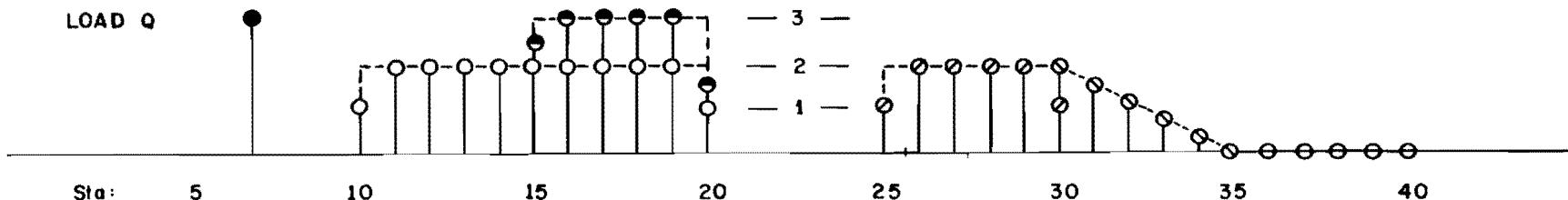
25		I=YES	0.0	2.0	
	30	I=YES	4.0	2.0	
	35	I=YES	2.0	0.0	
	40	O=NO	2.0		

Resulting Distribution of Data

STIFFNESS F



LOAD Q



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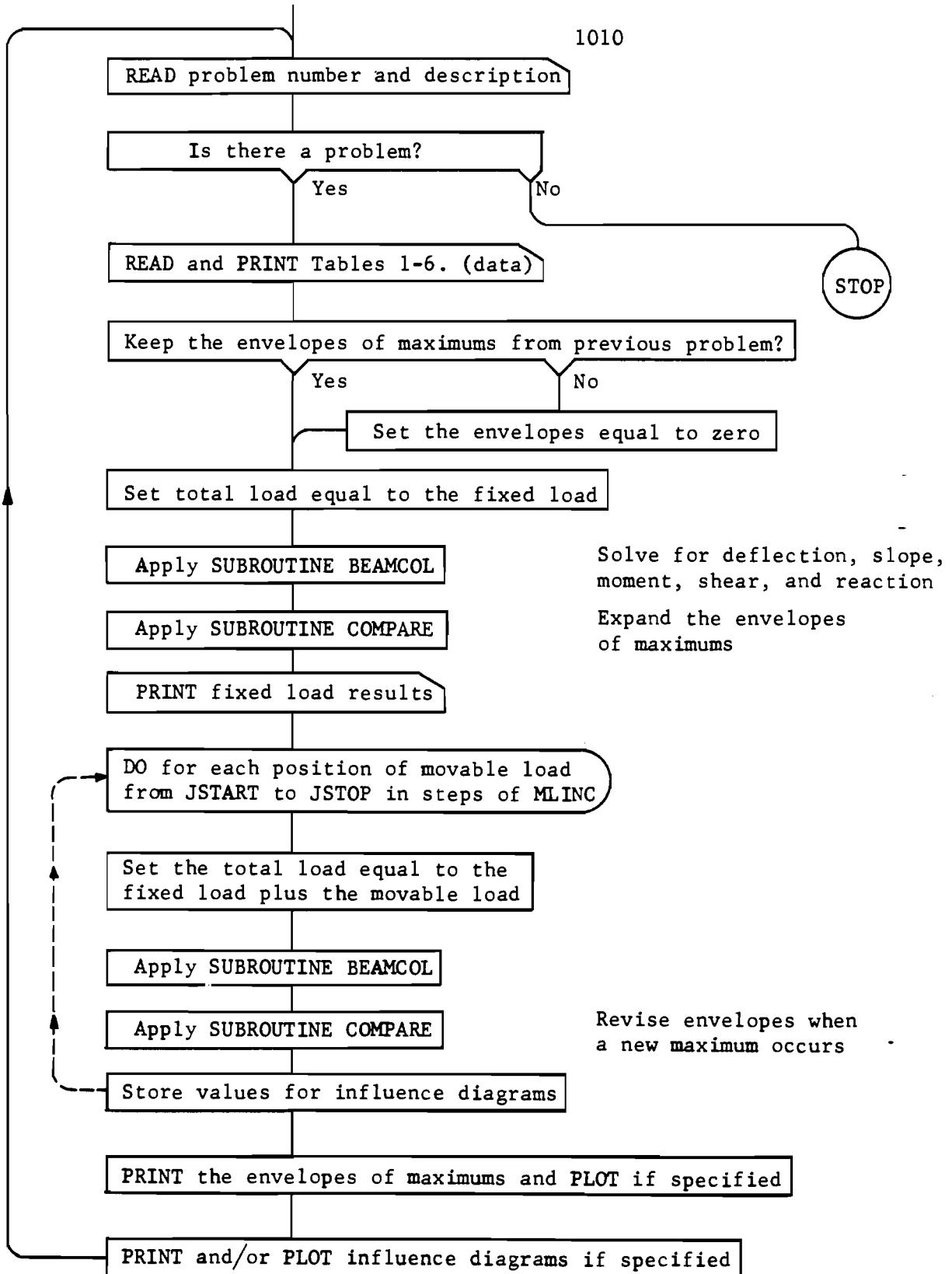
APPENDIX 3

FLOW DIAGRAMS FOR ECOL 43

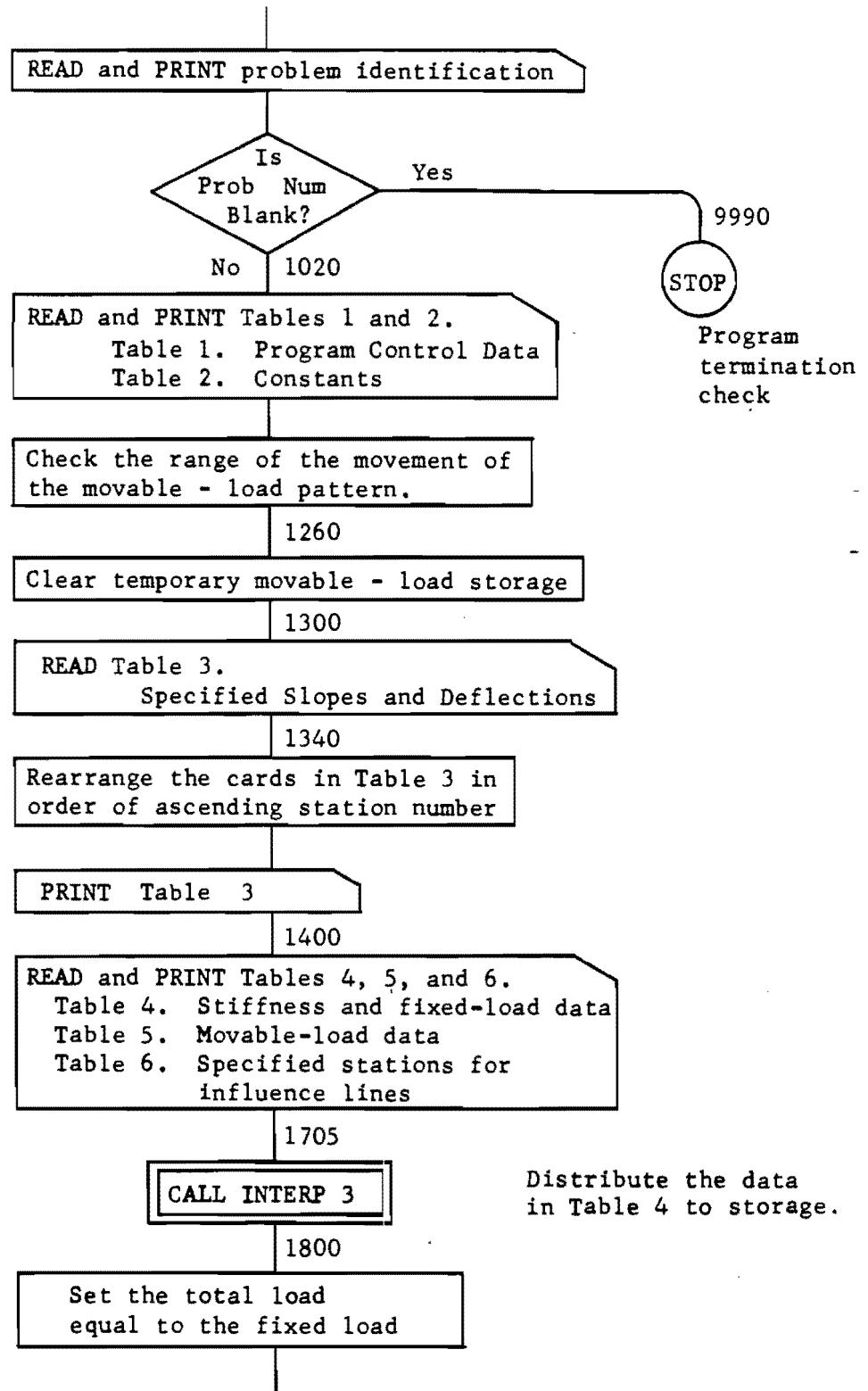
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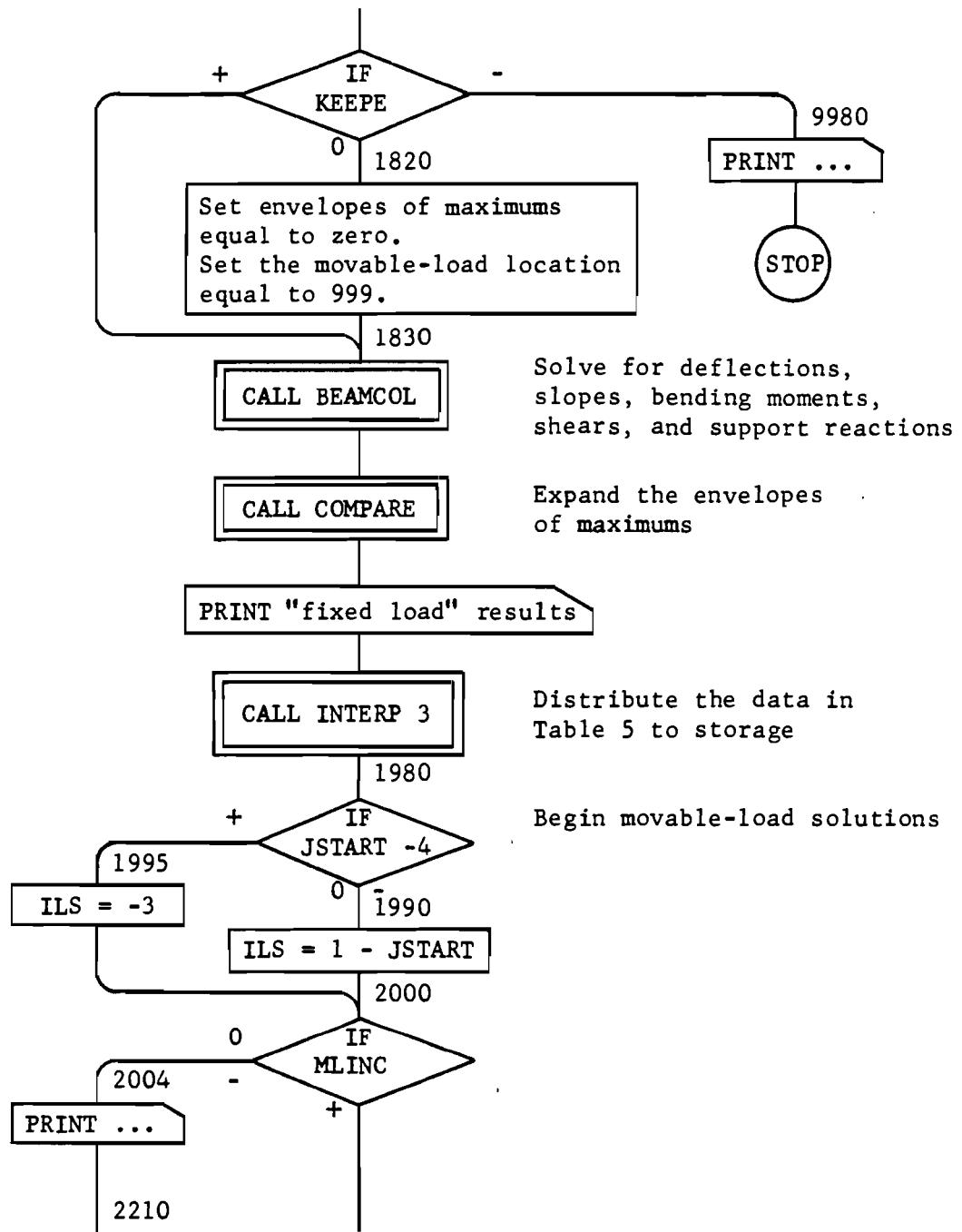
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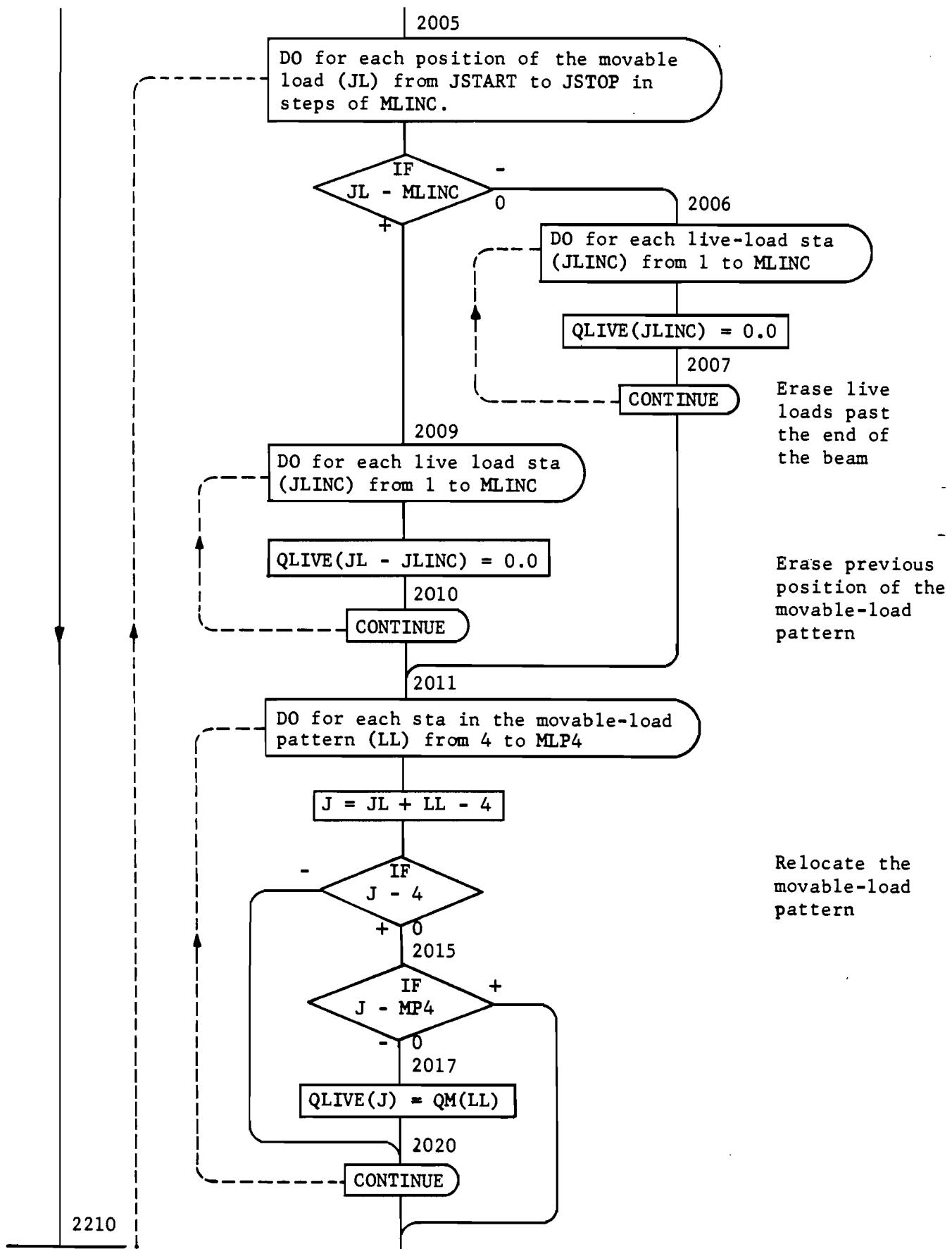
SUMMARY FLOW DIAGRAM FOR PROGRAM BMCOL 43

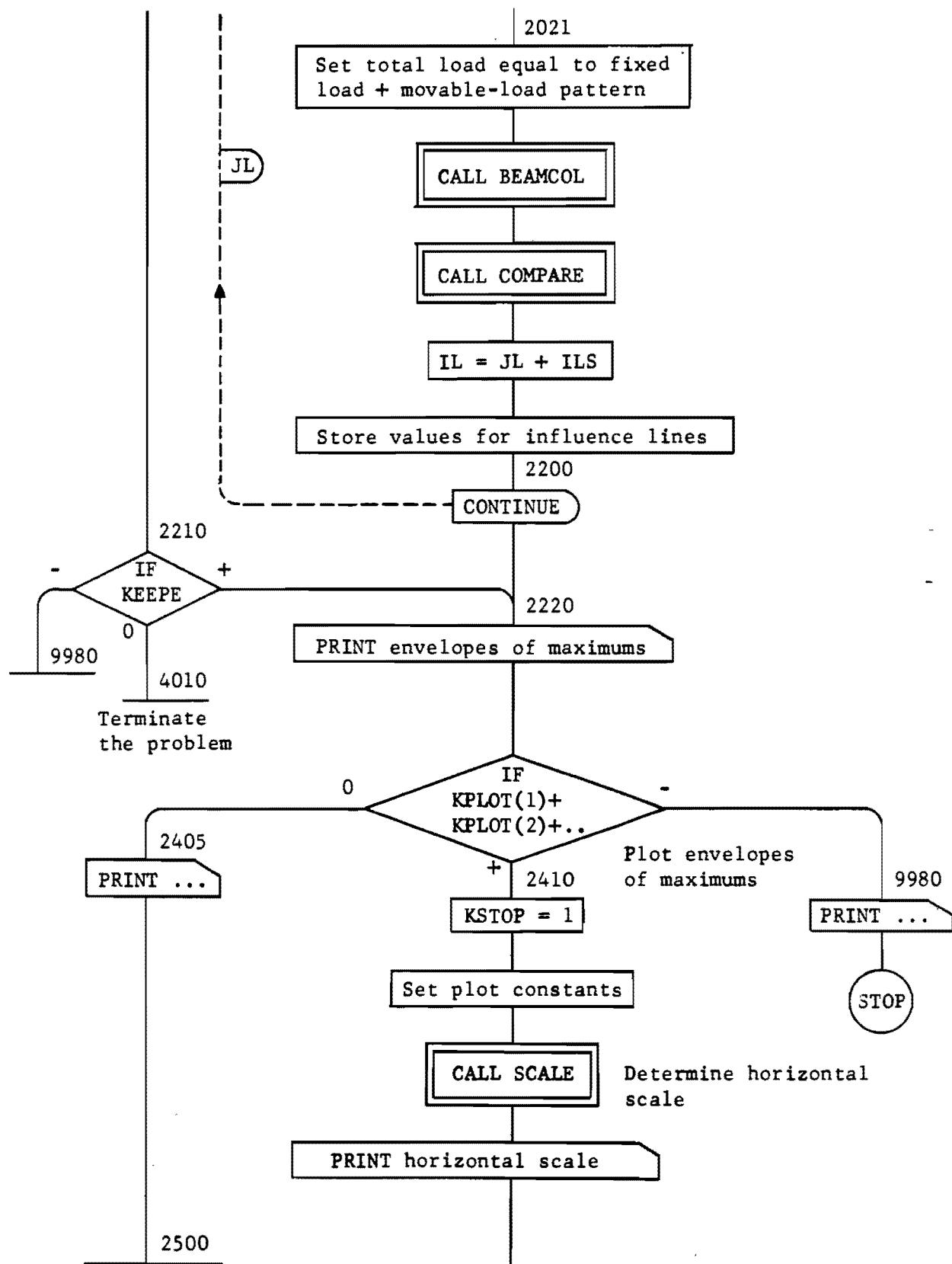


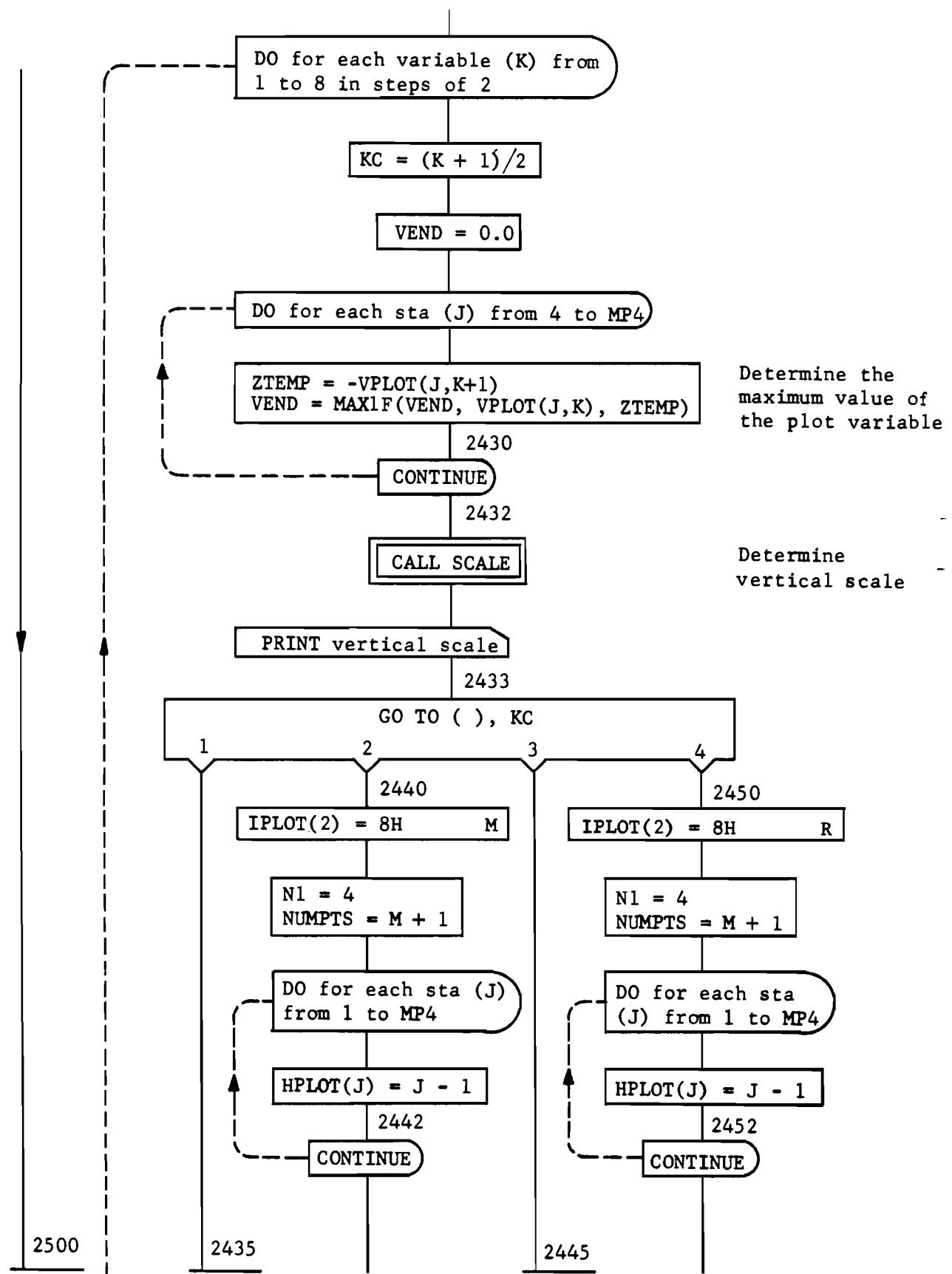
GENERAL FLOW DIAGRAM FOR PROGRAM BMCOL 43

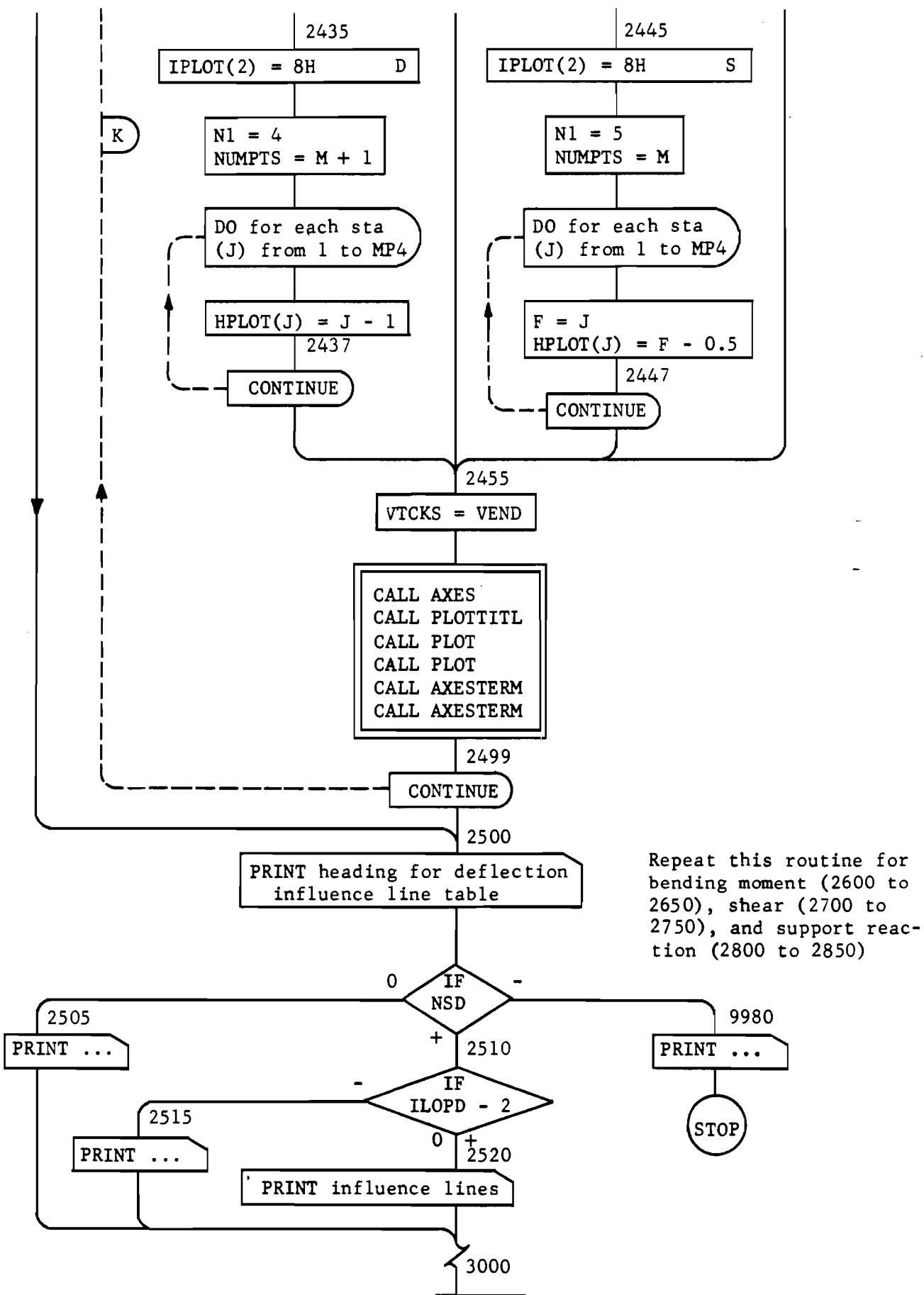


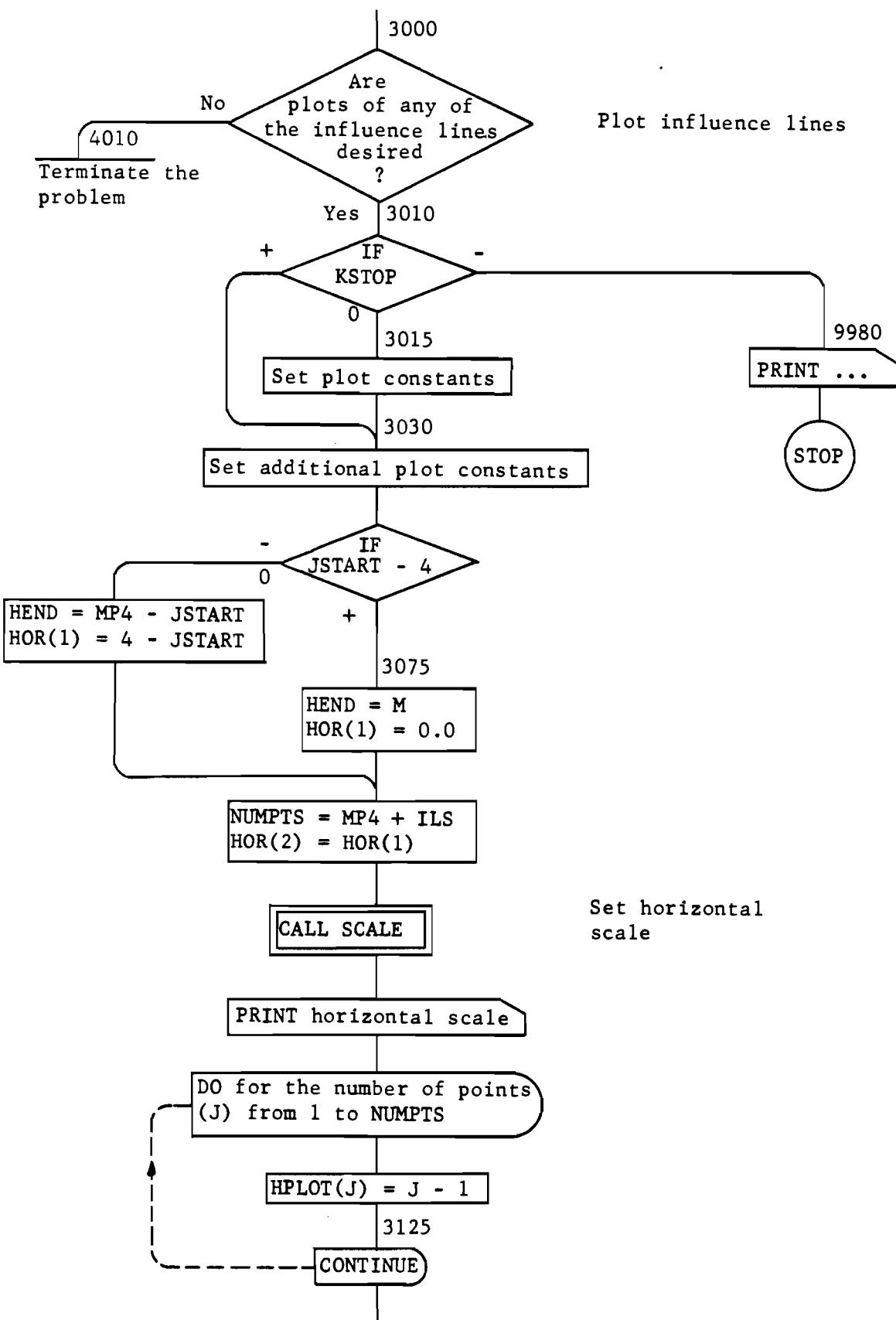


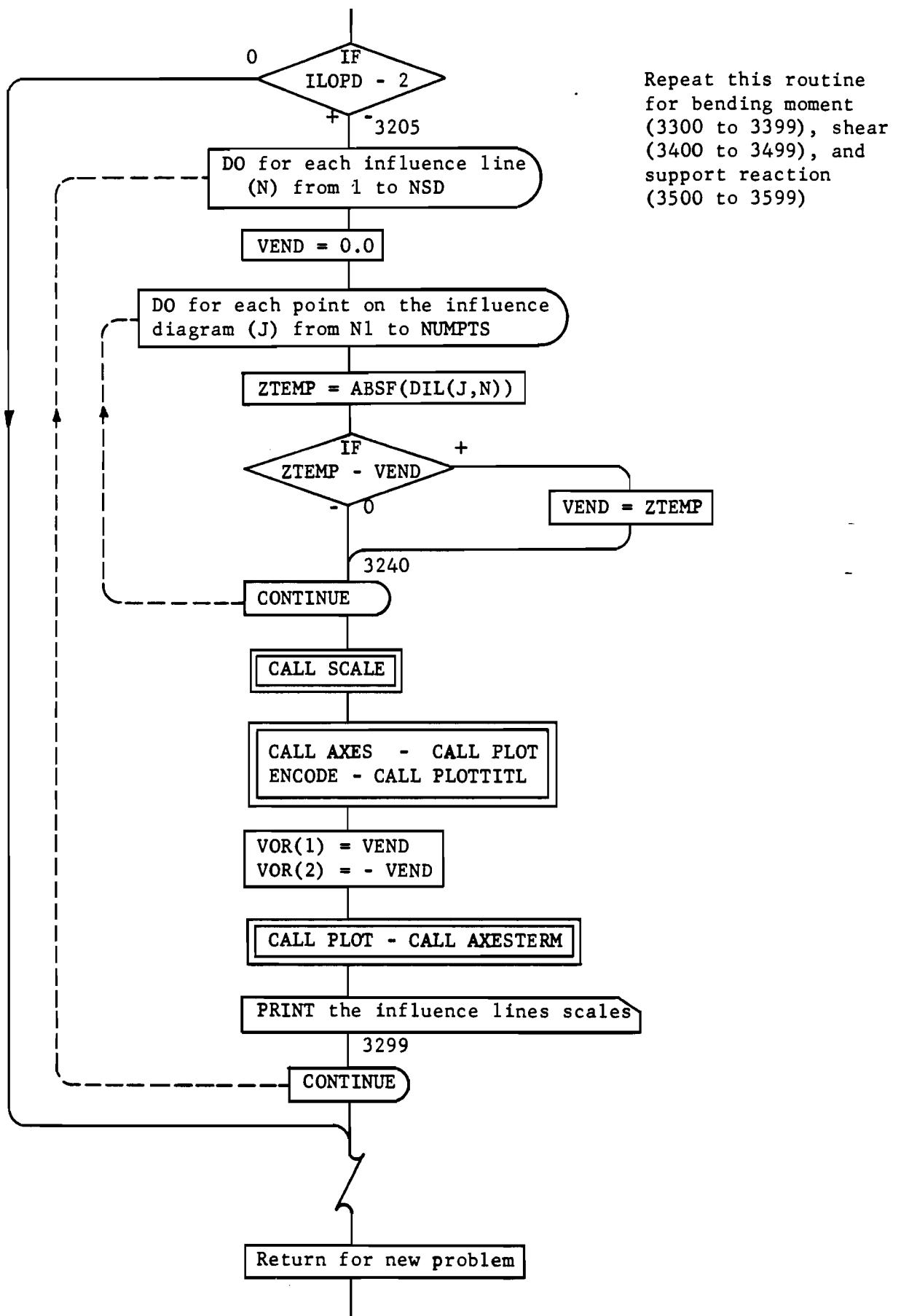




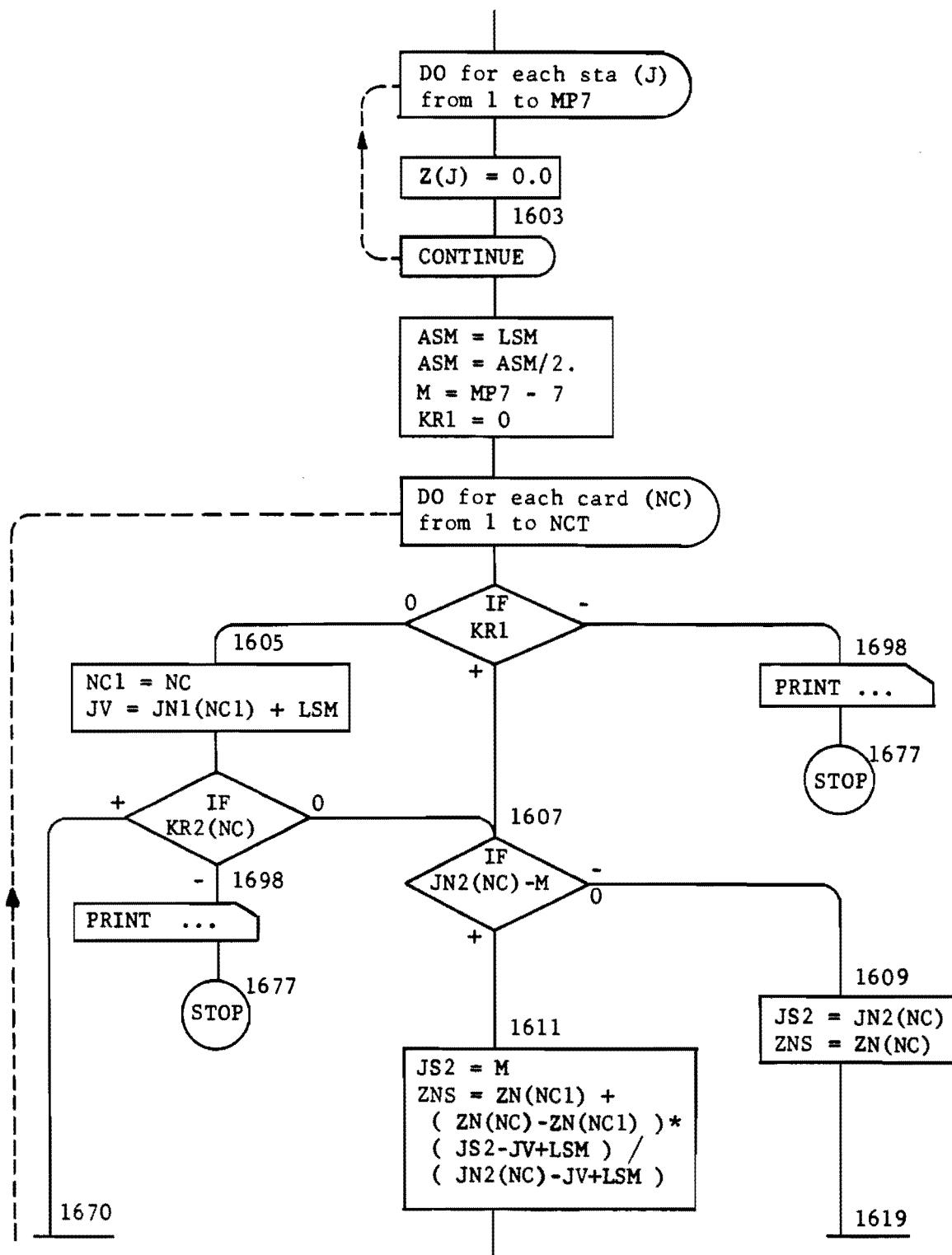


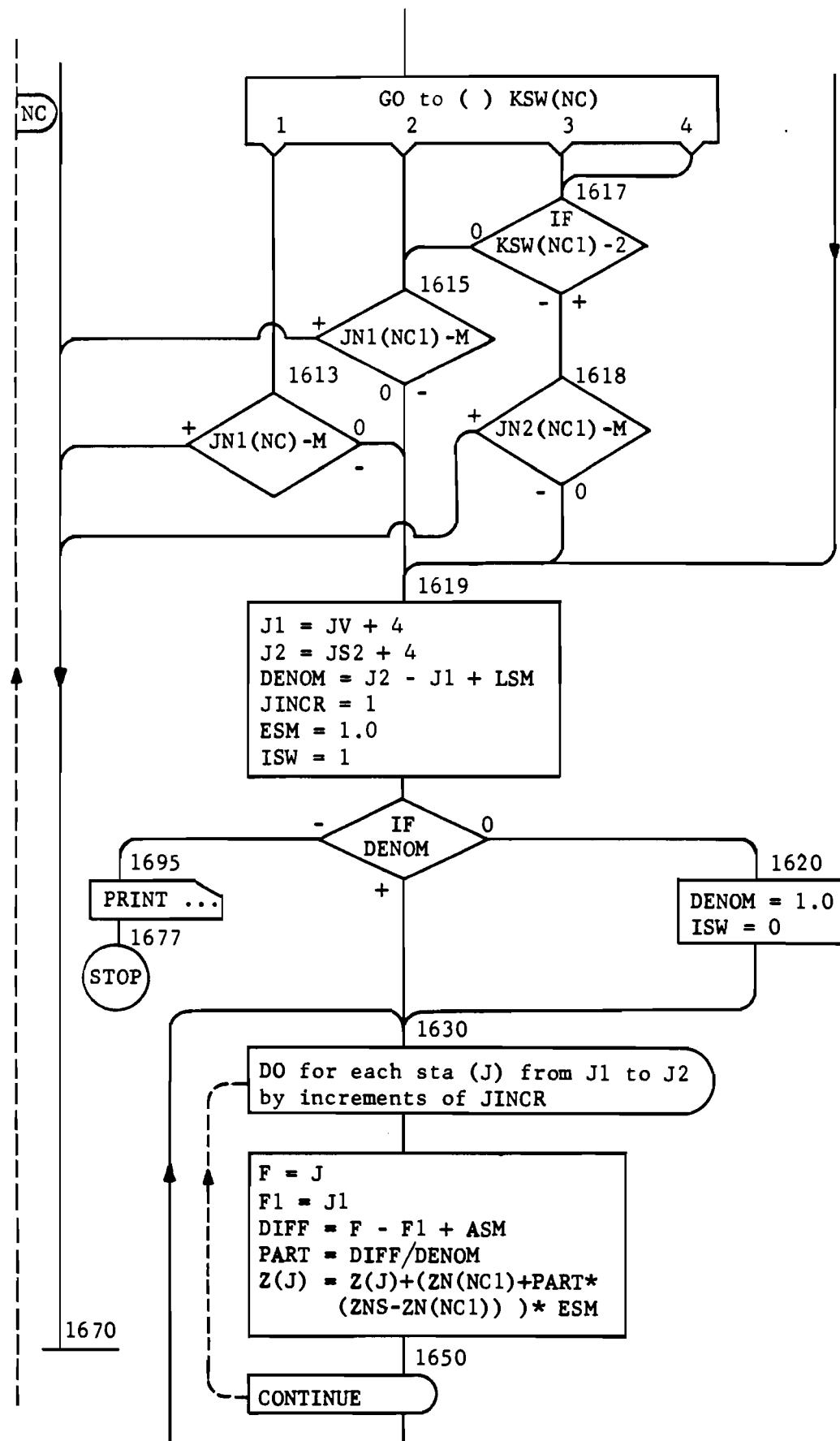


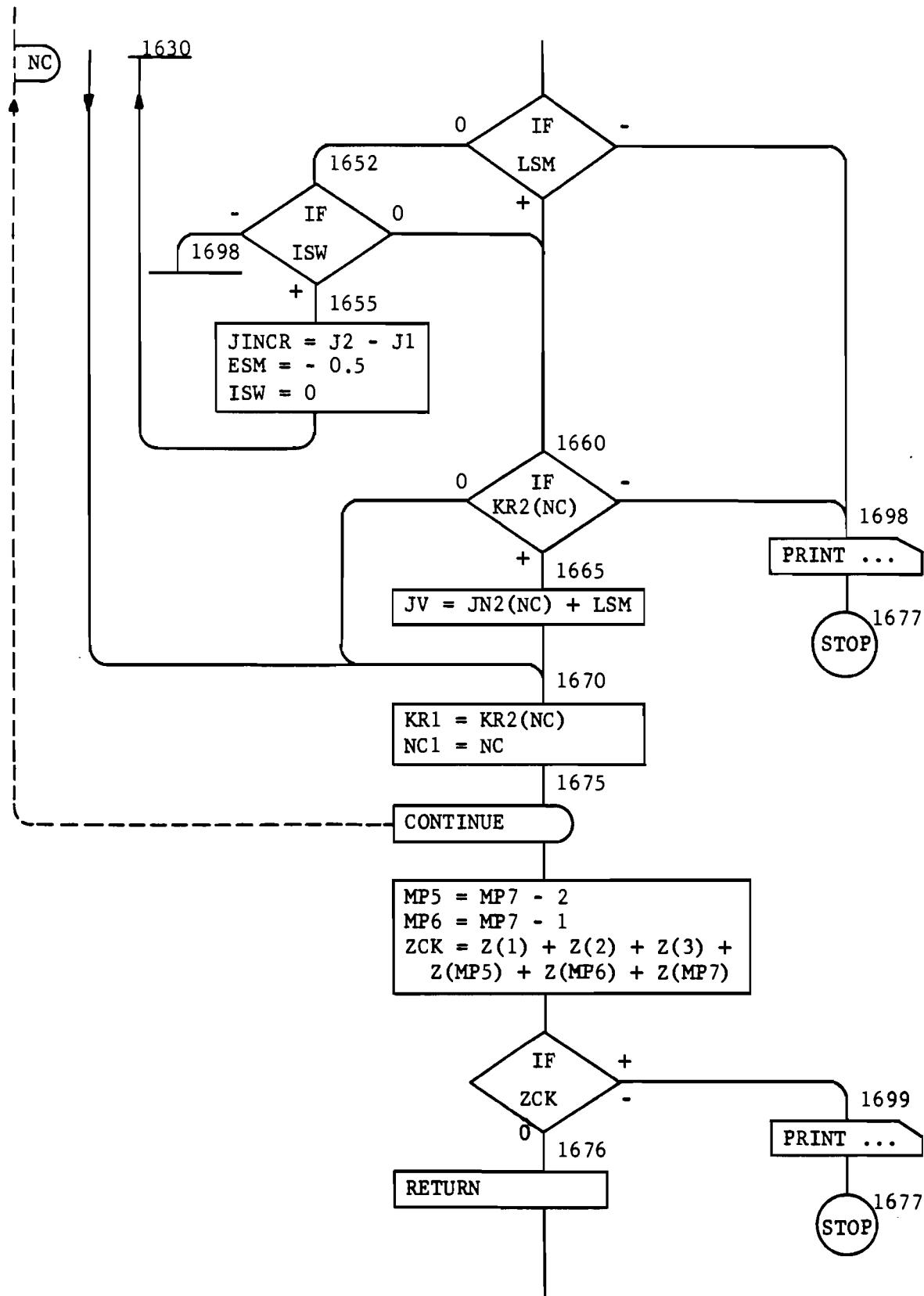


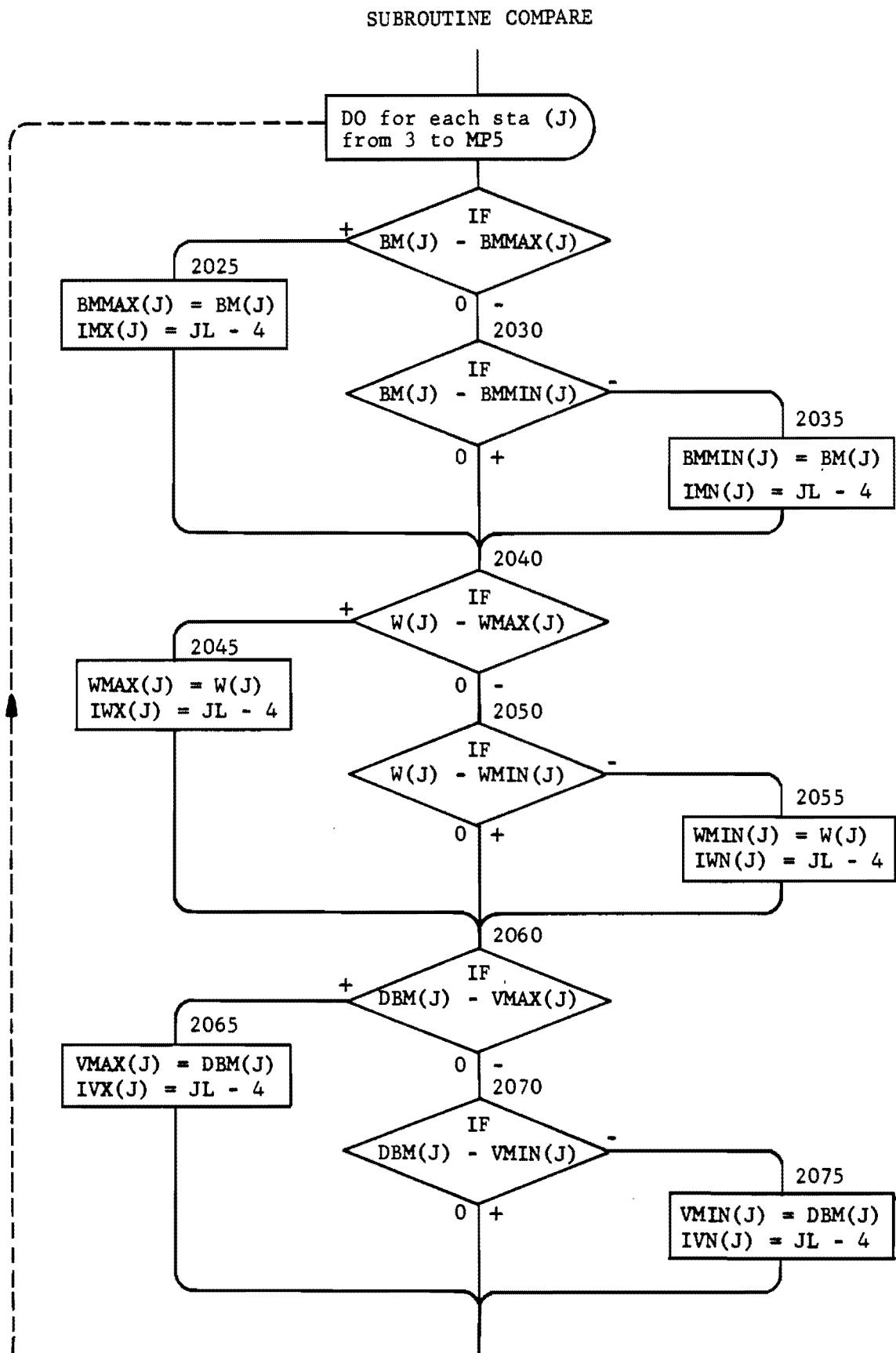


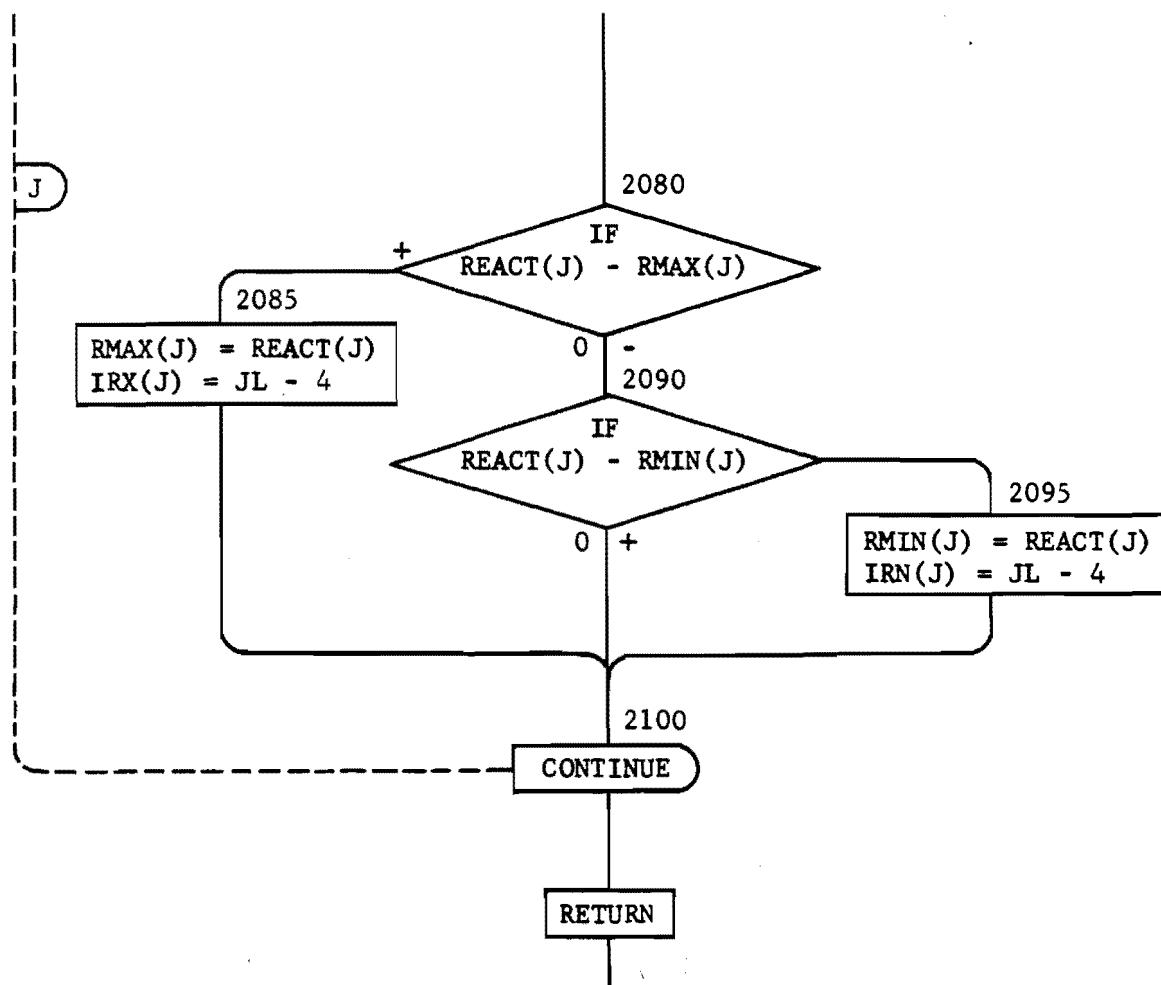
SUBROUTINE INTERP 3



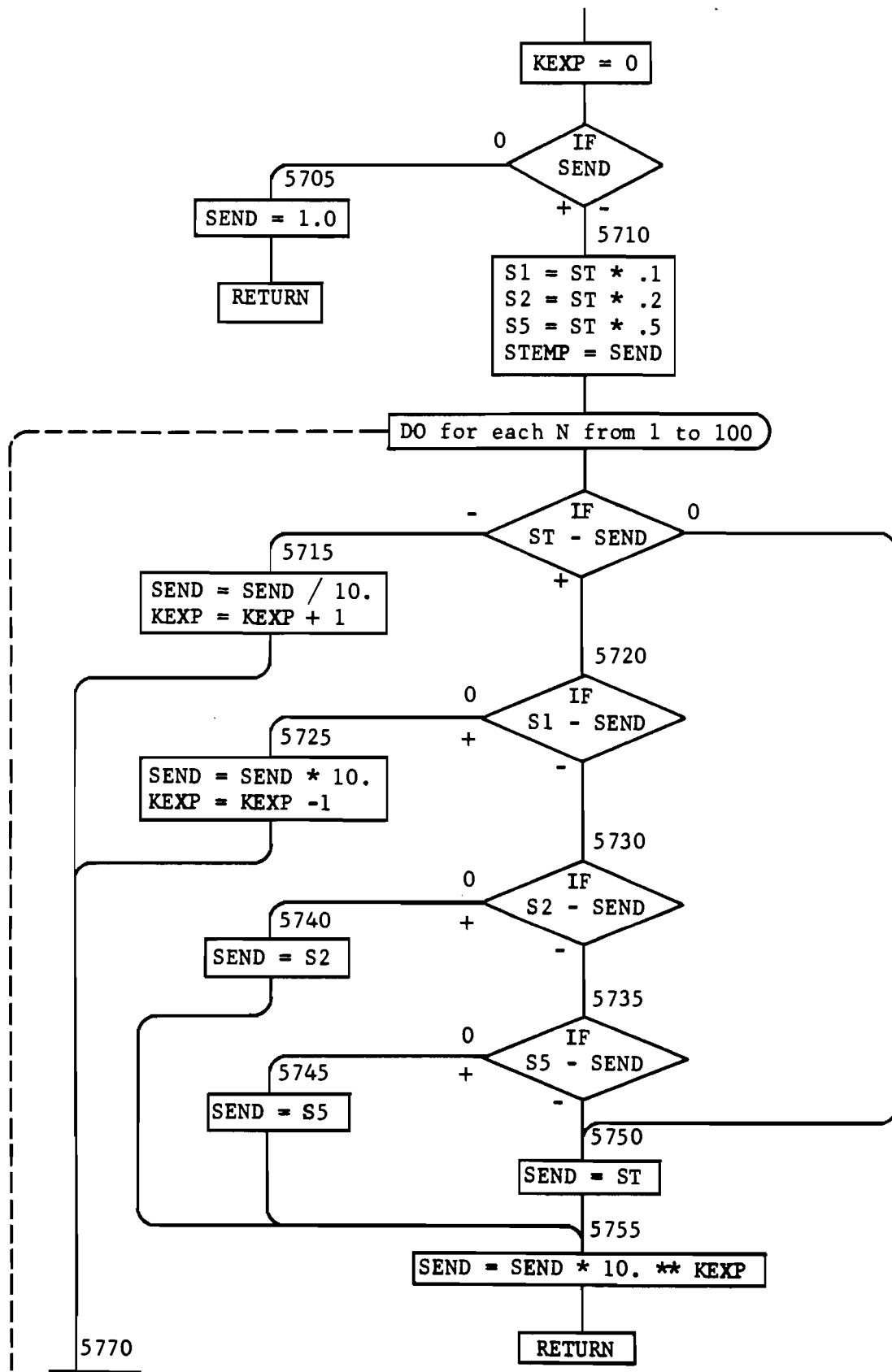


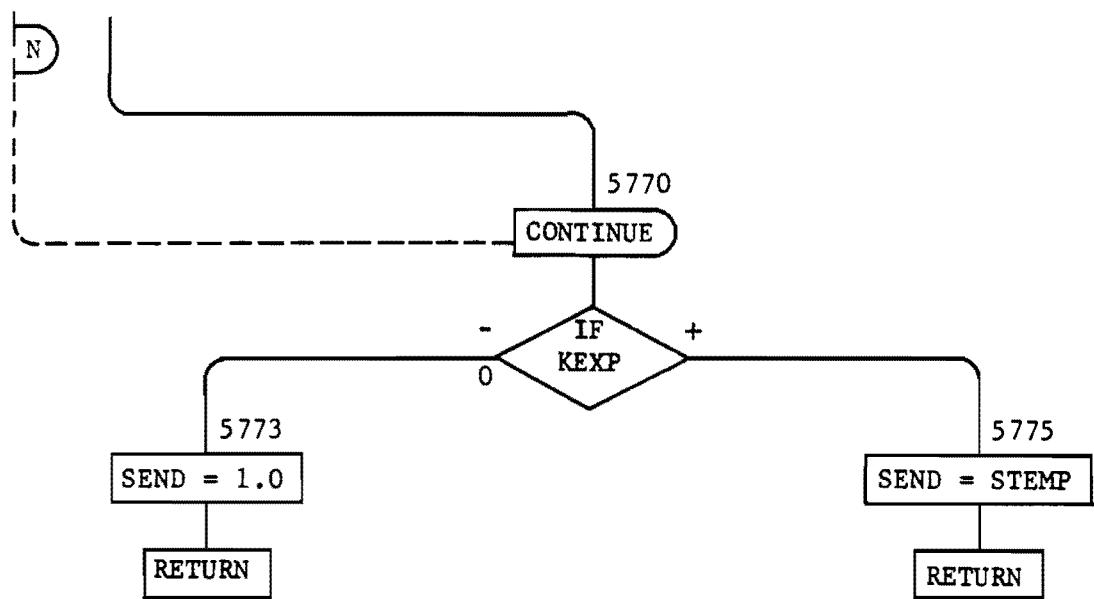




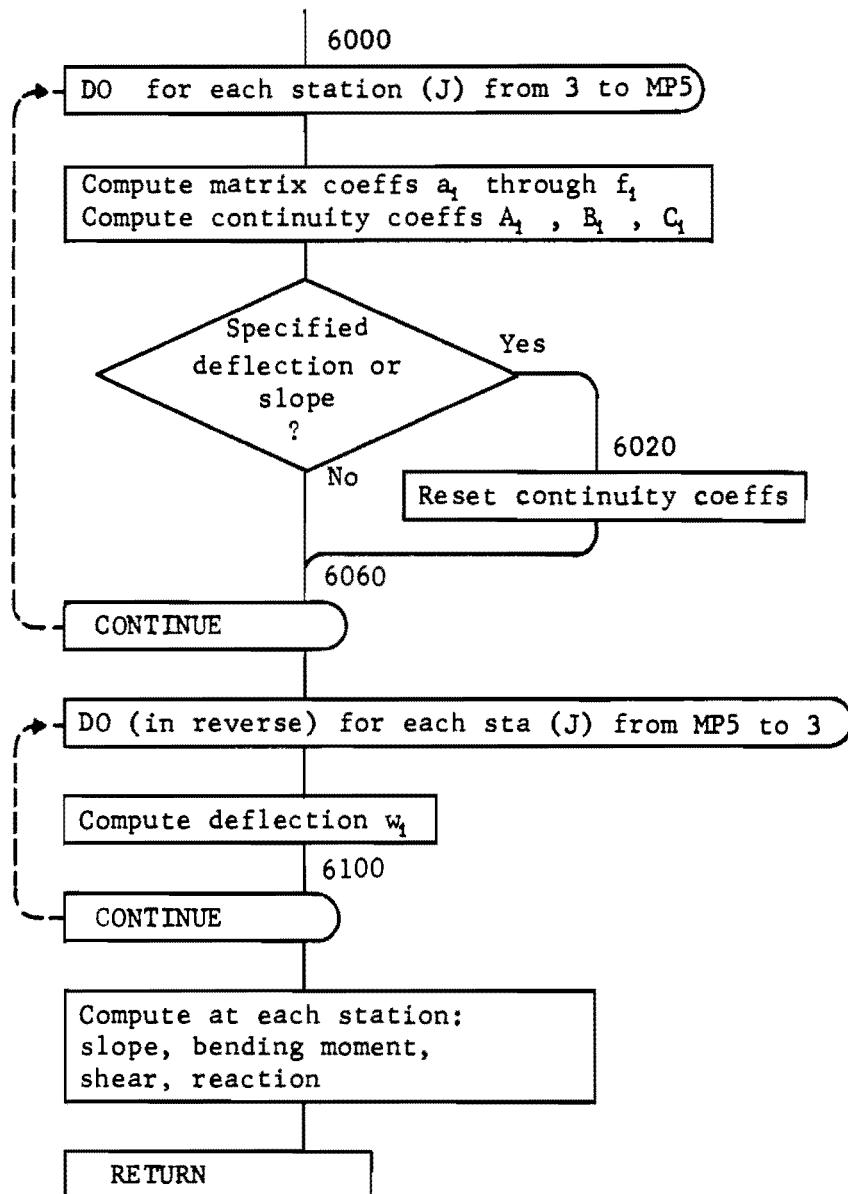


SUBROUTINE SCALE





SUBROUTINE BEAMCOL



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APPENDIX 4

GLOSSARY OF NOTATION FOR ENCOL 43

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C-----NOTATION FOR BMCOL 43

C	AN1(), AN2()	IDENTIFICATION AND REMARKS (ALPHA-NUM)	07JE5
C	BMI()	BENDING MOMENT	02JE5
C	BMF()	FIXED LOAD BENDING MOMENT	24JA7
C	BMIL()	INFLUENCE LINE FOR MOMENT	02JE5
C	BMMAX()	MAX VALUE OF POSITIVE BENDING MOMENT	02JE5
C	BMMIN()	MAX VALUE OF NEGATIVE BENDING MOMENT	02JE5
C	DBMI()	SHEAR BETWEEN ADJACENT STATIONS	08MR8
C	DIL()	INFLUENCE LINE FOR DEFLECTION	02JE5
C	DBMF()	FIRST DERIV OF FIXED LOAD BENDING MOMENT	24JA7
C	DWI()	FIRST DERIV OF BMCOL DEFL	07JE5
C	DWSI()	VALUE OF SPECIFIED SLOPE	02JE5
C	DWSAV	TEMPORARY VALUE OF DWS()	07JE5
C	F()	FLEXURAL STIFFNESS (TOTAL PER STA)	02JE5
C	FN2()	FLEXURAL STIFFNESS (INPUT)	02JE5
C	H	INCREMENT LENGTH	12JE3
C	HCOR	HORIZONTAL COORDINATE OF THE FIRST SYMBOL IN THE PLOT TITLE	15JL4
C	HEND	VALUE ASSIGNED TO END OF HORIZONTAL-AXIS	200C5
C	HE2	H SQUARED	12JE3
C	HE3	H CUBED	12JE3
C	HNEG	LENGTH, IN INCHES, OF THE NEGATIVE H-AXIS.	14AP4
C	THIS VARIABLE MUST NOT ITSELF BE NEGATIVE.	14AP4	
C	HOR	LOCATION OF THE VERTICAL AXIS	02JE5
C	HPLOT()	NAME OF ARRAY TO BE PLOTTED ON H-AXIS	02JE5
C	HPOS	LENGTH, IN INCHES, OF THE POSITIVE H-AXIS	14AP4
C	HTCKS	INCREMENT LENGTH BETWEEN TICK MARKS ON H-AXIS IN TERMS OF HEND	14AP4
C	HT2	H TIMES 2	27AP4
C	IL	INFLUENCE LINE STORAGE LOCATION RELATED TO THE POSITION OF THE MOVABLE LOAD	04JE5
C	ILOPD, ILOPM, ILOPR, ILOPV	INFLUENCE LINE OUTPUT OPTIONS	200C5
C		IF = 1, PLOT THE INFLUENCE DIAGRAM	24JA7
C		IF = 2, TABULATE THE INFLUENCE LINE	200C5
C		IF = 3, PLOT AND TABULATE THE INFLUENCE LINE	200C5
C	ILS	INTERNAL ADJUSTMENT FOR THE POSITION OF MOVABLE LOAD WHICH PREVENTS TABULATION OF	200C5
C		INFLUENCE LINES AT NEG STORAGE LOCATIONS	02JE5
C	IMNI()	LOCATION OF MOVABLE-LOAD STA ZERO THAT PRODUCES THE MAX NEGATIVE BENDING MOMENT	02JE5
C	IMX()	LOCATION OF MOVABLE-LOAD STA ZERO THAT PRODUCES THE MAX POSITIVE BENDING MOMENT	02JE5
C	INSAV	TEMPORARY VALUE OF IN13()	02JE5
C	IN13()	EXTERNAL STA NUMBER FOR SPECIFIED SLOPE OR DEFLECTION	02JE5
C	IN14()	INITIAL EXTERNAL STA USED IN TABLE 4	12FE5
C	IN15()	INITIAL EXTERNAL STA USED IN TABLE 5	02JE5
C	IN24()	FINAL EXTERNAL STA USED IN TABLE 4	02JE5
C	IN25()	FINAL EXTERNAL STA USED IN TABLE 5	02JE5
C	IOR	ORIENTATION OF THE TITLE, = PARALLEL TO THE AXIS	15JL4
C	IPLOT()	VARIABLE USED TO PLOT TITLES ON PLOTS	10JE5
C	IRN()	LOCATION FOR MAX NEGATIVE REACTION	02JE5

C	IRX()	LOCATION FOR MAX POSITIVE REACTION	02JE5
C	ISD()	DESIGNATED STAS FOR INFLUENCE LINES FOR	02JE5
C		DEFLECTION	02JE5
C	ISIZE	SIZE OF THE LETTERS IN THE PLOT TITLE	15JL4
C	ISM()	DESIGNATED STAS FOR INFLUENCE LINES FOR	02JE5
C		MOMENT	02JE5
C	ISR()	DESIGNATED STAS FOR INFLUENCE LINES FOR	03JE5
C		SUPPORT REACTION	03JE5
C	ISTA	OUTPUT VALUE OF STA NUMBER	03JE5
C	ISV()	DESIGNATED STAS FOR INFLUENCE LINES FOR	03JE5
C		SHEAR	03JE5
C	ITEST	= 5 ALPHANUMERIC BLANKS USED TO TERMINATE	12FE5
C		THE PROGRAM	12FE5
C	IT1S	PLOT SYMBOL (-8), PRODUCES A LINE PLOT	10JE5
C	IVN()	LOCATION FOR MAX NEGATIVE SHEAR	03JE5
C	IVX()	LOCATION FOR MAX POSITIVE SHEAR	03JE5
C	IWN()	LOCATION FOR MAX NEGATIVE DEFLECTION	03JE5
C	IWX()	LOCATION FOR MAX POSITIVE DEFLECTION	03JE5
C	J	DO LOOP INDEX, = STA NUMBER	03JE5
C	JA	INDEX IN TABLE 3 SORTING PROCEDURE	12FE5
C	JL	POSITION OF MOVABLE-LOAD STA ZERO	12FE5
C	JLINC	INDEX USED IN MOVING THE MOVABLE LOAD	10JE5
C	JM1	JA - 1	12FE5
C	JS	STA OF SPECIFIED DEFLECTION OR SLOPE	05JE3
C	JSTA	EXTERNAL VALUE OF JSTART	12FE5
C	JSTART	INITIAL POSITION OF MOVABLE-LOAD STA ZERO	12FE5
C	JSTO	EXTERNAL VALUE OF JSTOP	12FE5
C	JSTOP	FINAL POSITION OF MOVABLE-LOAD STA ZERO	12FE5
C	K	DO LOOP INDEX	03JE5
C	KASE	CASE NUM FOR SPECIFIED CONDITIONS	07JE3
C	KASS	TEMPORARY VALUE FOR KASE, KSW4, OR KSW5	24JA7
C	KAXES	INDEX USED TO DESCRIBE MANNER IN WHICH	10JL4
C		AXES SHOULD BE PLACED ON PAPER	14AP4
C	KC	NUMBER OF PLOT VARIABLE	03JE5
C	KEEP1	IF = 1, KEEP PRIOR ENVELOPES OF MAXIMUMS	10JE5
C	KEEP2 THRU KEEP6	IF = 1, KEEP PRIOR DATA, TABLES 2-6	24JA7
C	KEY()	ROUTING SWITCH FOR SPECIFIED CONDITIONS	24JA7
C	KPLOT()	PLOT OPTIONS FOR THE ENVELOPES OF MAXIMUMS	03JE5
C	KPS	PLOT SYMBOL TO BE USED	14AP4
C		1 DENOTES SMALL PLUS SIGN	14AP4
C		2 DENOTES SMALL X	14AP4
C		3 DENOTES SMALL SQUARE	14AP4
C		4 DENOTES SMALL STAR (COMBINATION OF 1, 2)	14AP4
C		8 DENOTES NO SYMBOL TO BE PLOTTED, BUT	14AP4
C		PEN TO BE MOVED	14AP4
C		9 DENOTES LARGE PLUS SIGN	14AP4
C		10 DENOTES LARGE X.	14AP4
C		11 DENOTES LARGE SQUARE	14AP4
C		12 DENOTES LARGE STAR (COMBINATION OF 9	14AP4
C		AND 10)	14AP4
C		A NEGATIVE NUMBER WILL MAKE THE SAME	14AP4
C		SYMBOLS AS THE POSITIVE NUMBERS, BUT THE	14AP4
C		POINTS WILL BE CONNECTED WITH STRAIGHT	14AP4
C		LINES. THUS IF KPS = -8 THE POINTS WILL	14AP4
C		BE CONNECTED BY STRAIGHT LINES WITHOUT	14AP4
C		THE POINTS BEING MARKED BY PLOT SYMBOLS	14AP4

C	KR1	PRIOR VALUE OF KR2	19MR4
C	KR24(), KR25()	CONTINUE SWITCHES IN TABLES 4 AND 5	03JE5
C	KSAV	TEMPORARY VALUE OF KASE()	03JE5
C	KSTOP	ROUTING SWITCH TO SKIP PLOT TAPE	07JE5
C	KSW4(), KSW5()	ROUTING SWITCH IN TABLES 4 AND 5	03JE5
C	LL	MOVABLE-LOAD STA	03JE5
C	LSM	SWITCH WHICH CAUSES THE AXIAL LOADS TO BE DISTRIBUTED TO HALF-STAS	03JE5
C	M	TOTAL NUMBER OF INCREMENTS OF BMCOL	12JE3
C	ML	TOTAL MOVABLE-LOAD WIDTH IN INCREMENTS	12FE5
C	MLINC	NUMBER OF STAS BETWEEN EACH POSITION OF	12FE5
C	MLP4	MOVABLE LOAD	10JE5
C	MP4 THRU MP7	ML + 4	12FE5
C	N	M + 4 THRU M + 7	12JE3
C	NAME()	MISCL INDEX	12JE3
C	NCD2 THRU NCD6	VARIABLE NAME - OUTPUT IN TABLES 9 AND 11	03JE5
C	NCT3, NCT4, NCT5	NUM CARDS IN TABLES 2 THRU 6, THIS PROB	03JE5
C	NC14, NC15	TOTAL NUMBER OF CARDS IN THE PARTICULAR TABLE	12FE5
C	NPROB	INITIAL INDEX VALUE FOR THE INPUT TO THE PARTICULAR TABLE	24JA7
C	NPTS	PROBLEM NUMBER (PROG STOPS IF BLANK)	24JA7
C	NSD	NUMBER OF POINTS TO BE PLOTTED, = 2	10JE5
C	NSM	NUMBER OF DEFLECTION INFLUENCE LINES	03JE5
C	NSR	NUMBER OF MOMENT INFLUENCE LINES	03JE5
C	NSV	NUMBER OF SUPPORT REACTION INFLUENCE LINES	03JE5
C	NSYMB	NUMBER OF SHEAR INFLUENCE LINES	03JE5
C	NUMPTS	NUM OF SYMBOLS TO BE PLOTTED IN THE TITLE	15JL4
C	N1	NUMBER OF POINTS TO BE PLOTTED	14AP4
C	P()	STORAGE LOCATION OF FIRST PLOT POINT	07JE5
C	PN2()	AXIAL FORCE (TOTAL PER STA)	03JE5
C	Q()	AXIAL FORCE (INPUT)	03JE5
C	QD()	TRANSVERSE FORCE (TOTAL PER STA)	03JE5
C	QDN2()	FIXED LOAD (TOTAL PER STA)	03JE5
C	QLIVE()	INPUT VALUE OF FIXED LOAD	10JE5
C	QM()	TEMPORARY STORAGE FOR MOVABLE-LOAD PATTERN	10FE6
C	QMN2()	MOVABLE LOAD (TOTAL PER STA)	03JE5
C	R()	INPUT VALUE OF MOVABLE LOAD	10JE5
C	REACT()	ROTATIONAL RESTRAINT (TOTAL PER STA)	03JE5
C	REACTF()	SUPPORT REACTION	24JA7
C	RIL()	FIXED LOAD SUPPORT REACTION	24JA7
C	RMAX()	INFLUENCE LINE FOR SUPPORT REACTION	03JE5
C	RMIN()	MAX VALUE OF POSITIVE REACTION	03JE5
C	RN2()	MAX VALUE OF NEGATIVE REACTION	03JE5
C	S()	ROTATIONAL RESTRAINT (INPUT)	03JE5
C	SN2()	SPRING SUPPORT STIFFNESS (TOTAL PER STA)	03JE5
C	SPACE	SPRING SUPPORT STIFFNESS (INPUT)	03JE5
C	T()	DISTANCE, IN INCHES, FROM LEFT EDGE OF	14AP4
C	TN2()	PAPER TO END OF NEGATIVE H-AXIS	14AP4
C	T1 THRU T5	TRANSVERSE TORQUE (TOTAL PER STA)	03JE5
C	VCOR	TRANSVERSE TORQUE (INPUT)	03JE5
C		PLACES BLANK OR ASTERICK BESIDE LOCATION INDICATOR IN ENVELOPES OF MAXIMUMS	24JA7
C		VERTICAL COORDINATE OF THE BOTTOM OF THE PLOT TITLE	15JL4
C			15JL4

C	VEND	VERTICAL AXIS VALUE AT 2 INCHES	24AP4
C	VIL()	INFLUENCE LINE FOR SHEAR	03JE5
C	VMAX()	MAX VALUE OF POSITIVE SHEAR	03JE5
C	VMIN()	MAX VALUE OF NEGATIVE SHEAR	03JE5
C	VNEG	LENGTH, IN INCHES, OF THE NEGATIVE V-AXIS.14AP4 THIS VARIABLE MUST NOT ITSELF BE NEGATIVE.14AP4	
C	VOR	VALUE AT THE END OF THE VERTICAL AXIS	03JE5
C	VPLOT	NAME OF ARRAY TO BE PLOTTED ON V-AXIS	03JE5
C	VPOS	LENGTH, IN INCHES, OF THE POSITIVE V-AXIS 14AP4	
C	VTCKS	INCREMENT LENGTH BETWEEN TICK MARKS ON V-AXIS IN TERMS OF VEND	14AP4
C	W()	LATERAL DEFLECTION OF BMCOL AT STA J	16AP4
C	WF()	FIXED LOAD LATERAL DEFLECTION	03JE5
C	WMAX()	MAX VALUE OF POSITIVE DEFLECTION	24JA7
C	WMIN()	MAX VALUE OF NEGATIVE DEFLECTION	03JE5
C	WS()	SPECIFIED VALUE OF DEFL AT STA JS	03JE5
C	WSAV	TEMPORARY VALUE OF WS()	07JE5
C	X	DISTANCE ALONG THE BMCOL	30MY3
C	ZTEMP	TEMPORARY MAX VALUE	10JE5
C	Z1	DECIMAL VALUE FOR ISTA	25JL3

C-----NOTATION FOR SUBROUTINE INTERP 3		03JE5
C	ASM	FACTOR ADDED TO THE STA NUMBER SO THAT THE03JE5
C		AXIAL LOADS WILL BE TABULATED AT HALF-STAS03JE5
C	DENOM	DENOMINATOR 07JE3
C	DIFF	DIFFERENCE 12JE3
C	ESM	MULTIPLIER FOR HALF VALUES AT END STAS 03JE5
C	F	FLOATING POINT STA NUM 03JE5
C	F1	FLOATING POINT VALUE OF J1 03JE5
C	ISW	ROUTING SWITCH 03JE5
C	J	DO LOOP INDEX, = STA NUMBER 03JE5
C	JINCR	INCREMENTATION INDEX 03JE5
C	JS2	TEMPORARY VALUE OF JN2() USED FOR ERASING 10FE6
C		LOADS PAST THE END OF THE BEAM 10FE6
C	JV	INITIAL STA NUMBER ON THE PREVIOUS CARD 03JE5
C	J1	INITIAL STA IN THE DISTRIBUTION SEQUENCE 03JE5
C	J2	FINAL STA IN THE DISTRIBUTION SEQUENCE 03JE5
C	JN1()	INITIAL EXTERNAL STA 07JE5
C	JN2()	FINAL EXTERNAL STA 03JE5
C	KASS	TEMPORARY VALUE FOR KASE, KSW4, OR KSW5 24JA7
C	KR2()	CONTINUE SWITCH 03JE5
C	KR1	PRIOR VALUE OF KR2() 03JE5
C	KSW()	VALUE OF KSW4() OR KSW5() IN SUBROUTINE 24JA7
C	LSM	SWITCH WHICH CAUSES THE AXIAL LOADS TO BE 03JE5
C		DISTRIBUTED TO HALF-STAS 03JE5
C	M	TOTAL NUMBER OF INCREMENTS OF BMCOL 12JE3
C	MP7	NUMBER OF INCREMENTS + 7 03JE5
C	MP5	MP7 - 2 03JE5
C	MP6	MP7 - 1 03JE5
C	NC	NUMBER OF CARD THAT IS BEING INTERPOLATED 07JE5
C	NCT	NUMBER OF CARDS TO BE INTERPOLATED 03JE5
C	NC1	PREVIOUS CARD NUMBER 03JE5
C	PART	INTERPOLATION FRACTION 03JE5
C	Z()	INTERPOLATED VALUE 03JE5
C	ZCK	SUM OF THE VALUES AT THE OUTRIGGER STA 10JE5
C	ZNI()	INPUT VALUE 03JE5
C	ZNS	TEMPORARY VALUE OF ZN2() USED FOR ERASING 10FE6
C		LOADS PAST THE END OF THE BEAM 10FE6

C-----NOTATION FOR SUBROUTINE COMPARE		08MR8
C	BM()	BENDING MOMENT
C	BMMAX()	MAX VALUE OF POSITIVE BENDING MOMENT
C	BMMIN()	MAX VALUE OF NEGATIVE BENDING MOMENT
C	DBM()	SHEAR BETWEEN ADJACENT STATIONS
C	DW()	FIRST DERIV OF BMCOL DEFL
C	DWS()	VALUE OF SPECIFIED SLOPE
C	F()	FLEXURAL STIFFNESS (TOTAL PER STA)
C	H	INCREMENT LENGTH
C	HE2	H SQUARED
C	HE3	H CUBED
C	HT2	H TIMES 2
C	IMN()	LOCATION OF MOBILE-LOAD STA ZERO THAT PRODUCES THE MAX NEGATIVE BENDING MOMENT
C	IMX()	LOCATION OF MOBILE-LOAD STA ZERO THAT PRODUCES THE MAX POSITIVE BENDING MOMENT
C	IRN()	LOCATION FOR MAX NEGATIVE REACTION
C	IRX()	LOCATION FOR MAX POSITIVE REACTION
C	IVN()	LOCATION FOR MAX NEGATIVE SHEAR
C	IVX()	LOCATION FOR MAX POSITIVE SHEAR
C	IWN()	LOCATION FOR MAX NEGATIVE DEFLECTION
C	IWX()	LOCATION FOR MAX POSITIVE DEFLECTION
C	J	DO LOOP INDEX, = STA NUMBER
C	JL	POSITION OF MOBILE-LOAD STA ZERO
C	KEY()	ROUTING SWITCH FOR SPECIFIED CONDITIONS
C	M	TOTAL NUMBER OF INCREMENTS OF BMCOL
C	MP4, MP5	M + 4, M + 5
C	REACT()	NET REACTION ON THE BMCOL AT EACH STA
C	P()	AXIAL FORCE (TOTAL PER STA)
C	Q()	TRANSVERSE FORCE (TOTAL PER STA)
C	R()	ROTATIONAL RESTRAINT (TOTAL PER STA)
C	RMAX()	MAX VALUE OF POSITIVE REACTION
C	RMIN()	MAX VALUE OF NEGATIVE REACTION
C	S()	SPRING SUPPORT STIFFNESS (TOTAL PER STA)
C	T()	TRANSVERSE TORQUE (TOTAL PER STA)
C	VMAX()	MAX VALUE OF POSITIVE SHEAR
C	VMIN()	MAX VALUE OF NEGATIVE SHEAR
C	W()	LATERAL DEFLECTION OF BMCOL AT STA J
C	WMAX()	MAX VALUE OF POSITIVE DEFLECTION
C	WMIN()	MAX VALUE OF NEGATIVE DEFLECTION
C	WS()	SPECIFIED VALUE OF DEFL AT STA JS

C-----NOTATION FOR SUBROUTINE SCALE		08MR8
C	KEXP	EXponent OF THE MAX VALUE OF THE VARIABLE
C	N	DO LOOP INDEX
C	SEND	MAXIMUM VALUE OF THE VARIABLE
C	ST	LENGTH OF THE AXIS
C	STEMP	TEMPORARY VALUE OF SEND
C	S1	ST * .1
C	S2	ST * .2
C	S5	ST * .5

C-----NOTATION FOR SUBROUTINE BEAMCOL		08MR8
C	A(), ATEMP, AREV	CONTINUITY COEFFICIENT 02JE5
C	AA	COEFF IN STIFFNESS MATRIX 12JE3
C	B(), BTEMP, BREV	CONTINUITY COEFFICIENT 02JE5
C	BB	COEFF IN STIFFNESS MATRIX 12JE3
C	BMI()	BENDING MOMENT 02JE5
C	BMAX()	MAX VALUE OF POSITIVE BENDING MOMENT 02JE5
C	BMIN()	MAX VALUE OF NEGATIVE BENDING MOMENT 02JE5
C	C(), CTEMP, CREV	CONTINUITY COEFFICIENT 02JE5
C	CC	COEFF IN STIFFNESS MATRIX 12JE3
C	D, DTEMP, DREV	MULTIPLIER IN CONTINUITY COEFF EQS 12JE3
C	DBM()	SHEAR BETWEEN ADJACENT STATIONS 08MR8
C	DD	COEFF IN STIFFNESS MATRIX 12JE3
C	DENOM	DENOMINATOR 07JE3
C	DW()	FIRST DERIV OF BMCOL DEFL 07JE5
C	DWS()	VALUE OF SPECIFIED SLOPE 02JE5
C	E	TERM IN CONTINUITY COEFF EQS 12JE3
C	EE	COEFF IN STIFFNESS MATRIX 12JE3
C	F()	FLEXURAL STIFFNESS (TOTAL PER STA) 02JE5
C	FF	COEFF IN LOAD MATRIX 12JE3
C	H	INCREMENT LENGTH 12JE3
C	HE2	H SQUARED 12JE3
C	HE3	H CUBED 12JE3
C	HT2	H TIMES 2 12JE3
C	IMN()	LOCATION OF MOVABLE-LOAD STA ZERO THAT PRODUCES THE MAX NEGATIVE BENDING MOMENT 02JE5
C	IMX()	LOCATION OF MOVABLE-LOAD STA ZERO THAT PRODUCES THE MAX POSITIVE BENDING MOMENT 12FE5
C	IRN()	LOCATION FOR MAX NEGATIVE REACTION 02JE5
C	IRX()	LOCATION FOR MAX POSITIVE REACTION 02JE5
C	IVN()	LOCATION FOR MAX NEGATIVE SHEAR 03JE5
C	IVX()	LOCATION FOR MAX POSITIVE SHEAR 03JE5
C	IWN()	LOCATION FOR MAX NEGATIVE DEFLECTION 03JE5
C	IWX()	LOCATION FOR MAX POSITIVE DEFLECTION 03JE5
C	J	DO LOOP INDEX, = STA NUMBER 03JE5
C	JL	POSITION OF MOVABLE-LOAD STA ZERO 12FE5
C	KEY(), KEYJ	ROUTING SWITCH FOR SPECIFIED CONDITIONS 03JE5
C	L	MISCL INDEX 12JE3
C	M	TOTAL NUMBER OF INCREMENTS OF BMCOL 12JE3
C	MP4, MP5	M + 4 , M + 5 24JA7
C	NS	INDEX NUM FOR SPECIFIED CONDITIONS 05JE3
C	P()	AXIAL FORCE (TOTAL PER STA) 03JE5
C	Q()	TRANSVERSE FORCE (TOTAL PER STA) 03JE5
C	R()	ROTATIONAL RESTRAINT (TOTAL PER STA) 03JE5
C	REACT()	NET REACTION ON THE BMCOL AT EACH STA 10JE5
C	RMAX()	MAX VALUE OF POSITIVE REACTION 03JE5
C	RMIN()	MAX VALUE OF NEGATIVE REACTION 03JE5
C	S()	SPRING SUPPORT STIFFNESS (TOTAL PER STA) 03JE5
C	T()	TRANSVERSE TORQUE (TOTAL PER STA) 03JE5
C	VMAX()	MAX VALUE OF POSITIVE SHEAR 03JE5
C	VMIN()	MAX VALUE OF NEGATIVE SHEAR 03JE5
C	VPLOT	NAME OF ARRAY TO BE PLOTTED ON V-AXIS 03JE5
C	W()	LATERAL DEFLECTION OF BMCOL AT STA J 03JE5
C	WMAX()	MAX VALUE OF POSITIVE DEFLECTION 03JE5
C	WMIN()	MAX VALUE OF NEGATIVE DEFLECTION 03JE5
C	WS()	SPECIFIED VALUE OF DEFL AT STA JS 03JE5

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APPENDIX 5

LISTING OF PROGRAM DECK OF EBCOL 43

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PROGRAM BMCOL 43 (INPUT,OUTPUT)

C-----FOR COMPUTERS WITH LESS THAN ELEVEN DIGIT ACCURACY, IT IS
 C RECOMMENDED THAT THE FOLLOWING VARIABLES BE MADE DOUBLE
 C PRECISION IN THE MAIN PROGRAM AND IN SUBROUTINE BEAMCOL.
 C W, DW, BM, DBM, REACT, DENOM, A, B, C, D, E, ATEMP,
 C BTEMP, CTEMP, DTEMP, AREV, BREV, CREV, DREV

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1 FORMAT ( 52H      PROGRAM BMCOL 43 -      - MATLOCK-TAYLOR - 03137
1                               284 REVISION DATE = 08 MAR 68 ) 18FE5 ID
1 DIMENSION   AN1(32), AN2(14),
1             F(207), Q(207), S(207), T(207), R(207), P(207),
1             KEY(207), IN13(20), KASE(20), WS(20), DWS(20),
1             IN14(100), IN24(100), KR24(100), FN2(100),
1             QDN2(100), SN2(100), TN2(100), RN2(100), PN2(100),
1             KSW4(100), IN15(100), IN25(100), KR25(100),
1             QMN2(100), KSW5(100), QD(207), QM(207),
1             QLIVE(207), BM(207), W(207), DBM(207), REACT(207) 25MR5
1             IMX(207), IMN(207), IWX(207), IWN(207), IVX(207),
1             IVN(207), IRX(207), IRN(207), DW(207), 25MY5
1             KPLOT(4), IPLDT(3), HPLOT(407), VPLOT(207,8),
1             NAME(4),ISM(5),ISD(5),ISV(5),ISR(5), 27MR5
1             BMIL(407,5),DIL(407,5),VIL(407,5),RIL(407,5),
1             HOR(2), VOR(2),WF(5),BMF(5),DBMF(5),REACTF(5) 08JE5
1             COMMON F, Q, S, T, R, P, KEY, WS, DWS, W, DW, BM, DBM, REACT,
1             IMX, IMN, IWX, IWN, IVX, IVN, IRX, IRN, JL, H, HE2, HE3, 26OC4
1             HT2, M, MP4, MPS 27MR5
1             COMMON/A/VPLOT 27MR5
10 FORMAT ( 5H      , 80X, 10H!----TRIM ) 18JA7
11 FORMAT ( 5H1     , 80X, 10H!----TRIM ) 27FE4 ID
12 FORMAT ( 16A5   ) 04MY3 ID
13 FORMAT (      5X, 16A5 ) 27FE4 ID
14 FORMAT ( A5, 5X, 14A5 ) 18FE5 ID
15 FORMAT (///10H    PROB , /5X, A5, 5X, 14A5 ) 18FE5 ID
15 FORMAT (///17H    PROB (CONTD), /5X, A5, 5X, 14A5 ) 18FE5 ID
19 FORMAT (///48H    RETURN THIS PAGE TO TIME RECORD FILE -- HM ) 12MR5 ID
20 FORMAT ( 5X, 15I5 ) 13MR5
21 FORMAT ( 5X, I5, 10X, E10.3, 10X, 4I5 ) 16FE5
31 FORMAT ( 2(5X, I5), 2E10.3 ) 23MR4
41 FORMAT ( 5X, 3I5, 6E10.3 ) 23MR4
51 FORMAT ( 5X, 3I5, 10X, E10.3 ) 25MR5
60 FORMAT ( I5 ) 22MR5
100 FORMAT (///35H   TABLE 1 - PROGRAM-CONTROL DATA 24JA4*
1   / 43X, 35H   ENVELOPES      TABLE NUMBER 15MR5
2   / 43X, 40H   OF MAXIMUMS   2   3   4   5   5 15MR5
3  // 46H        HOLD FROM PRECEDING PROBLEM (1=HOLD), 4X, 13MR5
4           15, 3X, 5I5, 13MR5
5   / 38H        NUM CARDS INPUT THIS PROBLEM, 20X, 5I5 ) 13MR5
101 FORMAT ( / 58X, 25H   DEFL MOM SHR RCT 23MR5
1           / 5X, 47H   OPTION (IF=1) TO PLOT ENVELOPES OF MAXIMUM 15MR5
2           1HS, 10X, 4I5 ) 23MR5
110 FORMAT (///51H   TABLE 10A -- INFLUENCE DIAGRAMS FOR DEFLECTION) 10FE6
111 FORMAT (///50H   TABLE 10B -- INFLUENCE DIAGRAMS FOR MOMENT ) 10FE6
112 FORMAT (///50H   TABLE 10C -- INFLUENCE DIAGRAMS FOR SHEAR 10FE6
1   / 15X, 45H   ( SHEAR IS COMPUTED ONE HALF INCREMENT 20AG5
2   / 15X, 45H   TO THE LEFT OF THE DESIGNATED STATION ) ) 20AG5
113 FORMAT (///52H   TABLE 10D -- INFLUENCE DIAGRAMS FOR SUPPORT REA10FE6

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1      5HCTION )
115 FORMAT ( / 52H           LOCATION          DESIGNATED STATIONS F029MY5
1           21HR INFLUENCE DIAGRAMS   /
2           52H           OF LOAD        STA       STA       STA 23JA67
3           25H           STA           STA       )           29MY5
117 FORMAT ( 15X,           5( 7X, 15 ) )           23JA67
118 FORMAT ( 10X, I5, 5X, 5E12.3 )           29MY5
120 FORMAT (///51H           TABLE 11 -- SCALES FOR INFLUENCE DIAGRAM PLOTS) 10FE6
123 FORMAT ( 30H           HORIZONTAL SCALE   /
1           24H           10 INCHES =, F4.0, 9H STATIONS )           25MY5
124 FORMAT ( / 30H           VERTICAL SCALES   /
1           49H           LENGTH         MAXIMUM   / 24MR5
2           49H           STA VARIABLE   OF AXIS     VALUE   / 25MY5
125 FORMAT ( 10X, I5, 2X, A8, 12H 2 INCHES =, E10.3 )           24MR5
200 FORMAT (///24H           TABLE 2 - CONSTANTS / )           23MR4*
201 FORMAT ( 28H           NUM INCREMENTS , 50X, I5,           31MR4*
1           / 28H           INCREMENT LENGTH , 45X, E10.3,           260C4
2           / 47H           NUMBER OF INCREMENTS FOR MOBILE LOAD,           230C4
331X, I5, / 51H           INITIAL POSITION OF MOBILE LOAD STA ZERO, 230C4
427X, I5, / 49H           FINAL POSITION OF MOBILE LOAD STA ZERO, 230C4
529X, I5, / 52H           NUMBER OF INCREMENTS BETWEEN EACH POSITION 230C4
6           16H OF MOBILE LOAD, 10X, I5 )           230C4
300 FORMAT (///47H           TABLE 3 - SPECIFIED DEFLECTIONS AND SLOPES 20JA4*
1 // 5X, 48H           STA CASE DEFLECTION SLOPE )           25MY5
311 FORMAT ( 10X, I3, 7X, I2, 8X, E10.3, 9X, 4HNONE, 5X, A5 )           02FE5
312 FORMAT ( 10X, I3, 7X, I2, 11X, 4HNONE, BX, E10.3, 5X, A5 )           02FE5
313 FORMAT ( 10X, I3, 7X, I2, 3X, 2(5X, E10.3), 5X, A5 )           02FE5
400 FORMAT (///48H           TABLE 4 - STIFFNESS AND FIXED-LOAD DATA 230C4
1 // 51H           FROM TO CONTD F QF S           09FE5
2           28H           T R P / )           13JE3
411 FORMAT ( 5X, 2I4, I3, 1X, 6E11.3 )           23MR4*
412 FORMAT ( 5X, I4, 4X, I3, 1X, 6E11.3 )           23MR4*
413 FORMAT ( 9X, I4, I3, 1X, 6E11.3 )           23MR4*
500 FORMAT (///40H           TABLE 5 - MOBILE-LOAD DATA           230C4
1           //25H           FROM TO CONTD QM / )           230C4
600 FORMAT (///47H           TABLE 6 - SPECIFIED STATIONS FOR INFLUENCE 27MR5
1           9H DIAGRAMS
2           / 5X, 51H           ( SHEAR IS COMPUTED ONE HALF INCREMENT           20AG5
3           / 10X, 45H           TO THE LEFT OF THE DESIGNATED STATION ) / )           20AG5
601 FORMAT ( 40H           NUM OF TYPE OF / )           24JA67
1           52H           VARIABLE DIAGRAMS OUTPUT DESIGNAT 24JA67
2           15HED STATIONS / )           24JA67
605 FORMAT ( 5X, 2I5, 5X, 5I5 )           24JA67
610 FORMAT ( 10X, A8, 2X, 2( I5, 5X), 5I5 )           16JA67
690 FORMAT ( / 43H           ALLOWABLE NUMBER OF INFLUENCE DIAGRAMS           10FE6
1           10H EXCEEDED )
700 FORMAT (// 35H           TABLE 7 - FIXED-LOAD RESULTS )           29MR5
701 FORMAT ( / 48H           STA I DIST DEFL SLOPE           25FE6
1           36H           MOM SHEAR SUP REACT / )           25FE6
711 FORMAT ( 5X, I4, 2X, 2E12.3, 10X, E12.3, 10X, E12.3 )           06MY5
712 FORMAT ( 34X, E12.3, 10X, E12.3 )           03MY5
800 FORMAT (///40H           TABLE 8A- ENVELOPES OF MAXIMUMS           07JA67
1           40H * = HELD FROM PRIOR PROBLEM )           07JA67
801 FORMAT ( / 52H           STA MAX +DEFL LOC MAX -DEFL LOC M10FE6
1           30HAX +MOM LOC MAX -MOM LOC )           10FE6
810 FORMAT ( 5X, I5, 4X, 4( E10.3, I4, A1, 3X )/ )           24JA67
820 FORMAT (///40H           TABLE 8B- ENVELOPES OF MAXIMUMS           07JA67
1           40H * = HELD FROM PRIOR PROBLEM )           07JA67
830 FORMAT ( / 52H           STA MAX +SHEAR LOC MAX -SHEAR LOC MA27AP5
1           30HX +REACT LOC MAX -REACT LOC )           27A75

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835 FORMAT ( 5X, I5, 40X, 2( E10.3, I4, A1, 3X ) )          24JA67
840 FORMAT ( 14X, 2( E10.3, I4, A1, 3X ) )          24JA67
900 FORMAT ( //50H      TABLE 9 -- SCALES FOR PLOTS OF THE ENVELOPES 25MY5
1           12HOF MAXIMUMS   / )          25MY5
901 FORMAT ( / 30H      VERTICAL SCALES   / )          25MY5
1           45H      LENGTH      MAXIMUM   / )          25MY5
2           45H      VARIABLE OF AXIS   VALUE   / )          26MY5
902 FORMAT ( 13X, A8, 12H 2 INCHES =, E10.3 )          25MY5
903 FORMAT ( / 25H      NONE )          04FE4*
904 FORMAT ( // 40H      TOO MUCH DATA FOR AVAILABLE STORAGE // ) 04FE4*
905 FORMAT ( 46H      USING DATA FROM THE PREVIOUS PROBLEM ) 05FE4*
907 FORMAT ( / 52H      ERROR STOP -- SLOPES AND DEFLECTIONS IMPROPERLY31DE4
1           10H SPECIFIED )          31DE4
909 FORMAT ( //50H      TABULATION OF INFLUENCE DIAGRAMS OMITTED )10FE6
910 FORMAT ( 43H      ADDITIONAL DATA FOR THIS PROBLEM ) 31DE4
911 FORMAT ( 30H      NO PLOTS SPECIFIED )          19AG5
940 FORMAT ( //36H      MOBILE LOAD SOLUTIONS OMITTED ) 07JA5
950 FORMAT ( //52H      ALLOWABLE RANGE OF MOVEMENT OF MOBILE LOAD EXC12N04
1           5HEEDED )          12N04
980 FORMAT (///50H      UNDESIGNATED ERROR STOP )          31DE4
C-----START EXECUTION OF PROGRAM - SEE GENERAL FLOW DIAGRAM 10FE6
ITEST = 5H          18FE5 ID
KSTOP = 0          12MR5
NCT3 = 0          18N04
NCT4 = 0          19OC4
NCT5 = 0          23OC4 -
NSD = 0          01JE5
NSM = 0          01JE5
NSV = 0          01JE5
NSR = 0          01JE5
ILOPD = 0          11JE5
ILOPM = 0          11JE5
ILOPV = 0          11JE5
ILOPR = 0          11JE5
1000 PRINT 10          12JL3 ID
CALL TIC TOC (1)          08MR8
C-----PROGRAM AND PROBLEM IDENTIFICATION          04MY3 ID
READ 12, ( AN1(N), N = 1, 32 )          18FE5 ID
1010 READ 14, NPROB, ( AN2(N), N = 1, 14 )          28AG3 ID
IF ( NPROB - ITEST ) 1020, 9990, 1020          26FE5 ID
1020 PRINT 11          26AG3 ID
PRINT 1          18FE5 ID
PRINT 13, ( AN1(N), N = 1, 32 )          18FE5 ID
PRINT 15, NPROB, ( AN2(N), N = 1, 14 )          26AG3 ID
C-----INPUT TABLE 1          10JE3
1100 READ 20, KEEPE, KEEP2, KEEP3, KEEP4, KEEP5, KEEP6, NCD2, NCD3,
1           NCD4, NCD5, NCD6, ( KPLOT(N), N = 1, 4 ) 13MR5
PRINT 100, KEEPE, KEEP2, KEEP3, KEEP4, KEEP5, KEEP6, NCD2, NCD3,
1           NCD4, NCD5, NCD6          13MR5
PRINT 101, ( KPLOT(N), N = 1, 4 )          13MR5
C-----INPUT TABLE 2          10JE3
1200 PRINT 200
IF ( KEEP2 ) 9980, 1210, 1255          15JA5
1210 READ 21, M, H, ML, JSTART, JSTOP, MLINC          19OC4
C-----COMPUTE CONSTANTS AND INDEXES          10JE3
HT2 = H + H          11JA5
HE2 = H * H          30MY3
HE3 = H * HE2          30MY3
MP4 = M + 4          30MY3
MP5 = M + 5          30MY3

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MP6 = M + 6          10JE3
MP7 = M + 7          30MY3
MLP4 = ML + 4        31DE4
JSTA = JSTART        19JA5
JSTO = JSTOP         19JA5
JSTART = JSTART + 4  15JA5
JSTOP = JSTOP + 4   11NO4
NAME(1) = 8HDEFLECT 25MY5
NAME(2) = 8HMOMENT  25MY5
NAME(3) = 8HSHEAR   13MR5
NAME(4) = 8HREACTION 13MR5
C-----LIMIT THE RANGE OF MOVEMENT OF MOBILE-LOAD TO REASONABLE VALUES 01JE5
    IF ( JSTART + ML + MLINC - 4 ) 1235, 1225, 1225 15JA5
1225    IF ( JSTOP - JSTART ) 1235, 1230, 1230 15JA5
1230    IF ( JSTOP - M - MLINC - 4 ) 1260, 1260, 1235 15JA5
1235 PRINT 950      15JA5
    GO TO 9990       26MR5
1255 PRINT 905      15JA5
1260 PRINT 201, M, H, ML, JSTA, JSTO, MLINC 19JA5
    DO 1270 J = 1, MP7 18NO4
    QLIVE(J) = 0.0   11NO4
1270 CONTINUE        18NO4
C-----INPUT TABLE 3  01JE5
    PRINT 300         11JA5
    DO 1303 J = 1, MP7 23FE5
    KEY(J) = 1        03JE3
1303 CONTINUE        23FE5
    IF ( KEEP3 ) 9980, 1310, 1305 26MY5
1305 PRINT 905      26MY5
    GO TO 1326       26MY5
1310    IF ( NCD3 - 20 ) 1312, 1312, 1311 25MY5
1311 PRINT 904      25MY5
    GO TO 9990       25MY5
1312    NCT3 = NCD3  25MY5
    IF ( NCT3 - 1 ) 1326, 1320, 1320 08MR8
1320    DO 1325 N = 1, NCT3 26SE7
    READ 31, IN13(N), KASE(N), WS(N), DWS(N) 18NO4
1325 CONTINUE        02FE5
1326    IF ( NCT3 ) 9980, 1327, 1328 25MY5
1327 PRINT 903      25MY5
    GO TO 1400       25MY5
1328    IF ( NCT3 - 2 ) 1355, 1329, 1329 08MR8
1329    DO 1350 JA = 2, NCT3 08MR8
    JM1 = JA - 1     31DE4
    DO 1345 N = JA, NCT3 02FE5
    IF ( IN13(JM1) - IN13(N) ) 1345, 1330, 1340 02FE5
1330 PRINT 907      25MY5
    GO TO 9990       25MY5
C-----ARRANGE THE CARDS IN ASCENDING ORDER OF STA NUMBER 01JE5
1340    INSAV = IN13(JM1) 02FE5
    KSAV = KASE(JM1)   31DE4
    WSAV = WS(JM1)    31DE4
    DWSAV = DWS(JM1)  31DE4
    IN13(JM1) = IN13(N) 31DE4
    KASE(JM1) = KASE(N) 31DE4
    WS(JM1) = WS(N)   31DE4
    DWS(JM1) = DWS(N)  31DE4
    IN13(N) = INSAV   31DE4
    KASE(N) = KSAV   31DE4
    WS(N) = WSAV    31DE4

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          DWS(N) = DWSAV      31DE4
1345    CONTINUE      02FE5
1350    CONTINUE      02FE5
1355    DO 1380 N = 1, NCT3      23JA67
           JS = IN13(N) + 4      31DE4
           IF( KASE(N) - 2 ) 1360,1365,1370      26SE7
1360        KEY(JS)= 2      25MY5
       PRINT 311, IN13(N), KASE(N), WS(N)      25MY5
           GO TO 1380      02FE5
1365        IF( KEY(JS-1) - 1 ) 9980, 1366, 1375      02FE5
1366        IF ( KEY(JS) - 1 ) 9980, 1367, 1375      02FE5
1367 PRINT 312, IN13(N), KASE(N), DWS(N)      26MY5
           KEY(JS-1) = 3      02FE5
           KEY(JS+1) = 5      05JE3
           GO TO 1380      02FE5
1370        IF( KEY(JS-1) - 1 ) 9980, 1371, 1375      02FE5
1371        IF ( KEY(JS) - 1 ) 9980, 1372, 1375      02FE5
1372 PRINT 313, IN13(N), KASE(N), WS(N), DWS(N)      26MY5
           KEY(JS-1) = 3      02FE5
           KEY(JS) = 4      05JE3
           KEY(JS+1) = 5      05JE3
           GO TO 1380      02FE5
1375 PRINT 907      02FE5
           GO TO 9990      26MR5
1380    CONTINUE      02FE5
C-----INPUT TABLE 4      10JE3
1400 PRINT 400      04JE3
           IF ( KEEP4 ) 9980, 1401, 1410      04JA5
1401        NC14 = 1      26MR5-
           NCT4 = NCD4      19OC4
           GO TO 1430      04JA5
1410 PRINT 905      04JA5
           IF( NCT4 - 1 ) 1426,1411,1411      26SE7
1411        DO 1425 N = 1, NCT4      26SE7
           IF( KSW4(N) - 2 ) 1413,1417,1421      26SE7
1413 PRINT 411, IN14(N), IN24(N), KR24(N), FN2(N), QDN2(N), SN2(N),
     1           TN2(N), RN2(N), PN2(N)      04JA5
           GO TO 1425      04JA5
1417 PRINT 412, IN14(N), KR24(N), FN2(N), QDN2(N), SN2(N), TN2(N),
     1           RN2(N), PN2(N)      04JA5
           GO TO 1425      04JA5
1421 PRINT 413, IN24(N), KR24(N), FN2(N), QDN2(N), SN2(N), TN2(N),
     1           RN2(N), PN2(N)      04JA5
1425    CONTINUE      04JA5
1426    CONTINUE      21SE66
       PRINT 910      04JA5
           NC14 = NCT4 + 1      21OC4
           NCT4 = NCT4 + NCD4      19OC4
1430        IF ( NCT4 - 100 ) 1435, 1435, 1433      04JA5
1433 PRINT 904      04JA5
           GO TO 9990      26MR5
1435        IF ( NCD4 ) 9980, 1437, 1440      04JA5
1437 PRINT 903      04JA5
           GO TO 1500      04JA5
1440        KR1 = 0      04JA5
           IF( NCT4 - NC14 ) 1500,1445,1445      26SE7
1445        DO 1470 N = NC14, NCT4      26SE7
       READ 41, IN14(N), IN24(N), KR24(N), FN2(N), QDN2(N), SN2(N),
     1           TN2(N), RN2(N), PN2(N)      19OC4
           KSW4(N) = 1 + KR24(N) + 2 * KR1      19OC4
                                         23OC4

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        KR1 = KR24(N)          05N04
        IF( KSW4(N) - 2 ) 1450,1455,1460 26SE7
1450 PRINT 411, IN14(N), IN24(N), KR24(N), FN2(N), QDN2(N), SN2(N),
      1           TN2(N), RN2(N), PN2(N) 04JA5
      GO TO 1470 04JA5
1455 PRINT 412, IN14(N), KR24(N), FN2(N), QDN2(N), SN2(N), TN2(N),
      1           RN2(N), PN2(N) 04JA5
      GO TO 1470 190C4
1460 PRINT 413, IN24(N), KR24(N), FN2(N), QDN2(N), SN2(N), TN2(N),
      1           RN2(N), PN2(N) 04JA5
1470   CONTINUE 03N04
C----INPUT TABLE 5 04JA5
1500 PRINT 500 01JE5
      IF ( KEEP5 ) 9980, 1501, 1510 190C4
1501     NC15 = 1 04JA5
      NCT5 = NCD5 26MR5
      GO TO 1530 190C4
1510 PRINT 905 04JA5
      IF( NCT5 - 1 ) 1526,1511,1511 26SE7
1511     DO 1525 N = 1, NCT5 26SE7
      IF( KSW5(N) - 2 ) 1513,1517,1521 26SE7
1513 PRINT 411, IN15(N), IN25(N), KR25(N), QMN2(N) 04JA5
      GO TO 1525 04JA5
1517 PRINT 412, IN15(N), KR25(N), QMN2(N) 04JA5
      GO TO 1525 04JA5
1521 PRINT 413, IN25(N), KR25(N), QMN2(N) 04JA5
1525   CONTINUE 04JA5
1526   CONTINUE 21SE66
      PRINT 910 20JA5
      NC15 = NCT5 + 1 06N04
      NCT5 = NCT5 + NCD5 190C4
1530   IF ( NCT5 - 100 ) 1535, 1535, 1533 04JA5
1533 PRINT 904 04JA5
      GO TO 9990 26MR5
1535   IF ( NCD5 ) 9980, 1537, 1540 04JA5
1537 PRINT 903 04JA5
      GO TO 1600 07JE5
1540     KR1 = 0 04JA5
      DO 1570 N = NC15, NCT5 04JA5
      READ 51, IN15(N), IN25(N), KR25(N), QMN2(N) 13MR5
      KSW5(N) = 1 + KR25(N) + 2 * KR1 190C4
      KR1 = KR25(N) 05N04
      IF( KSW5(N) - 2 ) 1550,1555,1560 26SE7
1550 PRINT 411, IN15(N), IN25(N), KR25(N), QMN2(N) 04JA5
      GO TO 1570 04JA5
1555 PRINT 412, IN15(N), KR25(N), QMN2(N) 07JA5
      GO TO 1570 04JA5
1560 PRINT 413, IN25(N), KR25(N), QMN2(N) 04JA5
1570   CONTINUE 04JA5
C----INPUT TABLE 6 01JE5
1600 PRINT 600 07JE5
      IF ( KEEP6 ) 9980, 1605, 1602 29MR5
1602 PRINT 905 29MR5
      GO TO 1619 01JE5
1605   IF ( NCD6 ) 9980, 1660, 1610 29MR5
1610 READ 605, NSD, ILOPD, ( ISD(N), N = 1, NSD ) 25MY5
      READ 605, NSM, ILOPM, ( ISM(N), N = 1, NSM ) 25MY5
      READ 605, NSV, ILOPV, ( ISV(N), N = 1, NSV ) 25MY5
      READ 605, NSR, ILOPR, ( ISR(N), N = 1, NSR ) 26MY5
      IF ( NSD - 5 ) 1612, 1612, 1615 01JE5

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1612      IF ( NSM - 5 ) 1613, 1613, 1615          01JE5
1613      IF ( NSV - 5 ) 1614, 1614, 1615          01JE5
1614      IF ( NSR - 5 ) 1619, 1619, 1615          01JE5
1615 PRINT 690
      GO TO 9990
1619 PRINT 601
      PRINT 610, NAME(1), NSD, ILOPD, ( ISD(N), N = 1, NSD )
      PRINT 610, NAME(2), NSM, ILOPM, ( ISM(N), N = 1, NSM )
      PRINT 610, NAME(3), NSV, ILOPV, ( ISV(N), N = 1, NSV )
      PRINT 610, NAME(4), NSR, ILOPR, ( ISR(N), N = 1, NSR ) 16JA67
1620      DO 1640 N = 1, 5                         16JA67
      DO 1630 J = 1, MP7                         16JA67
      DIL(J,N) = 0.0                           16JA67
      BMIL(J,N) = 0.0                          16JA67
      VIL(J,N) = 0.0                           16JA67
      RIL(J,N) = 0.0                           16JA67
1630      CONTINUE
1640      CONTINUE
      GO TO 1700
1660 PRINT 903
      NSD = 0                                     18MR5
      NSM = 0                                     24MR5
      NSV = 0                                     18MR5
      NSR = 0                                     18MR5
      ILOPD = 0                                    11JE5
      ILOPM = 0                                    11JE5
      ILOPV = 0                                    11JE5
      ILOPR = 0                                    11JE5
C-----INTERPOLATE AND DISTRIBUTE VALUES FROM TABLE 4
1700      LSM = 0                                     01JE5
1705 CALL INTERP3 ( MP7, NCT4, IN14, IN24, KR24, FN2, F, LSM, KSW4 ) 16JE5
      CALL INTERP3 ( MP7, NCT4, IN14, IN24, KR24, QDN2, QD, LSM, KSW4 ) 09MR6
      CALL INTERP3 ( MP7, NCT4, IN14, IN24, KR24, SN2, S, LSM, KSW4 ) 09MR6
      CALL INTERP3 ( MP7, NCT4, IN14, IN24, KR24, TN2, T, LSM, KSW4 ) 09MR6
      CALL INTERP3 ( MP7, NCT4, IN14, IN24, KR24, RN2, R, LSM, KSW4 ) 09MR6
      LSM = 1                                     12AP5
      CALL INTERP3 ( MP7, NCT4, IN14, IN24, KR24, PN2, P, LSM, KSW4 ) 09MR6
C-----SET TOTAL LOAD EQUAL TO FIXED LOAD
1800      DO 1810 J = 1, MP7
      Q(J) = QD(J)
1810      CONTINUE
      IF ( KEEPE ) 9980, 1820, 1828          11JA67
C-----THE VARIABLE VPLOT IS A 207 X 8 ARRAY WHERE EACH COLUMN IS
C      RESPECTIVELY EQUAL TO THE 207 X 1 ARRAYS WMAX,WMIN,BMAX,BMMIN,
C      VMAX,VMIN,RMAX,RMIN THEREFORE VPLOT( N*207 + 1 ) = WMAX( 1 ) 23JA67
C      FOR N=0, WMIN( 1 ) FOR N=1, ETC ..... , RMIN( 1 ) FOR N=7 23JA67
1820      DO 1825 J = 1, MP7
      VPLOT(J) = 0.0                           09FE5
      VPLOT(J+ 207) = 0.0                      20JA67
      VPLOT(J+ 414) = 0.0                      20JA67
      VPLOT(J+ 621) = 0.0                      20JA67
      VPLOT(J+ 828) = 0.0                      20JA67
      VPLOT(J+1035) = 0.0                      20JA67
      VPLOT(J+1242) = 0.0                      20JA67
      VPLOT(J+1449) = 0.0                      20JA67
      IMX(J) = 999                            26MR5
      IMN(J) = 999                            26MR5
      IWX(J) = 999                            26MR5
      IWN(J) = 999                            26MR5
      IVX(J) = 999                            26MR5

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        IVN(J) = 999          26MRS
        IRX(J) = 999          26MRS
        IRN(J) = 999          26MRS
1825      CONTINUE          09FE5
        GO TO 1830          17JA67
C-----THESE INDICATORS ADDED FOR ASTERICKS, STATEMENTS 2220 THRU 2350 24JA67
1828      DO 1829 J = 1, MP7 11JA67
            IMX(J) = IMX(J) + 10000 11JA67
            IMN(J) = IMN(J) + 10000 11JA67
            IWX(J) = IWX(J) + 10000 11JA67
            IWN(J) = IWN(J) + 10000 11JA67
            IVX(J) = IVX(J) + 10000 11JA67
            IVN(J) = IVN(J) + 10000 11JA67
            IRX(J) = IRX(J) + 10000 11JA67
            IRN(J) = IRN(J) + 10000 11JA67
1829      CONTINUE          11JA67
C-----FIXED-LOAD SOLUTION          24JA67
1830      CALL BEAMCOL          09FE5
            JL = 999 + 4          26MRS
            CALL COMPARE          13JA67
C-----PRINT FIXED LOAD RESULTS          24JA67
    PRINT 11          08MY3 ID
    PRINT 1          18FE5 ID
    PRINT 13, ( AN1(N), N = 1, 32 ) 18FE5 ID
    PRINT 16, NPROB, ( AN2(N), N = 1, 14 ) 28AG3 ID
    PRINT 700          27MRS
    PRINT 701          27MRS
            ISTA = -1          09AP5
            Z1 = ISTA          09AP5
            X = Z1 * H          09AP5
    PRINT 711, ISTA, X, W(3), BM(3), REACT(3) 09AP5
            DO 1850 J = 4, MP5 09AP5
            ISTA = J - 4          23OC4
            Z1 = ISTA          26OC4
            X = Z1 * H          25OC4
C-----STORE FIXED LOAD VALUES AT INFLUENCE LINE STATIONS          24JA67
    IF( NSD )9980,1834,1831          24JA67
1831      DO 1833 N = 1,NSD          20DE66
            IF ( ISTA - ISD(N) ) 1833,1832,1833          20DE66
1832      WF(N) = W(J)          20DE66
        GO TO 1834          20DE66
1833      CONTINUE          20DE66
1834      IF( NSM )9980,1838,1835          13JA67
1835      DO 1837 N = 1,NSM          20DE66
            IF ( ISTA - ISM(N) ) 1837,1836,1837          20DE66
1836      BMF(N) = BM(J)          20DE66
        GO TO 1838          20DE66
1837      CONTINUE          20DE66
1838      IF( NSV )9980,1842,1839          13JA67
1839      DO 1841 N = 1,NSV          20DE66
            IF ( ISTA - ISV(N) ) 1841,1840,1841          20DE66
1840      DBMF(N) = DBM(J)          20DE66
        GO TO 1842          20DE66
1841      CONTINUE          20DE66
1842      IF( NSR )9980,1847,1843          13JA67
1843      DO 1846 N = 1,NSR          20DE66
            IF ( ISTA - ISR(N) ) 1846,1844,1846          20DE66
1844      REACTF(N) = REACT(J)          20DE66
        GO TO 1847          20DE66
1845      CONTINUE          20DE66

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1847 PRINT 712, DW(J), DBM(J) 09AP5
      PRINT 711, ISTA, X, W(J), BM(J), REACT(J) 09AP5
1850   CONTINUE 230C4
C----INTERPOLATE AND DISTRIBUTE VALUES FROM TABLE 5 02JE5
1900     LSM = 0 16JE5
      CALL INTERP3 ( MP7, NCT5, IN15, IN25, KR25, QMN2, QM, LSM, KSW5 ) 09MR6
C----MOVABLE-LOAD SOLUTIONS 01JE5
1990   IF ( JSTART - 4 ) 1990, 1990, 1995 03JE5
1990     ILS = 1 - JSTART 25MY5
      GO TO 2000 25MY5
1995     ILS = -3 25MY5
2000   IF ( MLINC ) 2004, 2004, 2005 09FE5
2004 PRINT 940 13JA5
      GO TO 2210 13JA5
2005   DO 2200 JL = JSTART, JSTOP, MLINC 13JA5
C----CLEAR MOVABLE-LOAD VALUES FROM TEMPORARY LIVE LOAD STORAGE 01JE5
      IF ( JL - MLINC ) 2006, 2006, 2009 12ND4
2006   DO 2007 JLINC = 1, MLINC 12ND4
          QLIVE(JLINC) = 0.0 12ND4
2007   CONTINUE 12ND4
      GO TO 2011 12ND4
2009   DO 2010 JLINC = 1, MLINC 12ND4
          QLIVE(JL - JLINC) = 0.0 200C4
2010   CONTINUE 200C4
C----RELOCATE MOVABLE-LOAD PATTERN 01JE5
2011   DO 2020 LL = 4, MLP4 12ND4
          J = JL + LL - 4 200C4
          IF(J - 4) 2020, 2015, 2015 12ND4
2015   IF ( J - MP4 ) 2017, 2017, 2021 12NQ4
2017     QLIVE(J) = QM(LL) 12ND4
2020   CONTINUE 200C4
C----SET TOTAL LOAD EQUAL TO FIXED LOAD PLUS THE MOVABLE-LOAD PATTERN 10FE6
2021   DO 2022 J = 1, MP7 02JE5
          Q(J) = QD(J) + QLIVE(J) 200C4
2022   CONTINUE 200C4
      CALL BEAMCOL 210C4
      CALL COMPARE 13JA67
C----STORE VALUES OF INFLUENCE LINES AND SUBTRACT FIXED LOADS 02JE5
      IL = JL + ILS 25MY5
      IF( NSD - 1 ) 2041,2035,2035 26SE7
2035   DO 2040 N = 1, NSD 28SE7
          ISTA = ISD(N) + 4 06AP5
          DIL(IL,N) = W(ISTA) - WF(N) 13JA67
2040   CONTINUE 18MR5
2041   IF( NSM - 1 ) 2051,2045,2045 28SE7
2045   DO 2050 N = 1, NSM 26SE7
          ISTA = ISM(N) + 4 06AP5
          BMIL(IL,N) = BM(ISTA) - BMF(N) 13JA67
2050   CONTINUE 18MR5
2051   IF( NSV - 1 ) 2061,2055,2055 25SE7
2055   DO 2060 N = 1, NSV 26SE7
          ISTA = ISV(N) + 4 06AP5
          VIL(IL,N) = DBM(ISTA) - DBMF(N) 13JA67
2060   CONTINUE 18MR5
2061   IF( NSR - 1 ) 2200,2065,2065 26SE7
2065   DO 2070 N = 1, NSR 26SE7
          ISTA = ISR(N) + 4 06AP5
          RIL(IL,N) = REACT(ISTA) - REACTF(N) 13JA67
2070   CONTINUE 18MR5
2200   CONTINUE 200C4

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      GO TO 2220          09FE5
2210      IF ( KEEPE ) 9980, 4010, 2220 27MR5
C----PRINT ENVELOPES OF MAXIMUMS 10FE6
2220 PRINT 11          08MY3 ID
      PRINT 15, NPROB, ( AN2(N), N = 1, 14 ) 28AG3 ID
      PRINT 800          27MR5
      PRINT 801          27MR5
      DO 2299 J = 3, MP5 13MR5
          T1 = 1H          04JA67
          T2 = 1H          04JA67
          T3 = 1H          04JA67
          T4 = 1H          04JA67
          ISTA = J - 4    23OC4
      IF ( IWX(J)- 8000 ) 2230,2230,2225 11JA67
2225      IWX(J) = IWX(J) - 10000 11JA67
          T1 = 1H*          04JA67
2230      IF(IWN(J)-8000 ) 2240,2240,2235 04JA67
2235      IWN(J) = IWN(J) - 10000 11JA67
          T2 = 1H*          04JA67
2240      IF(IMX(J)-8000 ) 2250,2250,2245 04JA67
2245      IMX(J) = IMX(J) - 10000 11JA67
          T3 = 1H*          04JA67
2250      IF(IMN(J)- 8000 ) 2260,2260,2255 04JA67
2255      IMN(J) = IMN(J) - 10000 11JA67
          T4 = 1H*          04JA67
2260 PRINT 810,ISTA,VPL0T(J),IWX(J),T1,VPL0T(J+207),IWN(J),T2, 20JA67
1          VPL0T(J+414),IMX(J),T3,VPL0T(J+621),IMN(J),T4 20JA67
2299      CONTINUE          13MR5
      PRINT 820          27MR5
      PRINT 830          27MR5
          ISTA = -1          09AP5
          IRX(3) = 999        17JA67
          IRN(3) = 999        17JA67
      IF(KEEPE ) 9980,2300,2301          13JA67
2300      T5 = 1H          13JA67
      GO TO 2302          13JA67
2301      T5 = 1H*          17JA67
2302 PRINT 835,ISTA,VPL0T(1245),IRX(3),T5,VPL0T(1452),IRN(3),T5 20JA67
      DO 2399 J = 4, MP5 09AP5
          T1 = 1H          04JA67
          T2 = 1H          04JA67
          ISTA = J - 4    23OC4
      IF(IVX(J)-8000 ) 2305,2305,2303 13JA67
2303      IVX(J) = IVX(J) - 10000 11JA67
          T1 = 1H*          04JA67
2305      IF(IVN(J)-8000 ) 2315,2315,2310 04JA67
2310      IVN(J) = IVN(J) - 10000 11JA67
          T2 = 1H*          04JA67
2315 PRINT 840, VPL0T(J+828),IVX(J),T1,VPL0T(J+1035),IVN(J),T2 20JA67
          T1 = 1H          04JA67
          T2 = 1H          04JA67
      IF( IRX(J)-8000 ) 2340,2340,2335 04JA67
2335      IRX(J) = IRX(J) - 10000 11JA67
          T1 = 1H*          04JA67
2340      IF(IRN(J)-8000 ) 2350,2350,2345 04JA67
2345      IRN(J) = IRN(J) - 10000 11JA66
          T2 = 1H*          04JA67
2350 PRINT 835, ISTA,VPL0T(J+1242),IRX(J),T1,VPL0T(J+1449),IRN(J),T2 20JA67
2399      CONTINUE          13MR5
C----PLOT ENVELOPES OF MAXIMUMS 01JE5

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      PRINT 900
      IF( KPLOT(1) + KPLOT(2) + KPLOT(3) + KPLOT(4) ) 9980,2405,2410 29MY5
2405 PRINT 911
      GO TO 2500
C----SET PLOT CONSTANTS
2410      KSTOP = 1          29MY5
          HPOS = 10.0         12MR5
          HNEG = 0.0          12MR5
          SPACE = 0.5         12MR5
          VPOS = 2.0          12MR5
          VNEG = 2.0          12MR5
          KAXES = 0            12MR5
          HEND = M             15MR5
          NSYMB = 20           23JA67
          IOR = 0              12MR5
          ISIZE = 2             12MR5
          HCOR = 2.0           12MR5
          VCOR = -2.25          12MR5
          KPS = 9               12MR5
          IPLOT(1) = NPROB     27MR5
C----FOLLOWING CARD IS ADDED FOR MICROFILM PLOT ON CDC 6600. THE USER
C      MAY PULL OUT THIS CARD FOR PLOT ON PAPER.
          CALL FILMSET          08MR8
C----DETERMINE HORIZONTAL SCALE
          CALL SCALE ( HEND, HPOS ) 10FE6
          HTCKS = HEND          12MR5
          PRINT 123, HEND        12MR5
          PRINT 901               27MY5
          DO 2499  K = 1, 8, 2   25MY5
              KC = ( K + 1 ) / 2
              IF ( KPLOT(KC) ) 9980, 2499, 2420
2420      VEND = 0.0          12MR5
C----DETERMINE THE MAXIMUM VALUE OF THE PLOT VARIABLE
          DO 2430  J = 4, MP4    02JE5
              ZTEMP = -VPLOT(J,K+1)
              VEND = MAX1F( VEND, VPLOT(J,K), ZTEMP )
2430      CONTINUE          12MR5
2432      CALL SCALE ( VEND, VPOS ) 23MR5
          PRINT 902, NAME(KC), VEND 25MY5
2433      GO TO ( 2435, 2440, 2445, 2450 ), KC 23MR5
2435      IPLOT(2) =10H      DEFL
          N1 = 4                23JA67
          NUMPTS = M + 1        12AP5
          DO 2437  J = 1, MP4    12AP5
              HPLOT(J) = J - 1    12AP5
2437      CONTINUE          12AP5
          GO TO 2455          12MR5
2440      IPLOT(2) =10H      MOM
          N1 = 4                23JA67
          NUMPTS = M + 1        12AP5
          DO 2442  J = 1, MP4    12AP5
              HPLOT(J) = J - 1    12AP5
2442      CONTINUE          12AP5
          GO TO 2455          12MR5
2445      IPLOT(2) =10H      SHEAR
          N1 = 5                23JA67
          NUMPTS = M             12AP5
          DO 2447  J = 1, MP4    12AP5
              F = J                12AP5
              HPLOT(J) = F - 0.5  12AP5

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2447    CONTINUE          12AP5
        GO TO 2455        12MR5
2450      IPLOT(2) =10H    REACT      23JA67
        N1 = 4             12AP5
        NUMPTS = M + 1     12AP5
        DO 2452 J = 1, MP4   12AP5
        HPLOT(J) = J - 1   12AP5
2452    CONTINUE          12AP5
2455      VTCKS = VEND   27MR5
        CALL AXES ( HEND, HPOS, HNEG, SPACE, VEND, VPOS, VNEG, HTCKS,
1           VTCKS, KAXES ) 12MR5
        CALL PLDTITL ( IPLOT, NSYMB, IOR, ISIZE, HCOR, VCOR ) 12MR5
        CALL PLOT ( HPLOT, VPLOT(N1,K), NUMPTS, KPS ) 05JA67
        CALL PLOT ( HPLOT, VPLOT(N1,K+1), NUMPTS, KPS ) 12AP5
        CALL AXTERM ( 1 )   12AP5
        CALL AXTERM ( 1 )   05JA67
2499    CONTINUE          05JA67
C----TABULATE THE INFLUENCE DIAGRAMS
2500 PRINT 11          10FE6
        PRINT 15, NPROB, ( AN2(N), N = 1, 14 ) 08MY3 ID
        PRINT 110           28AG3 ID
        PRINT 115           29MY5
        IF( NSD ) 19980,2555,2503 27MY5
2503 PRINT 117, ( ISD(N), N = 1, NSD ) 23JA67
2510  IF ( ILOPD - 2 ) 2515, 2520, 2520 25MY5
2515 PRINT 909          25MY5
        GO TO 2600          25MY5
2520  DO 2550 JL = JSTART, JSTOP 25MY5
        IL = JL + ILS       25MY5
        ISTA = JL - 4       25MY5
        PRINT 118, ISTA, ( DIL(IL,N), N = 1, NSD ) 25MY5
2550    CONTINUE          25MY5
        GO TO 2600          25MY5
2555 PRINT 903          23JA67
2600 PRINT 111          25MY5
        PRINT 115           27MY5
        IF( NSM ) 19980,2655,2603 23JA67
2603 PRINT 117, ( ISM(N), N = 1, NSM ) 23JA67
2610  IF ( ILOPM - 2 ) 2615, 2620, 2620 25MY5
2615 PRINT 909          25MY5
        GO TO 2700          25MY5
2620  DO 2650 JL = JSTART, JSTOP 25MY5
        IL = JL + ILS       25MY5
        ISTA = JL - 4       25MY5
        PRINT 118, ISTA, ( BMIL(IL,N), N = 1, NSM ) 25MY5
2650    CONTINUE          25MY5
        GO TO 2700          25MY5
2655 PRINT 903          23JA67
2700 PRINT 112          25MY5
        PRINT 115           27MY5
        IF( NSV ) 19980,2755,2703 23JA67
2703 PRINT 117, ( ISV(N), N = 1, NSV ) 23JA67
2710  IF ( ILOPV - 2 ) 2715, 2720, 2720 26MY5
2715 PRINT 909          25MY5
        GO TO 2800          25MY5
2720  DO 2750 JL = JSTART, JSTOP 25MY5
        IL = JL + ILS       25MY5
        ISTA = JL - 4       25MY5
        PRINT 118, ISTA, ( VIL(IL,N), N = 1, NSV ) 25MY5
2750    CONTINUE          25MY5

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        GO TO 2800          25MY5
2755 PRINT 903          23JA67
2800 PRINT 113          25MY5
    PRINT 115          27MY5
        IF( NSR ) 9980,2855,2803          23JA67
2803 PRINT 117, ( ISR(N), N = 1, NSR )          23JA67
2810     IF ( ILOPR - 2 ) 2815, 2820, 2820          25MY5
2815 PRINT 909          25MY5
        GO TO 3000          25MY5
2820     DO 2850 JL = JSTART, JSTOP          25MY5
        IL = JL + ILS          25MY5
        ISTA = JL - 4          25MY5
    PRINT 118, ISTA, ( RIL(IL,N), N = 1, NSR )          25MY5
2850     CONTINUE          25MY5
        GO TO 3000          25MY5
2855 PRINT 903          23JA67
C----PLOT INFLUENCE LINES          01JE5
3000     IF ( NSM + NSD + NSV + NSR ) 9980, 4010, 3002          19AG5
3002     IF ( ILOPD - 2 ) 3010, 3004, 3010          19AG5
3004     IF ( ILOPM - 2 ) 3010, 3006, 3010          19AG5
3006     IF ( ILOPV - 2 ) 3010, 3008, 3010          19AG5
3008     IF ( ILOPR - 2 ) 3010, 3009, 3010          19AG5
3009 PRINT 120          19AG5
    PRINT 911          19AG5
        GO TO 4010          19AG5
3010     IF ( KSTOP ) 9980, 3015, 3030          22MR5
C----SET PLOT CONSTANTS          10FE6
3015     KSTOP = 1          22MR5
        HPOS = 10.0          22MR5
        HNEG = 0.0          22MR5
        SPACE = 0.5          22MR5
        VPOS = 2.0          22MR5
        VNEG = 2.0          22MR5
        IOR = 0          22MR5
        ISIZE = 2          22MR5
        VCOR = -2.25          22MR5
        KPS = 9          22MR5
3030     IPLOT(1) = NPROB          25MY5
        KAXES = 16          25MY5
        N1 = 1          25MY5
        ITIS = -8          07JE5
        NPTS = 2          07JE5
        NSYMB = 30          23JA67
        IF ( JSTART - 4 ) 3050, 3050, 3075          25MY5
3050     HEND = MP4 - JSTART          25MY5
        HOR(1) = 4 - JSTART          07JE5
        GO TO 3100          25MY5
3075     HEND = M          25MY5
        HOR(1) = 0.0          07JE5
3100     NUMPTS = MP4 + ILS          31MY5
        HOR(2) = HOR(1)          07JE5
C----SET HORIZONTAL SCALE          10FE6
    CALL SCALE ( HEND, HPOS )          22MR5
        HTCKS = HEND          28MY5
        HCOR = 10. * HOR / HEND + 2.0          03JE5
PRINT 120          25MY5
PRINT 123, HEND          25MY5
PRINT 124          25MY5
    DO 3125 J = 1, NUMPTS          25MY5
        HPLOT(J) = J - 1          25MY5

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3125    CONTINUE          25MY5
        IF ( ILOPD - 2 ) 3205, 3300, 3205      26MY5
3205    IPLOT(2) =10H     DEFL             23JA67
        IF ( NSD ) 9980, 3300, 3206      24JA67
3206    DO 3299 N = 1, NSD           21JA67
            VEND = 0.0
        DO 3240 J = N1, NUMPTS      25MY5
            ZTEMP = ABSF ( DIL(J,N) )
        IF ( ZTEMP - VEND ) 3240, 3240, 3235      25MY5
3235    VEND = ZTEMP          25MY5
3240    CONTINUE          25MY5
C----SET VERTICAL SCALE          10FE6
    CALL SCALE ( VEND, VPOS )      22MR5
        VTCKS = VEND             22MR5
    CALL AXES ( HEND, HPOS, HNEG, SPACE, VEND, VPOS, VNEG, HTCKS,      22MR5
1       VTCKS, KAXES )          22MR5
    CALL PLOT ( HPLOT, DIL(N1,N), NUMPTS, KPS )      12AP5
    ENCODE (10, 60, IPLOT(3) ) ISD(N)           23JA67
    CALL PLOTITL ( IPLOT, NSYMB, IOR, ISIZE, HCOR, VCOR )
        VOR(1) = VEND          05JA67
        VOR(2) = -VEND         07JE5
    CALL PLOT ( HOR, VOR, NPTS, IT1S )      25MY5
    CALL AXTERM   ( 1 )          05JA67
    CALL AXTERM   ( 1 )          05JA67
    PRINT 125, ISD(N), NAME(1), VEND      27MY5
3299    CONTINUE          26MY5
3300    IF ( ILOPM - 2 ) 3305, 3400, 3305      26MY5
3305    IPLOT(2) =10H     MOM             23JA67
        IF( NSM ) 9980, 3400, 3306      24JA67
3306    DO 3399 N = 1, NSM           21JA67
            VEND = 0.0
        DO 3340 J = N1, NUMPTS      22MR5
            ZTEMP = ABSF ( BMIL(J,N) )
        IF ( ZTEMP - VEND ) 3340, 3340, 3335      25MY5
3335    VEND = ZTEMP          25MY5
3340    CONTINUE          25MY5
    CALL SCALE ( VEND, VPOS )      22MR5
        VTCKS = VEND             22MR5
    CALL AXES ( HEND, HPOS, HNEG, SPACE, VEND, VPOS, VNEG, HTCKS,      22MR5
1       VTCKS, KAXES )          22MR5
    CALL PLDT ( HPLOT, BMIL(N1,N), NUMPTS, KPS )      12AP5
    ENCODE (10, 60, IPLOT(3) ) ISM(N)           23JA67
    CALL PLOTITL ( IPLOT, NSYMB, IOR, ISIZE, HCOR, VCOR )
        VOR(1) = VEND          05JA67
        VOR(2) = -VEND         07JE5
    CALL PLOT ( HOR, VOR, NPTS, IT1S )      25MY5
    CALL AXTERM   ( 1 )          05JA67
    CALL AXTERM   ( 1 )          05JA67
    PRINT 125, ISM(N), NAME(2), VEND      27MY5
3399    CONTINUE          26MY5
3400    IF ( ILOPV - 2 ) 3405, 3500, 3405      27MY5
3405    IPLOT(2) =10H     SHEAR          23JA67
        IF( NSV ) 9980, 3500, 3406      24JA67
3406    DO 3499 N = 1, NSV           21JA67
            VEND = 0.0
        DO 3440 J = N1, NUMPTS      22MR5
            ZTEMP = ABSF ( VIL(J,N) )
        IF ( ZTEMP - VEND ) 3440, 3440, 3435      25MY5
3435    VEND = ZTEMP          25MY5
3440    CONTINUE          25MY5

```

```

CALL SCALE ( VEND, VPOS )          22MR5
    VTCKS = VEND
CALL AXES ( HEND, HPOS, HNEG, SPACE, VEND, VPOS, VNEG, HTCKS,
1      VTCKS, KAXES )             22MR5
CALL PLDT ( HPLOT, VIL(N1,V), NUMPTS, KPS )           12AP5
ENCODE (10, 60, IPLOT(3) ) ISV(N)                   23JA67
CALL PLDTITL ( IPLOT, NSYMB, IOR, ISIZE, HCOR, VCOR )
    VOR(1) = VEND
    VOR(2) = -VEND
CALL PLDT ( HOR, VOR, NPTS, IT1S )                 25MY5
CALL AXTERM   ( 1 )                         05JA67
CALL AXTERM   ( 1 )                         05JA67
PRINT 125, ISV(N), NAME(3), VEND            24MR5
3499  CONTINUE                               26MY5
3500  IF ( ILOPR - 2 ) 3505, 4010, 3505       27MY5
3505  IPLOT(2) =10H   REACT                  23JA67
    IF( NSR ) 9980, 4010, 3506               24JA67
3506  DO 3599 N = 1, NSR                    21JA67
    VEND = 0.0
    DO 3540 J = N1, NUMPTS                22MR5
        ZTEMP = ABSF ( RIL(J,N) )           25MY5
        IF ( ZTEMP - VEND ) 3540, 3540, 3535 25MY5
3535  VEND = ZTEMP                         25MY5
3540  CONTINUE                               25MY5
CALL SCALE ( VEND, VPOS )          22MR5
    VTCKS = VEND
CALL AXES ( HEND, HPOS, HNEG, SPACE, VEND, VPOS, VNEG, HTCKS,
1      VTCKS, KAXES )             22MR5
CALL PLDT ( HPLOT, RIL(N1,V), NUMPTS, KPS )           12AP5
ENCODE (10, 60, IPLOT(3) ) ISR(N)                  23JA67
CALL PLDTITL ( IPLOT, NSYMB, IOR, ISIZE, HCOR, VCOR )
    VOR(1) = VEND
    VOR(2) = -VEND
CALL PLDT ( HOR, VOR, NPTS, IT1S )                 25MY5
CALL AXTERM   ( 1 )                         05JA67
CALL AXTERM   ( 1 )                         05JA67
PRINT 125, ISR(N), NAME(4), VEND            24MR5
3599  CONTINUE                               26MY5
4010 CALL TIC TOC (4)                      08MR8
C----RETURN TO TAKE NEXT PROBLEM
    GO TO 1010
1010 PRINT 980                                10JE3
C----COMPLETION OF RUN
9990  CONTINUE                               26AG3 ID
    IF ( KSTOP ) 9995, 9995, 9992          31DE4
9992 CALL AXTERM   ( 0 )                      10JE3
9995  CONTINUE                               12MR5 ID
9999  CONTINUE                               22MR5
    PRINT 11
    PRINT 1
    PRINT 13, ( AN1(N), N = 1, 32 )
    PRINT 19
    END                                     04MY3 ID
                                            08MY3 ID
                                            18FE5 ID
                                            18FE5 ID
                                            26AG3 ID
                                            04MY3 ID

```

```

SUBROUTINE INTERP3 ( MP7, NCT, JN1, JN2, KR2, ZN, Z, LSM, KSW ) 10FE6
DIMENSION JN1(100), JN2(100), KR2(100), Z(207), KSW(100) 10FE6
905 FORMAT ( //40H      ERROR STOP -- STATIONS NOT IN ORDER ) 14MY5
908 FORMAT ( //43H      UNDESIGNATED ERROR STOP IN SUBROUTINE ) 14MY5
909 FORMAT ( //46H      ERROR -- NON-ZERO DATA BEYOND END OF BEAM ) 14MY5
      DO 1603 J = 1, MP7
            Z(J) = 0.0
1603    CONTINUE
            ASM = LSM
            ASM = ASM / 2.
            M = MP7 - 7
            KR1 = 0
            IF( NCT - 1 ) 1674,1604,1604
1604    DO 1675 NC = 1, NCT
            IF ( KR1 ) 1698, 1605, 1607
1605    NC1 = NC
            JV = JN1(NC1) + LSM
            IF ( KR2(NC) ) 1698, 1607, 1670
1607    IF ( JN2(NC) - M ) 1609, 1609, 1611
1609    JS2 = JN2(NC)
            ZNS = ZN(NC)
            GO TO 1619
1611    JS2 = M
            ZNS = ZN(NC1) + ( ZN(NC) - ZN(NC1) ) * ( JS2 - JV + LSM ) 10MR6
1       / ( JN2(NC) - JV + LSM )
            KASS = KSW(NC)
            GO TO ( 1613, 1615, 1617, 1617 ), KASS 21SE66
1613    IF ( JN1(NC) - M ) 1619, 1619, 1670 10FE6
1615    IF ( JN1(NC1) - M ) 1619, 1619, 1670 10FE6
1617    IF ( KSW(NC1) - 2 ) 1618, 1615, 1618 09MR6
1618    IF ( JN2(NC1) - M ) 1619, 1619, 1670 09MR6
1619    J1 = JV + 4
            J2 = JS2 + 4
            DENOM = J2 - J1 + LSM
            JINCR = 1
            ESM = 1.0
            ISW = 1
            IF ( DENOM ) 1695, 1620, 1630 12JA5
1620    DENOM = 1.0
            ISW = 0
            IF ( J2 - J1 ) 1651, 1630, 1630 08MR8
1630    DO 1650 J = J1, J2, JINCR
            F = J
            F1 = J1
            DIFF = F - F1 + ASM
            PART = DIFF / DENOM
            Z(J) = Z(J) + ( ZN(NC1) + PART * ( ZNS - ZN(NC1) ) ) * ESM 10FE6
1650    CONTINUE
1651    IF ( LSM ) 1698, 1652, 1660 08MR8
1652    IF ( ISW ) 1698, 1660, 1655 09AP5
1655    JINCR = J2 - J1
            ESM = -0.5
            ISW = 0
            GO TO 1630
1660    IF ( KR2(NC) ) 1698, 1670, 1665 12JA5
1665    JV = JN2(NC) + LSM 12AP5
1670    KR1 = KR2(NC) 12JA5
            NC1 = NC 12JA5

```

1675	CONTINUE	12JA5
1674	CONTINUE	21SE66
	MP5 = MP7 - 2	20MY5
	MP6 = MP7 - 1	20MY5
C-----	TEST FOR DATA ERRONEOUSLY STORED BEYOND ENDS OF REAL BEAM	08MR8
	ZCK = Z(1) + Z(2) + Z(3) + Z(MP5) + Z(MP6) + Z(MP7)	14MY5
	IF (ZCK) 1699, 1676, 1699	14MY5
1675	RETURN	14MY5
1695	PRINT 905	14MY5
	GO TO 1799	05JA67
1698	PRINT 908	14MY5
	GO TO 1799	05JA67
1699	PRINT 909	14MY5
1799	CONTINUE	05JA67
	END	12JA5

SUBROUTINE COMPARE

13JA67

```

DIMENSION F(207), Q(207), S(207), T(207), R(207), P(207), WS(20), 13MR5
1      DWS(20), KEY(207), W(207), DW(207), BM(207), DBM(207), 13MR5
2      REACT(207), BMMAX(207), BMMIN(207), WMAX(207), 13MR5
3      WMIN(207), VMAX(207), VMIN(207), RMAX(207), RMIN(207), 13MR5
4      IMX(207), IMN(207), IWX(207), IWN(207), IVX(207), 13MR5
5      IVN(207), IRX(207), IRN(207) 28MY5
COMMON F, Q, S, T, R, P, KEY, WS, DWS, W, DW, BM, DBM, REACT, 26OC4
2      IMX, IMN, IWX, IWN, IVX, IVN, IRX, IRN, JL, H, HE2, HE3, 27MR5
3      HT2, M, MP4, MP5 27MR5
COMMON/A/WMAX,WMIN,BMMAX,BMMIN,VMAX,VMIN,RMAX,RMIN 28JA7
DO 2100 J = 3, MP5 26OC4
   IF ( BM(J) - BMMAX(J) ) 2030, 2030, 2025 200C4
2025      BMMAX(J) = BM(J) 200C4
      IMX(J) = JL - 4 26MR5
      GO TO 2040 200C4
2030      IF ( BM(J) - BMMIN(J) ) 2035, 2040, 2040 200C4
2035      BMMIN(J) = BM(J) 200C4
      IMN(J) = JL - 4 26MR5
2040      IF ( W(J) - WMAX(J) ) 2050, 2050, 2045 200C4
2045      WMAX(J) = W(J) 200C4
      IWX(J) = JL - 4 26MR5
      GO TO 2060 200C4
2050      IF ( W(J) - WMIN(J) ) 2055, 2060, 2060 200C4
2055      WMIN(J) = W(J) 200C4
      IWN(J) = JL - 4 26MR5
2060      IF ( DBM(J) - VMAX(J) ) 2070, 2070, 2065 200C4
2065      VMAX(J) = DBM(J) 200C4
      IVX(J) = JL - 4 26MR5
      GO TO 2080 200C4
2070      IF ( DBM(J) - VMIN(J) ) 2075, 2080, 2080 200C4
2075      VMIN(J) = DBM(J) 200C4
      IVN(J) = JL - 4 26MR5
2080      IF ( REACT(J) - RMAX(J) ) 2090, 2090, 2085 200C4
2085      RMAX(J) = REACT(J) 200C4
      IRX(J) = JL - 4 26MR5
      GO TO 2100 200C4
2090      IF ( REACT(J) - RMIN(J) ) 2095, 2100, 2100 200C4
2095      RMIN(J) = REACT(J) 200C4
      IRN(J) = JL - 4 26MR5
2100      CONTINUE 200C4
      RETURN
      END 30ND4

```

SUBROUTINE SCALE (SEND, ST)	14MY5
	11MRS
5705 KEXP = 0	12AP5
IF (SEND) 5710, 5705, 5710	12AP5
SEND = 1.0	12AP5
RETURN	14MY5
5710 S1 = ST * .1	11MRS
S2 = ST * .2	11MRS
S5 = ST * .5	11MRS
STEMP = SEND	29AP5
DO 5770 N = 1, 100	11MRS
IF (ST - SEND) 5715, 5750, 5720	11MRS
5715 SEND = SEND / 10.	11MRS
KEXP = KEXP + 1	11MRS
GO TO 5770	11MRS
5720 IF (S1 - SEND) 5730, 5725, 5725	11MRS
5725 SEND = SEND * 10.	11MRS
KEXP = KEXP - 1	11MRS
GO TO 5770	11MRS
5730 IF (S2 - SEND) 5735, 5740, 5740	11MRS
5735 IF (S5 - SEND) 5750, 5745, 5745	11MRS
5740 SEND = S2	11MRS
GO TO 5755	11MRS
5745 SEND = S5	11MRS
GO TO 5755	11MRS
5750 SEND = ST	11MRS
5755 SEND = SEND * 10. ** KEXP	11MRS
RETURN	11MRS
5770 CONTINUE	11MRS
IF (KEXP) 5773, 5773, 5775	12AP5
5773 SEND = 1.0	12AP5
RETURN	27AP5
5775 SEND = STEMP	28AP5
RETURN	12AP5
END	11MRS

SUBROUTINE BEAMCOL	21OC4
DIMENSION F(207), Q(207), S(207), T(207), R(207), P(207), WS(20),	13MR5
1 DWS(20), KEY(207), W(207), DW(207), BM(207), DBM(207),	15MR5
2 REACT(207), BMMAX(207), BMMIN(207), WMAX(207),	13MR5
3 WMIN(207), VMAX(207), VMIN(207), RMAX(207), RMIN(207),	13MR5
4 IMX(207), IMN(207), IWX(207), IWN(207), IVX(207),	13MR5
5 IVN(207), IRX(207), IRN(207), A(207), B(207), C(207),	28MY5
6 VPLOT(207,8)	28MY5
COMMON F, Q, S, T, R, P, KEY, WS, DWS, W, DW, BM, DBM, REACT,	26OC4
2 IMX, IMN, IWX, IWN, IVX, IVN, IRX, IRN, JL, H, HE2, HE3,	27MR5
3 HT2, M, MP4, MP5	27MR5
C----START OF BEAM-COLUMN SOLUTION	10JE3
6000 NS = 1	04JE3
A(1) = 0	01SE66
A(2) = 0	01SE66
B(1) = 0	01SE66
B(2) = 0	01SE66
C(1) = 0	01SE66
C(2) = 0	01SE66
DO 5060 J = 3, MP5	04JE3
C----COMPUTE MATRIX COEFFS AT EACH STA J	10JE3
AA = F(J-1) - .25 * H * R(J-1)	14AP5
BB = -2.0 * (F(J-1) + F(J)) - HE2 * P(J)	14AP5
CC = F(J-1) + 4.0 * F(J) + F(J+1) + HE3 * S(J) +	14AP5
1 .25 * H * (R(J-1) + R(J+1)) + HE2 * (P(J) +	19AP5
2 P(J+1))	19AP5
DD = -2.0 * (F(J) + F(J+1)) - HE2 * P(J+1)	14AP5
EE = F(J+1) - .25 * H * R(J+1)	14AP5
FF = HE3 * Q(J) - 0.5 * HE2 * (T(J-1) - T(J+1))	14JE3
C----COMPUTE RECURSION OR CONTINUITY COEFFS AT EACH STA	10JE3
E = AA * B(J-2) + BB	28MY3
DENOM = E * B(J-1) + AA * C(J-2) + CC	28MY3
IF (DENOM) 6010, 6005, 6010	28MY3
C----NOTE IF DENOM IS ZERO, BEAM DOES NOT EXIST, D = 0 SETS DEFL = 0.	10JE3
6005 D = 0.0	28MY3
GO TO 6015	28MY3
6010 D = - 1.0 / DENOM	28MY3
6015 C(J) = D * EE	28MY3
B(J) = D * (E * C(J-1) + DD)	28MY3
A(J) = D * (E * A(J-1) + AA * A(J-2) - FF)	28MY3
C----CONTROL RESET ROUTINES FOR SPECIFIED CONDITIONS	10JE3
KEYJ = KEY(J)	04JE3
GO TO (6060, 6020, 6030, 6020, 6050), KEYJ	20JA4
C----RESET FOR SPECIFIED DEFLECTION	20JA4
6020 C(J) = 0.0	05JE3
B(J) = 0.0	28MY3
A(J) = WS(NS)	05JE3
IF (KEYJ - 3) 6059, 6030, 6060	20JA4
C----RESET FOR SPECIFIED SLOPE AT NEXT STA	17JA4
6030 DTEMP = D	05JE3
CTEMP = C(J)	28MY3
BTEMP = B(J)	28MY3
ATEMP = A(J)	28MY3
C(J) = 1.0	28MY3
B(J) = 0.0	28MY3
A(J) = - HT2 * DWS(NS)	05JE3
GO TO 6060	04JE3
C----RESET FOR SPECIFIED SLOPE AT PRECEDING STATION	23MR4

```

6050      DREV = 1.0 / ( 1.0 - ( BTEMP * B(J-1) + CTEMP - 1.0 ) * 05JE3
1          D / DTEMP ) 04JE3
      CREV = DREV * C(J) 28MY3
      BREV = DREV * ( B(J) + ( BTEMP * C(J-1) ) * D / DTEMP ) 28MY3
      AREV = DREV * ( A(J) + ( HT2 * DWS(NS) + ATEMP + BTEMP 05JE3
1          * A(J-1) ) * D / DTEMP ) 04JE3
      C(J) = CREV 28MY3
      B(J) = BREV 28MY3
      A(J) = AREV 28MY3
6059      NS = NS+1 19AG6
6060      CONTINUE 19AG6
C-----COMPUTE DEFLECTIONS 19AG6
      W(M+6) = 0 30MR7
      W(M+7) = 0 30MR7
      DO 6100 L = 3, MP5 19AG6
      J = M + 8 - L 19AG6
      W(J) = A(J) + B(J) * W(J+1) + C(J) * W(J+2) 19AG6
6100      CONTINUE 19AG6
      W(2) = 2.0 * W(3) - W(4) 19AG6
      W(M+6) = 2.0 * W(MP5) - W(MP4) 19AG6
      DO 8100 J = 3, MP5 19AG6
      DW(J) = ( -W(J-1) + W(J) ) / H 19AG6
      BM(J) = F(J) * ( W(J-1) - 2.0 * W(J) + W(J+1) ) / HE2 19AG6
8100      CONTINUE 19AG6
      BM(2) = 0.0 19AG6
      BM(M+6) = 0.0 19AG6
      DO 8200 J = 3, MP5 19AG6
      DBM(J) = ( -BM(J-1) + BM(J) ) / H 19AG6
1          -P(J) * ( -W(J-1) + W(J) ) / H 19AG6
      KEYJ = KEY(J) 23SE66
      GO TO ( 8140, 8120, 8140, 8120, 8140 ), KEYJ 23SE66
8120      REACT(J) = ( BM(J-1) - 2.0 * BM(J) + BM(J+1) ) / H 19AG6
1          - Q(J) + ( T(J-1) - T(J+1) ) / ( 2.0 * H ) 19AG6
2          - ( R(J-1) * W(J-2) - R(J-1) * W(J) - R(J+1) 19AG6
3          * W(J) + R(J+1)*W(J+2) ) / ( 4.0 * HE2 ) 19AG6
4          - ( P(J) * W(J-1) - P(J) * W(J) - P(J+1) 19AG6
5          * W(J) + P(J+1) * W(J+1) ) / H 19AG6
      GO TO 8200 19AG6
8140      REACT(J) = -S(J) * W(J) 30MR7
8200      CONTINUE 19AG6
      RETURN
      END 19AG6

```

SUBROUTINE TIC TOC (J)	240C66
C-----	
C TIC TOC (1) = COMPILE TIME	08MR8
C TIC TOC (2) = ELAPSED CPU TIME	08MR8
C TIC TOC (3) = TIME FOR THIS PROBLEM	08MR8
C TIC TOC (4) = TIME FOR THIS PROBLEM AND ELAPSED CPU TIME	08MR8
10 FORMAT(//30X19HELAPSED CPU TIME = I5,8H MINUTESF9.3,8H SECONDS)	25SE66
11 FORMAT(//30X15HCOMPILE TIME = ,I5,8H MINUTES,F9.3,8H SECONDS)	25SE66
12 FORMAT(//30X24HTIME FOR THIS PROBLEM = ,I5,8H MINUTES,F9.3,	25SE66
1 8H SECONDS)	25SE66
I = J - 2	240C66
30 IFI(I-1) 40,30,30	25SE66
40 CALL SECOND (F)	25SE66
I11 = F	25SE66
I1 = I11 / 60	25SE66
FI2 = F - I1*60	25SE66
IFI(I) 50,70,60	25SE66
50 PRINT 11, I1,FI2	25SE66
GO TO 990	25SE66
60 FI3 = F - FI4	25SE66
I2 = FI3 / 60	25SE66
FI3 = FI3 - I2*60	25SE66
PRINT 12, I2, FI3	25SE66
IFI(I-1) 990,990,70	25SE66
70 PRINT 10, I1,FI2	25SE66
990 CONTINUE	25SE66
RETURN	25SE66
END	25SE66

APPENDIX 6

LISTING OF DATA FOR EXAMPLE PROBLEMS

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CHG CE244010 CODED BY JJP RUN ON 23 APR 68
EXAMPLE PROBLEMS FOR BIMCOL43 FINAL REPORT

1 THREE SPAN STRUCTURE, FIXED LOAD ONLY

1 4 9 0 0 1 1 1 1

5 TWO SPAN GIRDER, RAILROAD LOADING
 1 3 1 14 0 1 1 1 1
 86 1.200E+01 79 -80 86 2
 0 1 0.000E 00
 43 1 0.000E 00
 86 1 0.000E 00
 0 86 2.440E 11-1.000E 03 -3.300E 04
 0 0 -1.000E 04
 8 8 -2.000E 04
 13 13 -2.000E 04
 18 18 -2.000E 04
 23 23 -2.000E 04
 32 32 -1.300E 04
 37 37 -1.300E 04
 43 43 -1.300E 04
 48 48 -1.300E 04
 56 56 -1.000E 04
 64 64 -2.000E 04
 69 69 -2.000E 04
 74 74 -2.000E 04
 79 79 -2.000E 04

APPENDIX 7

COMPUTED RESULTS FOR EXAMPLE PROBLEMS

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PROGRAM BMCOL 43 - - MATLOCK-TAYLOR - REVISION DATE = 08 MAR 58
 CHG CE244010 CODED BY JJP RUN ON 23 APR 68
 EXAMPLE PROBLEMS FOR BMCOL43 FINAL REPORT

PROB

1 THREE SPAN STRUCTURE, FIXED LOAD ONLY

TABLE 1 - PROGRAM-CONTROL DATA

	ENVELOPES OF MAXIMUMS	TABLE NUMBER			
	2	3	4	5	,6
HOLD FROM PRECEDING PROBLEM (1=HOLD)	0	0	0	0	0
NUM CARDS INPJT THIS PROBLEM	1	4	9	3	0
OPTION (IF=1) TO PLOT ENVELOPES OF MAXIMUMS		DEFL	MOM	SHR	RCT
		1	1	1	1

TABLE 2 - CONSTANTS

NUM INCREMENTS	80
INCREMENT LENGTH	2.400E+01
NUMBER OF INCREMENTS FOR MOBILE LOAD	0
INITIAL POSITION OF MOBILE LOAD STA ZERO	0
FINAL POSITION OF MOBILE LOAD STA ZERO	0
NUMBER OF INCREMENTS BETWEEN EACH POSITION OF MOBILE LOAD	1

TABLE 3 - SPECIFIED DEFLECTIONS AND SLOPES

STA	CASE	DEFLECTION	SLOPE
0	1	0.	NONE
25	1	0.	NONE
55	1	0.	NONE
80	1	0.	NONE

TABLE 4 - STIFFNESS AND FIXED-LOAD DATA

FROM	TO	CONT'D	F	QF	S	T	R	P
0	80	-0	4.147E+10	-6.000E+02	-0.	-0.	-0.	-0.
19	1	0.	0.	-0.	-0.	-0.	-0.	-0.
25	1	0.	3.953E+10	-1.500E+02	-0.	-0.	-0.	-0.
31	-0	0.	0.	-0.	-0.	-0.	-0.	-0.
49	1	0.	0.	-0.	-0.	-0.	-0.	-0.
55	1	0.	3.953E+10	-1.500E+02	-0.	-0.	-0.	-0.
61	-0	0.	0.	-0.	-0.	-0.	-0.	-0.
25	25	-0	-0.	-0.	-0.	-0.	3.873E+07	-0.
55	55	-0	-0.	-0.	-0.	-0.	3.873E+07	-0.

TABLE 5 - MOVABLE-LOAD DATA

FROM TO CONTD QM

NONE

TABLE 6 - SPECIFIED STATIONS FOR INFLUENCE DIAGRAMS
(SHEAR IS COMPUTED ONE HALF INCREMENT
TO THE LEFT OF THE DESIGNATED STATION)

NONE

PROGRAM BMCOL 43 - - MATLOCK-TAYLOR - REVISION DATE = 08 MAR 68
 CHG CE244010 CODED BY JJP RUN ON 23 APR 58
 EXAMPLE PROBLEMS FOR BMCOL43 FINAL REPORT

PROB (CONT'D)
 1 THREE SPAN STRUCTURE, FIXED LOAD ONLY

TABLE 7 - FIXED-LOAD RESULTS

STA I	DIST	DEFL	SLOPE	MOM	SHEAR	SUP REACT
-1	-2.400E+01	6.103E-02	-2.543E-03	0.	-3.331E-10	-0.
0	0.	0.	-2.543E-03	-7.993E-09	5.176E+03	5.476E+03
1	2.400E+01	-6.103E-02	-2.471E-03	1.242E+05	4.576E+03	-0.
2	4.800E+01	-1.203E-01	-2.336E-03	2.340E+05	3.976E+03	-0.
3	7.200E+01	-1.764E-01	-2.145E-03	3.294E+05	3.376E+03	-0.
4	9.600E+01	-2.279E-01	-1.908E-03	4.105E+05	2.776E+03	-0.
5	1.200E+02	-2.737E-01	-1.631E-03	4.771E+05	2.176E+03	-0.
6	1.440E+02	-3.128E-01	-1.325E-03	5.293E+05	1.576E+03	-0.
7	1.680E+02	-3.446E-01	-9.969E-04	5.671E+05	9.756E+02	-0.
8	1.920E+02	-3.685E-01	-6.552E-04	5.905E+05	3.756E+02	-0.
9	2.160E+02	-3.843E-01	-3.082E-04	5.995E+05	-2.244E+02	-0.
10	2.400E+02	-3.917E-01	3.564E-05	5.941E+05	-8.244E+02	-0.
11	2.640E+02	-3.908E-01	3.680E-04	5.744E+05	-1.424E+03	-0.
12	2.880E+02	-3.820E-01	6.807E-04	5.402E+05	-2.024E+03	-0.
13	3.120E+02	-3.656E-01	9.651E-04	4.916E+05	-2.624E+03	-0.
14	3.360E+02	-3.425E-01	1.213E-03	4.286E+05	-3.224E+03	-0.
15	3.600E+02	-3.134E-01	1.416E-03	3.512E+05	-3.824E+03	-0.
16	3.840E+02	-2.794E-01	1.567E-03	2.594E+05	-4.424E+03	-0.
17	4.080E+02	-2.418E-01	1.655E-03	1.532E+05	-5.024E+03	-0.
18	4.320E+02	-2.020E-01	1.674E-03	3.265E+04	-5.624E+03	-0.
19	4.560E+02	-1.619E-01	1.615E-03	-1.023E+05	-6.224E+03	-0.
20	4.800E+02	-1.231E-01	1.489E-03	-2.517E+05	-6.849E+03	-0.
21	5.040E+02	-8.736E-02	1.306E-03	-4.161E+05	-7.499E+03	-0.

22	5.280E+02	-5.601E-02		-5.961E+05		-0.
23	5.520E+02	-3.026E-02	1.073E-03	-7.923E+05	-8.174E+03	-0.
24	5.760E+02	-1.124E-02	7.925E-04	-1.005E+06	-8.874E+03	-0.
25	6.000E+02	0.	4.683E-04	-1.230E+06	-9.369E+03	-0.
26	6.240E+02	2.491E-03	1.038E-04	-1.007E+06	9.306E+03	1.942E+04
27	6.480E+02	-2.811E-03	-2.209E-04	-8.064E+05	8.350E+03	-0.
28	6.720E+02	-1.496E-02	-5.063E-04	-6.228E+05	7.650E+03	-0.
29	6.960E+02	-3.297E-02	-7.503E-04	-4.554E+05	6.975E+03	-0.
30	7.200E+02	-5.578E-02	-9.503E-04	-3.036E+05	6.325E+03	-0.
31	7.440E+02	-8.222E-02	-1.102E-03	-1.668E+05	5.700E+03	-0.
32	7.680E+02	-1.110E-01	-1.198E-03	-4.437E+04	5.100E+03	-0.
33	7.920E+02	-1.404E-01	-1.187E-03	6.363E+04	4.500E+03	-0.
34	8.160E+02	-1.689E-01	-1.096E-03	1.572E+05	3.900E+03	-0.
35	8.400E+02	-1.952E-01	-9.595E-04	2.364E+05	3.300E+03	-0.
36	8.640E+02	-2.182E-01	-7.852E-04	3.012E+05	2.700E+03	-0.
37	8.880E+02	-2.370E-01	-5.817E-04	3.516E+05	2.100E+03	-0.
38	9.120E+02	-2.510E-01	-3.573E-04	3.876E+05	1.500E+03	-0.
39	9.360E+02	-2.596E-01	-1.205E-04	4.092E+05	9.000E+02	-0.
40	9.600E+02	-2.625E-01	1.205E-04	4.164E+05	3.000E+02	-0.
41	9.840E+02	-2.596E-01	3.573E-04	4.092E+05	-3.000E+02	-0.
42	1.008E+03	-2.510E-01	5.817E-04	3.876E+05	-9.000E+02	-0.
43	1.032E+03	-2.370E-01	7.852E-04	3.516E+05	-1.500E+03	-0.
44	1.056E+03	-2.182E-01	9.595E-04	3.012E+05	-2.100E+03	-0.
45	1.080E+03	-1.952E-01	1.096E-03	2.364E+05	-2.700E+03	-0.
46	1.104E+03	-1.689E-01	1.187E-03	1.572E+05	-3.300E+03	-0.
47	1.128E+03	-1.404E-01	1.224E-03	6.363E+04	-3.900E+03	-0.
48	1.152E+03	-1.110E-01	1.198E-03	-4.437E+04	-4.500E+03	-0.
49	1.176E+03	-8.222E-02	1.102E-03	-1.668E+05	-5.100E+03	-0.
50	1.200E+03	-5.578E-02	9.503E-04	-3.036E+05	-5.700E+03	-0.
51	1.224E+03	-3.297E-02	7.503E-04	-4.554E+05	-6.325E+03	-0.
52	1.248E+03	-1.496E-02		-6.228E+05	-6.975E+03	-0.

53	1.272E+03	-2.811E-03	5.063E-04	-8.064E+05	-7.650E+03	-0.
54	1.296E+03	2.491E-03	2.209E-04	-1.007E+06	-8.350E+03	-0.
55	1.320E+03	0.	-1.038E-04	-1.230E+06	-9.306E+03	1.942E+04
56	1.344E+03	-1.124E-02	-4.683E-04	-1.005E+06	9.369E+03	-0.
57	1.368E+03	-3.026E-02	-7.925E-04	-7.923E+05	8.874E+03	-0.
58	1.392E+03	-5.601E-02	-1.073E-03	-1.005E+06	8.174E+03	-0.
59	1.416E+03	-8.736E-02	-1.306E-03	-4.161E+05	7.499E+03	-0.
60	1.440E+03	-1.231E-01	-1.489E-03	-2.517E+05	6.849E+03	-0.
61	1.464E+03	-1.619E-01	-1.615E-03	-1.023E+05	6.224E+03	-0.
62	1.488E+03	-2.020E-01	-1.674E-03	3.265E+04	5.624E+03	-0.
63	1.512E+03	-2.418E-01	-1.655E-03	1.532E+05	5.024E+03	-0.
64	1.536E+03	-2.794E-01	-1.567E-03	2.594E+05	4.424E+03	-0.
65	1.560E+03	-3.134E-01	-1.416E-03	3.512E+05	3.824E+03	-0.
66	1.584E+03	-3.425E-01	-1.213E-03	4.286E+05	3.224E+03	-0.
67	1.608E+03	-3.656E-01	-9.651E-04	4.916E+05	2.624E+03	-0.
68	1.632E+03	-3.820E-01	-6.807E-04	5.402E+05	2.024E+03	-0.
69	1.656E+03	-3.908E-01	-3.680E-04	5.744E+05	1.424E+03	-0.
70	1.680E+03	-3.917E-01	-3.564E-05	5.941E+05	8.244E+02	-0.
71	1.704E+03	-3.843E-01	3.082E-04	5.995E+05	2.244E+02	-0.
72	1.728E+03	-3.685E-01	6.552E-04	5.905E+05	-3.756E+02	-0.
73	1.752E+03	-3.446E-01	9.969E-04	5.671E+05	-9.756E+02	-0.
74	1.776E+03	-3.128E-01	1.325E-03	5.293E+05	-1.576E+03	-0.
75	1.800E+03	-2.737E-01	1.631E-03	4.771E+05	-2.176E+03	-0.
76	1.824E+03	-2.279E-01	1.908E-03	4.105E+05	-2.776E+03	-0.
77	1.848E+03	-1.764E-01	2.145E-03	3.294E+05	-3.376E+03	-0.
78	1.872E+03	-1.203E-01	2.336E-03	2.340E+05	-3.976E+03	-0.
79	1.896E+03	-6.103E-02	2.471E-03	1.242E+05	-4.576E+03	-0.
80	1.920E+03	0.	2.543E-03	0.	-5.176E+03	5.476E+03
81	1.944E+03	6.103E-02	2.543E-03	0.	0.	-0.

PROB (CONT'D)

1 THREE SPAN STRUCTURE, FIXED LOAD ONLY

TABLE 8A- ENVELOPES OF MAXIMUMS * = HELD FROM PRIOR PROBLEM

STA	MAX +DEFL	LOC	MAX -DEFL	LOC	MAX +MOM	LOC	MAX -MOM	LOC
-1	6.103E-02	999	0.	999	0.	999	-7.993E-09	999
0	0.	999	0.	999	0.	999	-7.993E-09	999
1	0.	999	-6.103E-02	999	1.242E+05	999	0.	999
2	0.	999	-1.203E-01	999	2.340E+05	999	0.	999
3	0.	999	-1.764E-01	999	3.294E+05	999	0.	999
4	0.	999	-2.279E-01	999	4.105E+05	999	0.	999
5	0.	999	-2.737E-01	999	4.771E+05	999	0.	999
6	0.	999	-3.128E-01	999	5.293E+05	999	0.	999
7	0.	999	-3.446E-01	999	5.671E+05	999	0.	999
8	0.	999	-3.685E-01	999	5.905E+05	999	0.	999
9	0.	999	-3.843E-01	999	5.995E+05	999	0.	999
10	0.	999	-3.917E-01	999	5.941E+05	999	0.	999
11	0.	999	-3.908E-01	999	5.744E+05	999	0.	999
12	0.	999	-3.820E-01	999	5.402E+05	999	0.	999
13	0.	999	-3.656E-01	999	4.916E+05	999	0.	999
14	0.	999	-3.425E-01	999	4.286E+05	999	0.	999
15	0.	999	-3.134E-01	999	3.512E+05	999	0.	999
16	0.	999	-2.794E-01	999	2.594E+05	999	0.	999
17	0.	999	-2.418E-01	999	1.532E+05	999	0.	999
18	0.	999	-2.020E-01	999	3.265E+04	999	0.	999
19	0.	999	-1.619E-01	999	0.	999	-1.023E+05	999
20	0.	999	-1.231E-01	999	0.	999	-2.517E+05	999
21	0.	999	-8.736E-02	999	0.	999	-4.161E+05	999
22	0.	999	-5.601E-02	999	0.	999	-5.961E+05	999
23	0.	999	-3.026E-02	999	0.	999	-7.923E+05	999

24	0.	999	-1.124E-02	999	0.	999	-1.005E+06	999
25	0.	999	0.	999	0.	999	-1.230E+06	999
26	2.491E-03	999	0.	999	0.	999	-1.007E+06	999
27	0.	999	-2.811E-03	999	0.	999	-8.064E+05	999
28	0.	999	-1.496E-02	999	0.	999	-5.228E+05	999
29	0.	999	-3.297E-02	999	0.	999	-4.554E+05	999
30	0.	999	-5.578E-02	999	0.	999	-3.036E+05	999
31	0.	999	-8.222E-02	999	0.	999	-1.668E+05	999
32	0.	999	-1.110E-01	999	0.	999	-4.437E+04	999
33	0.	999	-1.404E-01	999	6.363E+04	999	0.	999
34	0.	999	-1.689E-01	999	1.572E+05	999	0.	999
35	0.	999	-1.952E-01	999	2.364E+05	999	0.	999
36	0.	999	-2.182E-01	999	3.012E+05	999	0.	999
37	0.	999	-2.370E-01	999	3.516E+05	999	0.	999
38	0.	999	-2.510E-01	999	3.876E+05	999	0.	999
39	0.	999	-2.596E-01	999	4.092E+05	999	0.	999
40	0.	999	-2.625E-01	999	4.164E+05	999	0.	999
41	0.	999	-2.596E-01	999	4.092E+05	999	0.	999
42	0.	999	-2.510E-01	999	3.876E+05	999	0.	999
43	0.	999	-2.370E-01	999	3.516E+05	999	0.	999
44	0.	999	-2.182E-01	999	3.012E+05	999	0.	999
45	0.	999	-1.952E-01	999	2.364E+05	999	0.	999
46	0.	999	-1.689E-01	999	1.572E+05	999	0.	999
47	0.	999	-1.404E-01	999	6.363E+04	999	0.	999
48	0.	999	-1.110E-01	999	0.	999	-4.437E+04	999
49	0.	999	-8.222E-02	999	0.	999	-1.668E+05	999
50	0.	999	-5.578E-02	999	0.	999	-3.036E+05	999
51	0.	999	-3.297E-02	999	0.	999	-4.554E+05	999
52	0.	999	-1.496E-02	999	0.	999	-5.228E+05	999
53	0.	999	-2.811E-03	999	0.	999	-8.064E+05	999

54	2.491E-03	999	0.	999	0.	999	-1.007E+06	999
55	0.	999	0.	999	0.	999	-1.230E+06	999
56	0.	999	-1.124E-02	999	0.	999	-1.005E+06	999
57	0.	999	-3.026E-02	999	0.	999	-7.923E+05	999
58	0.	999	-5.601E-02	999	0.	999	-5.961E+05	999
59	0.	999	-8.736E-02	999	0.	999	-4.161E+05	999
60	0.	999	-1.231E-01	999	0.	999	-2.517E+05	999
61	0.	999	-1.619E-01	999	0.	999	-1.023E+05	999
62	0.	999	-2.020E-01	999	3.265E+04	999	0.	999
63	0.	999	-2.418E-01	999	1.532E+05	999	0.	999
64	0.	999	-2.794E-01	999	2.594E+05	999	0.	999
65	0.	999	-3.134E-01	999	3.512E+05	999	0.	999
66	0.	999	-3.425E-01	999	4.286E+05	999	0.	999
67	0.	999	-3.656E-01	999	4.916E+05	999	0.	999
68	0.	999	-3.820E-01	999	5.402E+05	999	0.	999
69	0.	999	-3.908E-01	999	5.744E+05	999	0.	999
70	0.	999	-3.917E-01	999	5.941E+05	999	0.	999
71	0.	999	-3.843E-01	999	5.995E+05	999	0.	999
72	0.	999	-3.685E-01	999	5.905E+05	999	0.	999
73	0.	999	-3.446E-01	999	5.671E+05	999	0.	999
74	0.	999	-3.128E-01	999	5.293E+05	999	0.	999
75	0.	999	-2.737E-01	999	4.771E+05	999	0.	999
76	0.	999	-2.279E-01	999	4.105E+05	999	0.	999
77	0.	999	-1.764E-01	999	3.294E+05	999	0.	999
78	0.	999	-1.203E-01	999	2.340E+05	999	0.	999
79	0.	999	-6.103E-02	999	1.242E+05	999	0.	999
80	0.	999	0.	999	0.	999	0.	999
81	6.103E-02	999	0.	999	0.	999	0.	999

TABLE 8B- ENVELOPES OF MAXIMUMS * = HELD FROM PRIOR PROBLEM

STA	MAX +SHEAR LOC	MAX -SHEAR LOC	MAX +REACT LOC	MAX -REACT LOC
-1	0. 999	-3.331E-10 999	0. 999	0. 999
0	5.176E+03 999	0. 999	5.476E+03 999	0. 999
1	4.576E+03 999	0. 999	0. 999	0. 999
2	3.976E+03 999	0. 999	0. 999	0. 999
3	3.376E+03 999	0. 999	0. 999	0. 999
4	2.776E+03 999	0. 999	0. 999	0. 999
5	2.176E+03 999	0. 999	0. 999	0. 999
6	1.576E+03 999	0. 999	0. 999	0. 999
7	9.756E+02 999	0. 999	0. 999	0. 999
8	3.756E+02 999	0. 999	0. 999	0. 999
9	0. 999	-2.244E+02 999	0. 999	0. 999
10	0. 999	-8.244E+02 999	0. 999	0. 999
11	0. 999	-1.424E+03 999	0. 999	0. 999
12	0. 999	-2.024E+03 999	0. 999	0. 999
13	0. 999	-2.624E+03 999	0. 999	0. 999
14	0. 999	-3.224E+03 999	0. 999	0. 999
15	0. 999	-3.824E+03 999	0. 999	0. 999
16	0. 999	-4.424E+03 999	0. 999	0. 999
17	0. 999	-5.024E+03 999	0. 999	0. 999
18	0. 999	-5.624E+03 999	0. 999	0. 999
19	0. 999	-6.224E+03 999	0. 999	0. 999
20	0. 999	-6.849E+03 999	0. 999	0. 999
21	0. 999	-7.499E+03 999	0. 999	0. 999
22	0. 999	-8.174E+03 999	0. 999	0. 999
23	0. 999	-8.874E+03 999	0. 999	0. 999
24	0. 999	-9.369E+03 999	0. 999	0. 999
25	9.306E+03 999	0. 999	1.942E+04 999	0. 999
26	8.350E+03 999	0. 999	0. 999	0. 999
27	7.650E+03 999	0. 999	0. 999	0. 999
28	6.975E+03 999	0. 999	0. 999	0. 999

29	6.325E+03	999	0.	999	0.	999	0.	999
30	5.700E+03	999	0.	999	0.	999	0.	999
31	5.100E+03	999	0.	999	0.	999	0.	999
32	4.500E+03	999	0.	999	0.	999	0.	999
33	3.900E+03	999	0.	999	0.	999	0.	999
34	3.300E+03	999	0.	999	0.	999	0.	999
35	2.700E+03	999	0.	999	0.	999	0.	999
36	2.100E+03	999	0.	999	0.	999	0.	999
37	1.500E+03	999	0.	999	0.	999	0.	999
38	9.000E+02	999	0.	999	0.	999	0.	999
39	3.000E+02	999	0.	999	0.	999	0.	999
40	0.	999	-3.000E+02	999	0.	999	0.	999
41	0.	999	-9.000E+02	999	0.	999	0.	999
42	0.	999	-1.500E+03	999	0.	999	0.	999
43	0.	999	-2.100E+03	999	0.	999	0.	999
44	0.	999	-2.700E+03	999	0.	999	0.	999
45	0.	999	-3.300E+03	999	0.	999	0.	999
46	0.	999	-3.900E+03	999	0.	999	0.	999
47	0.	999	-4.500E+03	999	0.	999	0.	999
48	0.	999	-5.100E+03	999	0.	999	0.	999
49	0.	999	-5.700E+03	999	0.	999	0.	999
50	0.	999	-6.325E+03	999	0.	999	0.	999
51	0.	999	-6.975E+03	999	0.	999	0.	999
52	0.	999	-7.650E+03	999	0.	999	0.	999
53	0.	999	-8.350E+03	999	0.	999	0.	999
54	0.	999	-9.306E+03	999	1.942E+04	999	0.	999
55	9.369E+03	999	0.	999	0.	999	0.	999
56	8.874E+03	999	0.	999	0.	999	0.	999
57	8.174E+03	999	0.	999	0.	999	0.	999
58	7.499E+03	999	0.	999	0.	999	0.	999
59					0.	999	0.	999

	6.849E+03	999	0.	999	0.	999	0.	999
60	6.224E+03	999	0.	999	0.	999	0.	999
61	5.624E+03	999	0.	999	0.	999	0.	999
62	5.024E+03	999	0.	999	0.	999	0.	999
63	4.424E+03	999	0.	999	0.	999	0.	999
64	3.824E+03	999	0.	999	0.	999	0.	999
65	3.224E+03	999	0.	999	0.	999	0.	999
66	2.624E+03	999	0.	999	0.	999	0.	999
67	2.024E+03	999	0.	999	0.	999	0.	999
68	1.424E+03	999	0.	999	0.	999	0.	999
69	8.244E+02	999	0.	999	0.	999	0.	999
70	2.244E+02	999	0.	999	0.	999	0.	999
71	0.	999	-3.756E+02	999	0.	999	0.	999
72	0.	999	-9.756E+02	999	0.	999	0.	999
73	0.	999	-1.576E+03	999	0.	999	0.	999
74	0.	999	-2.176E+03	999	0.	999	0.	999
75	0.	999	-2.776E+03	999	0.	999	0.	999
76	0.	999	-3.376E+03	999	0.	999	0.	999
77	0.	999	-3.976E+03	999	0.	999	0.	999
78	0.	999	-4.576E+03	999	0.	999	0.	999
79	0.	999	-5.176E+03	999	0.	999	0.	999
80	0.	999	0.	999	5.476E+03	999	0.	999
81					0.	999	0.	999

TABLE 9 -- SCALES FOR PLOTS OF THE ENVELOPES OF MAXIMUMS

HORIZONTAL SCALE
10 INCHES = 100 STATIONS

VERTICAL SCALES

VARIABLE	LENGTH OF AXIS	MAXIMUM VALUE
DEFLECT	2 INCHES	= 4.000E-01
MOMENT	2 INCHES	= 2.000E+06
SHEAR	2 INCHES	= 1.000E+04
REACTION	2 INCHES	= 2.000E+04

PROB (CONT'D)

1 THREE SPAN STRUCTURE, FIXED LOAD ONLY

TABLE 10A -- INFLUENCE DIAGRAMS FOR DEFLECTION

LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TABLE 10B -- INFLUENCE DIAGRAMS FOR MOMENT

LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TABLE 10C -- INFLUENCE DIAGRAMS FOR SHEAR

(SHEAR IS COMPUTED ONE HALF INCREMENT
 TO THE LEFT OF THE DESIGNATED STATION)

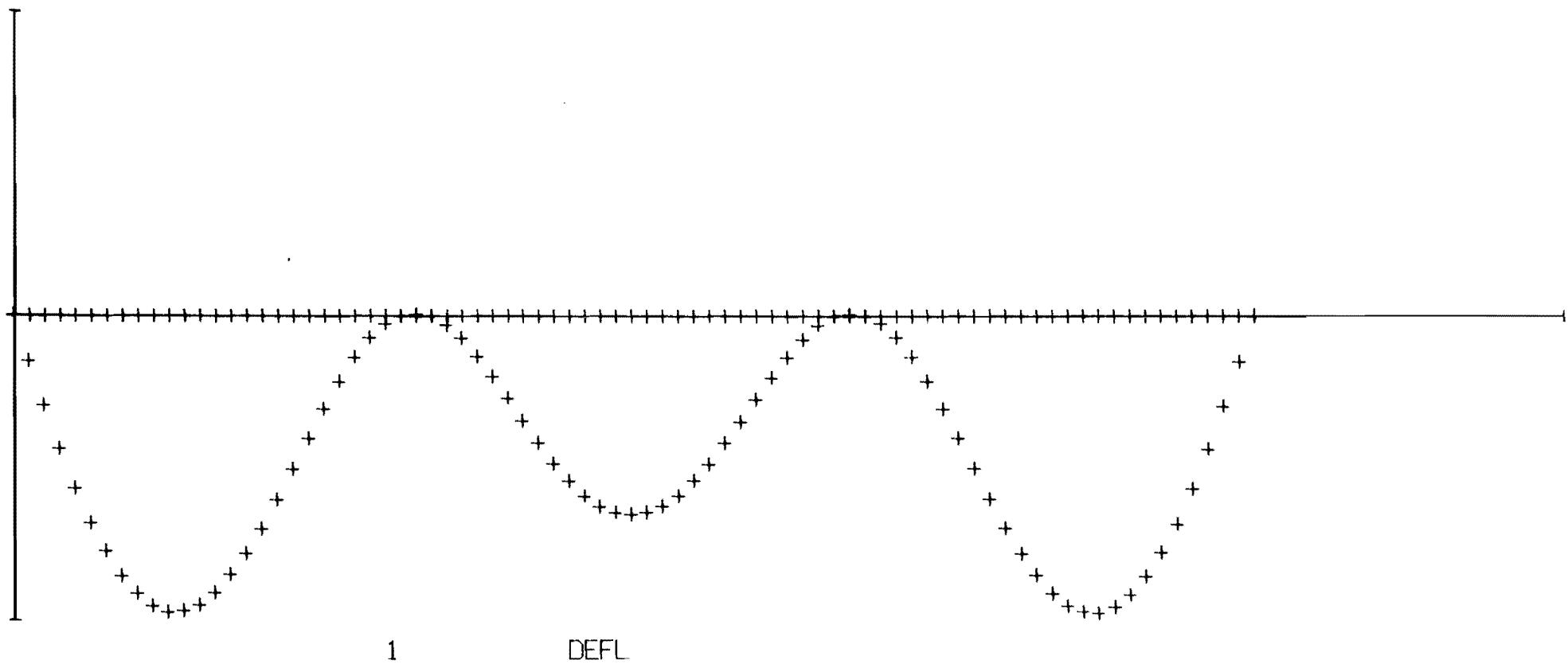
LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TABLE 10D -- INFLUENCE DIAGRAMS FOR SUPPORT REACTION

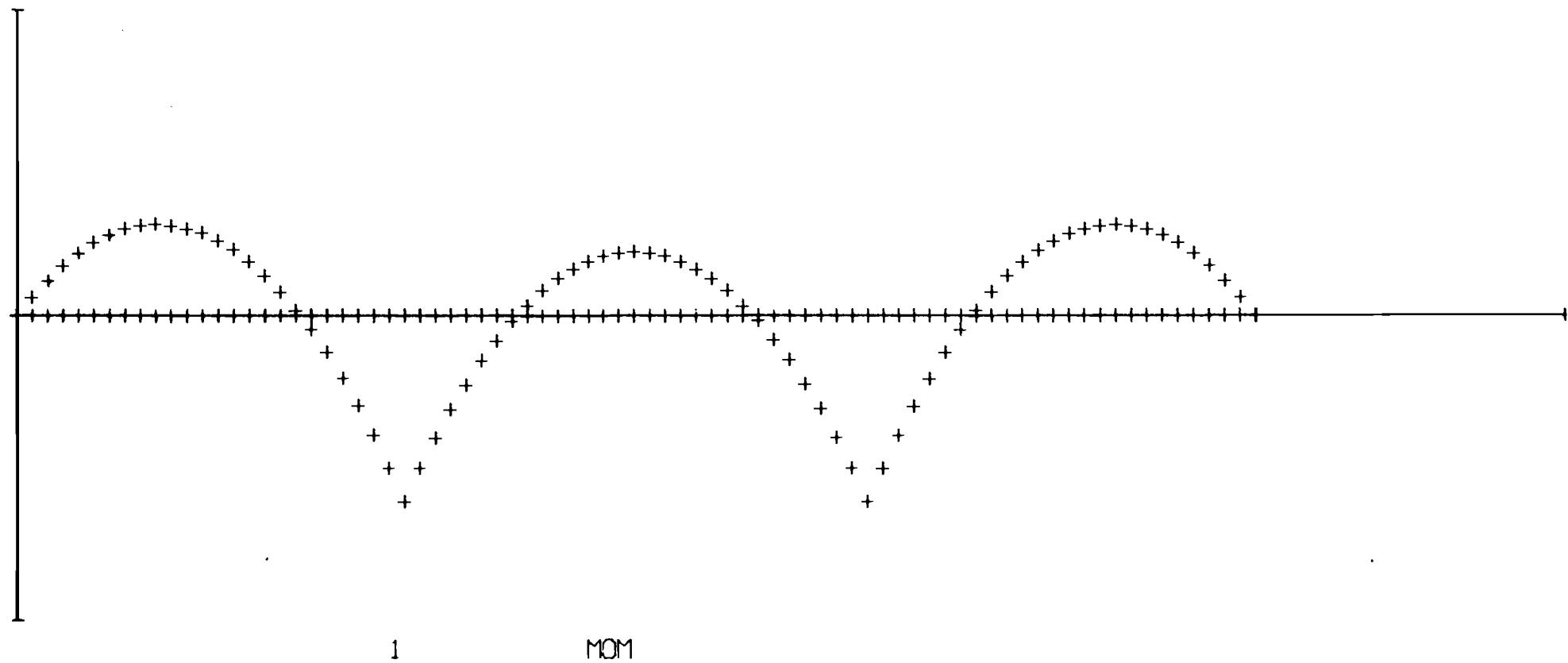
LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TIME FOR THIS PROBLEM = 0 MINUTES 3.258 SECONDS

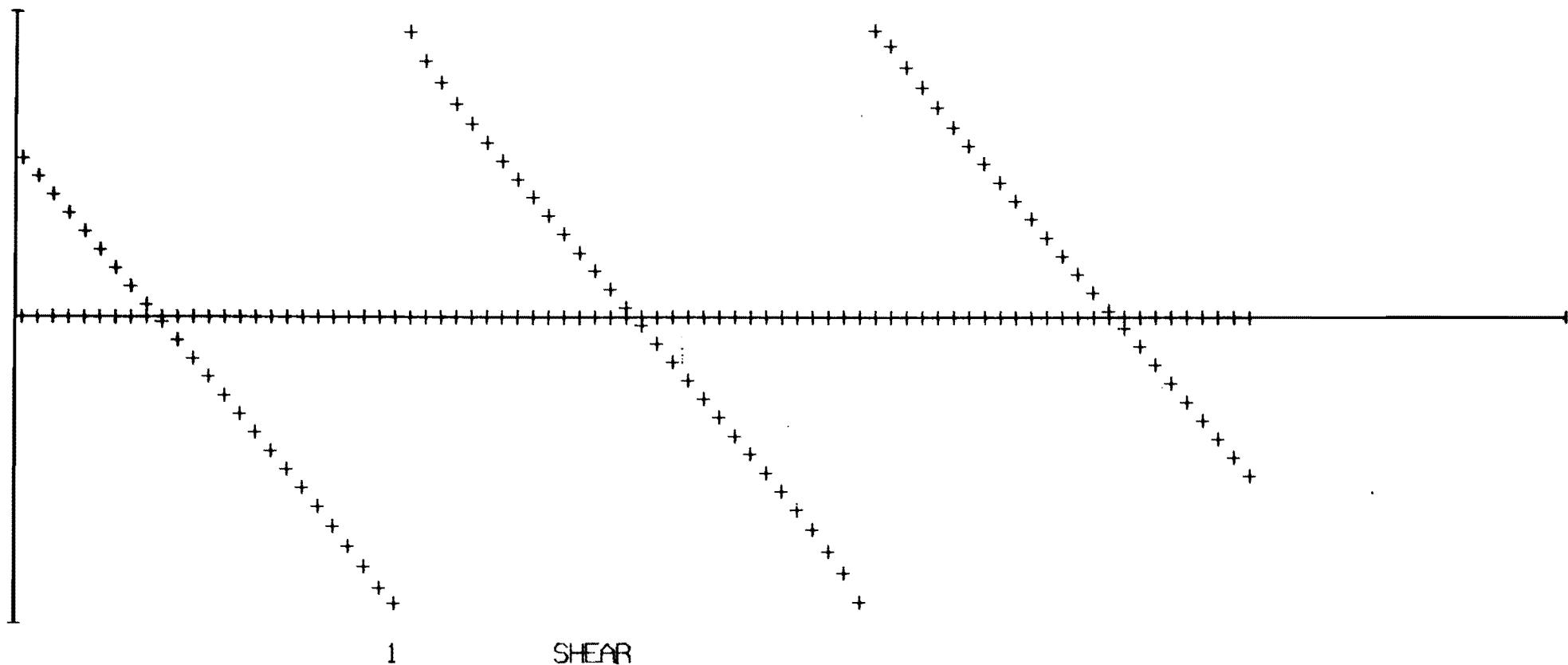
ELAPSED CPU TIME = 0 MINUTES 17.277 SECONDS



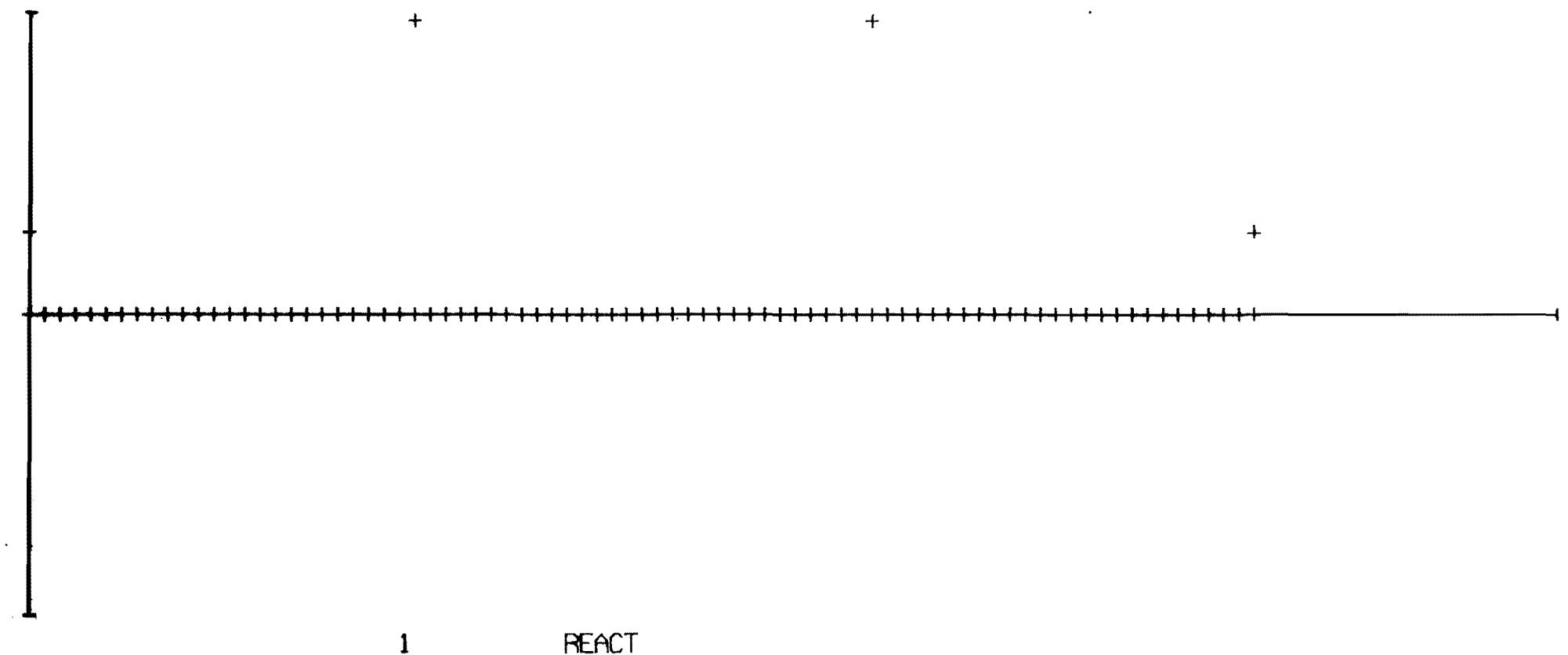
Prob 1. Three-span structure, fixed load only.
Envelope of maximum deflections.



**Prob 1. Three-span structure, fixed load only.
Envelope of maximum moments.**



Prob 1. Three-span structure, fixed load only.
Envelope of maximum shears.



Prob 1. Three-span structure, fixed load only.
Envelope of maximum reactions.

PROGRAM BMCOL 43 - - MATLOCK-TAYLOR - REVISION DATE = 08 MAR 68
 CHG CE244010 CODED BY JJP RUN ON 23 APR 58
 EXAMPLE PROBLEMS FOR BMCOL43 FINAL REPORT

PROB

2 THREE SPAN STRUCTURE, FIXED LOADS PLUS MOVING LOADS

TABLE 1 - PROGRAM-CONTROL DATA

	ENVELOPES OF MAXIMUMS	TABLE NUMBER				
	2	3	4	5	6	
HOLD FROM PRECEDING PROBLEM (1=HOLD)	0	0	1	1	0	0
NUM CARDS INPJT THIS PROBLEM		1	0	0	3	0
OPTION (IF=1) TO PLOT ENVELOPES OF MAXIMUMS		DEFL	MOM	SHR	RCT	
		1	1	1	1	

TABLE 2 - CONSTANTS

NUM INCREMENTS	80
INCREMENT LENGTH	2.400E+01
NUMBER OF INCREMENTS FOR MOVABLE LOAD	14
INITIAL POSITION OF MOVABLE LOAD STA ZERO	-14
FINAL POSITION OF MOVABLE LOAD STA ZERO	80
NUMBER OF INCREMENTS BETWEEN EACH POSITION OF MOVABLE LOAD	1

TABLE 3 - SPECIFIED DEFLECTIONS AND SLOPES

STA	CASE	DEFLECTION	SLOPE
USING DATA FROM THE PREVIOUS PROBLEM			
0	1	0.	NONE
25	1	0.	NONE
55	1	0.	NONE
80	1	0.	NONE

TABLE 4 - STIFFNESS AND FIXED-LOAD DATA

FROM	TO	CONTD	F	QF	S	T	R	P
USING DATA FROM THE PREVIOUS PROBLEM								
0	80	-0	4.147E+10	-6.000E+02	-0.	-0.	-0.	-0.
19	1	0.	0.	-0.	-0.	-0.	-0.	-0.
25	1	3.953E+10	-1.500E+02	-0.	-0.	-0.	-0.	-0.
31	-0	0.	0.	-0.	-0.	-0.	-0.	-0.
49	1	0.	0.	-0.	-0.	-0.	-0.	-0.
55	1	3.953E+10	-1.500E+02	-0.	-0.	-0.	-0.	-0.
61	-0	0.	0.	-0.	-0.	-0.	-0.	-0.
25	25	-0.	-0.	-0.	-0.	-0.	3.873E+07	-0.
55	55	-0.	-0.	-0.	-0.	-0.	3.873E+07	-0.
ADDITIONAL DATA FOR THIS PROBLEM								
NONE								

TABLE 5 - MOVABLE-LOAD DATA

FROM TO CONTD QM

0	0	-0	-7.300E+02
7	7	-0	-2.920E+03
14	14	-0	-2.920E+03

TABLE 6 - SPECIFIED STATIONS FOR INFLUENCE DIAGRAMS
(SHEAR IS COMPUTED ONE HALF INCREMENT
TO THE LEFT OF THE DESIGNATED STATION)

NONE

PROGRAM BMCOL 43 - - MATLOCK-TAYLOR - REVISION DATE = 08 MAR 68
 CHG CE244010 CODED BY JJP RUN ON 23 APR 68
 EXAMPLE PROBLEMS FOR BMCOL43 FINAL REPORT

PROB (CONT'D)

2 THREE SPAN STRUCTURE, FIXED LOADS PLUS MOVING LOADS

TABLE 7 - FIXED-LOAD RESULTS

STA I	DIST	DEFL	SLOPE	MOM	SHEAR	SUP REACT
-1	-2.400E+01	6.103E-02	-2.543E-03	0.	-3.331E-10	-0.
0	0.	0.	-2.543E-03	-7.993E-09	5.176E+03	5.476E+03
1	2.400E+01	-6.103E-02	-2.471E-03	1.242E+05	4.576E+03	-0.
2	4.800E+01	-1.203E-01	-2.336E-03	2.340E+05	3.976E+03	-0.
3	7.200E+01	-1.764E-01	-2.145E-03	3.294E+05	3.376E+03	-0.
4	9.600E+01	-2.279E-01	-1.908E-03	4.105E+05	2.776E+03	-0.
5	1.200E+02	-2.737E-01	-1.631E-03	4.771E+05	2.176E+03	-0.
6	1.440E+02	-3.128E-01	-1.325E-03	5.293E+05	1.576E+03	-0.
7	1.680E+02	-3.446E-01	-9.969E-04	5.671E+05	9.756E+02	-0.
8	1.920E+02	-3.685E-01	-6.552E-04	5.905E+05	3.756E+02	-0.
9	2.160E+02	-3.843E-01	-3.082E-04	5.995E+05	-2.244E+02	-0.
10	2.400E+02	-3.917E-01	3.564E-05	5.941E+05	-8.244E+02	-0.
11	2.640E+02	-3.908E-01	3.680E-04	5.744E+05	-1.424E+03	-0.
12	2.880E+02	-3.820E-01	6.807E-04	5.402E+05	-2.024E+03	-0.
13	3.120E+02	-3.656E-01	9.651E-04	4.916E+05	-2.624E+03	-0.
14	3.360E+02	-3.425E-01	1.213E-03	4.286E+05	-3.224E+03	-0.
15	3.600E+02	-3.134E-01	1.416E-03	3.512E+05	-3.824E+03	-0.
16	3.840E+02	-2.794E-01	1.567E-03	2.594E+05	-4.424E+03	-0.
17	4.080E+02	-2.418E-01	1.655E-03	1.532E+05	-5.024E+03	-0.
18	4.320E+02	-2.020E-01	1.674E-03	3.265E+04	-5.624E+03	-0.
19	4.560E+02	-1.619E-01	1.615E-03	-1.023E+05	-6.224E+03	-0.
20	4.800E+02	-1.231E-01	1.489E-03	-2.517E+05	-6.849E+03	-0.
21	5.040E+02	-8.736E-02	1.306E-03	-4.161E+05	-7.499E+03	-0.

22	5.280E+02	-5.601E-02		-5.961E+05	-8.174E+03	-0.
23	5.520E+02	-3.026E-02	1.073E-03	-7.923E+05	-8.874E+03	-0.
24	5.760E+02	-1.124E-02	7.925E-04	-1.005E+06	-9.369E+03	-0.
25	6.000E+02	0.	4.683E-04	-1.230E+06	9.306E+03	1.942E+04
26	6.240E+02	2.491E-03	1.038E-04	-1.007E+06	8.350E+03	-0.
27	6.480E+02	-2.811E-03	-2.209E-04	-8.064E+05	7.650E+03	-0.
28	6.720E+02	-1.496E-02	-5.063E-04	-6.228E+05	6.975E+03	-0.
29	6.960E+02	-3.297E-02	-9.503E-04	-4.554E+05	6.325E+03	-0.
30	7.200E+02	-5.578E-02	-1.102E-03	-3.036E+05	5.700E+03	-0.
31	7.440E+02	-8.222E-02	-1.198E-03	-1.668E+05	5.100E+03	-0.
32	7.680E+02	-1.110E-01	-1.224E-03	-4.437E+04	4.500E+03	-0.
33	7.920E+02	-1.404E-01	-1.187E-03	6.363E+04	3.900E+03	-0.
34	8.160E+02	-1.689E-01	-1.096E-03	1.572E+05	3.300E+03	-0.
35	8.400E+02	-1.952E-01	-9.595E-04	2.364E+05	2.700E+03	-0.
36	8.640E+02	-2.182E-01	-7.852E-04	3.012E+05	2.100E+03	-0.
37	8.880E+02	-2.370E-01	-5.817E-04	3.516E+05	1.500E+03	-0.
38	9.120E+02	-2.510E-01	-3.573E-04	3.876E+05	9.000E+02	-0.
39	9.360E+02	-2.596E-01	-1.205E-04	4.092E+05	3.000E+02	-0.
40	9.600E+02	-2.625E-01	1.205E-04	4.164E+05	-3.000E+02	-0.
41	9.840E+02	-2.596E-01	3.573E-04	4.092E+05	-9.000E+02	-0.
42	1.008E+03	-2.510E-01	5.817E-04	3.876E+05	-1.500E+03	-0.
43	1.032E+03	-2.370E-01	7.852E-04	3.516E+05	-2.100E+03	-0.
44	1.056E+03	-2.182E-01	9.595E-04	3.012E+05	-2.700E+03	-0.
45	1.080E+03	-1.952E-01	1.096E-03	2.364E+05	-3.300E+03	-0.
46	1.104E+03	-1.689E-01	1.187E-03	1.572E+05	-3.900E+03	-0.
47	1.128E+03	-1.404E-01	1.224E-03	6.363E+04	-4.500E+03	-0.
48	1.152E+03	-1.110E-01	1.198E-03	-4.437E+04	-5.100E+03	-0.
49	1.176E+03	-8.222E-02	1.102E-03	-1.668E+05	-5.700E+03	-0.
50	1.200E+03	-5.578E-02	9.503E-04	-3.036E+05	-6.325E+03	-0.
51	1.224E+03	-3.297E-02	7.503E-04	-4.554E+05	-6.975E+03	-0.
52	1.248E+03	-1.496E-02		-6.228E+05		-0.

53	1.272E+03	-2.811E-03	5.063E-04	-8.064E+05	-7.650E+03	-0.
54	1.296E+03	2.491E-03	2.209E-04	-1.007E+06	-8.350E+03	-0.
55	1.320E+03	0.	-1.038E-04	-1.230E+06	-9.306E+03	1.942E+04
56	1.344E+03	-1.124E-02	-4.683E-04	-1.005E+06	9.369E+03	-0.
57	1.368E+03	-3.026E-02	-7.925E-04	-7.923E+05	8.874E+03	-0.
58	1.392E+03	-5.601E-02	-1.073E-03	-5.961E+05	8.174E+03	-0.
59	1.416E+03	-8.736E-02	-1.306E-03	-4.161E+05	7.499E+03	-0.
60	1.440E+03	-1.231E-01	-1.489E-03	-2.517E+05	6.849E+03	-0.
61	1.464E+03	-1.619E-01	-1.615E-03	-1.023E+05	6.224E+03	-0.
62	1.488E+03	-2.020E-01	-1.674E-03	3.265E+04	5.624E+03	-0.
63	1.512E+03	-2.418E-01	-1.655E-03	1.532E+05	5.024E+03	-0.
64	1.536E+03	-2.794E-01	-1.567E-03	2.594E+05	4.424E+03	-0.
65	1.560E+03	-3.134E-01	-1.416E-03	3.512E+05	3.824E+03	-0.
66	1.584E+03	-3.425E-01	-9.651E-04	4.286E+05	2.624E+03	-0.
67	1.608E+03	-3.656E-01	-6.807E-04	4.916E+05	2.024E+03	-0.
68	1.632E+03	-3.820E-01	-3.680E-04	5.402E+05	1.424E+03	-0.
69	1.656E+03	-3.908E-01	-3.564E-05	5.744E+05	8.244E+02	-0.
70	1.680E+03	-3.917E-01	3.082E-04	5.941E+05	2.244E+02	-0.
71	1.704E+03	-3.843E-01	6.552E-04	5.995E+05	-3.756E+02	-0.
72	1.728E+03	-3.685E-01	9.969E-04	5.905E+05	-9.756E+02	-0.
73	1.752E+03	-3.446E-01	1.325E-03	5.671E+05	-1.576E+03	-0.
74	1.776E+03	-3.128E-01	1.631E-03	5.293E+05	-2.176E+03	-0.
75	1.800E+03	-2.737E-01	1.908E-03	4.771E+05	-2.776E+03	-0.
76	1.824E+03	-2.279E-01	2.145E-03	4.105E+05	-3.376E+03	-0.
77	1.848E+03	-1.764E-01	2.336E-03	3.294E+05	-3.976E+03	-0.
78	1.872E+03	-1.203E-01	2.471E-03	2.340E+05	-4.576E+03	-0.
79	1.896E+03	-6.103E-02	2.543E-03	1.242E+05	-5.176E+03	-0.
80	1.920E+03	0.	2.543E-03	0.	0.	5.476E+03
81	1.944E+03	6.103E-02	0.	0.	0.	-0.

PROB (CONT'D)

2 THREE SPAN STRUCTURE, FIXED LOADS PLUS MOVING LOADS

TABLE 8A- ENVELOPES OF MAXIMUMS * = HELD FROM PRIOR PROBLEM

STA	MAX 1.129E-01	+DEFL LOC 1	MAX 0.	-DEFL LOC 999	MAX 0.	+MOM LOC 999	MAX 0.	-MOM LOC 999
0	0.	999	0.	999	0.	999	-1.599E-08	-13
1	0.	999	-1.129E-01	1	2.320E+05	-6	0.	999
2	0.	999	-2.230E-01	1	4.354E+05	-5	0.	999
3	0.	999	-3.279E-01	1	6.107E+05	-4	0.	999
4	0.	999	-4.255E-01	1	7.581E+05	-3	0.	999
5	0.	999	-5.138E-01	1	8.781E+05	-2	0.	999
6	0.	999	-5.910E-01	1	9.712E+05	-1	0.	999
7	0.	999	-6.557E-01	1	1.038E+06	0	0.	999
8	0.	999	-7.071E-01	2	1.089E+06	1	0.	999
9	0.	999	-7.441E-01	2	1.114E+06	2	0.	999
10	0.	999	-7.657E-01	2	1.112E+06	3	0.	999
11	0.	999	-7.721E-01	2	1.085E+06	4	0.	999
12	0.	999	-7.638E-01	2	1.034E+06	5	0.	999
13	0.	999	-7.417E-01	2	9.602E+05	6	0.	999
14	0.	999	-7.067E-01	3	8.797E+05	0	0.	999
15	0.	999	-6.604E-01	3	7.803E+05	1	0.	999
16	0.	999	-6.036E-01	3	6.554E+05	2	0.	999
17	0.	999	-5.380E-01	3	5.061E+05	3	-9.202E+04	28
18	0.	999	-4.654E-01	3	3.333E+05	4	-2.270E+05	28
19	0.	999	-3.890E-01	4	1.382E+05	5	-3.764E+05	28
20	1.376E-02	28	-3.120E-01	4	0.	999	-5.403E+05	28
21	2.813E-02	28	-2.368E-01	4	0.	999	-7.191E+05	28
22	3.493E-02	28	-1.659E-01	4	0.	999	-9.135E+05	28
23	3.314E-02	28	-1.016E-01	4	0.	999	-1.124E+06	28

24	2.180E-02	28	-4.577E-02	4	0.	999	-1.352E+06	28
25	0.	999	0.	999	0.	999	-1.619E+06	28
26	3.437E-02	4	-3.331E-02	28	0.	999	-1.338E+06	4
27	5.838E-02	4	-7.690E-02	28	0.	999	-1.122E+06	4
28	7.286E-02	4	-1.293E-01	28	0.	999	-9.231E+05	4
29	7.866E-02	4	-1.887E-01	28	0.	999	-7.403E+05	4
30	7.665E-02	4	-2.536E-01	28	0.	999	-5.732E+05	4
31	6.778E-02	4	-3.218E-01	28	4.513E+04	24	-4.210E+05	4
32	5.305E-02	4	-3.909E-01	28	2.263E+05	25	-2.832E+05	4
33	3.440E-02	4	-4.587E-01	29	3.871E+05	26	-1.598E+05	4
34	1.352E-02	4	-5.226E-01	29	5.268E+05	27	-5.087E+04	4
35	0.	999	-5.802E-01	29	6.450E+05	28	0.	999
36	0.	999	-6.294E-01	29	7.411E+05	29	0.	999
37	0.	999	-6.694E-01	30	8.149E+05	30	0.	999
38	0.	999	-6.989E-01	30	8.662E+05	31	0.	999
39	0.	999	-7.166E-01	30	8.950E+05	32	0.	999
40	0.	999	-7.224E-01	30	9.013E+05	33	0.	999
41	0.	999	-7.169E-01	31	8.853E+05	34	0.	999
42	0.	999	-7.000E-01	31	8.473E+05	35	0.	999
43	0.	999	-6.716E-01	31	7.992E+05	29	0.	999
44	0.	999	-6.325E-01	31	7.296E+05	30	0.	999
45	0.	999	-5.840E-01	32	6.373E+05	31	0.	999
46	1.027E-02	57	-5.278E-01	32	5.226E+05	32	-4.715E+04	57
47	3.128E-02	57	-4.644E-01	32	3.857E+05	33	-1.558E+05	57
48	5.012E-02	57	-3.963E-01	33	2.271E+05	34	-2.789E+05	57
49	6.510E-02	57	-3.269E-01	33	4.745E+04	35	-4.164E+05	57
50	7.429E-02	57	-2.580E-01	33	0.	999	-5.683E+05	57
51	7.666E-02	57	-1.922E-01	33	0.	999	-7.352E+05	57
52	7.129E-02	57	-1.317E-01	33	0.	999	-9.177E+05	57
53	5.728E-02	57	-7.842E-02	33	0.	999	-1.116E+06	57

54	3.380E-02	57	-3.403E-02	33	0.	999	-1.340E+06	32
55	0.	999	0.	999	0.	999	-1.626E+06	33
56	2.247E-02	33	-4.515E-02	57	0.	999	-1.358E+06	33
57	3.442E-02	33	-1.003E-01	57	0.	999	-1.131E+06	33
58	3.677E-02	33	-1.639E-01	57	0.	999	-9.199E+05	33
59	3.046E-02	33	-2.340E-01	57	0.	999	-7.252E+05	33
60	1.651E-02	33	-3.087E-01	57	0.	999	-5.461E+05	33
61	0.	999	-3.856E-01	57	1.313E+05	54	-3.820E+05	33
62	0.	999	-4.619E-01	57	3.292E+05	55	-2.323E+05	33
63	0.	999	-5.352E-01	58	5.061E+05	56	-9.696E+04	33
64	0.	999	-6.024E-01	58	6.611E+05	57	0.	999
65	0.	999	-6.610E-01	58	7.932E+05	58	0.	999
66	0.	999	-7.086E-01	58	9.013E+05	59	0.	999
67	0.	999	-7.439E-01	58	9.846E+05	60	0.	999
68	0.	999	-7.668E-01	59	1.042E+06	61	0.	999
69	0.	999	-7.757E-01	59	1.073E+06	62	0.	999
70	0.	999	-7.701E-01	59	1.090E+06	56	0.	999
71	0.	999	-7.495E-01	59	1.101E+06	57	0.	999
72	0.	999	-7.140E-01	59	1.088E+06	58	0.	999
73	0.	999	-6.636E-01	59	1.049E+06	59	0.	999
74	0.	999	-5.989E-01	60	9.833E+05	60	0.	999
75	0.	999	-5.217E-01	60	8.907E+05	61	0.	999
76	0.	999	-4.326E-01	60	7.703E+05	62	0.	999
77	0.	999	-3.336E-01	60	6.216E+05	63	0.	999
78	0.	999	-2.269E-01	60	4.440E+05	64	0.	999
79	0.	999	-1.148E-01	60	2.369E+05	65	0.	999
80	0.	999	0.	999	7.993E-09	27	-1.599E-08	59
81	1.148E-01	60	0.	999	0.	999	0.	999

TABLE 8B- ENVELOPES OF MAXIMUMS * = HELD FROM PRIOR PROBLEM

STA	MAX +SHEAR LOC	MAX -SHEAR LOC	MAX +REACT LOC	MAX -REACT LOC
-1	0.	999	-6.661E-10	-13
0	9.665E+03	-6	0.	999
1	8.771E+03	-5	0.	999
2	7.882E+03	-4	0.	999
3	6.997E+03	-3	0.	999
4	6.118E+03	-2	0.	999
5	5.244E+03	-1	0.	999
6	4.378E+03	0	0.	999
7	3.481E+03	1	-7.855E+01	-7
8	2.593E+03	2	-9.754E+02	-6
9	1.714E+03	3	-1.869E+03	-5
10	8.449E+02	4	-2.758E+03	-4
11	0.	999	-3.643E+03	-3
12	0.	999	-4.522E+03	-2
13	0.	999	-5.396E+03	-1
14	0.	999	-6.262E+03	0
15	0.	999	-7.159E+03	1
16	0.	999	-8.047E+03	2
17	0.	999	-8.926E+03	3
18	0.	999	-9.795E+03	4
19	0.	999	-1.065E+04	5
20	0.	999	-1.152E+04	6
21	0.	999	-1.241E+04	7
22	0.	999	-1.331E+04	8
23	0.	999	-1.422E+04	9
24	0.	999	-1.414E+04	10
25	1.396E+04	19	0.	999
26	1.336E+04	20	0.	999
27	1.247E+04	21	0.	999
28	1.159E+04	22	0.	999

29					0.	999	0.	999
30	1.072E+04	23	0.	999	0.	999	0.	999
31	9.878E+03	24	0.	999	0.	999	0.	999
32	9.047E+03	25	0.	999	0.	999	0.	999
33	8.210E+03	26	0.	999	0.	999	0.	999
34	7.366E+03	27	0.	999	0.	999	0.	999
35	6.518E+03	28	0.	999	0.	999	0.	999
36	5.666E+03	29	0.	999	0.	999	0.	999
37	4.812E+03	30	0.	999	0.	999	0.	999
38	3.957E+03	31	0.	999	0.	999	0.	999
39	3.103E+03	32	-7.621E+02	24	0.	999	0.	999
40	2.251E+03	33	-1.593E+03	25	0.	999	0.	999
41	1.402E+03	34	-2.430E+03	26	0.	999	0.	999
42	5.588E+02	35	-3.274E+03	27	0.	999	0.	999
43	0.	999	-4.122E+03	28	0.	999	0.	999
44	0.	999	-4.974E+03	29	0.	999	0.	999
45	0.	999	-5.828E+03	30	0.	999	0.	999
46	0.	999	-6.683E+03	31	0.	999	0.	999
47	0.	999	-7.537E+03	32	0.	999	0.	999
48	0.	999	-8.389E+03	33	0.	999	0.	999
49	0.	999	-9.238E+03	34	0.	999	0.	999
50	0.	999	-1.008E+04	35	0.	999	0.	999
51	0.	999	-1.094E+04	36	0.	999	0.	999
52	0.	999	-1.182E+04	37	0.	999	0.	999
53	0.	999	-1.272E+04	38	0.	999	0.	999
54	0.	999	-1.363E+04	39	0.	999	0.	999
55	0.	999	-1.400E+04	40	2.575E+04	46	0.	999
56	1.408E+04	49	0.	999	0.	999	0.	999
57	1.394E+04	50	0.	999	0.	999	0.	999
58	1.305E+04	51	0.	999	0.	999	0.	999
59	1.218E+04	52	0.	999	0.	999	0.	999

60	1.132E+04	53	0.	999	0.	999	0.	999
61	1.048E+04	54	0.	999	0.	999	0.	999
62	9.643E+03	55	0.	999	0.	999	0.	999
63	8.797E+03	56	0.	999	0.	999	0.	999
64	7.941E+03	57	0.	999	0.	999	0.	999
65	7.074E+03	58	0.	999	0.	999	0.	999
66	6.197E+03	59	0.	999	0.	999	0.	999
67	5.312E+03	60	0.	999	0.	999	0.	999
68	4.418E+03	61	0.	999	0.	999	0.	999
69	3.516E+03	62	-1.638E+02	54	0.	999	0.	999
70	2.606E+03	63	-9.975E+02	55	0.	999	0.	999
71	1.691E+03	64	-1.843E+03	56	0.	999	0.	999
72	7.700E+02	65	-2.699E+03	57	0.	999	0.	999
73	0.	999	-3.566E+03	58	0.	999	0.	999
74	0.	999	-4.443E+03	59	0.	999	0.	999
75	0.	999	-5.328E+03	60	0.	999	0.	999
76	0.	999	-6.222E+03	61	0.	999	0.	999
77	0.	999	-7.124E+03	62	0.	999	0.	999
78	0.	999	-8.034E+03	63	0.	999	0.	999
79	0.	999	-8.949E+03	64	0.	999	0.	999
80	0.	999	-9.870E+03	65	1.050E+04	66	0.	999
81	6.661E-10	59	-3.331E-10	27	0.	999	0.	999

TABLE 9 -- SCALES FOR PLOTS OF THE ENVELOPES OF MAXIMUMS

HORIZONTAL SCALE
10 INCHES = 100 STATIONS

VERTICAL SCALES

VARIABLE	LENGTH OF AXIS	MAXIMUM VALUE
DEFLECT	2 INCHES	= 1.000E+00
MOMENT	2 INCHES	= 2.000E+06
SHEAR	2 INCHES	= 2.000E+04
REACTION	2 INCHES	= 4.000E+04

PROB (CONT'D)

2 THREE SPAN STRUCTURE, FIXED LOADS PLUS MOVING LOADS

TABLE 10A -- INFLUENCE DIAGRAMS FOR DEFLECTION

LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TABLE 10B -- INFLUENCE DIAGRAMS FOR MOMENT

LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TABLE 10C -- INFLUENCE DIAGRAMS FOR SHEAR

(SHEAR IS COMPUTED ONE HALF INCREMENT
TO THE LEFT OF THE DESIGNATED STATION)

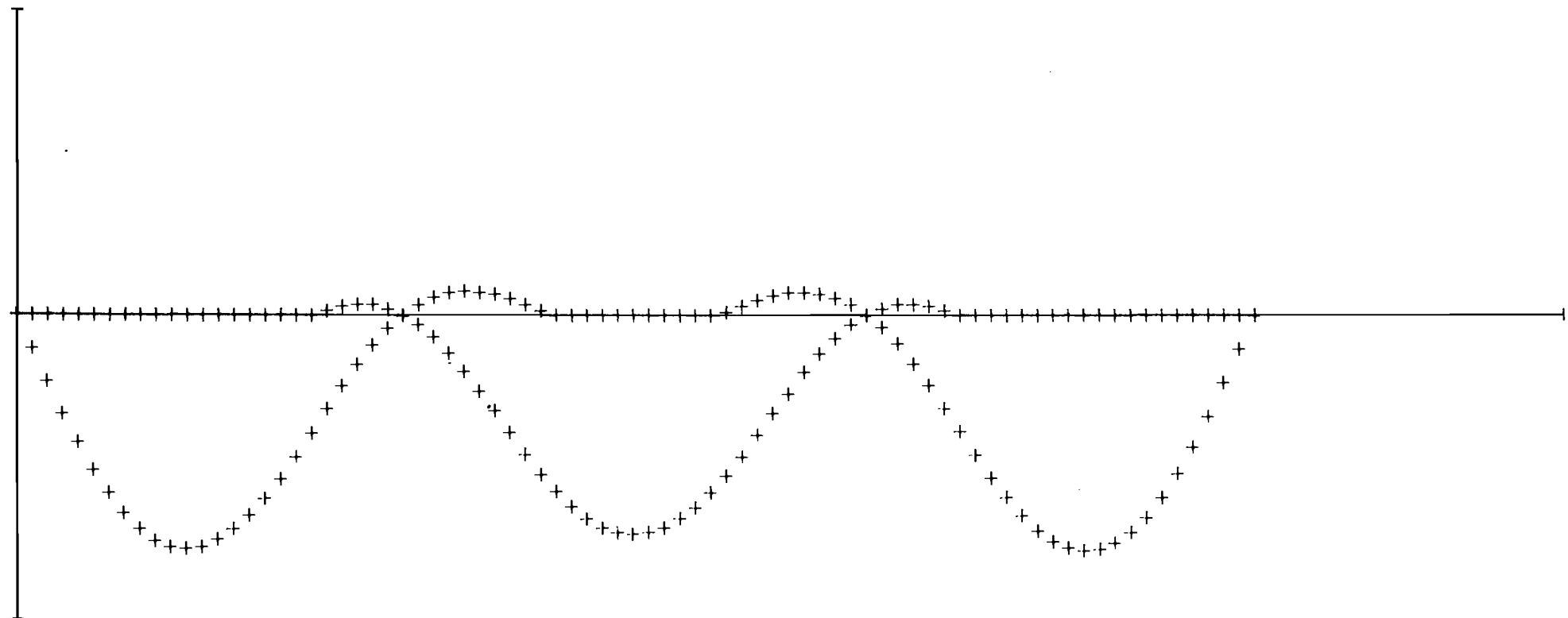
LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TABLE 10D -- INFLUENCE DIAGRAMS FOR SUPPORT REACTION

LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TIME FOR THIS PROBLEM = 0 MINUTES 5.081 SECONDS

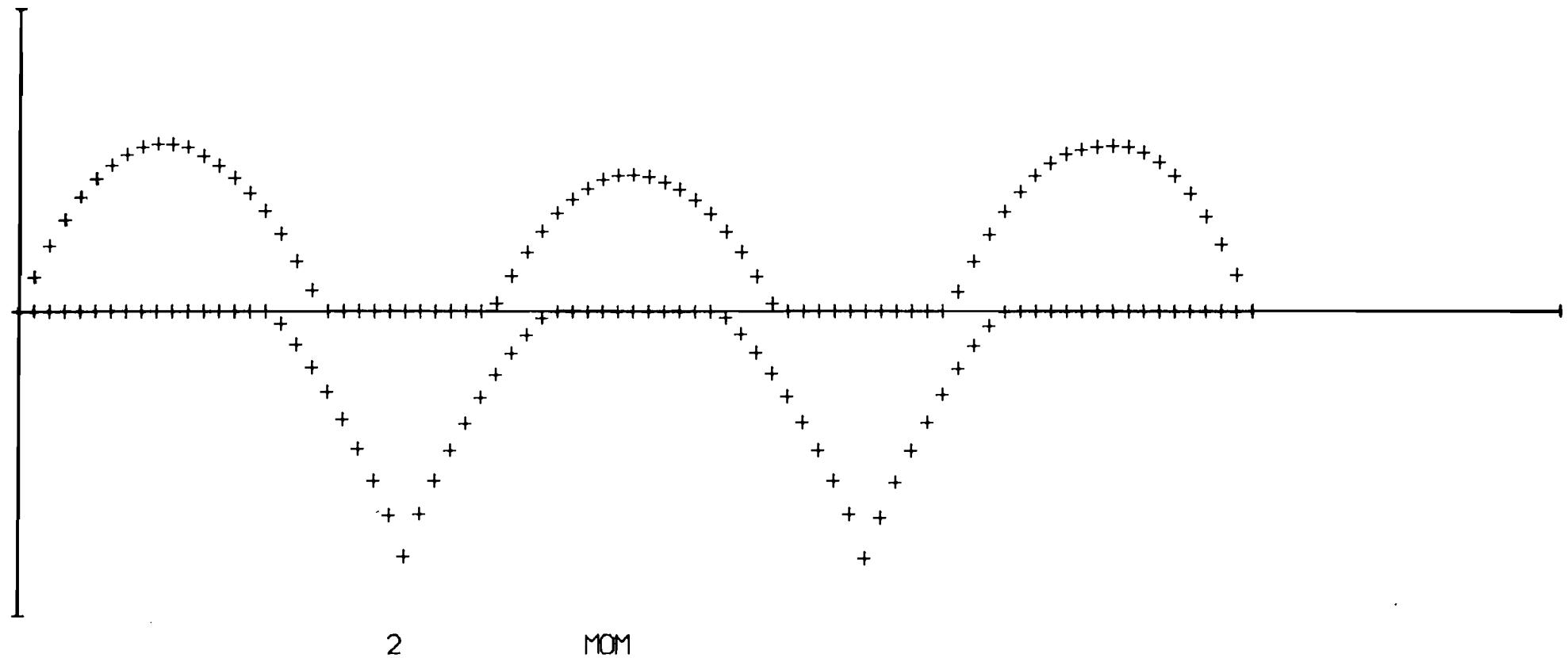
ELAPSED CPU TIME = 0 MINUTES 22.358 SECONDS



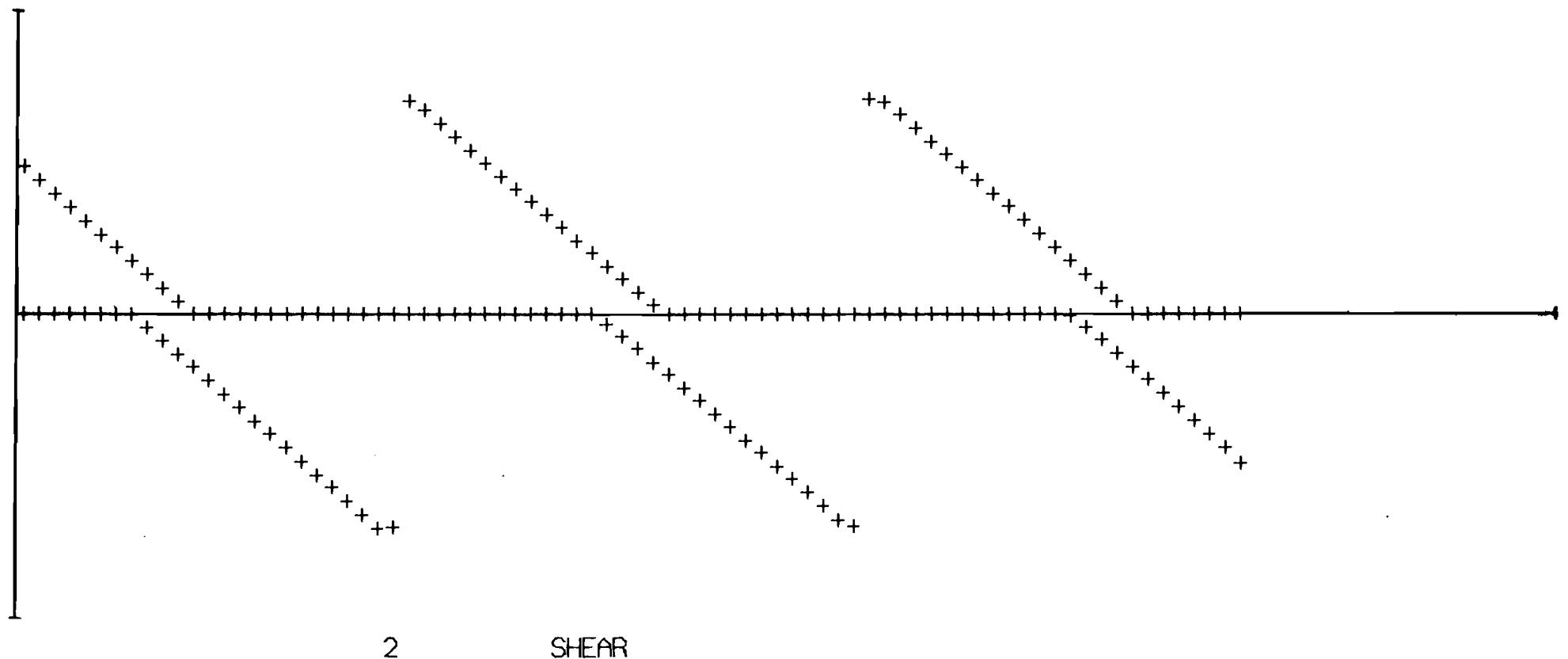
2

DEFL.

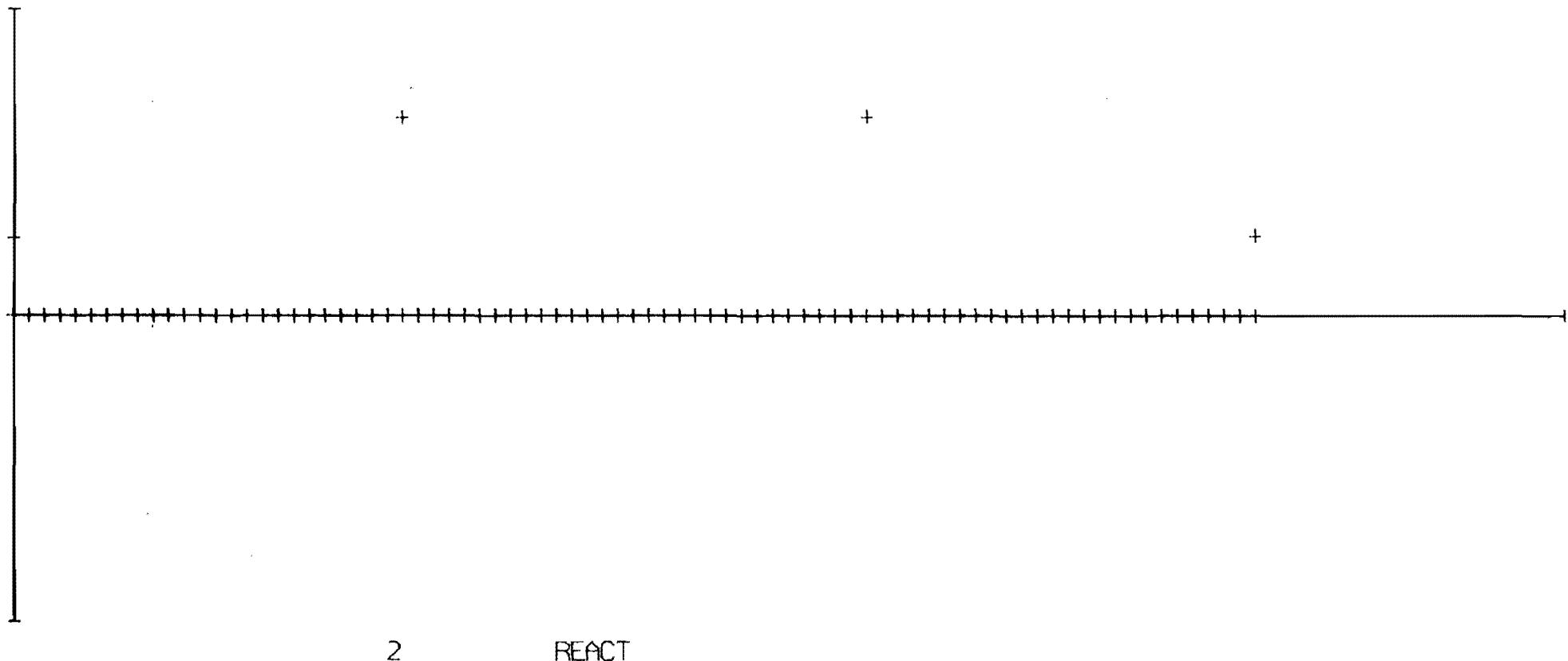
Prob 2. Three-span structure, fixed loads plus moving loads.
Envelope of maximum deflections.



Prob 2. Three-span structure, fixed loads plus moving loads.
Envelope of maximum moments.



Prob 2. Three-span structure, fixed loads plus moving loads.
Envelope of maximum shears.



2

REACT

Prob 2. Three-span structure, fixed loads plus moving loads.
Envelope of maximum reactions.

PROGRAM BMCOL 43 - - MATLOCK-TAYLOR - REVISION DATE = 08 MAR 68
 CHG CE244010 CODED BY JJP RUN ON 23 APR 68
 EXAMPLE PROBLEMS FOR BMCOL43 FINAL REPORT

PROB
 3 THREE SPAN STRUCTURE, ALL PREVIOUS LOADS PLUS SUPPORT SETTLEMENTS

TABLE 1 - PROGRAM-CONTROL DATA

	ENVELOPES OF MAXIMUMS	TABLE NUMBER				
	2	3	4	5	6	
HOLD FROM PRECEDING PROBLEM (1=HOLD)	1	1	0	1	1	0
NUM CARDS INPJT THIS PROBLEM	0	4	0	0	0	0
OPTION (IF=1) TO PLOT ENVELOPES OF MAXIMUMS		DEFL	MOM	SHR	RCT	
		1	1	1	1	

TABLE 2 - CONSTANTS

USING DATA FROM THE PREVIOUS PROBLEM	
NUM INCREMENTS	80
INCREMENT LENGTH	2.400E+01
NUMBER OF INCREMENTS FOR MOBILE LOAD	14
INITIAL POSITION OF MOBILE LOAD STA ZERO	-14
FINAL POSITION OF MOBILE LOAD STA ZERO	80
NUMBER OF INCREMENTS BETWEEN EACH POSITION OF MOBILE LOAD	1

TABLE 3 - SPECIFIED DEFLECTIONS AND SLOPES

STA	CASE	DEFLECTION	SLOPE
0	1	0.	NONE
25	1	-1.250E+00	NONE
55	1	-6.250E-01	NONE
80	1	0.	NONE

TABLE 4 - STIFFNESS AND FIXED-LOAD DATA

FROM	TO	CONTD	F	QF	S	T	R	P
USING DATA FROM THE PREVIOUS PROBLEM								
0	80	-0	4.147E+10	-6.000E+02	-0.	-0.	-0.	-0.
19	1	0.	0.	-0.	-0.	-0.	-0.	-0.
25	1	1	3.953E+10	-1.500E+02	-0.	-0.	-0.	-0.
31	-0	0.	0.	-0.	-0.	-0.	-0.	-0.
49	1	0.	0.	-0.	-0.	-0.	-0.	-0.
55	1	1	3.953E+10	-1.500E+02	-0.	-0.	-0.	-0.
61	-0	0.	0.	-0.	-0.	-0.	-0.	-0.
25	25	-0.	-0.	-0.	-0.	-0.	3.873E+07	-0.
55	55	-0.	-0.	-0.	-0.	-0.	3.873E+07	-0.
ADDITIONAL DATA FOR THIS PROBLEM								
NONE								

TABLE 5 - MOVABLE-LOAD DATA

FROM TO CONTD QM

USING DATA FROM THE PREVIOUS PROBLEM

0 0 -0 -7.300E+02

7 7 -0 -2.920E+03

14 14 -0 -2.920E+03

ADDITIONAL DATA FOR THIS PROBLEM

NONE

TABLE 6 - SPECIFIED STATIONS FOR INFLUENCE DIAGRAMS

(SHEAR IS COMPUTED ONE HALF INCREMENT
TO THE LEFT OF THE DESIGNATED STATION)

NONE

PROGRAM BMCOL 43 - - MATLOCK-TAYLOR - REVISION DATE = 08 MAR 68
 CHG CE244010 CODED BY JJP RUN ON 23 APR 68
 EXAMPLE PROBLEMS FOR BMCOL43 FINAL REPORT

PROB (CONT'D)

3 THREE SPAN STRUCTURE, ALL PREVIOUS LOADS PLUS SUPPORT SETTLEMENTS

TABLE 7 - FIXED-LOAD RESULTS

STA I	DIST	DEFL	SLOPE	MOM	SHEAR	SUP REACT
-1	-2.400E+01	1.323E-01	-5.514E-03	0.	-1.332E-09	-0.
0	0.	0.	-5.514E-03	-3.197E-08	5.811E+03	6.111E+03
1	2.400E+01	-1.323E-01	-5.434E-03	1.395E+05	5.211E+03	-0.
2	4.800E+01	-2.627E-01	-5.281E-03	2.645E+05	4.611E+03	-0.
3	7.200E+01	-3.895E-01	-5.063E-03	3.752E+05	4.011E+03	-0.
4	9.600E+01	-5.110E-01	-4.791E-03	4.714E+05	3.411E+03	-0.
5	1.200E+02	-6.260E-01	-4.470E-03	5.533E+05	2.811E+03	-0.
6	1.440E+02	-7.333E-01	-4.111E-03	6.207E+05	2.211E+03	-0.
7	1.680E+02	-8.319E-01	-3.721E-03	6.738E+05	1.611E+03	-0.
8	1.920E+02	-9.212E-01	-3.309E-03	7.125E+05	1.011E+03	-0.
9	2.160E+02	-1.001E+00	-2.882E-03	7.367E+05	4.107E+02	-0.
10	2.400E+02	-1.070E+00	-2.450E-03	7.466E+05	-1.893E+02	-0.
11	2.640E+02	-1.129E+00	-2.021E-03	7.420E+05	-7.893E+02	-0.
12	2.880E+02	-1.177E+00	-1.602E-03	7.231E+05	-1.389E+03	-0.
13	3.120E+02	-1.216E+00	-1.203E-03	6.897E+05	-1.989E+03	-0.
14	3.360E+02	-1.244E+00	-8.318E-04	6.420E+05	-2.589E+03	-0.
15	3.600E+02	-1.264E+00	-4.962E-04	5.799E+05	-3.189E+03	-0.
16	3.840E+02	-1.276E+00	-2.049E-04	5.033E+05	-3.789E+03	-0.
17	4.080E+02	-1.281E+00	3.376E-05	4.124E+05	-4.389E+03	-0.
18	4.320E+02	-1.280E+00	2.114E-04	3.070E+05	-4.989E+03	-0.
19	4.560E+02	-1.275E+00	3.198E-04	1.873E+05	-5.589E+03	-0.
20	4.800E+02	-1.268E+00	3.464E-04	5.315E+04	-6.214E+03	-0.
21	5.040E+02	-1.259E+00	3.042E-04	-9.600E+04	-6.864E+03	-0.

22	5.280E+02	-1.252E+00		-2.607E+05		-0.
23	5.520E+02	-1.247E+00	2.020E-04	-4.417E+05	-7.539E+03	-0.
24	5.760E+02	-1.246E+00	4.573E-05	-6.394E+05	-8.239E+03	-0.
25	6.000E+02	-1.250E+00	-1.605E-04	-8.601E+05	-9.197E+03	-0.
26	6.240E+02	-1.260E+00	-4.154E-04	-6.637E+05	8.185E+03	1.813E+04
27	6.480E+02	-1.275E+00	-6.294E-04	-4.791E+05	7.692E+03	-0.
28	6.720E+02	-1.294E+00	-7.989E-04	-3.113E+05	6.992E+03	-0.
29	6.960E+02	-1.316E+00	-9.209E-04	-1.597E+05	6.317E+03	-0.
30	7.200E+02	-1.340E+00	-9.911E-04	-2.364E+04	5.667E+03	-0.
31	7.440E+02	-1.364E+00	-1.003E-03	9.738E+04	5.042E+03	-0.
32	7.680E+02	-1.387E+00	-9.465E-04	2.040E+05	4.442E+03	-0.
33	7.920E+02	-1.407E+00	-8.284E-04	2.962E+05	3.842E+03	-0.
34	8.160E+02	-1.423E+00	-6.570E-04	3.740E+05	3.242E+03	-0.
35	8.400E+02	-1.433E+00	-4.406E-04	4.374E+05	2.642E+03	-0.
36	8.640E+02	-1.438E+00	-1.874E-04	4.865E+05	2.042E+03	-0.
37	8.880E+02	-1.435E+00	9.414E-05	5.211E+05	1.442E+03	-0.
38	9.120E+02	-1.426E+00	3.957E-04	5.413E+05	8.424E+02	-0.
39	9.360E+02	-1.409E+00	7.090E-04	5.471E+05	2.424E+02	-0.
40	9.600E+02	-1.384E+00	1.026E-03	5.385E+05	-3.576E+02	-0.
41	9.840E+02	-1.352E+00	1.337E-03	5.155E+05	-9.576E+02	-0.
42	1.008E+03	-1.313E+00	1.636E-03	4.782E+05	-1.558E+03	-0.
43	1.032E+03	-1.267E+00	1.912E-03	4.264E+05	-2.158E+03	-0.
44	1.056E+03	-1.215E+00	2.159E-03	3.602E+05	-2.758E+03	-0.
45	1.080E+03	-1.158E+00	2.368E-03	2.796E+05	-3.358E+03	-0.
46	1.104E+03	-1.098E+00	2.529E-03	1.846E+05	-3.958E+03	-0.
47	1.128E+03	-1.034E+00	2.636E-03	7.525E+04	-4.558E+03	-0.
48	1.152E+03	-9.701E-01	2.680E-03	-4.854E+04	-5.158E+03	-0.
49	1.176E+03	-9.064E-01	2.652E-03	-1.867E+05	-5.758E+03	-0.
50	1.200E+03	-8.454E-01	2.544E-03	-3.393E+05	-6.358E+03	-0.
51	1.224E+03	-7.884E-01	2.374E-03	-5.069E+05	-6.983E+03	-0.
52	1.248E+03	-7.368E-01	2.152E-03	-6.901E+05	-7.633E+03	-0.

53	1.272E+03	-6.916E-01	1.881E-03	-8.308E+03	-0.
54	1.296E+03	-6.540E-01	1.566E-03	-9.008E+03	-0.
55	1.320E+03	-6.250E-01	1.210E-03	-1.106E+06	-0.
56	1.344E+03	-6.053E-01	8.188E-04	-8.914E+03	2.019E+04
57	1.368E+03	-5.940E-01	4.746E-04	-1.052E+04	-0.
58	1.392E+03	-5.898E-01	1.734E-04	8.982E+03	-0.
59	1.416E+03	-5.918E-01	-8.244E-05	8.282E+03	-0.
60	1.440E+03	-5.987E-01	-2.889E-04	-6.527E+05	-0.
61	1.464E+03	-6.093E-01	-4.403E-04	7.607E+03	-0.
62	1.488E+03	-6.219E-01	-5.278E-04	6.957E+03	-0.
63	1.512E+03	-6.348E-01	-5.357E-04	6.332E+03	-0.
64	1.536E+03	-6.461E-01	-4.723E-04	5.732E+03	-0.
65	1.560E+03	-6.544E-01	-3.460E-04	5.132E+03	-0.
66	1.584E+03	-6.584E-01	-1.650E-04	4.532E+03	-0.
67	1.608E+03	-6.569E-01	6.216E-05	3.926E+05	-0.
68	1.632E+03	-6.491E-01	3.273E-04	2.732E+03	-0.
69	1.656E+03	-6.341E-01	6.221E-04	2.132E+03	-0.
70	1.680E+03	-6.116E-01	9.381E-04	1.532E+03	-0.
71	1.704E+03	-5.812E-01	1.267E-03	9.316E+02	-0.
72	1.728E+03	-5.428E-01	1.601E-03	5.684E+05	-0.
73	1.752E+03	-4.965E-01	1.930E-03	3.316E+02	-0.
74	1.776E+03	-4.425E-01	2.248E-03	-2.684E+02	-0.
75	1.800E+03	-3.814E-01	2.546E-03	5.699E+05	-0.
76	1.824E+03	-3.139E-01	2.814E-03	-8.684E+02	-0.
77	1.848E+03	-2.408E-01	3.046E-03	5.491E+05	-0.
78	1.872E+03	-1.632E-01	3.232E-03	-1.468E+03	-0.
79	1.896E+03	-8.244E-02	3.365E-03	5.139E+05	-0.
80	1.920E+03	0.	3.435E-03	-2.068E+03	-0.
81	1.944E+03	8.244E-02	0.	-3.268E+03	-0.
			0.	-3.868E+03	-0.
			0.	-4.468E+03	-0.
			0.	-5.068E+03	-0.
			0.	5.368E+03	-0.
			0.	-0.	-0.

PROB (CONT'D)

3 THREE SPAN STRUCTURE, ALL PREVIOUS LOADS PLUS SUPPORT SETTLEMENTS

TABLE 8A- ENVELOPES OF MAXIMUMS * = HELD FROM PRIOR PROBLEM

STA	MAX +DEFL	LOC	MAX -DEFL	LOC	MAX +MOM	LOC	MAX -MOM	LOC
-1	1.842E-01	1	0.	999*	0.	999*	0.	999*
0	0.	999*	0.	999*	0.	999*	-3.197E-08	999
1	0.	999*	-1.842E-01	1	2.472E+05	-6	0.	999*
2	0.	999*	-3.654E-01	1	4.659E+05	-5	0.	999*
3	0.	999*	-5.409E-01	1	6.564E+05	-4	0.	999*
4	0.	999*	-7.086E-01	1	8.191E+05	-3	0.	999*
5	0.	999*	-8.661E-01	1	9.543E+05	-2	0.	999*
6	0.	999*	-1.011E+00	1	1.063E+06	-1	0.	999*
7	0.	999*	-1.143E+00	1	1.145E+06	0	0.	999*
8	0.	999*	-1.260E+00	2	1.211E+06	1	0.	999*
9	0.	999*	-1.360E+00	2	1.251E+06	2	0.	999*
10	0.	999*	-1.444E+00	2	1.264E+06	3	0.	999*
11	0.	999*	-1.510E+00	2	1.253E+06	4	0.	999*
12	0.	999*	-1.559E+00	2	1.217E+06	5	0.	999*
13	0.	999*	-1.592E+00	2	1.158E+06	6	0.	999*
14	0.	999*	-1.609E+00	3	1.093E+06	0	0.	999*
15	0.	999*	-1.611E+00	3	1.009E+06	1	0.	999*
16	0.	999*	-1.601E+00	3	8.993E+05	2	0.	999*
17	0.	999*	-1.578E+00	3	7.652E+05	3	-9.202E+04	28*
18	0.	999*	-1.544E+00	3	6.077E+05	4	-2.270E+05	28*
19	0.	999*	-1.503E+00	4	4.279E+05	5	-3.764E+05	28*
20	1.376E-02	28*	-1.457E+00	4	2.269E+05	6	-5.403E+05	28*
21	2.813E-02	28*	-1.409E+00	4	4.947E+03	7	-7.191E+05	28*
22	3.493E-02	28*	-1.362E+00	4	0.	999*	-9.135E+05	28*
23	3.314E-02	28*	-1.319E+00	4	0.	999*	-1.124E+06	28*

24	2.180E-02	28*	-1.281E+00	4	0.	999*	-1.352E+06	28*
25	0.	999*	-1.250E+00	999	0.	999*	-1.619E+06	28*
26	3.437E-02	4*	-1.296E+00	28	0.	999*	-1.338E+06	4*
27	5.838E-02	4*	-1.349E+00	28	0.	999*	-1.122E+06	4*
28	7.286E-02	4*	-1.409E+00	28	0.	999*	-9.231E+05	4*
29	7.866E-02	4*	-1.472E+00	28	0.	999*	-7.403E+05	4*
30	7.665E-02	4*	-1.538E+00	28	1.244E+05	23	-5.732E+05	4*
31	6.778E-02	4*	-1.604E+00	28	3.093E+05	24	-4.210E+05	4*
32	5.305E-02	4*	-1.667E+00	28	4.746E+05	25	-2.832E+05	4*
33	3.440E-02	4*	-1.725E+00	29	6.197E+05	26	-1.598E+05	4*
34	1.352E-02	4*	-1.776E+00	29	7.436E+05	27	-5.087E+04	4*
35	0.	999*	-1.818E+00	29	8.460E+05	28	0.	999*
36	0.	999*	-1.849E+00	29	9.264E+05	29	0.	999*
37	0.	999*	-1.868E+00	30	9.844E+05	30	0.	999*
38	0.	999*	-1.874E+00	30	1.020E+06	31	0.	999*
39	0.	999*	-1.866E+00	30	1.033E+06	32	0.	999*
40	0.	999*	-1.844E+00	30	1.023E+06	33	0.	999*
41	0.	999*	-1.810E+00	31	9.916E+05	34	0.	999*
42	0.	999*	-1.762E+00	31	9.379E+05	35	0.	999*
43	0.	999*	-1.702E+00	31	8.740E+05	29	0.	999*
44	0.	999*	-1.630E+00	31	7.886E+05	30	0.	999*
45	0.	999*	-1.547E+00	32	6.805E+05	31	0.	999*
46	1.027E-02	57*	-1.457E+00	32	5.500E+05	32	-4.715E+04	57*
47	3.128E-02	57*	-1.358E+00	32	3.973E+05	33	-1.558E+05	57*
48	5.012E-02	57*	-1.255E+00	33	2.271E+05	34*	-2.831E+05	57
49	6.510E-02	57*	-1.151E+00	33	4.745E+04	35*	-4.364E+05	57
50	7.429E-02	57*	-1.048E+00	33	0.	999*	-6.041E+05	57
51	7.666E-02	57*	-9.476E-01	33	0.	999*	-7.867E+05	57
52	7.129E-02	57*	-8.535E-01	33	0.	999*	-9.850E+05	57
53	5.728E-02	57*	-7.672E-01	33	0.	999*	-1.200E+06	57

54	3.380E-02	57*	-6.906E-01	33	0.	999*	-1.439E+06	32
55	0.	999*	-6.250E-01	999	0.	999*	-1.716E+06	33
56	2.247E-02	33*	-6.393E-01	57	0.	999*	-1.420E+06	33
57	3.442E-02	33*	-6.640E-01	57	0.	999*	-1.190E+06	33
58	3.677E-02	33*	-6.976E-01	57	0.	999*	-9.765E+05	33
59	3.046E-02	33*	-7.384E-01	57	0.	999*	-7.792E+05	33
60	1.651E-02	33*	-7.843E-01	57	0.	999*	-5.975E+05	33
61	0.	999*	-8.330E-01	57	1.313E+05	54*	-4.308E+05	33
62	0.	999*	-8.818E-01	57	3.292E+05	55*	-2.786E+05	33
63	0.	999*	-9.282E-01	58	5.061E+05	56*	-1.407E+05	33
64	0.	999*	-9.692E-01	58	6.611E+05	57*	-1.720E+04	33
65	0.	999*	-1.002E+00	58	7.932E+05	58*	0.	999*
66	0.	999*	-1.024E+00	58	9.013E+05	59*	0.	999*
67	0.	999*	-1.035E+00	58	9.846E+05	60*	0.	999*
68	0.	999*	-1.034E+00	59	1.042E+06	61*	0.	999*
69	0.	999*	-1.019E+00	59	1.073E+06	62*	0.	999*
70	0.	999*	-9.900E-01	59	1.090E+06	56*	0.	999*
71	0.	999*	-9.465E-01	59	1.101E+06	57*	0.	999*
72	0.	999*	-8.883E-01	59	1.088E+06	58*	0.	999*
73	0.	999*	-8.155E-01	59	1.049E+06	59*	0.	999*
74	0.	999*	-7.286E-01	60	9.833E+05	60*	0.	999*
75	0.	999*	-6.294E-01	60	8.907E+05	61*	0.	999*
76	0.	999*	-5.186E-01	60	7.703E+05	62*	0.	999*
77	0.	999*	-3.980E-01	60	6.216E+05	63*	0.	999*
78	0.	999*	-2.698E-01	60	4.440E+05	64*	0.	999*
79	0.	999*	-1.362E-01	60	2.369E+05	65*	0.	999*
80	0.	999*	0.	999*	1.599E-08	0	-1.599E-08	59*
81	1.362E-01	60	0.	999*	0.	999*	0.	999*

TABLE 8B- ENVELOPES OF MAXIMUMS * = HELD FROM PRIOR PROBLEM

STA	MAX +SHEAR LOC	MAX -SHEAR LOC	MAX +REACT LOC	MAX -REACT LOC
-1	0.	999*	-1.332E-09	999
0	1.030E+04	-6	0.	999*
1	9.406E+03	-5	0.	999*
2	8.517E+03	-4	0.	999*
3	7.632E+03	-3	0.	999*
4	6.753E+03	-2	0.	999*
5	5.880E+03	-1	0.	999*
6	5.013E+03	0	0.	999*
7	4.117E+03	1	-7.855E+01	-7*
8	3.228E+03	2	-9.754E+02	-6*
9	2.349E+03	3	-1.869E+03	-5*
10	1.480E+03	4	-2.758E+03	-4*
11	6.220E+02	5	-3.643E+03	-3*
12	0.	999*	-4.522E+03	-2*
13	0.	999*	-5.396E+03	-1*
14	0.	999*	-6.262E+03	0*
15	0.	999*	-7.159E+03	1*
16	0.	999*	-8.047E+03	2*
17	0.	999*	-8.926E+03	3*
18	0.	999*	-9.795E+03	4*
19	0.	999*	-1.065E+04	5*
20	0.	999*	-1.152E+04	6*
21	0.	999*	-1.241E+04	7*
22	0.	999*	-1.331E+04	8*
23	0.	999*	-1.422E+04	9*
24	0.	999*	-1.414E+04	10*
25	1.396E+04	19*	0.	999*
26	1.336E+04	20*	0.	999*
27	1.247E+04	21*	0.	999*
28	1.159E+04	22*	0.	999*

29		1.072E+04	23*	0.	999*	0.	999*	0.	999*
30		9.878E+03	24*	0.	999*	0.	999*	0.	999*
31		9.047E+03	25*	0.	999*	0.	999*	0.	999*
32		8.210E+03	26*	0.	999*	0.	999*	0.	999*
33		7.366E+03	27*	0.	999*	0.	999*	0.	999*
34		6.518E+03	28*	0.	999*	0.	999*	0.	999*
35		5.666E+03	29*	0.	999*	0.	999*	0.	999*
36		4.812E+03	30*	0.	999*	0.	999*	0.	999*
37		3.957E+03	31*	-5.979E+02	23	0.	999*	0.	999*
38		3.103E+03	32*	-1.420E+03	24	0.	999*	0.	999*
39		2.251E+03	33*	-2.250E+03	25	0.	999*	0.	999*
40		1.402E+03	34*	-3.088E+03	26	0.	999*	0.	999*
41		5.588E+02	35*	-3.932E+03	27	0.	999*	0.	999*
42		0.	999*	-4.780E+03	28	0.	999*	0.	999*
43		0.	999*	-5.632E+03	29	0.	999*	0.	999*
44		0.	999*	-6.486E+03	30	0.	999*	0.	999*
45		0.	999*	-7.340E+03	31	0.	999*	0.	999*
46		0.	999*	-8.195E+03	32	0.	999*	0.	999*
47		0.	999*	-9.047E+03	33	0.	999*	0.	999*
48		0.	999*	-9.895E+03	34	0.	999*	0.	999*
49		0.	999*	-1.074E+04	35	0.	999*	0.	999*
50		0.	999*	-1.160E+04	36	0.	999*	0.	999*
51		0.	999*	-1.248E+04	37	0.	999*	0.	999*
52		0.	999*	-1.338E+04	38	0.	999*	0.	999*
53		0.	999*	-1.429E+04	39	0.	999*	0.	999*
54		0.	999*	-1.400E+04	40*	0.	999*	0.	999*
55		1.523E+04	49	0.	999*	2.652E+04	46	0.	999*
56		1.405E+04	50	0.	999*	0.	999*	0.	999*
57		1.316E+04	51	0.	999*	0.	999*	0.	999*
58		1.229E+04	52	0.	999*	0.	999*	0.	999*
59						0.	999*	0.	999*

	1.143E+04	53	0.	999*	0.	999*	0.	999*
60	1.058E+04	54	0.	999*	0.	999*	0.	999*
61	9.750E+03	55	0.	999*	0.	999*	0.	999*
62	8.904E+03	56	0.	999*	0.	999*	0.	999*
63	8.048E+03	57	0.	999*	0.	999*	0.	999*
64	7.181E+03	58	0.	999*	0.	999*	0.	999*
65	6.305E+03	59	0.	999*	0.	999*	0.	999*
66	5.419E+03	60	0.	999*	0.	999*	0.	999*
67	4.525E+03	61	0.	999*	0.	999*	0.	999*
68	3.623E+03	62	-1.638E+02	54*	0.	999*	0.	999*
69	2.714E+03	63	-9.975E+02	55*	0.	999*	0.	999*
70	1.798E+03	64	-1.843E+03	56*	0.	999*	0.	999*
71	8.771E+02	65	-2.699E+03	57*	0.	999*	0.	999*
72	0.	999*	-3.566E+03	58*	0.	999*	0.	999*
73	0.	999*	-4.443E+03	59*	0.	999*	0.	999*
74	0.	999*	-5.328E+03	60*	0.	999*	0.	999*
75	0.	999*	-6.222E+03	61*	0.	999*	0.	999*
76	0.	999*	-7.124E+03	62*	0.	999*	0.	999*
77	0.	999*	-8.034E+03	63*	0.	999*	0.	999*
78	0.	999*	-8.949E+03	64*	0.	999*	0.	999*
79	0.	999*	-9.870E+03	65*	0.	999*	0.	999*
80	6.661E-10	59*	-6.661E-10	0	1.050E+04	66*	0.	999*
81					0.	999*	0.	999*

TABLE 9 -- SCALES FOR PLOTS OF THE ENVELOPES OF MAXIMUMS

HORIZONTAL SCALE
10 INCHES = 100 STATIONS

VERTICAL SCALES

VARIABLE	LENGTH OF AXIS	MAXIMUM VALUE
DEFLECT	2 INCHES	= 2.000E+00
MOMENT	2 INCHES	= 2.000E+06
SHEAR	2 INCHES	= 2.000E+04
REACTION	2 INCHES	= 4.000E+04

PROB (CONTD)
 3 THREE SPAN STRUCTURE, ALL PREVIOUS LOADS PLUS SUPPORT SETTLEMENTS

TABLE 10A -- INFLUENCE DIAGRAMS FOR DEFLECTION

LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TABLE 10B -- INFLUENCE DIAGRAMS FOR MOMENT

LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TABLE 10C -- INFLUENCE DIAGRAMS FOR SHEAR
 (SHEAR IS COMPUTED ONE HALF INCREMENT
 TO THE LEFT OF THE DESIGNATED STATION)

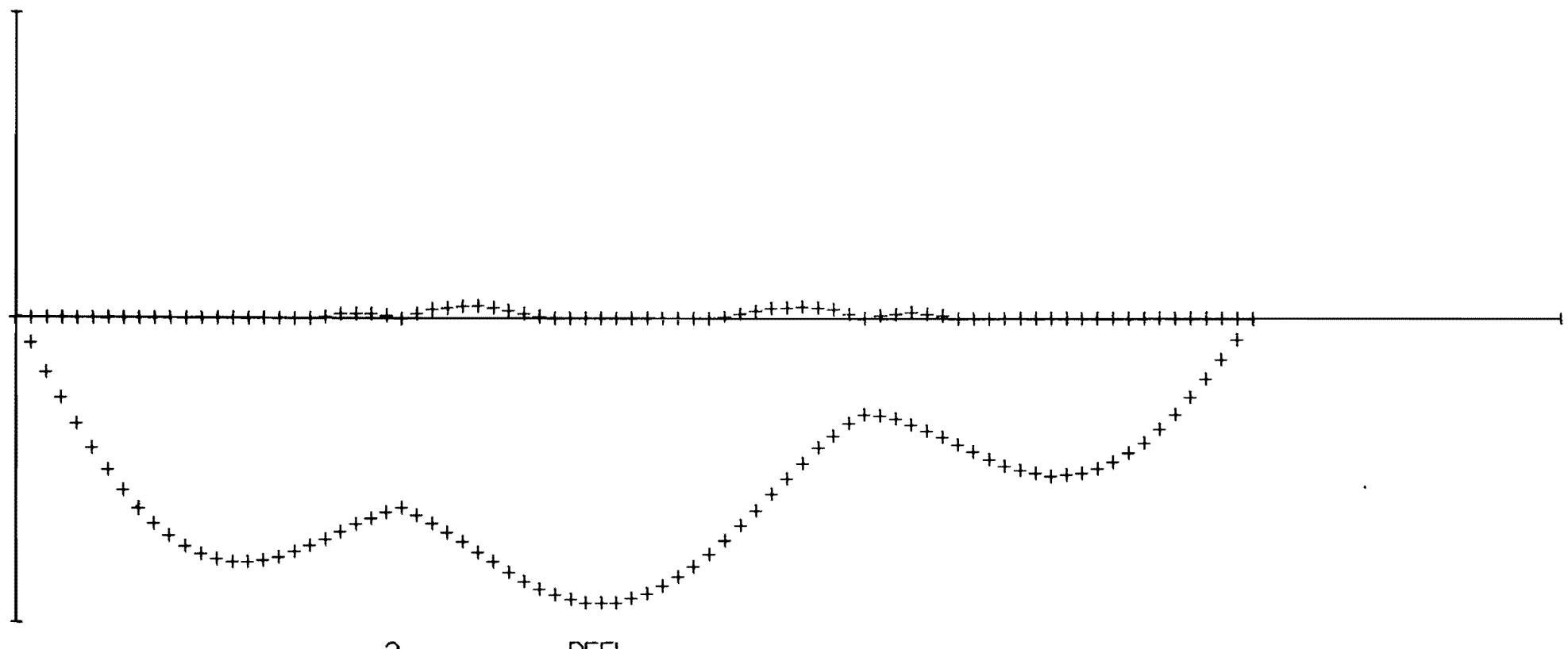
LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TABLE 10D -- INFLUENCE DIAGRAMS FOR SUPPORT REACTION

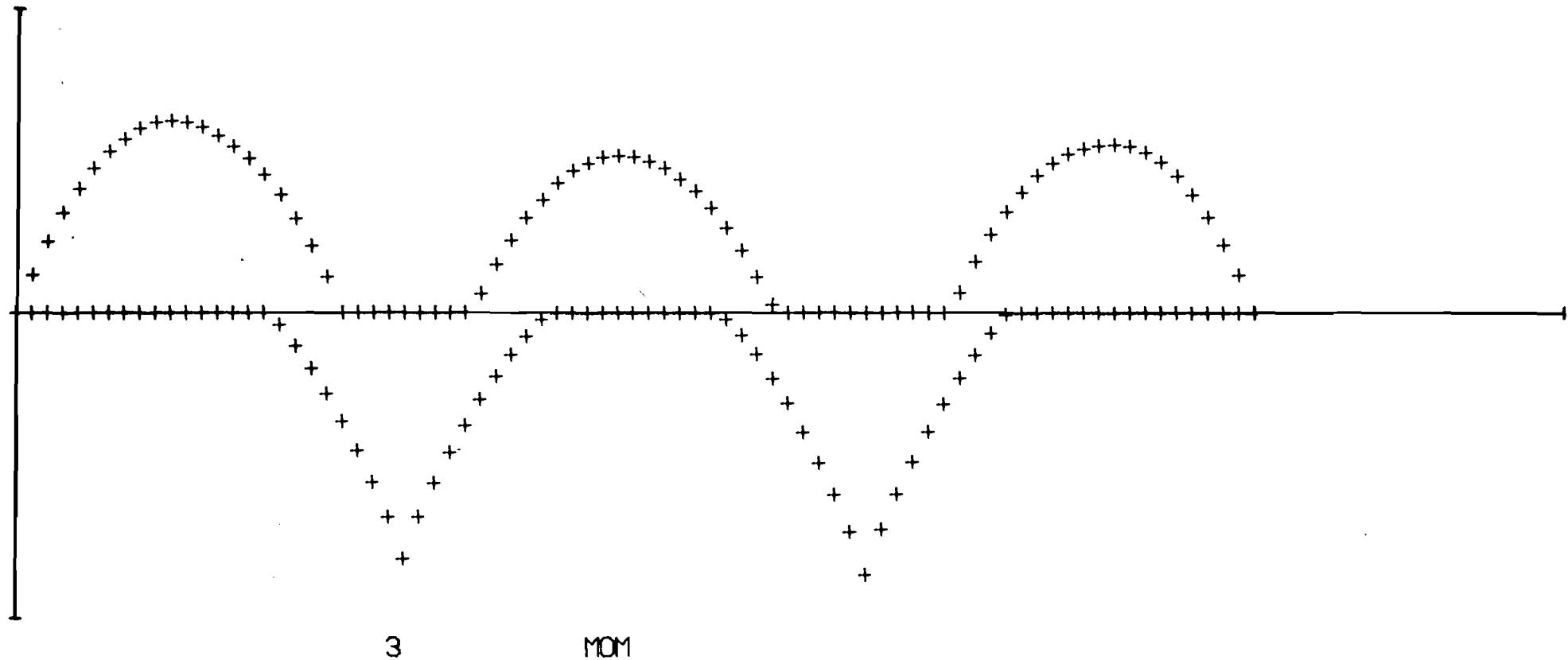
LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TIME FOR THIS PROBLEM = 0 MINUTES 5.083 SECONDS

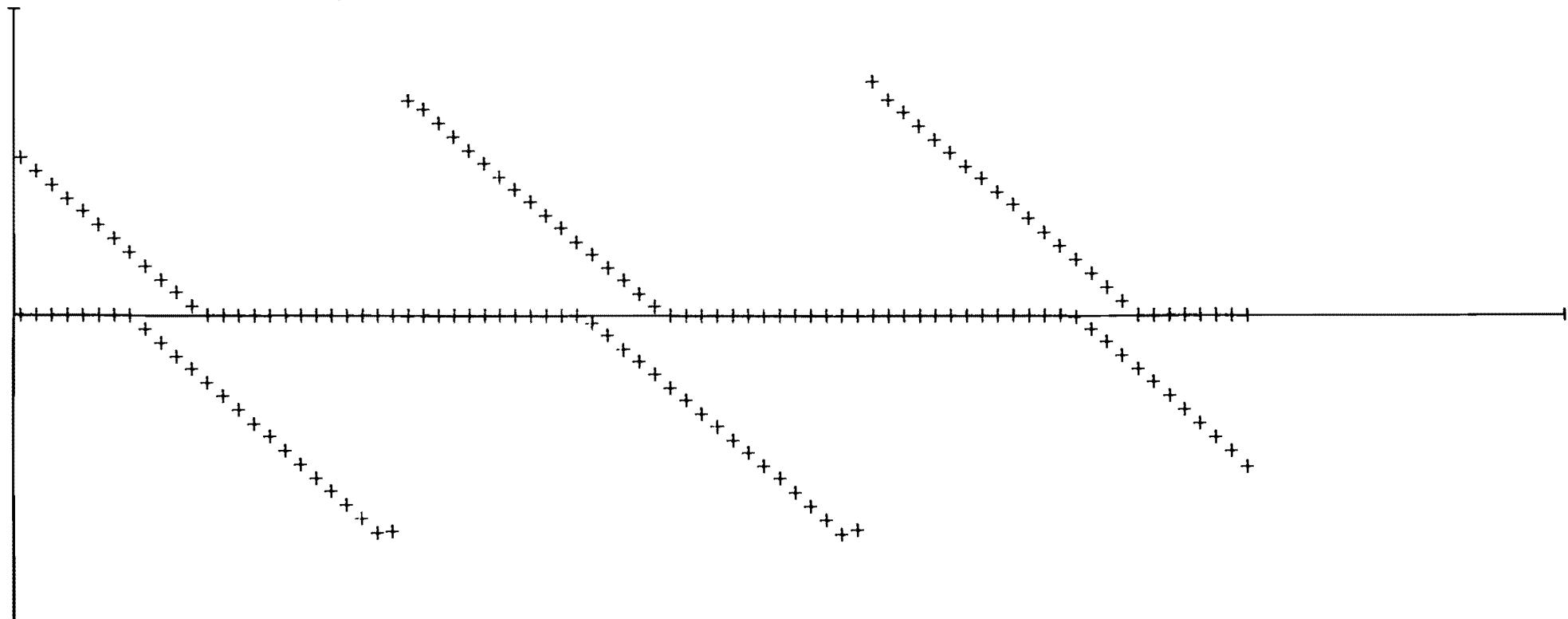
ELAPSED CPU TIME = 0 MINUTES 27.441 SECONDS



Prob 3. Three-span structure, all previous loads plus support settlements.
Envelope of maximum deflections.



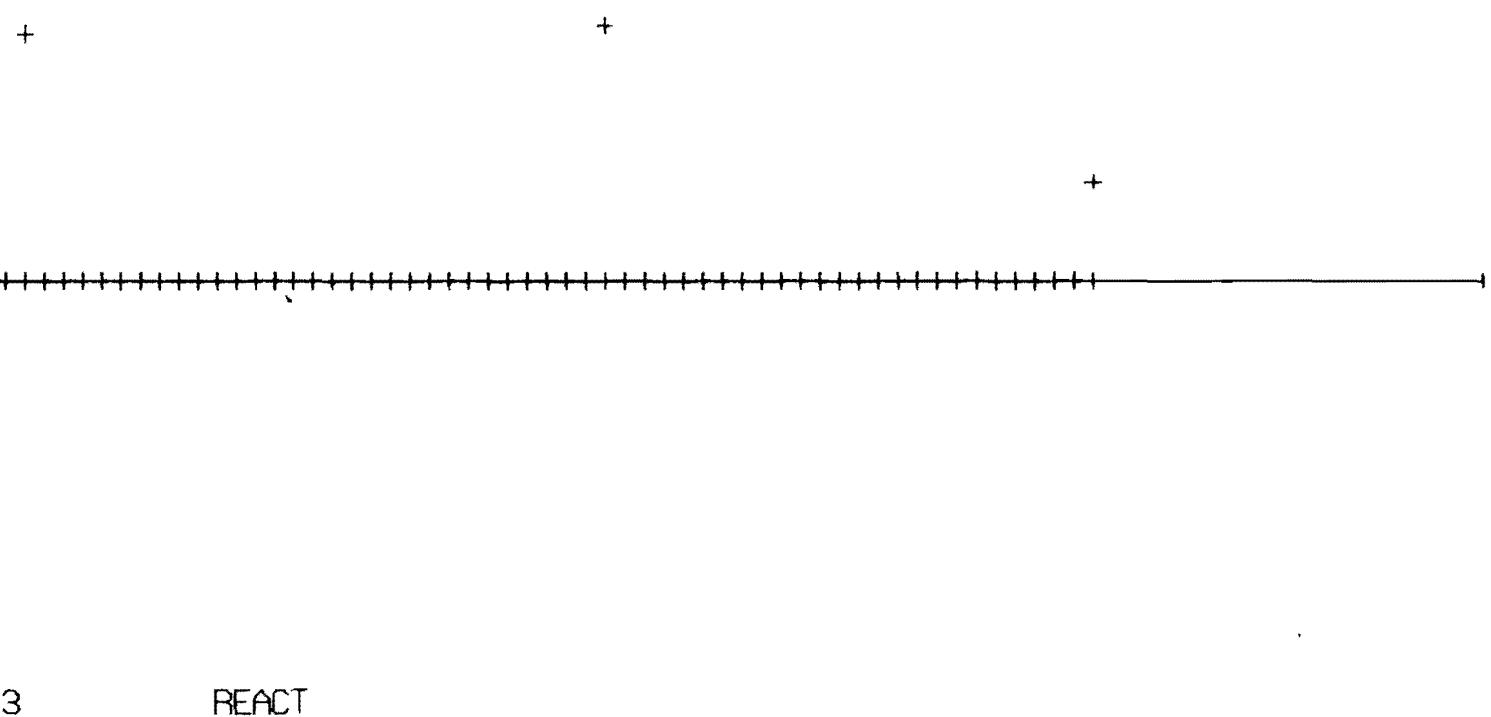
**Prob 3. Three-span structure, all previous loads plus support settlements.
Envelope of maximum moments.**



3

SHEAR

Prob 3. Three-span structure, all previous loads plus support settlements.
Envelope of maximum shears.



3

REACT

Prob 3. Three-span structure, all previous loads plus support settlements.
Envelope of maximum reactions.

PROGRAM HMCOL 43 - - MATLOCK-TAYLOR - REVISION DATE = 08 MAR 58
 CHG CE244010 CODED BY JJP RUN ON 23 APR 68
 EXAMPLE PROBLEMS FOR HMCOL43 FINAL REPORT

PROB
 4 THREE SPAN STRUCTURE, INFLUENCE DIAGRAMS FOR ONE MOVING LOAD

TABLE 1 - PROGRAM-CONTROL DATA

	ENVELOPES OF MAXIMUMS	TABLE NUMBER 2	TABLE NUMBER 3	TABLE NUMBER 4	TABLE NUMBER 5	TABLE NUMBER 6
HOLD FROM PRECEDING PROBLEM (1=HOLD)	0	0	0	0	0	0
NUM CARDS INPJT THIS PROBLEM	1	4	9	3	4	
OPTION (IF=1) TO PLOT ENVELOPES OF MAXIMUMS		DEFL	MOM	SHR	RCT	
		0	0	0	0	

TABLE 2 - CONSTANTS

NUM INCREMENTS	80
INCREMENT LENGTH	2.400E+01
NUMBER OF INCREMENTS FOR MOBILE LOAD	14
INITIAL POSITION OF MOBILE LOAD STA ZERO	-14
FINAL POSITION OF MOBILE LOAD STA ZERO	80
NUMBER OF INCREMENTS BETWEEN EACH POSITION OF MOBILE LOAD	1

TABLE 3 - SPECIFIED DEFLECTIONS AND SLOPES

STA	CASE	DEFLECTION	SLOPE
0	1	0.	NONE
25	1	0.	NONE
55	1	0.	NONE
80	1	0.	NONE

TABLE 4 - STIFFNESS AND FIXED-LOAD DATA

FROM	TO	COND	F	QF	S	T	R	P
0	80	-0	4.147E+10	-6.000E+02	-0.	-0.	-0.	-0.
19	1	0	0.	0.	-0.	-0.	-0.	-0.
25	1	3	3.953E+10	-1.500E+02	-0.	-0.	-0.	-0.
31	-0	0	0.	0.	-0.	-0.	-0.	-0.
49	1	0	0.	0.	-0.	-0.	-0.	-0.
55	1	3	3.953E+10	-1.500E+02	-0.	-0.	-0.	-0.
61	-0	0	0.	0.	-0.	-0.	-0.	-0.
25	25	-0	-0.	-0.	-0.	-0.	3.873E+07	-0.
55	55	-0	-0.	-0.	-0.	-0.	3.873E+07	-0.

TABLE 5 - MOVABLE-LOAD DATA

FROM TO CONTD QM

0	0	-0	-7.300E+02
7	7	-0	-2.920E+03
14	14	-0	-2.920E+03

TABLE 6 - SPECIFIED STATIONS FOR INFLUENCE DIAGRAMS
(SHEAR IS COMPUTED ONE HALF INCREMENT
TO THE LEFT OF THE DESIGNATED STATION)

VARIABLE	NJM OF DIAGRAMS	TYPE OF OUTPUT	DESIGNATED STATIONS		
DEFLECT	2	3	11	40	
MOMENT	3	3	9	25	40
SHEAR	0	0	-0		
REACTION	0	0	-0		

PROGRAM BMCOL 43 - - MATLOCK-TAYLOR - REVISION DATE = 08 MAR 58
 CHG CE244010 CODED BY JJP RUN ON 23 APR 68
 EXAMPLE PROBLEMS FOR BMCOL43 FINAL REPORT

PROB (CONT'D)

4 THREE SPAN STRUCTURE, INFLUENCE DIAGRAMS FOR ONE MOVING LOAD

TABLE 7 - FIXED-LOAD RESULTS

STA I	DIST	DEFL	SLOPE	MOM	SHEAR	SUP REACT
-1	-2.400E+01	6.103E-02	-2.543E-03	0.	-3.331E-10	-0.
0	0.	0.	-2.543E-03	-7.993E-09	5.176E+03	5.476E+03
1	2.400E+01	-6.103E-02	-2.471E-03	1.242E+05	5.176E+03	-0.
2	4.800E+01	-1.203E-01	-2.336E-03	2.340E+05	4.576E+03	-0.
3	7.200E+01	-1.764E-01	-2.145E-03	3.294E+05	3.976E+03	
4	9.600E+01	-2.279E-01				
5	1.200E+02					
6						

The tabulated output from Tables 7, 8A, and 8B
 is omitted since it is identical to that in
 Problem 2.

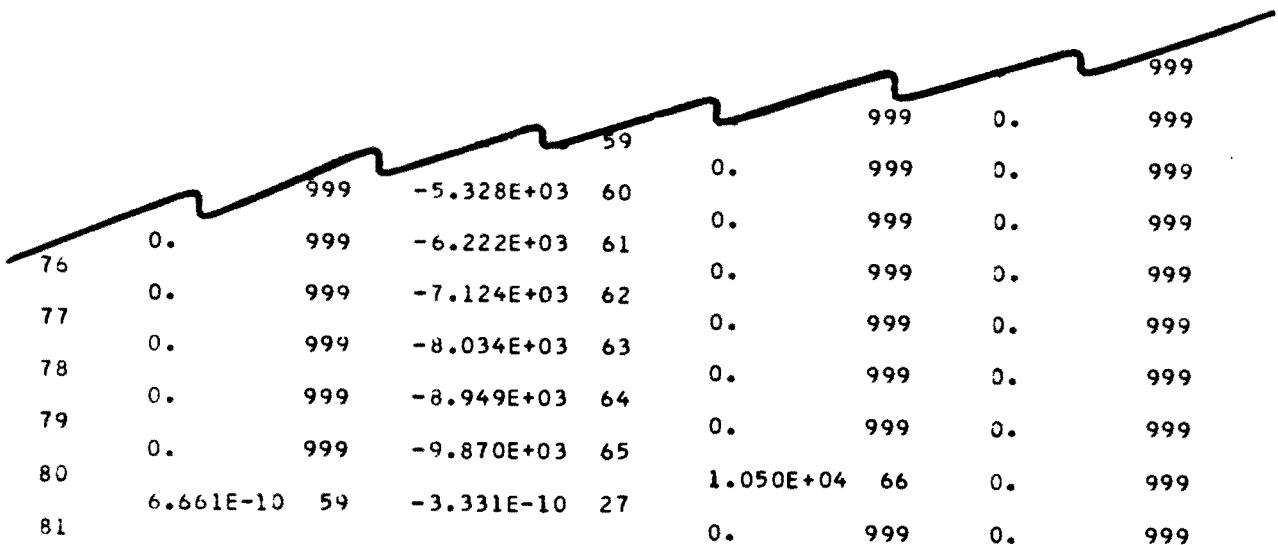


TABLE 9 -- SCALES FOR PLOTS OF THE ENVELOPES OF MAXIMUMS

NO PLOTS SPECIFIED

PROB (CONTD)

4 THREE SPAN STRUCTURE, INFLUENCE DIAGRAMS FOR ONE MOVING LOAD

TABLE 10A -- INFLUENCE DIAGRAMS FOR DEFLECTION

LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA 11	STA 40	STA	STA	STA
-14	0.	0.			
-13	-2.791E-02	9.629E-03			
-12	-5.538E-02	1.916E-02			
-11	-8.198E-02	2.850E-02			
-10	-1.073E-01	3.756E-02			
-9	-1.308E-01	4.623E-02			
-8	-1.522E-01	5.443E-02			
-7	-1.709E-01	6.205E-02			
-6	-2.145E-01	7.862E-02			
-5	-2.541E-01	9.434E-02			
-4	-2.889E-01	1.090E-01			
-3	-3.181E-01	1.224E-01			
-2	-3.407E-01	1.344E-01			
-1	-3.568E-01	1.448E-01			
0	-3.666E-01	1.533E-01			
1	-3.771E-01	1.623E-01			
2	-3.813E-01	1.690E-01			
3	-3.792E-01	1.733E-01			
4	-3.708E-01	1.749E-01			
5	-3.562E-01	1.736E-01			
6	-3.362E-01	1.693E-01			
7	-3.118E-01	1.619E-01			
8	-2.838E-01	1.515E-01			
9	-2.531E-01	1.381E-01			
10	-2.202E-01	1.215E-01			
11	-1.859E-01	1.018E-01			
12	-1.506E-01	7.884E-02			
13	-1.149E-01	5.248E-02			
14	-7.967E-02	2.306E-02			
15	-4.538E-02	-9.105E-03			
16	-1.262E-02	-4.367E-02			
17	1.810E-02	-8.024E-02			
18	4.631E-02	-1.183E-01			
19	7.189E-02	-1.575E-01			
20	9.484E-02	-1.973E-01			
21	1.151E-01	-2.370E-01			
22	1.326E-01	-2.758E-01			
23	1.472E-01	-3.130E-01			
24	1.589E-01	-3.478E-01			
25	1.676E-01	-3.791E-01			
26	1.736E-01	-4.061E-01			
27	1.769E-01	-4.277E-01			
28	1.778E-01	-4.439E-01			
29	1.765E-01	-4.547E-01			
30	1.731E-01	-4.599E-01			
31	1.679E-01	-4.595E-01			
32	1.610E-01	-4.535E-01			

33	1.527E-01	-4.415E-01
34	1.432E-01	-4.237E-01
35	1.327E-01	-4.007E-01
36	1.214E-01	-3.735E-01
37	1.096E-01	-3.428E-01
38	9.744E-02	-3.094E-01
39	8.507E-02	-2.738E-01
40	7.266E-02	-2.367E-01
41	6.038E-02	-1.986E-01
42	4.834E-02	-1.602E-01
43	3.667E-02	-1.220E-01
44	2.547E-02	-8.468E-02
45	1.486E-02	-4.873E-02
46	4.924E-03	-1.466E-02
47	-4.245E-03	1.707E-02
48	-1.255E-02	4.600E-02
49	-1.998E-02	7.205E-02
50	-2.657E-02	9.523E-02
51	-3.229E-02	1.155E-01
52	-3.715E-02	1.327E-01
53	-4.112E-02	1.468E-01
54	-4.420E-02	1.578E-01
55	-4.635E-02	1.655E-01
56	-4.764E-02	1.701E-01
57	-4.811E-02	1.718E-01
58	-4.782E-02	1.707E-01
59	-4.681E-02	1.671E-01
60	-4.513E-02	1.611E-01
61	-4.283E-02	1.529E-01
62	-3.994E-02	1.426E-01
63	-3.653E-02	1.304E-01
64	-3.266E-02	1.166E-01
65	-2.839E-02	1.013E-01
66	-2.377E-02	8.486E-02
67	-2.157E-02	7.701E-02
68	-1.913E-02	6.828E-02
69	-1.646E-02	5.878E-02
70	-1.362E-02	4.863E-02
71	-1.063E-02	3.796E-02
72	-7.529E-03	2.688E-02
73	-4.345E-03	1.551E-02
74	-3.811E-03	1.361E-02
75	-3.237E-03	1.156E-02
76	-2.630E-03	9.390E-03
77	-1.996E-03	7.126E-03
78	-1.342E-03	4.791E-03
79	-6.743E-04	2.407E-03
80	0.	0.

TABLE 10B -- INFLUENCE DIAGRAMS FOR MOMENT

LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA 9	STA 25	STA 40	STA	STA
-14	0.	0.	0.		
-13	3.692E+04	-2.055E+04	-6.380E+03		
-12	7.392E+04	-4.090E+04	-1.270E+04		
-11	1.111E+05	-6.083E+04	-1.889E+04		
-10	1.485E+05	-8.016E+04	-2.489E+04		

-9	1.862E+05	-9.867E+04	-3.063E+04
-8	2.243E+05	-1.162E+05	-3.606E+04
-7	2.629E+05	-1.324E+05	-4.111E+04
-6	3.389E+05	-1.678E+05	-5.209E+04
-5	4.157E+05	-2.013E+05	-6.251E+04
-4	4.232E+05	-2.326E+05	-7.223E+04
-3	4.318E+05	-2.613E+05	-8.112E+04
-2	4.415E+05	-2.869E+05	-8.906E+04
-1	4.526E+05	-3.090E+05	-9.593E+04
0	4.652E+05	-3.272E+05	-1.016E+05
1	4.887E+05	-3.463E+05	-1.075E+05
2	5.140E+05	-3.606E+05	-1.120E+05
3	4.712E+05	-3.698E+05	-1.148E+05
4	4.307E+05	-3.732E+05	-1.159E+05
5	3.924E+05	-3.706E+05	-1.150E+05
6	3.568E+05	-3.613E+05	-1.122E+05
7	3.236E+05	-3.456E+05	-1.073E+05
8	2.929E+05	-3.234E+05	-1.004E+05
9	2.647E+05	-2.947E+05	-9.148E+04
10	2.216E+05	-2.594E+05	-8.052E+04
11	1.810E+05	-2.173E+05	-6.747E+04
12	1.431E+05	-2.385E+05	-5.224E+04
13	1.077E+05	-2.525E+05	-3.459E+04
14	7.497E+04	-2.595E+05	-1.452E+04
15	4.473E+04	-2.595E+05	8.016E+03
16	1.703E+04	-2.525E+05	3.312E+04
17	-8.101E+03	-2.384E+05	6.095E+04
18	-3.059E+04	-2.170E+05	9.173E+04
19	-5.062E+04	-2.586E+05	1.255E+05
20	-6.826E+04	-2.932E+05	1.624E+05
21	-8.358E+04	-3.210E+05	2.025E+05
22	-9.664E+04	-3.421E+05	2.460E+05
23	-1.075E+05	-3.568E+05	2.929E+05
24	-1.161E+05	-3.648E+05	3.434E+05
25	-1.224E+05	-3.663E+05	3.978E+05
26	-1.268E+05	-3.792E+05	4.561E+05
27	-1.292E+05	-3.865E+05	4.482E+05
28	-1.298E+05	-3.884E+05	4.443E+05
29	-1.289E+05	-3.856E+05	4.443E+05
30	-1.264E+05	-3.783E+05	4.483E+05
31	-1.227E+05	-3.669E+05	4.564E+05
32	-1.176E+05	-3.519E+05	4.686E+05
33	-1.116E+05	-3.337E+05	4.849E+05
34	-1.046E+05	-3.129E+05	4.352E+05
35	-9.691E+04	-2.899E+05	3.897E+05
36	-8.869E+04	-2.653E+05	3.483E+05
37	-8.006E+04	-2.395E+05	3.107E+05
38	-7.117E+04	-2.129E+05	2.768E+05
39	-6.213E+04	-1.859E+05	2.464E+05
40	-5.307E+04	-1.588E+05	2.195E+05
41	-4.410E+04	-1.319E+05	1.783E+05
42	-3.531E+04	-1.056E+05	1.403E+05
43	-2.678E+04	-8.012E+04	1.054E+05
44	-1.861E+04	-5.566E+04	7.340E+04
45	-1.086E+04	-3.248E+04	4.423E+04
46	-3.597E+03	-1.076E+04	1.777E+04
47	3.101E+03	9.276E+03	-6.029E+03
48	9.163E+03	2.741E+04	-2.714E+04
49	1.460E+04	4.367E+04	-4.579E+04
50	1.941E+04	5.806E+04	-6.205E+04
51	2.359E+04	7.056E+04	-7.601E+04

52	2.713E+04	8.117E+04	-8.773E+04
53	3.004E+04	8.986E+04	-9.724E+04
54	3.228E+04	9.658E+04	-1.046E+05
55	3.386E+04	1.013E+05	-1.096E+05
56	3.480E+04	1.041E+05	-1.127E+05
57	3.514E+04	1.051E+05	-1.138E+05
58	3.493E+04	1.045E+05	-1.131E+05
59	3.419E+04	1.023E+05	-1.107E+05
60	3.296E+04	9.861E+04	-1.068E+05
61	3.128E+04	9.358E+04	-1.013E+05
62	2.917E+04	8.727E+04	-9.447E+04
63	2.668E+04	7.982E+04	-8.641E+04
64	2.385E+04	7.136E+04	-7.725E+04
65	2.073E+04	6.203E+04	-6.715E+04
66	1.736E+04	5.194E+04	-5.623E+04
67	1.576E+04	4.714E+04	-5.103E+04
68	1.397E+04	4.179E+04	-4.524E+04
69	1.203E+04	3.598E+04	-3.894E+04
70	9.949E+03	2.976E+04	-3.222E+04
71	7.766E+03	2.323E+04	-2.515E+04
72	5.499E+03	1.645E+04	-1.781E+04
73	3.173E+03	9.494E+03	-1.028E+04
74	2.784E+03	8.328E+03	-9.015E+03
75	2.365E+03	7.074E+03	-7.658E+03
76	1.921E+03	5.747E+03	-6.221E+03
77	1.458E+03	4.362E+03	-4.722E+03
78	9.801E+02	2.932E+03	-3.174E+03
79	4.925E+02	1.473E+03	-1.595E+03
80	0.	0.	0.

TABLE 10C -- INFLUENCE DIAGRAMS FOR SHEAR
 (SHEAR IS COMPUTED ONE HALF INCREMENT
 TO THE LEFT OF THE DESIGNATED STATION)

LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

TABLE 10D -- INFLUENCE DIAGRAMS FOR SUPPORT REACTION

LOCATION OF LOAD	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS				
	STA	STA	STA	STA	STA
NONE					

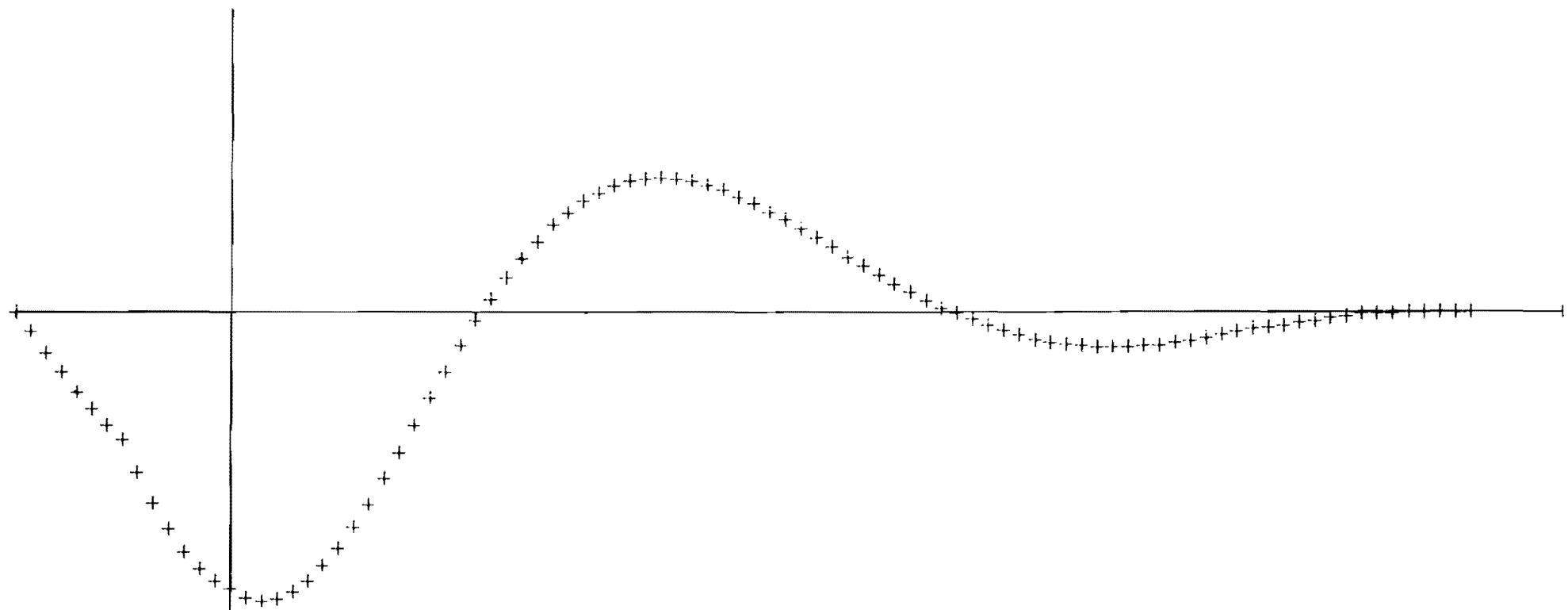
TABLE 11 -- SCALES FOR INFLUENCE DIAGRAM PLOTS
HORIZONTAL SCALE
10 INCHES = 100 STATIONS

VERTICAL SCALES

STA	VARIABLE	LENGTH OF AXIS	MAXIMUM VALUE
11	DEFLECT	2 INCHES	= 4.000E-01
40	DEFLECT	2 INCHES	= 1.000E+00
9	MOMENT	2 INCHES	= 1.000E+06
25	MOMENT	2 INCHES	= 4.000E+05
40	MOMENT	2 INCHES	= 1.000E+06

TIME FOR THIS PROBLEM = 0 MINUTES 4.905 SECONDS

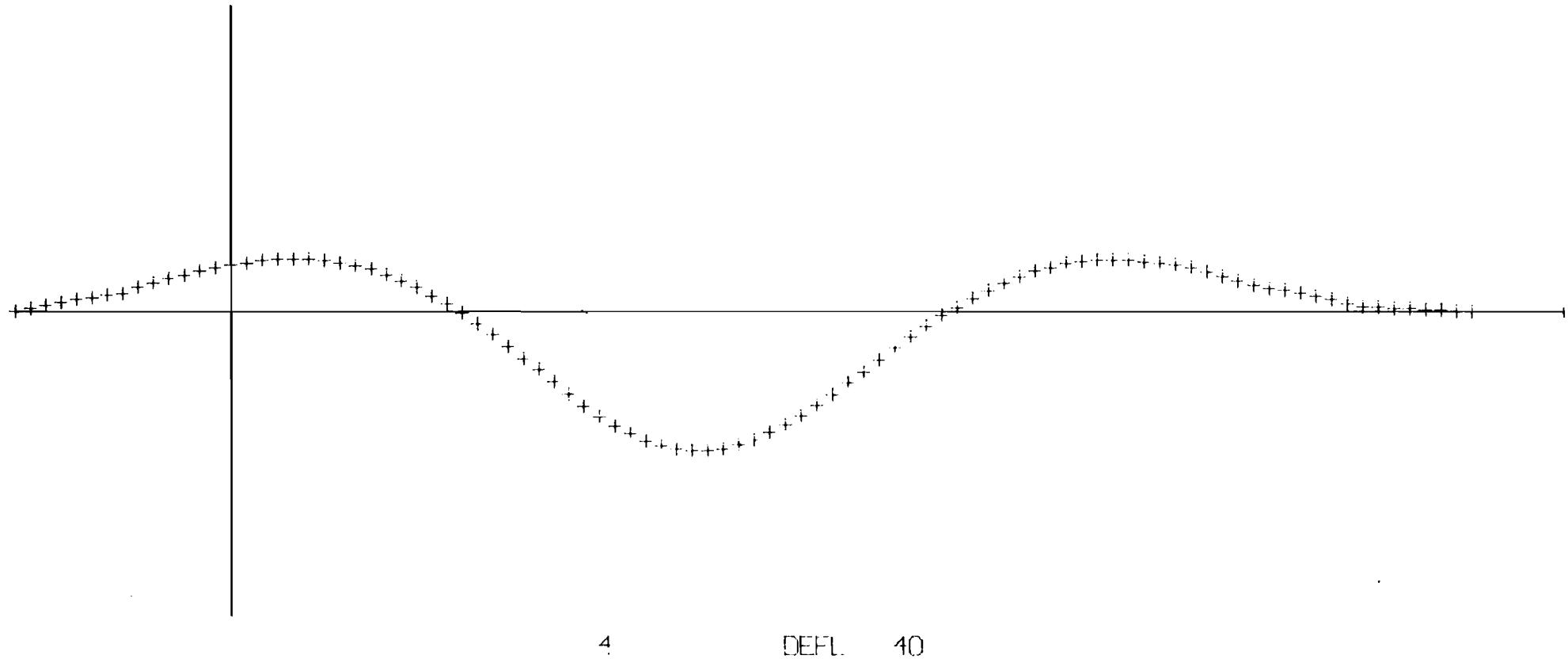
ELAPSED CPU TIME = 0 MINUTES 32.346 SECONDS



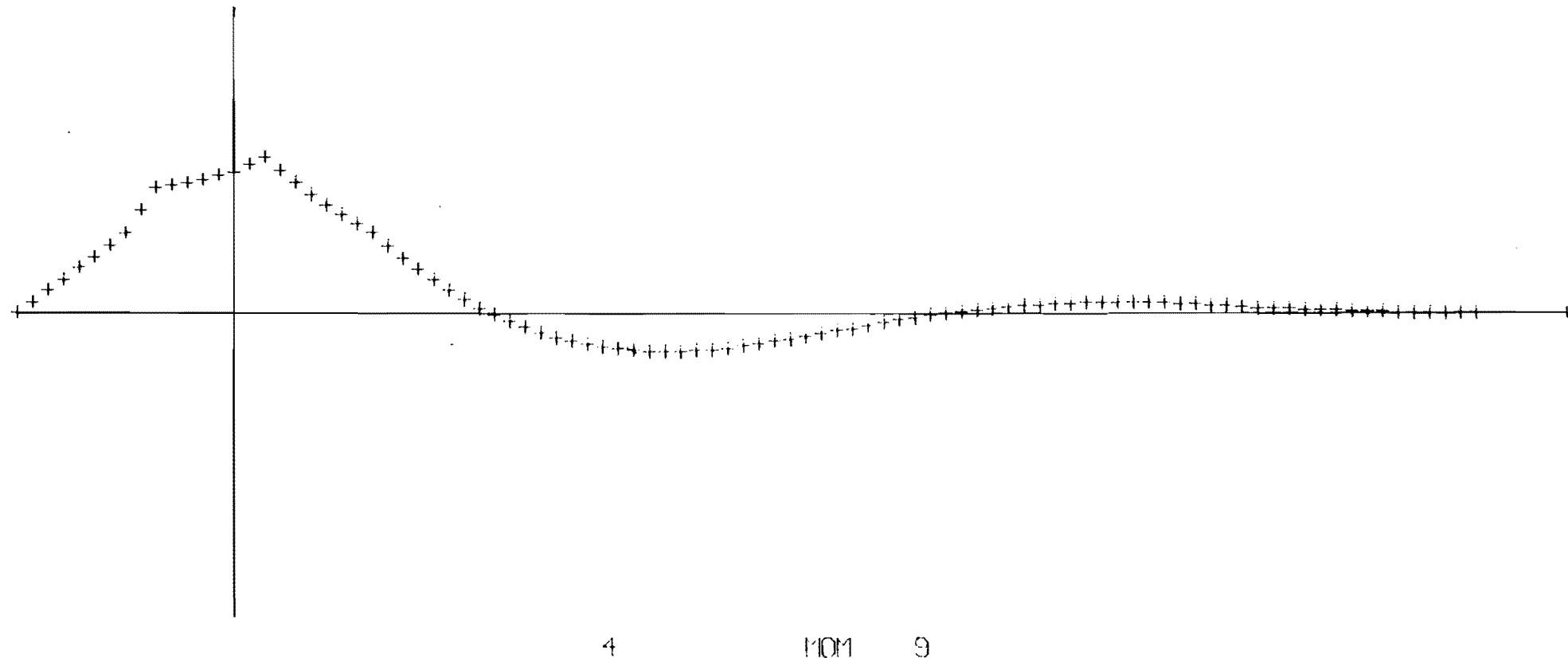
4

DEFL 11

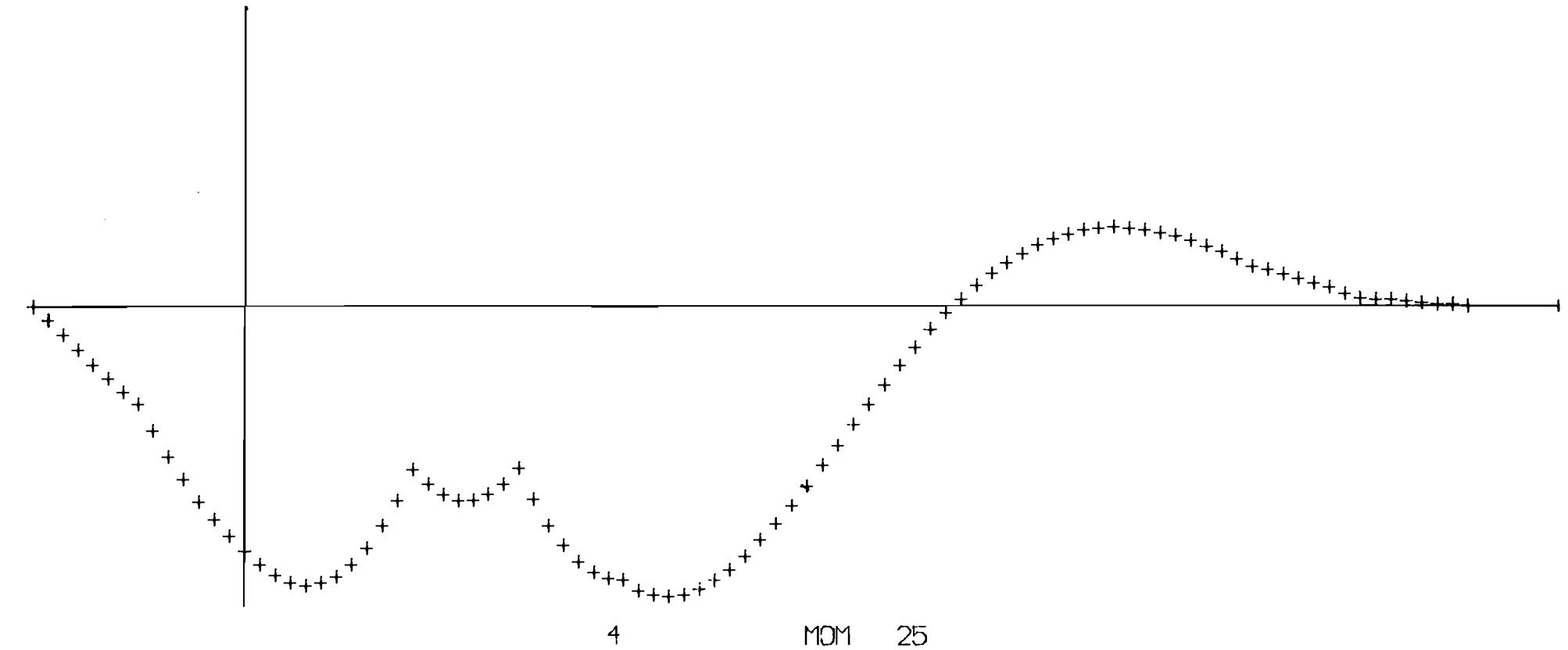
Prob 4. Three-span structure, influence diagrams for one moving load.
Influence diagram for deflection at Station 11.



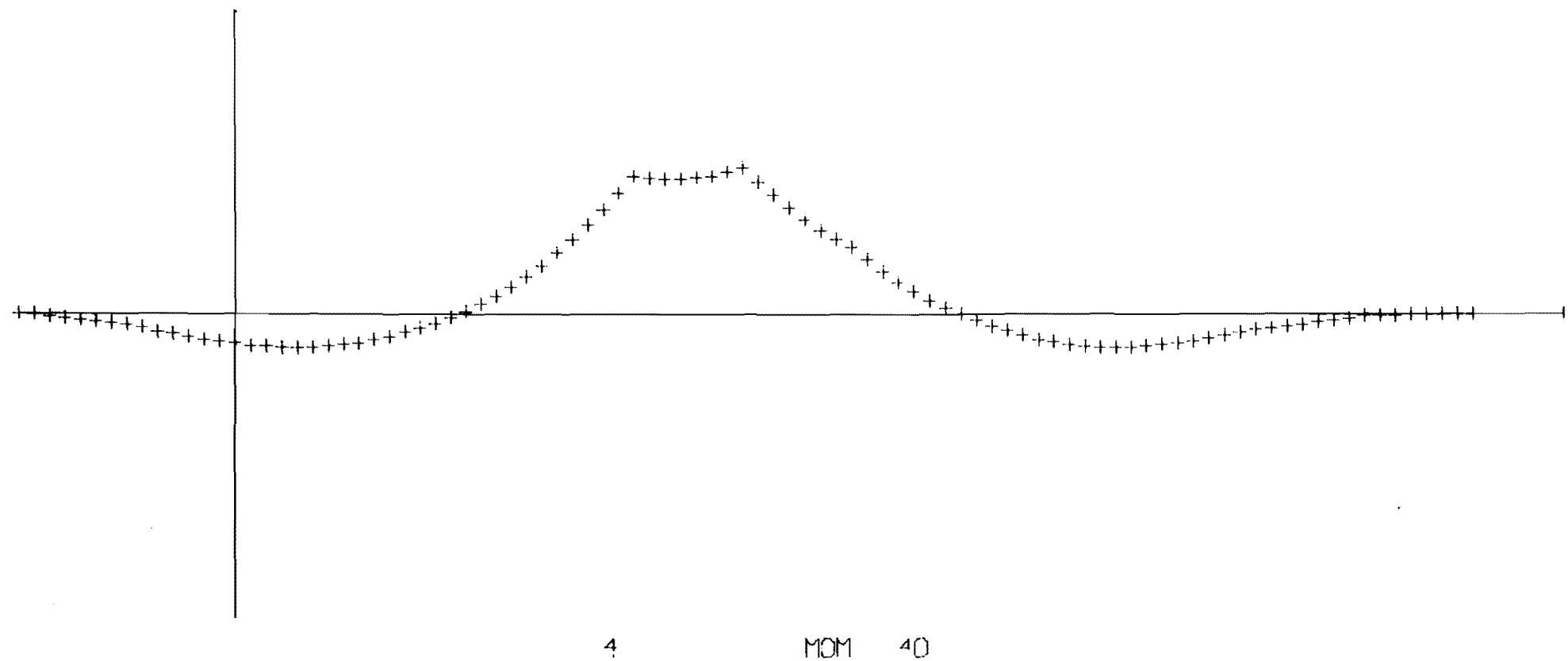
Prob 4. Three-span structure, influence diagrams for one moving load.
Influence diagram for deflection at Station 40.



Prob 4. Three-span structure, influence diagrams for one moving load.
Influence diagram for moment at Station 9.



Prob 4. Three-span structure, influence diagrams for one moving load.
Influence diagram for moment at Station 25.



Prob 4. Three-span structure, influence diagrams for one moving load.
Influence diagram for moment at Station 40.

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-- CTR Library Digitization Team

PROGRAM BMCOL 43 - - MATLOCK-TAYLOR - REVISION DATE = 08 MAR 68
 CHG CE244010 CODED BY JJP RUN ON 23 APR 58
 EXAMPLE PROBLEMS FOR BMCOL43 FINAL REPORT

PROB
 5 TWO SPAN GIRDER, RAILROAD LOADING

TABLE 1 - PROGRAM-CONTROL DATA

	ENVELOPES OF MAXIMUMS	TABLE NUMBER	2	3	4	5	6
HOLD FROM PRECEDING PROBLEM (1=HOLD)	0		0	0	0	0	0
NUM CARDS INPJT THIS PROBLEM			1	3	1	14	0
OPTION (IF=1) TO PLOT ENVELOPES OF MAXIMUMS			DEFL	MOM	SHR	RCT	
			1	1	1	1	1

TABLE 2 - CONSTANTS

NUM INCREMENTS	86
INCREMENT LENGTH	1.200E+01
NUMBER OF INCREMENTS FOR MOBILE LOAD	79
INITIAL POSITION OF MOBILE LOAD STA ZERO	-80
FINAL POSITION OF MOBILE LOAD STA ZERO	86
NUMBER OF INCREMENTS BETWEEN EACH POSITION OF MOBILE LOAD	2

TABLE 3 - SPECIFIED DEFLECTIONS AND SLOPES

STA	CASE	DEFLECTION	SLOPE
0	1	0.	NONE
43	1	0.	NONE
86	1	0.	NONE

TABLE 4 - STIFFNESS AND FIXED-LOAD DATA

FROM	TO	CONTD	F	QF	S	T	R	P
0	86	-0	2.440E+11	-1.000E+03	-0.	-0.	-0.	-3.300E+04

TABLE 5 - MOBILE-LOAD DATA

FROM TO CONTD QM

0	0	-0	-1.000E+04
8	8	-0	-2.000E+04
13	13	-0	-2.000E+04
18	18	-0	-2.000E+04
23	23	-0	-2.000E+04

32	32	-0	-1.300E+04
37	37	-0	-1.300E+04
43	43	-0	-1.300E+04
48	48	-0	-1.300E+04
56	56	-0	-1.000E+04
64	64	-0	-2.000E+04
69	69	-0	-2.000E+04
74	74	-0	-2.000E+04
79	79	-0	-2.000E+04

TABLE 6 - SPECIFIED STATIONS FOR INFLUENCE DIAGRAMS
(SHEAR IS COMPUTED ONE HALF INCREMENT
TO THE LEFT OF THE DESIGNATED STATION)

NONE

PROGRAM BMCOL 43 - - MATLOCK-TAYLOR - REVISION DATE = 08 MAR 58
 CHG CE244010 CODED BY JJP RUN ON 23 APR 58
 EXAMPLE PROBLEMS FOR BMCOL43 FINAL REPORT

PROB (CONT'D)
 5 TWO SPAN GIRDER, RAILROAD LOADING

TABLE 7 - FIXED-LOAD RESULTS

STA I	DIST	DEFL	SLOPE	MOM	SHEAR	SUP REACT
-1	-1.200E+01	1.175E-02	-9.796E-04	0.	-7.838E-09	-0.
0	0.	0.	-9.796E-04	-9.406E-08	1.562E+04	1.612E+04
1	1.200E+01	-1.175E-02	-9.703E-04	1.879E+05	1.462E+04	-0.
2	2.400E+01	-2.340E-02	-9.524E-04	3.637E+05	1.362E+04	-0.
3	3.600E+01	-3.483E-02	-9.265E-04	5.276E+05	1.262E+04	-0.
4	4.800E+01	-4.595E-02	-8.931E-04	6.794E+05	1.162E+04	-0.
5	6.000E+01	-5.666E-02	-8.528E-04	8.192E+05	1.062E+04	-0.
6	7.200E+01	-6.690E-02	-8.062E-04	9.471E+05	9.623E+03	-0.
7	8.400E+01	-7.657E-02	-7.539E-04	1.063E+06	8.623E+03	-0.
8	9.600E+01	-8.562E-02	-6.966E-04	1.167E+06	7.623E+03	-0.
9	1.080E+02	-9.398E-02	-6.347E-04	1.258E+06	6.623E+03	-0.
10	1.200E+02	-1.016E-01	-5.689E-04	1.338E+06	5.623E+03	-0.
11	1.320E+02	-1.084E-01	-4.997E-04	1.406E+06	4.623E+03	-0.
12	1.440E+02	-1.144E-01	-4.279E-04	1.461E+06	3.623E+03	-0.
13	1.560E+02	-1.196E-01	-3.538E-04	1.505E+06	2.623E+03	-0.
14	1.680E+02	-1.238E-01	-2.783E-04	1.537E+06	1.623E+03	-0.
15	1.800E+02	-1.271E-01	-2.017E-04	1.556E+06	6.229E+02	-0.
16	1.920E+02	-1.296E-01	-1.248E-04	1.564E+06	-3.771E+02	-0.
17	2.040E+02	-1.311E-01	-4.812E-05	1.559E+06	-1.377E+03	-0.
18	2.160E+02	-1.316E-01	2.776E-05	1.543E+06	-2.377E+03	-0.
19	2.280E+02	-1.313E-01	1.022E-04	1.514E+06	-3.377E+03	-0.
20	2.400E+02	-1.301E-01	1.747E-04	1.474E+06	-4.377E+03	-0.
21	2.520E+02	-1.280E-01	2.446E-04	1.421E+06	-5.377E+03	-0.

22	2.640E+02	-1.250E-01	3.113E-04	1.357E+06	-6.377E+03	-0.
23	2.760E+02	-1.213E-01	3.743E-04	1.280E+06	-7.377E+03	-0.
24	2.880E+02	-1.168E-01	4.329E-04	1.191E+06	-8.377E+03	-0.
25	3.000E+02	-1.116E-01	4.865E-04	1.091E+06	-9.377E+03	-0.
26	3.120E+02	-1.058E-01	5.346E-04	9.778E+05	-1.038E+04	-0.
27	3.240E+02	-9.937E-02	5.765E-04	8.531E+05	-1.138E+04	-0.
28	3.360E+02	-9.245E-02	6.118E-04	7.163E+05	-1.238E+04	-0.
29	3.480E+02	-8.511E-02	6.397E-04	5.676E+05	-1.338E+04	-0.
30	3.600E+02	-7.743E-02	6.597E-04	4.068E+05	-1.438E+04	-0.
31	3.720E+02	-6.952E-02	6.712E-04	2.340E+05	-1.538E+04	-0.
32	3.840E+02	-6.146E-02	6.736E-04	4.921E+04	-1.638E+04	-0.
33	3.960E+02	-5.338E-02	6.664E-04	-1.476E+05	-1.738E+04	-0.
34	4.080E+02	-4.538E-02	6.488E-04	-3.564E+05	-1.838E+04	-0.
35	4.200E+02	-3.760E-02	6.204E-04	-5.772E+05	-1.938E+04	-0.
36	4.320E+02	-3.015E-02	5.806E-04	-8.099E+05	-2.038E+04	-0.
37	4.440E+02	-2.318E-02	5.287E-04	-1.055E+06	-2.138E+04	-0.
38	4.560E+02	-1.684E-02	4.642E-04	-1.311E+06	-2.238E+04	-0.
39	4.680E+02	-1.127E-02	3.865E-04	-1.580E+06	-2.338E+04	-0.
40	4.800E+02	-6.628E-03	2.950E-04	-1.861E+06	-2.438E+04	-0.
41	4.920E+02	-3.088E-03	1.891E-04	-2.153E+06	-2.538E+04	-0.
42	5.040E+02	-8.187E-04	6.823E-05	-2.458E+06	-2.638E+04	-0.
43	5.160E+02	0.	-6.823E-05	-2.775E+06	5.375E+04	
44	5.280E+02	-8.187E-04	-1.891E-04	-2.458E+06	2.638E+04	-0.
45	5.400E+02	-3.088E-03	-2.950E-04	-2.153E+06	2.538E+04	-0.
46	5.520E+02	-6.628E-03	-3.865E-04	-1.861E+06	2.438E+04	-0.
47	5.640E+02	-1.127E-02	-4.642E-04	-1.580E+06	2.338E+04	-0.
48	5.760E+02	-1.684E-02	-5.287E-04	-1.311E+06	2.238E+04	-0.
49	5.880E+02	-2.318E-02	-5.806E-04	-1.055E+06	2.138E+04	-0.
50	6.000E+02	-3.015E-02	-6.204E-04	-8.099E+05	2.038E+04	-0.
51	6.120E+02	-3.760E-02	-6.488E-04	-5.772E+05	1.938E+04	-0.
52	6.240E+02	-4.538E-02		-3.564E+05	1.838E+04	-0.

53	6.360E+02	-5.338E-02	-6.664E-04	-1.476E+05	1.738E+04	-0.
54	6.480E+02	-6.146E-02	-6.736E-04	4.921E+04	1.638E+04	-0.
55	6.600E+02	-6.952E-02	-6.712E-04	2.340E+05	1.538E+04	-0.
56	6.720E+02	-7.743E-02	-6.597E-04	4.068E+05	1.438E+04	-0.
57	6.840E+02	-8.511E-02	-6.397E-04	5.676E+05	1.338E+04	-0.
58	6.960E+02	-9.245E-02	-6.118E-04	7.163E+05	1.238E+04	-0.
59	7.080E+02	-9.937E-02	-5.765E-04	8.531E+05	1.138E+04	-0.
60	7.200E+02	-1.058E-01	-4.865E-04	9.778E+05	1.038E+04	-0.
61	7.320E+02	-1.116E-01	-4.329E-04	1.091E+06	9.377E+03	-0.
62	7.440E+02	-1.168E-01	-3.743E-04	1.191E+06	8.377E+03	-0.
63	7.560E+02	-1.213E-01	-3.113E-04	1.280E+06	7.377E+03	-0.
64	7.680E+02	-1.250E-01	-2.446E-04	1.357E+06	6.377E+03	-0.
65	7.800E+02	-1.280E-01	-1.747E-04	1.421E+06	5.377E+03	-0.
66	7.920E+02	-1.301E-01	-1.022E-04	1.474E+06	4.377E+03	-0.
67	8.040E+02	-1.313E-01	-2.776E-05	1.514E+06	3.377E+03	-0.
68	8.160E+02	-1.316E-01	4.812E-05	1.543E+06	2.377E+03	-0.
69	8.280E+02	-1.311E-01	1.248E-04	1.559E+06	1.377E+03	-0.
70	8.400E+02	-1.296E-01	2.017E-04	1.564E+06	3.771E+02	-0.
71	8.520E+02	-1.271E-01	2.783E-04	1.556E+06	-6.229E+02	-0.
72	8.640E+02	-1.238E-01	3.538E-04	1.537E+06	-1.623E+03	-0.
73	8.760E+02	-1.196E-01	4.279E-04	1.505E+06	-2.623E+03	-0.
74	8.880E+02	-1.144E-01	4.997E-04	1.461E+06	-3.623E+03	-0.
75	9.000E+02	-1.084E-01	5.689E-04	1.406E+06	-4.623E+03	-0.
76	9.120E+02	-1.016E-01	6.347E-04	1.338E+06	-5.623E+03	-0.
77	9.240E+02	-9.398E-02	6.966E-04	1.258E+06	-6.623E+03	-0.
78	9.360E+02	-8.562E-02	7.539E-04	1.167E+06	-7.623E+03	-0.
79	9.480E+02	-7.657E-02	8.062E-04	1.063E+06	-8.623E+03	-0.
80	9.600E+02	-6.690E-02	8.528E-04	9.471E+05	-9.623E+03	-0.
81	9.720E+02	-5.666E-02	8.931E-04	8.192E+05	-1.062E+04	-0.
82	9.840E+02	-4.595E-02	9.265E-04	6.794E+05	-1.162E+04	-0.

83	9.960E+02	-3.483E-02	9.524E-04	5.276E+05	-1.362E+04	-0.
84	1.008E+03	-2.340E-02	9.703E-04	3.637E+05	-1.462E+04	-0.
85	1.020E+03	-1.175E-02	9.796E-04	1.879E+05	-1.562E+04	-0.
86	1.032E+03	0.	9.796E-04	4.703E-08	-3.919E-09	1.612E+04
87	1.044E+03	1.175E-02		0.		-0.

PROB (CONT'D)
 5 TWO SPAN GIRDER, RAILROAD LOADING

TABLE 8A- ENVELOPES OF MAXIMUMS * = HELD FROM PRIOR PROBLEM

STA	MAX +DEFL LOC	MAX -DEFL LOC	MAX +MOM LOC	MAX -MOM LOC
-1	6.229E-02 -52	-6.313E-03 46	0.	999
0	0. 999	0. 999	4.703E-08 38	-3.762E-07 -68
1	6.313E-03 46	-6.229E-02 -52	8.906E+05 -6	0. 999
2	1.268E-02 46	-1.241E-01 -52	1.769E+06 -6	0. 999
3	1.914E-02 46	-1.851E-01 -52	2.472E+06 -60	0. 999
4	2.574E-02 46	-2.448E-01 -52	3.272E+06 -60	0. 999
5	3.250E-02 46	-3.028E-01 -52	3.820E+06 -60	0. 999
6	3.945E-02 46	-3.588E-01 -52	4.522E+06 -58	0. 999
7	4.661E-02 46	-4.124E-01 -52	4.994E+06 -58	0. 999
8	5.399E-02 46	-4.633E-01 -52	5.522E+06 -56	0. 999
9	6.158E-02 46	-5.111E-01 -52	5.918E+06 -56	0. 999
10	6.939E-02 46	-5.557E-01 -52	6.450E+06 -54	0. 999
11	7.740E-02 46	-5.967E-01 -52	6.764E+06 -54	0. 999
12	8.559E-02 46	-6.340E-01 -50	7.115E+06 -52	0. 999
13	9.394E-02 46	-6.678E-01 -50	7.381E+06 -56	0. 999
14	1.024E-01 46	-6.975E-01 -50	7.635E+06 -54	0. 999
15	1.110E-01 46	-7.227E-01 -50	7.901E+06 -54	0. 999
16	1.196E-01 46	-7.433E-01 -50	7.979E+06 -52	-2.807E+04 46
17	1.282E-01 46	-7.594E-01 -50	8.165E+06 -52	-1.319E+05 46
18	1.367E-01 46	-7.707E-01 -50	8.098E+06 -52	-2.477E+05 46
19	1.450E-01 46	-7.773E-01 -50	8.183E+06 -50	-3.754E+05 46
20	1.532E-01 46	-7.790E-01 -50	8.038E+06 -50	-5.152E+05 46
21	1.610E-01 46	-7.760E-01 -50	7.969E+06 -48	-5.670E+05 46
22	1.684E-01 46	-7.684E-01 -50	7.746E+06 -48	-8.307E+05 46
23	1.754E-01 46	-7.562E-01 -50	7.638E+06 -46	-1.006E+06 46

24	1.817E-01	46	-7.416E-01 -46	7.335E+06	-50	-1.194E+06	46	
25	1.874E-01	46	-7.230E-01 -46	7.067E+06	-44	-1.394E+06	46	
26	1.922E-01	46	-7.002E-01 -46	6.735E+06	-48	-1.606E+06	46	
27	1.961E-01	46	-6.735E-01 -46	6.338E+06	-46	-1.829E+06	46	
28	1.989E-01	46	-6.431E-01 -46	5.982E+06	-46	-2.065E+06	46	
29	2.005E-01	46	-6.091E-01 -46	5.437E+06	-44	-2.312E+06	46	
30	2.007E-01	46	-5.720E-01 -46	4.999E+06	-44	-2.572E+06	46	
31	1.994E-01	46	-5.320E-01 -46	4.336E+06	-42	-2.843E+06	46	
32	1.964E-01	46	-4.896E-01 -46	3.813E+06	-42	-3.127E+06	46	
33	1.916E-01	46	-4.452E-01 -46	3.038E+06	-42	-3.422E+06	46	
34	1.847E-01	46	-3.991E-01 -46	2.419E+06	-40	-3.729E+06	46	
35	1.757E-01	46	-3.519E-01 -46	1.557E+06	-40	-4.049E+06	46	
36	1.642E-01	46	-3.046E-01 -44	8.166E+05	-38	-4.380E+06	46	
37	1.502E-01	46	-2.570E-01 -44	0.	999	-4.723E+06	46	
38	1.334E-01	46	-2.098E-01 -44	0.	999	-5.228E+06	-2	
39	1.136E-01	46	-1.636E-01 -44	0.	999	-6.208E+06	-2	
40	9.060E-02	46	-1.188E-01 -44	0.	999	-7.217E+06	0	
41	6.415E-02	46	-7.623E-02 -44	0.	999	-8.285E+06	0	
42	3.403E-02	46	-3.642E-02 -44	0.	999	-9.386E+06	2	
43	0.	999	0.	999	0.	999	-1.056E+07	-2
44	3.239E-02	-44	-3.817E-02	46	0.	999	-9.460E+06	-2
45	6.098E-02	-44	-7.984E-02	46	0.	999	-8.387E+06	0
46	8.603E-02	-44	-1.243E-01	46	0.	999	-7.371E+06	0
47	1.077E-01	-44	-1.710E-01	46	0.	999	-6.398E+06	-4
48	1.264E-01	-44	-2.194E-01	46	0.	999	-5.488E+06	-2
49	1.421E-01	-44	-2.688E-01	46	0.	999	-4.603E+06	-2
50	1.551E-01	-44	-3.187E-01	46	5.727E+05	42	-4.212E+06	-44
51	1.657E-01	-44	-3.686E-01	46	1.439E+06	38	-3.885E+06	-44
52	1.739E-01	-44	-4.179E-01	46	2.216E+06	40	-3.571E+06	-44
53	1.801E-01	-44	-4.662E-01	46	3.063E+06	40	-3.268E+06	-44

54	1.843E-01	-44	-5.128E-01	46	3.711E+06	42	-2.977E+05	-44
55	1.867E-01	-44	-5.572E-01	46	4.465E+06	42	-2.699E+06	-44
56	1.876E-01	-44	-5.992E-01	46	4.984E+06	44	-2.432E+06	-44
57	1.870E-01	-44	-6.383E-01	46	5.648E+06	44	-2.177E+06	-44
58	1.852E-01	-44	-6.742E-01	46	6.060E+06	44	-1.934E+06	-44
59	1.822E-01	-44	-7.065E-01	46	6.617E+06	46	-1.703E+06	-44
60	1.782E-01	-44	-7.349E-01	46	6.940E+06	46	-1.484E+06	-44
61	1.733E-01	-44	-7.593E-01	46	7.335E+06	48	-1.277E+06	-44
62	1.677E-01	-44	-7.793E-01	46	7.585E+06	44	-1.082E+06	-44
63	1.614E-01	-44	-7.949E-01	46	7.837E+06	50	-8.989E+05	-44
64	1.546E-01	-44	-8.058E-01	46	8.110E+06	46	-7.279E+05	-44
65	1.474E-01	-44	-8.127E-01	48	8.190E+06	48	-5.688E+05	-44
66	1.398E-01	-44	-8.149E-01	48	8.374E+06	48	-4.217E+05	-44
67	1.320E-01	-44	-8.121E-01	48	8.338E+06	50	-2.866E+05	-44
68	1.240E-01	-44	-8.044E-01	48	8.433E+06	50	-1.634E+05	-44
69	1.160E-01	-44	-7.919E-01	48	8.290E+06	52	-5.233E+04	-44
70	1.079E-01	-44	-7.746E-01	48	8.299E+06	52	0.	999
71	9.978E-02	-44	-7.525E-01	48	8.056E+06	52	0.	999
72	9.178E-02	-44	-7.258E-01	48	7.893E+06	54	0.	999
73	8.390E-02	-44	-6.951E-01	50	7.566E+06	56	0.	999
74	7.619E-02	-44	-6.601E-01	50	7.408E+06	56	0.	999
75	6.866E-02	-44	-6.209E-01	50	6.997E+06	56	0.	999
76	6.135E-02	-44	-5.778E-01	50	6.663E+06	58	0.	999
77	5.427E-02	-44	-5.311E-01	50	6.172E+06	58	0.	999
78	4.742E-02	-44	-4.811E-01	50	5.692E+06	56	0.	999
79	4.082E-02	-44	-4.281E-01	50	5.233E+06	56	0.	999
80	3.445E-02	-44	-3.724E-01	50	4.623E+06	58	0.	999
81	2.831E-02	-44	-3.142E-01	50	4.083E+06	58	0.	999
82	2.237E-02	-44	-2.540E-01	50	3.290E+06	58	0.	999
83	1.660E-02	-44	-1.921E-01	50	2.660E+06	60	0.	999
84	1.098E-02	-44	-1.288E-01	50	1.785E+06	60	0.	999

85	5.463E-03	-44	-6.462E-02	50	9.577E+05	62	0.	999
86	0.	999	0.	999	3.762E-07	56	-2.352E-08	-48
87	6.462E-02	50	-5.463E-03	-44	0.	999	0.	999

TABLE 8B- ENVELOPES OF MAXIMUMS * = HELD FROM PRIOR PROBLEM

STA	MAX +SHEAR LOC		MAX -SHEAR LOC		MAX +REACT LOC		MAX -REACT LOC	
-1	3.919E-09	38	-3.135E-08	-68	0.	999	0.	999
0	7.408E+04	-6	0.	999	8.096E+04	-8	0.	999
1	7.308E+04	-6	0.	999	0.	999	0.	999
2	6.753E+04	-60	0.	999	0.	999	0.	999
3	6.653E+04	-60	0.	999	0.	999	0.	999
4	6.116E+04	-58	0.	999	0.	999	0.	999
5	6.016E+04	-58	0.	999	0.	999	0.	999
6	5.487E+04	-56	0.	999	0.	999	0.	999
7	5.387E+04	-56	0.	999	0.	999	0.	999
8	4.810E+04	-54	-6.257E+02	46	0.	999	0.	999
9	4.710E+04	-54	-1.626E+03	46	0.	999	0.	999
10	4.143E+04	-52	-2.626E+03	46	0.	999	0.	999
11	4.043E+04	-52	-5.748E+03	-68	0.	999	0.	999
12	3.488E+04	-50	-6.748E+03	-68	0.	999	0.	999
13	3.388E+04	-50	-1.116E+04	-66	0.	999	0.	999
14	2.847E+04	-48	-1.216E+04	-66	0.	999	0.	999
15	2.747E+04	-48	-1.654E+04	-64	0.	999	0.	999
16	2.145E+04	-46	-1.754E+04	-64	0.	999	0.	999
17	2.045E+04	-46	-2.304E+04	-62	0.	999	0.	999
18	1.460E+04	-44	-2.404E+04	-62	0.	999	0.	999
19	1.360E+04	-44	-3.095E+04	-4	0.	999	0.	999
20	7.545E+03	-42	-3.195E+04	-4	0.	999	0.	999
21	6.545E+03	-42	-3.863E+04	-2	0.	999	0.	999
22	3.090E+02	-40	-3.963E+04	-2	0.	999	0.	999

23					0.	999	0.	999
24	0.	999	-4.596E+04	0	0.	999	0.	999
25	0.	999	-4.696E+04	0	0.	999	0.	999
26	0.	999	-5.353E+04	2	0.	999	0.	999
27	0.	999	-5.453E+04	2	0.	999	0.	999
28	0.	999	-6.074E+04	4	0.	999	0.	999
29	0.	999	-6.174E+04	4	0.	999	0.	999
30	0.	999	-6.760E+04	6	0.	999	0.	999
31	0.	999	-6.860E+04	6	0.	999	0.	999
32	0.	999	-7.423E+04	8	0.	999	0.	999
33	0.	999	-7.523E+04	8	0.	999	0.	999
34	0.	999	-8.063E+04	10	0.	999	0.	999
35	0.	999	-8.163E+04	10	0.	999	0.	999
36	0.	999	-8.668E+04	12	0.	999	0.	999
37	0.	999	-8.768E+04	12	0.	999	0.	999
38	0.	999	-9.261E+04	14	0.	999	0.	999
39	0.	999	-9.361E+04	14	0.	999	0.	999
40	0.	999	-9.821E+04	16	0.	999	0.	999
41	0.	999	-9.921E+04	16	0.	999	0.	999
42	0.	999	-1.037E+05	-38	0.	999	0.	999
43	0.	999	-1.047E+05	-38	1.920E+05	2	0.	999
44	1.109E+05	36	0.	999	0.	999	0.	999
45	1.039E+05	38	0.	999	0.	999	0.	999
46	1.029E+05	38	0.	999	0.	999	0.	999
47	9.645E+04	40	0.	999	0.	999	0.	999
48	9.545E+04	40	0.	999	0.	999	0.	999
49	8.875E+04	42	0.	999	0.	999	0.	999
50	8.775E+04	42	0.	999	0.	999	0.	999
51	8.196E+04	-12	0.	999	0.	999	0.	999
52	8.096E+04	-12	0.	999	0.	999	0.	999
53	7.644E+04	-10	0.	999	0.	999	0.	999

54	7.544E+04	-10	0.	999	0.	999	0.	999
55	7.059E+04	-8	0.	999	0.	999	0.	999
56	6.959E+04	-8	0.	999	0.	999	0.	999
57	6.464E+04	-6	0.	999	0.	999	0.	999
58	6.364E+04	-6	0.	999	0.	999	0.	999
59	5.834E+04	-4	0.	999	0.	999	0.	999
60	5.734E+04	-4	0.	999	0.	999	0.	999
61	5.170E+04	-2	0.	999	0.	999	0.	999
62	5.070E+04	-2	0.	999	0.	999	0.	999
63	4.471E+04	0	-8.606E+02	-18	0.	999	0.	999
64	4.371E+04	0	-4.636E+03	-16	0.	999	0.	999
65	3.748E+04	2	-5.636E+03	-16	0.	999	0.	999
66	3.648E+04	2	-9.733E+03	-14	0.	999	0.	999
67	2.990E+04	4	-1.073E+04	-14	0.	999	0.	999
68	2.890E+04	4	-1.579E+04	44	0.	999	0.	999
69	2.198E+04	6	-1.679E+04	44	0.	999	0.	999
70	2.098E+04	6	-2.317E+04	46	0.	999	0.	999
71	1.428E+04	8	-2.417E+04	46	0.	999	0.	999
72	1.328E+04	8	-3.073E+04	48	0.	999	0.	999
73	6.814E+03	10	-3.173E+04	48	0.	999	0.	999
74	5.814E+03	10	-3.808E+04	50	0.	999	0.	999
75	3.238E+03	-44	-3.908E+04	50	0.	999	0.	999
76	2.238E+03	-44	-4.522E+04	52	0.	999	0.	999
77	1.238E+03	-44	-4.622E+04	52	0.	999	0.	999
78	2.378E+02	-44	-5.249E+04	54	0.	999	0.	999
79	0.	999	-5.349E+04	54	0.	999	0.	999
80	0.	999	-5.913E+04	56	0.	999	0.	999
81	0.	999	-6.013E+04	56	0.	999	0.	999
82	0.	999	-6.588E+04	58	0.	999	0.	999
83	0.	999	-6.688E+04	58	0.	999	0.	999
	0.	999	-7.273E+04	60				

84	0.	999	-7.373E+04	60	0.	999	0.	999
85	0.	999	-7.966E+04	62	0.	999	0.	999
86	1.960E-09	-48	-3.135E-08	56	8.016E+04	62	0.	999
87					0.	999	0.	999

TABLE 9 -- SCALES FOR PLOTS OF THE ENVELOPES OF MAXIMUMS

HORIZONTAL SCALE
10 INCHES = 100 STATIONS

VERTICAL SCALES

VARIABLE	LENGTH OF AXIS	MAXIMUM VALUE
DEFLECT	2 INCHES	= 1.000E+00
MOMENT	2 INCHES	= 2.000E+07
SHEAR	2 INCHES	= 2.000E+05
REACTION	2 INCHES	= 2.000E+05

PROB (CONTD)

5 TWO SPAN GIRDER, RAILROAD LOADING

TABLE 10A -- INFLUENCE DIAGRAMS FOR DEFLECTION

LOCATION OF LOAD	STA	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS	STA	STA	STA
NONE					

TABLE 10B -- INFLUENCE DIAGRAMS FOR MOMENT

LOCATION OF LOAD	STA	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS	STA	STA	STA
NONE					

TABLE 10C -- INFLUENCE DIAGRAMS FOR SHEAR

(SHEAR IS COMPUTED ONE HALF INCREMENT
TO THE LEFT OF THE DESIGNATED STATION)

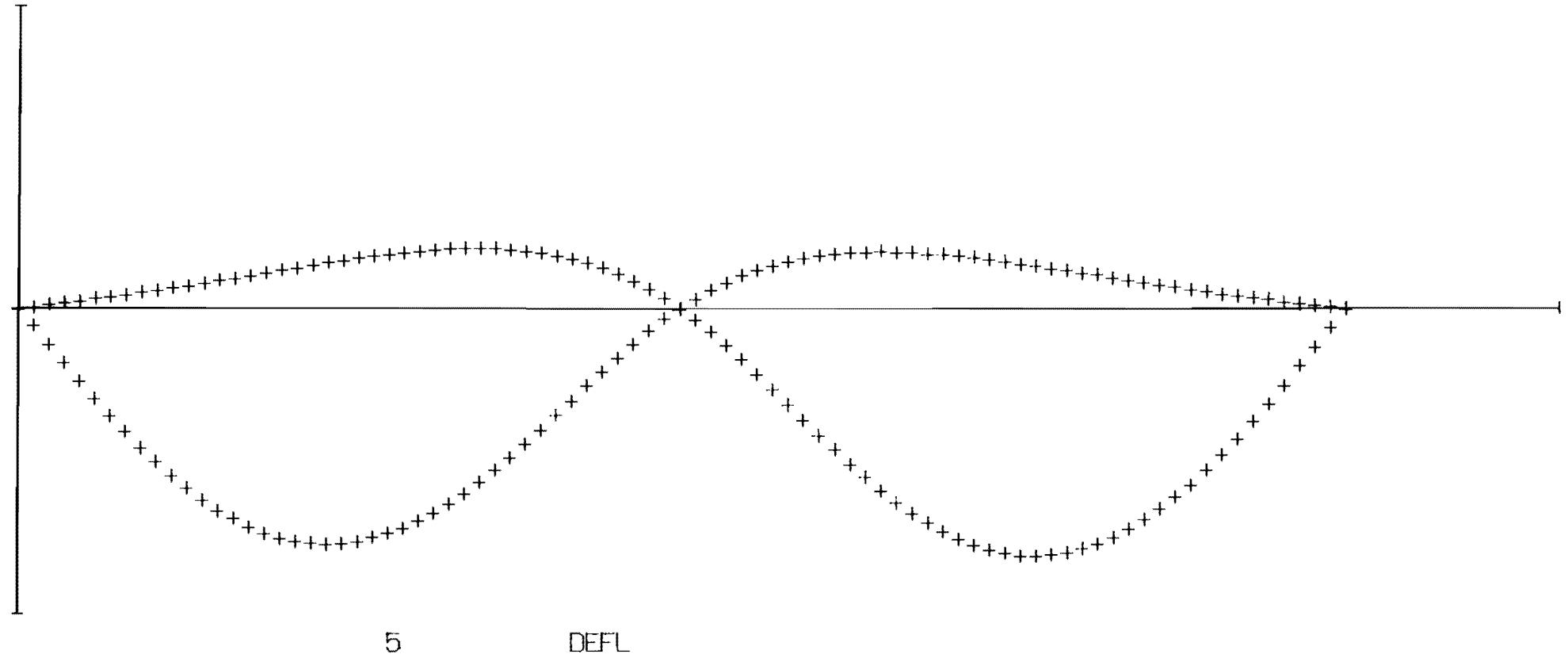
LOCATION OF LOAD	STA	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS	STA	STA	STA
NONE					

TABLE 10D -- INFLUENCE DIAGRAMS FOR SUPPORT REACTION

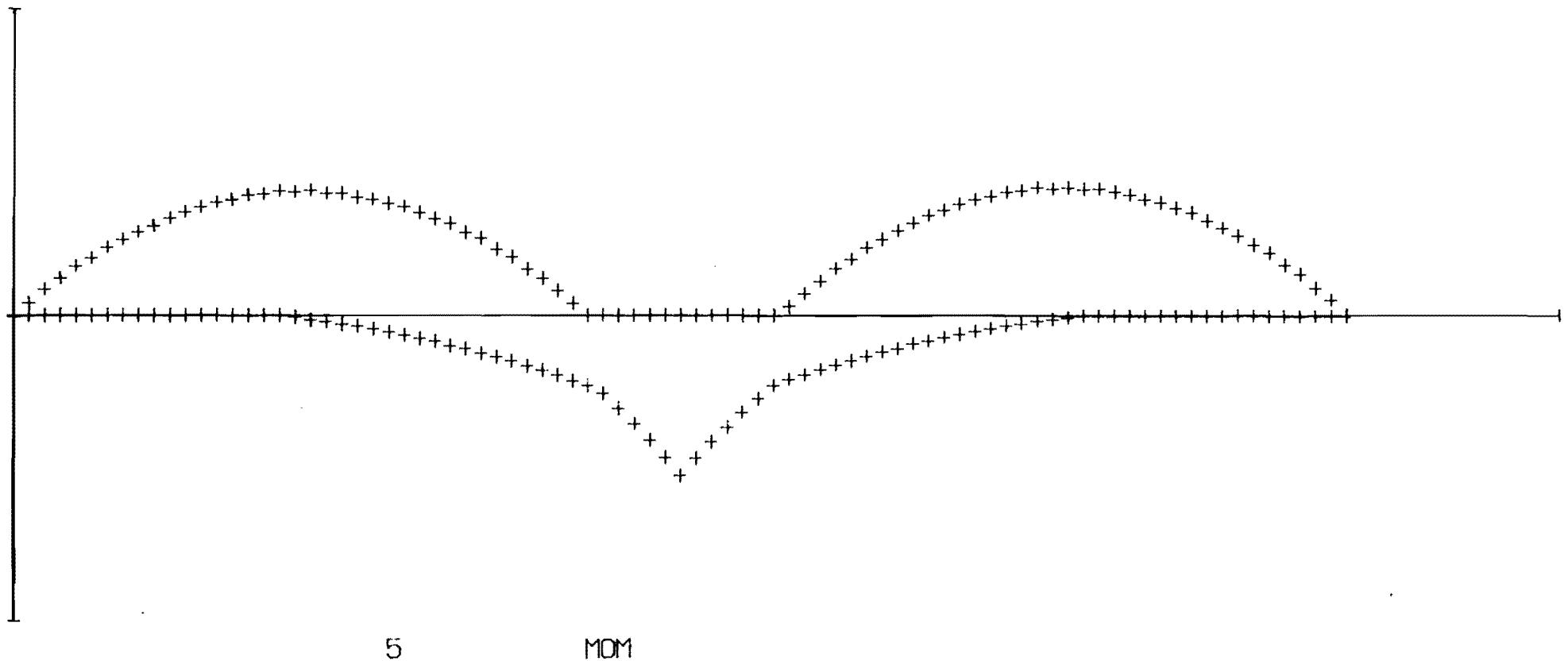
LOCATION OF LOAD	STA	DESIGNATED STATIONS FOR INFLUENCE DIAGRAMS	STA	STA	STA
NONE					

TIME FOR THIS PROBLEM = 0 MINUTES 5.207 SECONDS

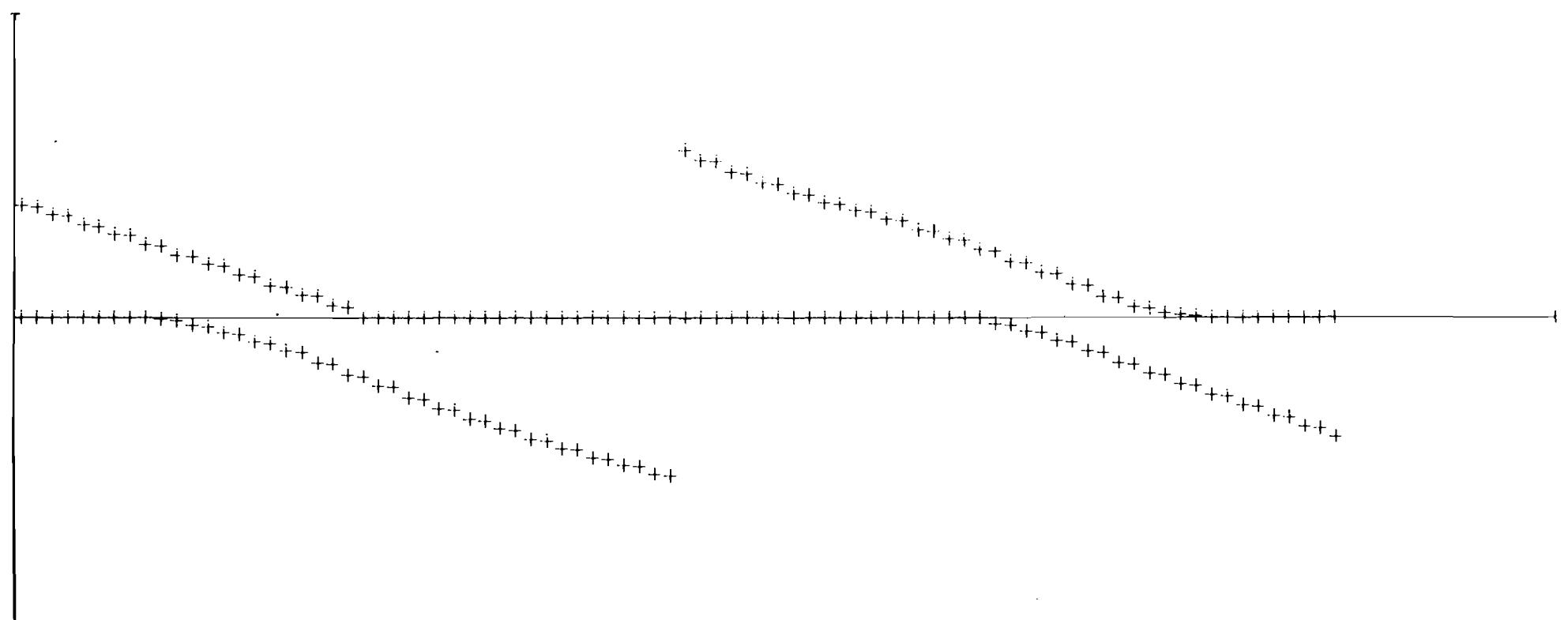
ELAPSED CPU TIME = 0' MINUTES 37.553 SECONDS



Prob 5. Two-span girder, railroad loading.
Envelope of maximum deflections.



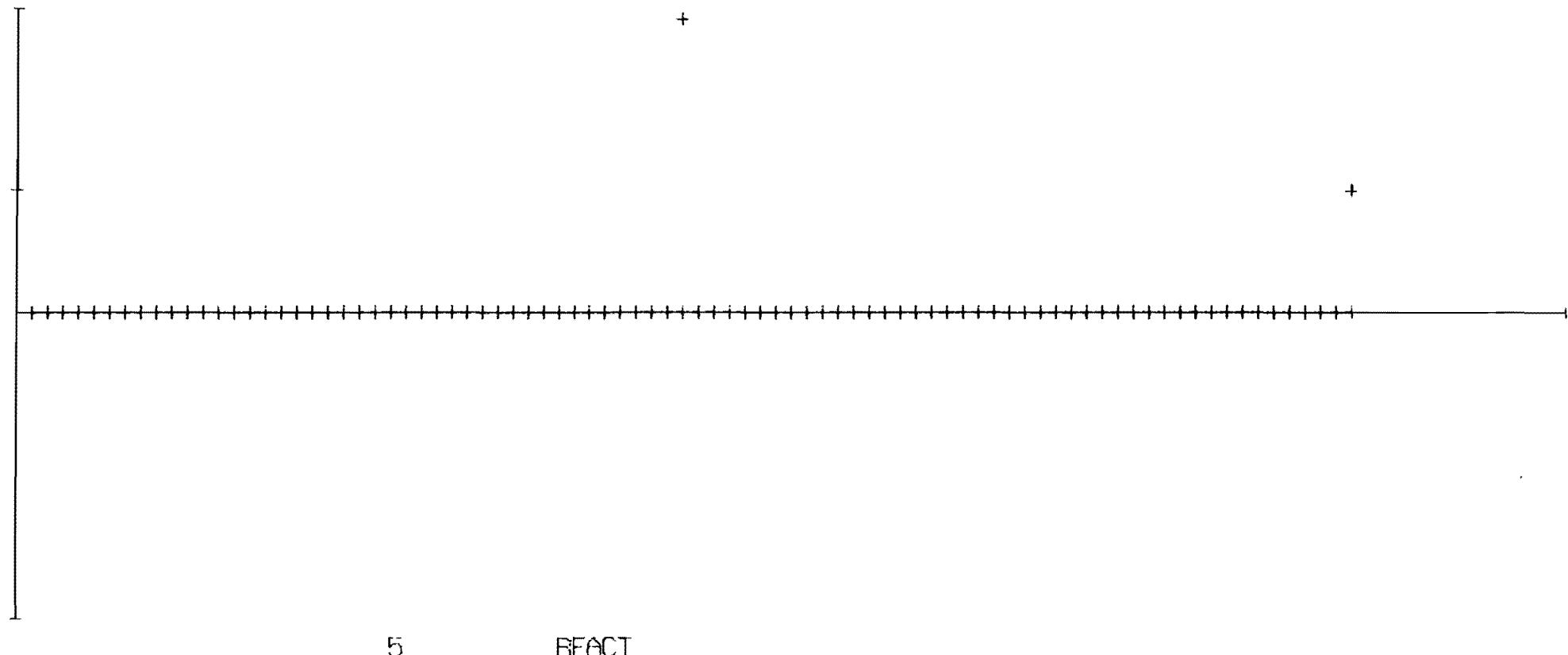
Prob 5. Two-span girder, railroad loading.
Envelope of maximum moments.



5

SHEAR

Prob 5. Two-span girder, railroad loading.
Envelope of maximum shears.



Prob 5. Two-span girder, railroad loading.
Envelope of maximum reactions.