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16. Abstract <p>In recent years, maximum legal truck size and weight limits have become major issues in the United States. The assessment of impacts due to changes in maximum limits is an ongoing dynamic problem faced by many highway departments and State legislatures. It has been difficult to predict future truck weight distribution patterns as affected by an alternative legislation governing truck weight. Consequently, it has become implausible to try to forecast precisely the benefits and costs associated with changes in weight limits.</p> <p>In the past, various methodologies for projecting truck weight distribution patterns have been developed. Each methodology makes some contributions to the assessment of changes in truck weight patterns. However, the precision of projection and the application of each methodology can yet be improved.</p> <p>In June 1977, the Texas SDHPT contracted the Center for Transportation Research to conduct a study into the truck size and weight issue. As a part of the truck study, a shifting methodology has been developed for the projection of future truck weight distribution patterns. This methodology can be applied either manually or by using a series of computer programs. It can be used to predict both gross vehicle weight and axle weight distributions.</p> <p>In this report, a brief review of available methodologies and a detailed discussion of the new methodology are presented. Illustrative applications of predicting gross vehicle weight and axle weight distributions as a result of changes in weight limits are presented in the text. Comparison of prediction results generated by all the available shifting methodologies is also included.</p>					
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TRUCK WEIGHT SHIFTING METHODOLOGY
FOR PREDICTING HIGHWAY LOADS

by

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Chien-pei Yu
Paul Ng

Research Report Number 241-5

Truck Use of Highways in Texas
Research Study Number 3-18-78-241

conducted for

Texas State Department of Highways and Public Transportation

by the

CENTER FOR TRANSPORTATION RESEARCH
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THE UNIVERSITY OF TEXAS AT AUSTIN

April 1983

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PREFACE

This is an interim report on Research Project 3-18-78-241, "Truck Use of Highways in Texas." This report represents one element of an ongoing study to assess the various issues and effects of an increase in truck size and weight on rural highways in Texas. Various joint interim reports,

- 231 "Effects of Heavy Trucks on Texas Highways,"
- 241-2 "An Assessment of Changes in Truck Dimensions on Highway Geometric Design Principles and Practices,"
- 241-3 "Operational Issues of Truck Terminals,"
- 241-4 "An Assessment of Recent State Truck Size and Weight Studies,"
- and 241-6F "An Assessment of the Enforcement of Truck Size and Weight Limitations in Texas."

have been published in the past or will be published in the very near future.

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ABSTRACT

In recent years, maximum legal truck size and weight limits have become major issues in the United States. The assessment of impacts due to changes in maximum limits is an ongoing dynamic problem faced by many highway departments and State legislatures. It has been difficult to predict future truck weight distribution patterns as affected by an alternative legislation governing truck weight. Consequently, it has become implausible to try to forecast precisely the benefits and costs associated with changes in weight limits.

In the past, various methodologies for projecting truck weight distribution patterns have been developed. Each methodology makes some contributions to the assessment of changes in truck weight patterns. However, the precision of projection and the application of each methodology can yet be improved.

In June 1977, the Texas SDHPT contracted the Center for Transportation Research to conduct a study into the truck size and weight issue. As a part of the truck study, a shifting methodology has been developed for the projection of future truck weight distribution patterns. This methodology can be applied either manually or by using a series of computer programs. It can be used to predict both gross vehicle weight and axle weight distributions.

In this report, a brief review of available methodologies and a detailed discussion of the new methodology are presented. Illustrative applications of predicting gross vehicle weight and axle weight distributions as a result of changes in weight limits are presented in the text. Comparison of prediction results generated by all the available shifting methodologies is also included.

KEY WORDS: truck, size, weight, motor carrier, shifting methodology, highway load, forecasting, load prediction, truck laws and regulations, inter- and intra-state commerce

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SUMMARY

One important element in the assessment of impacts due to changes in legal truck weight limits is the prediction of the shifting of future truck weight distribution as affected by the change. A number of methodologies have been developed for this purpose. However, with the availability of more recent truck weight data, most of these methodologies have been proven inadequate. A new methodology, known as the average GVW factor methodology, was developed. Both the development and the application of this methodology are discussed explicitly in this report.

The shifting methodology can be applied either automatically by computer software or manually with the aid of pocket calculators. For the former case, a series of computer programs was developed for immediate application. The shifting methodology, in general, can be divided into two phases; one phase to predict the average weight for the truck type under the proposed weight limits and another to shift a typical truck weight distribution curve to a new position so that the mean of the shifted curve is compatible with the average truck weight obtained in the first phase.

In predicting the average truck weight, a regression model can be constructed over the historical data and estimation performed thereafter. However, for a prediction of average truck weight affected by changes in legal weight limits, the expected value issued from the regression model may be purely a guess.

Based on extensive analyses of historical data and their relationship to past changes in legal weight limits, a ratio was found to remain quite stable regardless of the weight limits. The ratio is defined as the average GVW factor. It is the ratio between the current average GVW and the maximum practical GVW. An average GVW factor for each type of truck can be found. This finding is very significant in that once the maximum practical GVW is derived from the proposed weight limits, it is possible to find the expected average vehicle weight for the truck type. By applying this expected value

to the proposed shifting methodology, it may be possible to precisely predict the weight distribution for the truck type.

Another significance of the finding is the relationship discovered between the steering axle, tandem axle, and GVW distributions. For two representative types of trucks, 3A and 3-S2, it was found that the relationship of the three weight distribution types can be represented by the axle configurations. In other words, the tandem axle weight distributions for 3A can be constructed by the algebraic subtraction of the single (steering) axle weight distribution from the GVW distribution at the specific percent intervals. It was observed that the steering axle weight distributions for most of the truck types did not undergo significant changes in the past years. Based on these findings, it becomes possible to predict tandem axle weight distribution patterns for vehicles such as 3A and 3-S2. One may obtain a precise GVW distribution curve for either 3A or 3-S2 from the average GVW factor and the proposed shifting methodology. Then, by algebraic subtraction, one may obtain a precise tandem axle distribution for the truck type.

Although the methodology was developed by analyzing Texas data only, the principles behind the methodology can be applied to other states. Compared to other methodologies, the proposed one requires analysis of more historical data and the shifting procedure is quite time-consuming. However, with available computer software, this shortcoming can easily be overcome. The design of better roadway systems is based on precise prediction, and optimum design should, by no means, be sacrificed in the interest of reducing effort.

IMPLEMENTATION STATEMENT

This report deals with one element of the ongoing study to assess the various issues and effects of increased truck size and/or weight on the rural highways in Texas. This element is the methodology of predicting truck weight distribution patterns as a result of changes in weight legislation. This report should be used in concert with previous and/or subsequent reports as a guide in the consideration of the realism of issues surrounding vehicle size and/or weight limits. The methodology provided in this report will assist with the estimation of changes in truck weight distribution patterns associated with different degrees of changes in weight limits. It also provides a guide to the assessment of truck weight distribution patterns associated with various degrees of weight violation.

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DEFINITION OF TERMS AND ACRONYMS

AASHTO	The American Association of State Highway and Transportation Officials (formerly the AASHO: The American Association of State Highway Officials)
AGVWF	Average Gross Vehicle Weight Factor
CTR	Center for Transportation Research
18-KESAL	Eighteen-Kip Equivalent Single Axle Loads
FHWA	The Federal Highway Administration
GVW	Gross Vehicle Weight
NCHRP	The National Cooperative Highway Research Program
PMGVWF	Practical Maximum Gross Vehicle Weight (Future)
PMGVWP	Practical Maximum Gross Vehicle Weight (Present)
SAW	Single Axle Weight
SDHPT	The Texas State Department of Highways and Public Transportation
TAW	Tandem Axle Weight

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METRIC CONVERSION TABLE

1 pound force = 4.448 newtons

1 kip = 1,000 pounds = 4.448 kilonewtons

1 ton = 2 kips = 2,000 pounds = 8.896 kilonewtons

1 inch = 25.40 millimeters

1 foot = 12 inches = 304.8 millimeters

1 mile = 5,280 feet = 63,360 inches = 1.609 kilometers

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TABLE OF CONTENTS

PREFACE	iii
ABSTRACT	v
SUMMARY	vii
IMPLEMENTATION	ix
DEFINITION OF TERMS AND ACRONYMS	xi
METRIC CONVERSION TABLE	xiii
LIST OF TABLES	xix
LIST OF FIGURES	xxi
CHAPTER 1. INTRODUCTION	
Available Shifting Methodologies	3
Data Base of the Research	4
Organization of the Report	6
CHAPTER 2. EXISTING SHIFTING METHODOLOGIES AND THE DEVELOPMENT OF A NEW SHIFTING PROCEDURE	
Evaluation of Available Shifting Methodologies	7
Development of a New Shifting Procedure	8
Description of the Methodology	17
Estimators for the Shifting Procedure	19
Discussion of the New Shifting Procedure	23
Analysis of Historical Truck Weight Data	23
CHAPTER 3. A SHIFTING PROCEDURE TO INCORPORATE INFLUENCES DUE TO CHANGES IN TRUCK WEIGHT LIMITS	
Derivation of Average GVW Factors	27
One Sample t-test	32
Two Sample t-test	35
Relationship between Practical Maximum GVW's and Average GVW Factors	36
CHAPTER 4. MANUAL APPLICATION OF THE SHIFTING PROCEDURE	
Shifting of Truck Weight Distribution Curve	43
Preparation of a Cumulative Frequency Curve	45
Shifting of Curve to Obtain Expected Mean and Variance	45

Statistical Tests Applied in the Procedure	50
Student t-test of the Mean	50
Chi-squared (χ^2)-test of the Variance	51
General Discussion of the Methodology	53
CHAPTER 5. COMPUTERIZED SHIFTING METHODOLOGY IN THE ESTIMATION OF TRUCK WEIGHT DISTRIBUTION	
Modifications of Shifting Methodology to be Compatible with Automation	57
Iteration Method in the Shifting Procedure	61
Limitations of the Computer Program	63
Applications of the Computerized Shifting Methodologies . . .	64
Comparison of Various Shifting Methodologies	68
General Discussion of the Methodology	72
CHAPTER 6. PREDICTIONS OF AXLE WEIGHT DISTRIBUTION AND EQUIVALENT 18-KIP SINGLE AXLE LOAD	
Estimation of Tandem Axle Weight Distribution from Available Data	79
Prediction for Tandem Axle Weight Distribution under Proposed Truck Weight Limits	85
Example of Prediction of Tandem Axle Weight Distribution . . .	86
Calculation of Equivalent 18-Kip Single Axle Load	86
Comments on the Axle Weight Shifting Methodology	92
CHAPTER 7. SUMMARY AND RECOMMENDATIONS	
Summary of New Concepts Used in the Shifting Methodology . . .	99
Assumptions Made in the Development of the Shifting Methodology	100
Recommendations	101
REFERENCES	102
APPENDICES	
Appendix 1. Source Program of "MEANWGT"—To Compute Mean and Variance of Truck Weight Distribution Data	105
Appendix 2. Source Program of "SHIFTIN"—A Computerized Shifting Procedure	111
Appendix 3. Source Program of "TAWEXP"— Shifting Program for Tandem Axle Weight Distribution	145

Appendix 4.	Source Program for Shifting of Truck Weight Distribution Based on NCHRP/SDHPT Procedure . . .	153
Appendix 5.	Input Format and Illustration for "MEANWGT" . . .	157
Appendix 6.	Input Format and Illustration for "SHIFTIN" . . .	161
Appendix 7.	Input Format and Illustration for "TAWEXP" . . .	169
Appendix 8.	Sample Output from "SHIFTIN"	173

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LIST OF TABLES

<u>Table</u>		<u>Page</u>
1	Example of Computation of Mean and Variance for Truck Weight Distribution Data (3-S2, Texas Interstate Rural Highways, 1980)	24
2	Relationship of Average GVW and Practical Maximum GVW for 2D, Texas Interstate Rural Highways	28
3	Relationship of Average GVW and Practical Maximum GVW for 3A, Texas Interstate Rural Highways	29
4	Relationship of Average GVW and Practical Maximum GVW for 3-S2, Texas Interstate Rural Highways	30
5	Relationship of Average GVW and Practical Maximum GVW for 2-S1-2, Texas Interstate Rural Highways	31
6	Recommended Average GVW Factors for Four Types of Trucks Operating on Texas Interstate Rural Highways	32
7	Summary of Results from One Sample t-tests	33
8	Summary of Results from Two Sample t-tests	36
9	Practical maximum Steering Axle Limits for Trucks	41
10	Practical Maximum GVW for Trucks in Texas	41
11	Computation of Mean and Variance from an Estimated Cumulated Distribution Curve	49
12	Chi-squared Test on Observed and Predicted Truck Weight Distributions	54
13	Comparison of Accumulated Frequencies of Actual Field Data and Predicted from Shifting Model for 2D on Texas Interstate Rural Highways (Prediction is for 1974)	67
14	Comparison of Cumulative Frequencies for Truck Weight Distribution for 3-S2 Based on Actual Field Data and Prediction by Computerized Shifting Model (Prediction is for 1978)	70

<u>Table</u>	<u>Page</u>
15 Initial Weights of Four Truck Types Used in NCHRP and SDHPT Methodologies (Based on 1974 Truck Weight Data)	68
16 Comparison of Projected Cumulated Frequency Curves Generated from Available Shifting Methodologies and the Actual Field Data for 2D on Texas Interstate Rural Highways (Projection is for 1978)	74
17 Comparison of Projected Accumulated Frequency Curves Generated from Available Shifting Methodologies and the Actual Field Data for 3-S2 on Texas Interstate Rural Highways (Projection is for 1978)	76
18 Chi-square values to Show the Goodness-of-Fit Between Actual and Predicted Tandem Axle Weight Distribution Curves	84
19 Projected GVW Distribution for 1978, 3-S2, Texas Interstate Rural Highways (Input Data for Projection of TAW, 1978)	89
20 Single Axle Weight Distribution of 3-S2 on Texas Interstate Rural Highways (Input Data for Projection of TAW, 1978)	90
21 Prediction of 1978 Tandem Axle Weight Distribution Based on Projected 1978 GVW and Actual 1974 SAW Distribution Data	91
22 Example of Determination of Equivalent 18-Kip (80KN) Single Axle Loads from Loadometer Station Data	93
23 Computation of Actual and Predicted 18 KESAL for Flexible Pavement (3-S2, Texas Interstate Rural Highways)	95
24 Computation of Actual and Predicted 18 KESAL for Rigid Pavement (3-S2, Texas Interstate Rural Highways)	96

LIST OF FIGURES

<u>Figure</u>		<u>Page</u>
1	Influence flow between legal vehicle weight limits, basic elements, and benefit and costs to highways, users, and society	2
2	Vehicle configurations included in the study	5
3	Typical historical shifts in gross vehicle distributions	9
4	An example of multipliers adopted for shifting GVW distributions	10
5	Shifting of GVW distribution of 2D after weight limits changed in 1975	11
6	Shifting of GVW distribution of 3A after weight limits changed in 1975	12
7	Shifting of GVW distribution of 3-S2 after weight limits changed in 1975	13
8	Shifting of GVW distribution of 2-S1-2 after weight limits changed in 1975	14
9	Comparison of NCHRP, SDHPT, and actual multiplying factors for interstate rural highways	15
10	Comparison of NCHRP, SDHPT, and actual multiplying factors for other main rural highways	16
11	Average gross vehicle weight for four types of trucks on Texas and interstate rural highways	18
12	A set of typical truck weight distribution curves showing shifting occurred after weight limits changed in 1975	20
13	Trends of mean and variance of GVW distribution for 3-S2 on Texas interstate rural highways	21
14	Truck GVW distribution curves obtained before the 1975 weight law changes	22

<u>Figure</u>		<u>Page</u>
15	Distribution of I when H_0 is true	34
16	Axle weight analysis (single axle) for 3A, interstate rural, 1974-1979, state of Texas	38
17	Axle weight analysis (steering axle) for 3-S2, interstate rural, 1970-1973, state of Texas	39
18	Axle weight analysis (steering axle) for 3-S2, interstate rural, 1974-1979, state of Texas	40
19	Flowchart showing the manual application of the shifting procedure	44
20	First trial shifting from 1970 data for the projections of 1978 GVW distribution, 3-S2, Texas interstate rural highways	46
21	Acceptable shifting for the projection of 1978 GVW distribution, 3-S2, Texas interstate rural highways	48
22	Flowchart showing procedure of computation of mean and variance	58
23	Flowchart showing the computerized shifting methodology	62
24	Trends of mean and variance for truck GVW distribution (2D, Texas interstate rural highways)	65
25	Comparison of accumulated frequency distribution curves derived from actual field data and computerized shifting methodology	66
26	Comparison of actual and predicted GVW distribution curves for 3-S2 on Texas interstate rural highways	69
27	Comparison of GVW distribution curves derived from actual field data, NCHRP, SDHPT, and the computerized AGVWF shifting procedure	73
28	Comparison of GVW distribution curves derived from actual field data, NCHRP, SDHPT, and the computerized AGVWF shifting procedure	75
29	Schematic representation illustrating the procedure to estimate the additional pavement cost and highway rehabilitation cost for changing motor vehicle weight limits	80

<u>Figure</u>		<u>Page</u>
30	Comparison of actual and expected tandem axle distributions for 3A on Texas interstate rural highways	82
31	Comparison of actual and expected tandem axle distributions for 3-S2 on Texas interstate rural highways	83
32	Flowchart for predicting tandem axle weight distribution for 3A and 3-S2	87
33	Comparison of actual and predicted tandem axle weight distribution for 3-S2 on Texas interstate rural highways	88
34	Shifting procedure and computation of 18-Kip equivalent single axle loads	94

CHAPTER 1. INTRODUCTION

Changes in legal truck weight and size limits always result in a complicated interaction among highway systems, transport economics, industry, and society. The issue is a complex problem faced by state legislatures and highway engineers. Its complexity is due to the fact that the assessment of impacts of changes involves an interaction of a multitude of disciplines.

The impacts due to changes of legal weight and size limits may be quantified in monetary terms, which can be categorized into benefit and cost components. The major elements of benefit are social (i.e., decreased consumer prices) and motor freight industry benefits (i.e., decreased operating costs). The major elements of cost are highway costs, social costs, and user costs. The highway costs may be subdivided into bridge cost, pavement cost, and maintenance cost. Social costs may be subdivided into costs of noise pollution, air pollution, vibration, and, most important of all, the indirect cost of accidents. User costs have the elements of direct cost of accidents and cost of travel delays. The impact on these economical elements due to changes of legal vehicle limits can be assessed by studying the changes in vehicle operating characteristics, which are the initial and direct result of the legal limit changes. The relationship between these elements and the changes of legal vehicle weight limits is shown in Fig 1.

To evaluate the impact of proposed changes in legal weight limits, vehicle operating characteristics must be precisely forecast. One of the major elements in the vehicle operating characteristics is the future truck weight pattern. The pattern is composed of two components—namely, the gross vehicle weight (GVW) frequency distributions and axle weight frequency distributions. The prediction of the GVW distribution directly affects the assessment of the efficiency of truck operation. For an increase in allowable GVW, a given quantity of payload can possibly be hauled by fewer vehicles. In other words, fewer trips may be required as the payload per vehicle increases; thus, total costs incurred by the truckers might decrease. The prediction of

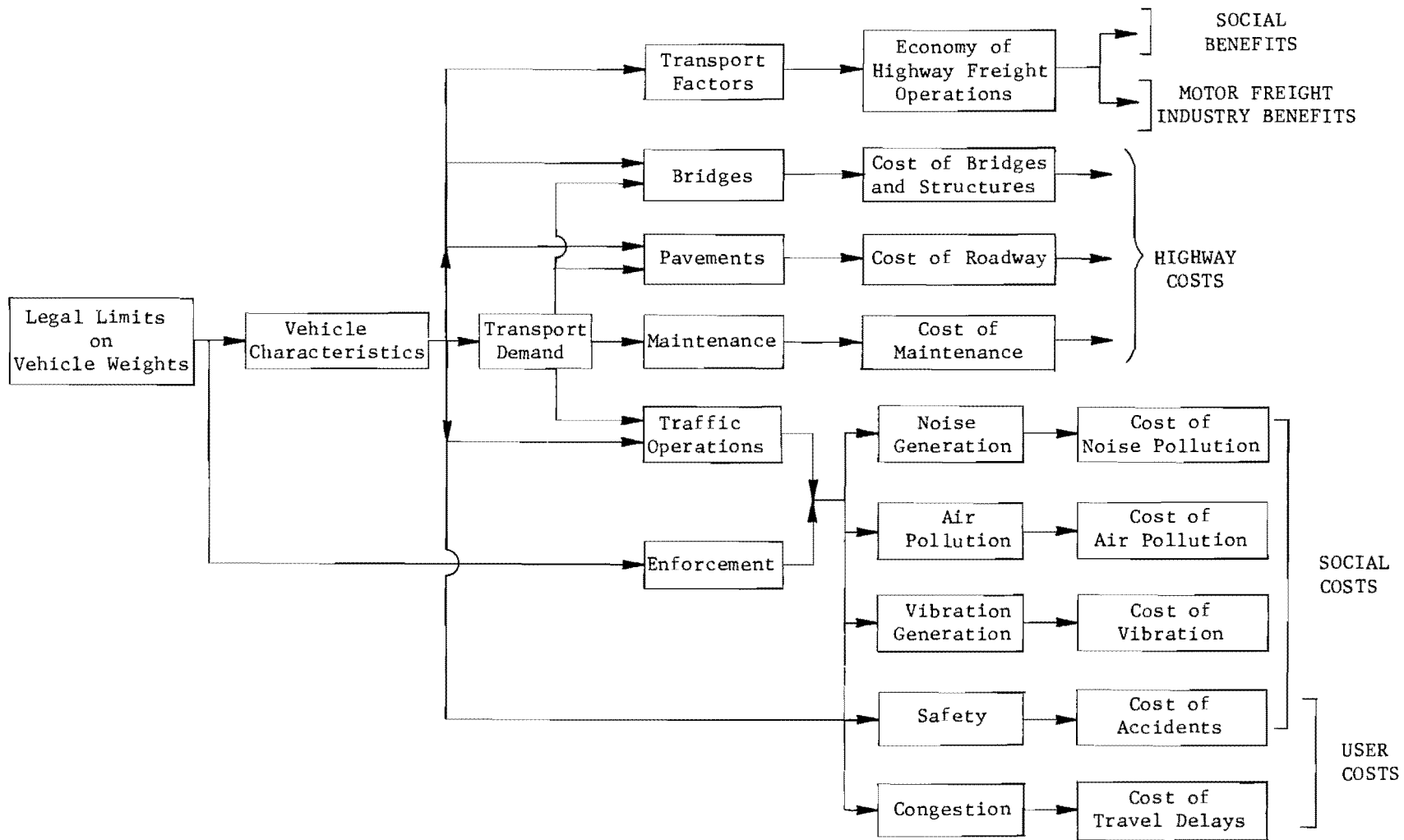


Fig 1. Influence flow between legal vehicle weight limits, basic elements, and benefits and costs to highways, users, and society (Ref 15).

axle weight distribution, which has a close relationship with GVW distribution, directly influences the calculation of highway costs. According to the methodology suggested by AASHTO in pavement design, a precise prediction of axle load distribution will yield a better and more efficient pavement system. Moreover, a precise prediction of axle load distribution will provide a better estimation of pavement cost and bridge cost as results of changes in legal weight limits.

AVAILABLE SHIFTING METHODOLOGIES

Since the prediction of future weight distributions is so vital to the evaluation of impacts due to changes in legal weight limits, a number of methodologies were developed in the past. The effort was focused on forecasting of future truck weight trends precisely so that engineers and planners "may adequately assess the impact of such legislation on the economic vitality of the states and the nation." (Ref 7). The process of predicting future truck weight trends is known as the shifting procedure. The term implies that a truck weight frequency distribution curve is shifted from one position to another as affected by the changes in weight limits. In the past, four different shifting procedures have been developed by federal or state transportation agencies:

- (1) first FHWA procedure,
- (2) second FHWA procedure,
- (3) NCHRP procedure, and
- (4) SDHPT procedure.

These procedures will be discussed in the next chapter with the evaluations of their precision and applications.

The impact of a change in legal vehicle size has an influence on the economic elements similar to that caused by the changes in weight limits. However, changes in dimensions cannot be easily forecast because of "the complexity of possible combinations of dimensions and their relationship to geometric design, highway operations, safety, etc" (Ref 15).

Thus, in this report, only the procedure for forecasting truck weight distributions will be presented. The data base for the analysis and research was provided by FHWA (Ref 11).

DATA BASE OF THE RESEARCH

The Transportation Planning Division of the Texas State Department of Highways and Public Transportation is responsible for collecting field truck weight data. These data are then turned over to FHWA, where they are processed and summarized into W-tables. These W-tables are generally published in seven different forms in the annual truck weight survey study report. The following information is obtained from each table (Ref 17).

- Table W-1. Location and time of operation of each truck weigh station and the description and the number of vehicles weighed by type compared to corresponding data from the previous year.
- Table W-1A. Gives the same type of information as W-1, except that W-1 is based on vehicles weighed, while W-1A is based on vehicles counted.
- Table W-2. Gives the number and percentage of vehicles of each type counted at truck weigh station by highway system.
- Table W-3. Gives the number of loaded and empty vehicles counted and average loads of vehicles of each type counted and weighed at the stations by highway system.
- Table W-4. Gives the axle weight distribution at various magnitudes of different truck types counted and weighed at truck weigh stations by highway system.
- Table W-5. Gives the distribution of GVW of different types of vehicles by stations and by highway system.
- Table W-6. This table shows the axle weight, axle spacing, and gross weight of trucks in violation of State limit based on AASHTO recommendations.
- Table W-7. Gives the number and accumulative percentage of vehicles at or below State limit based on AASHTO recommendations.

Based on data provided in W-tables, a study on the shifting procedure was conducted at the Center for Transportation Research. The shifting procedure was studied as a part of the research project entitled "Truck Use of Highways in Texas," which was sponsored by the Texas State Department of Highways and Public Transportation.

In this report, evaluations of several available shifting methodologies based on the truck weight W-tables published since 1959, up to 1979, are made. In the evaluations, four vehicle types were considered. The four vehicle types selected in the analysis constitute the majority of the payload carrying trucks operating on the Texas highway network. These truck types are

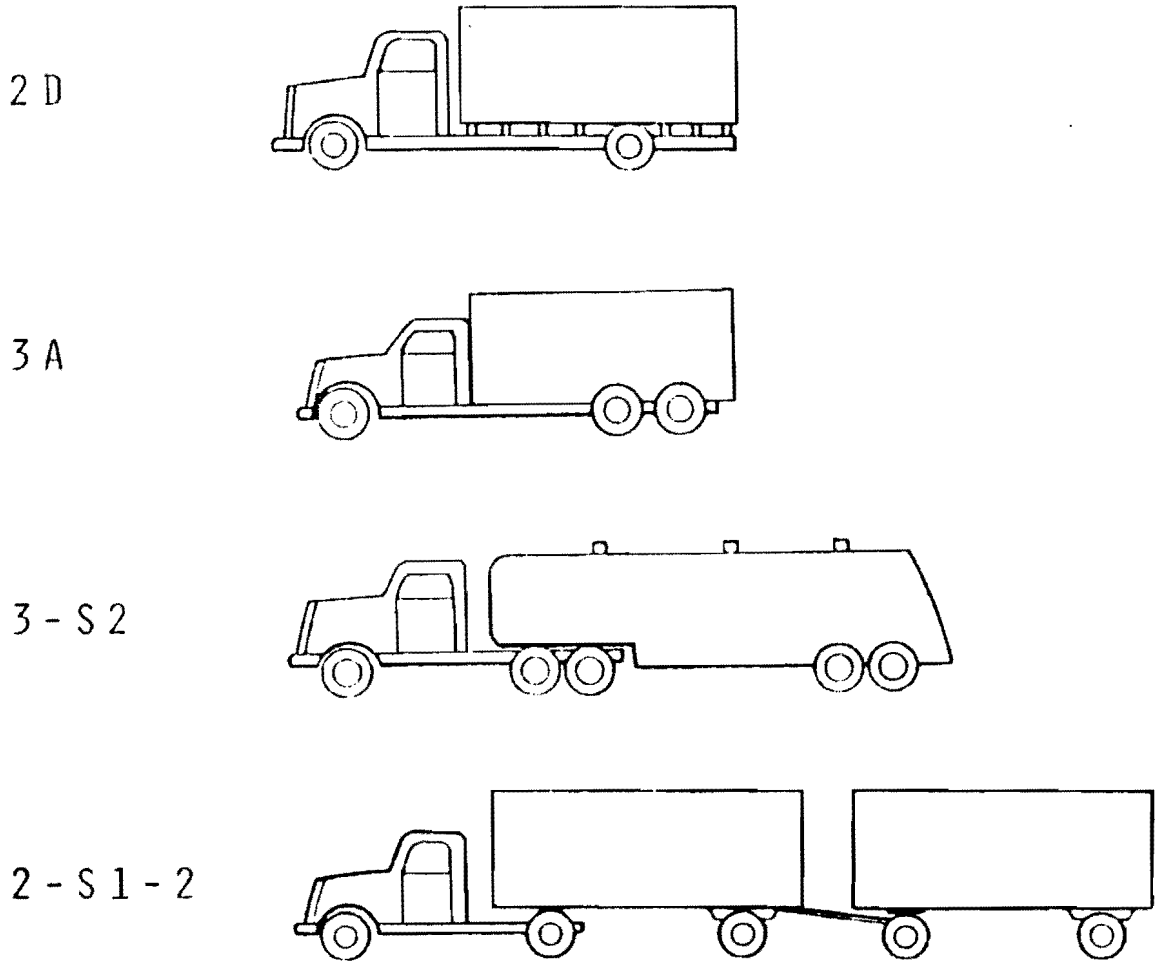


Fig 2. Vehicle configurations included in the study.

2D, 3A, 3-S2, and 2-S1-2. Diagrammatical presentations of these four truck types are provided in Fig 2. With extensive use of GVW distribution and axle weight distribution data, a new methodology known as the Average GVW Factor Procedure was developed at the Center. This procedure can be used in predicting both GVW and axle weight distributions as a result of changes in legal weight limits.

ORGANIZATION OF THE REPORT

In Chapter 2, the existing shifting methodologies will be discussed briefly. Based on the contribution of each shifting methodology, a new shifting procedure was developed. The development of this methodology is discussed explicitly.

The discussion in Chapter 3 relates to the shifting procedure with respect to forecasting truck weight distribution after changes in truck weight limits. A factor known as Average GVW Factor is used to forecast weight distribution trends under proposed limits. The derivation of this factor is also discussed.

Within Chapter 4, the application procedure for the shifting methodology is presented as an illustrative example.

In Chapter 5, the computer procedure of the shifting methodology is introduced. Modifications of the methodology to cope with computer application are discussed in the same chapter.

In Chapter 6, the application of this shifting procedure in forecasting axle weight distribution and the 18-kip equivalent single axle load applications is presented.

Summary and recommendations are provided in the last chapter.

CHAPTER 2. A NEW METHODOLOGY FOR ESTIMATING SHIFTS IN VEHICLE WEIGHT DISTRIBUTION—AVERAGE GVW FACTOR APPROACH

In light of the materials presented in the previous chapter, a brief summary of the evaluations of the available shifting methodologies is presented in the beginning of this chapter. In the second half of the chapter, a modified shifting methodology will be presented.

EVALUATION OF AVAILABLE SHIFTING METHODOLOGIES

The first procedure was published in 1970 by the FHWA in the report "Manual Procedures for Conducting Studies of the Desirable Limits of Dimensions and Weights of Motor Vehicles" (Ref 16). The procedure estimates axle weight distribution by resorting to data from states having higher vehicle size and weight limits. When other states have data for the proposed limits, this procedure may be a useful one, assuming other influences are similar or not significant. Otherwise, this procedure is not flexible enough to study size and weight limits that are not found to exist in other states' size and weight laws (Ref 7).

The second procedure was published in the same report (Ref 16). It uses existing data to predict weight redistribution under the proposed limits. This procedure assumes that "both vehicle empty weights and vehicle payloads will increase with an increase in gross vehicle weight limits and axle weight limits" (Ref 7). The procedure does not take into account commodities which reflect the volume and demand constraints on vehicle usage. Hence, it projects shifting in the lower portion of the weight distribution curve which may not occur (Ref 7).

The NCHRP procedure provides more flexibility in adjusting for volume and demand constraints (Ref 15). In this procedure, ratios of the practical maximum GVW under present and proposed limits are obtained. Through these ratios, or multiplying factors, the weight distribution under the present limit is shifted. The pattern of shift in the NCHRP Shifting Procedure is based on prior research, which indicates that the GVW distribution is shifted to the

right with an increase in GVW limit or axle weight limit. This pattern is illustrated in Fig 3. Nevertheless, "this model was based on 1962 truck weight study data and did not apply to the 1975 weight law change situation" (Ref 17). The NCHRP model was reviewed during the initial phase of the Texas truck weight study and modifications were recommended. These recommendations were then incorporated into the SDHPT model (Ref 17).

The SDHPT shifting model (named after the Texas State Department of Highways and Public Transportation) is different from the NCHRP model in the following major areas (Ref 17).

1. Heavily loaded vehicle trips would shift to a higher GVW in proportion to the ratio of practical maximum weight at the future upper limit and practical maximum weight at the present upper limit.
2. Empty and lightly loaded vehicles would be unaffected by the law change.

These differences are illustrated in Fig 4. The shifting produced by SDHPT does not occur immediately, but is introduced at levels which are most likely to be affected by law changes. The multiplying factors for the SDHPT procedure increase more rapidly than the NCHRP procedure.

Further study of existing truck weight data suggests that both NCHRP and SDHPT should be challenged for the following reasons:

1. The historical shift pattern shown in Fig 3 is not clearly observed in the cumulative frequency curves for most vehicle types (Figs 5, 6, 7, 8).
2. With respect to the actual multiplying factors obtained by NCHRP and SDHPT methodologies, a comparison of the differences between the actual and predicted factors proved significant (Figs 9, 10).

DEVELOPMENT OF A NEW SHIFTING PROCEDURE

More recent research surrounding the shifting methodology has been performed at the Center for Transportation Research. The work performed by Walton, Larkin, and Yu provided some very valuable recommendations for the improvement of the shifting procedure. In Walton and Larkin's study (Ref 7), it was observed that the multiplying factors for the 2D and 3A start increasing at a point at approximately 50 percent of the cumulative percentage curve, while 3-S2 and 2-S1-2 started from approximately the 33 percent point. The reason cited for such a difference was based on the observation of the differences in operating characteristics and the types of

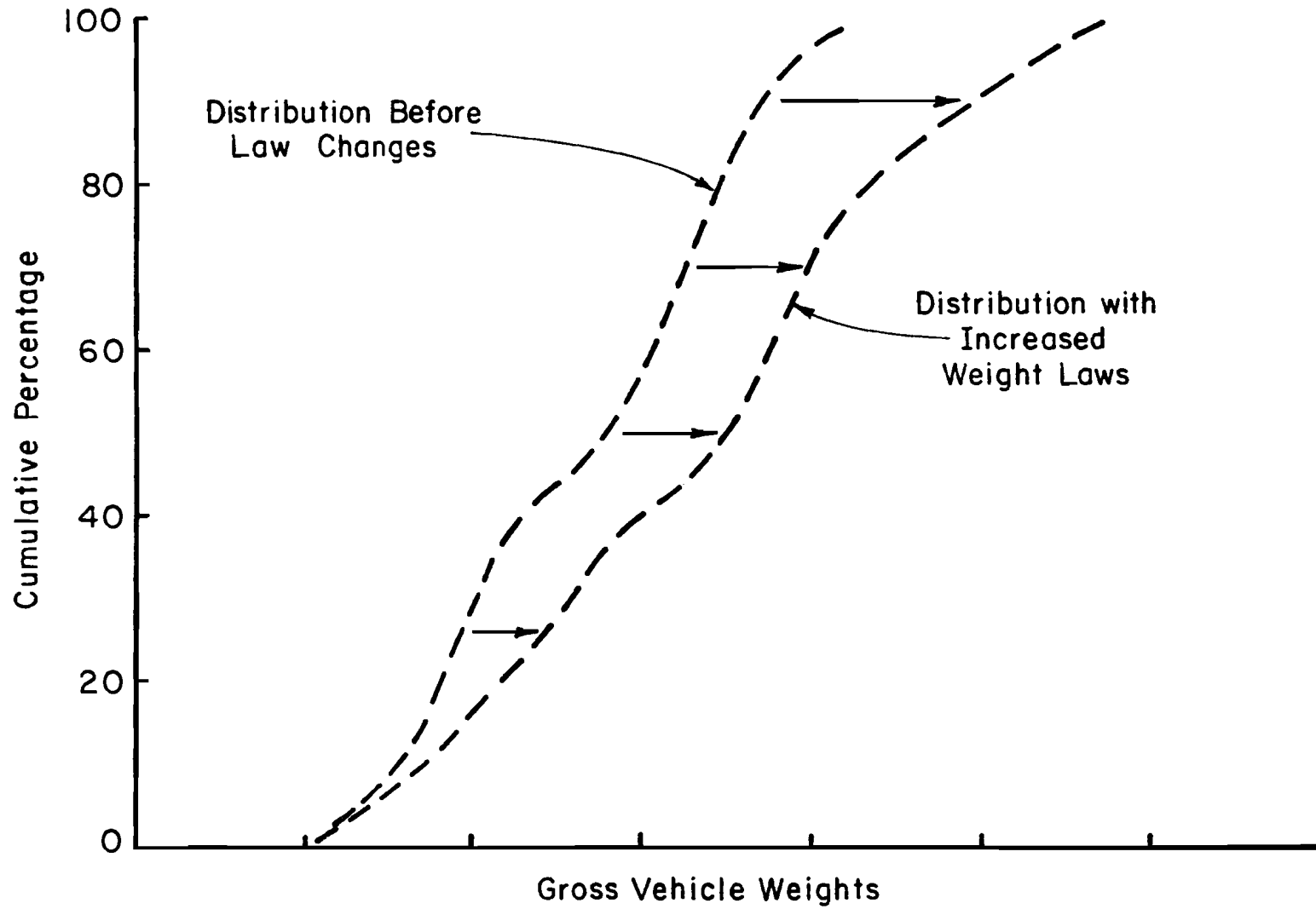


Fig 3. Typical historical shifts in gross vehicle weight distribution (Ref 7).

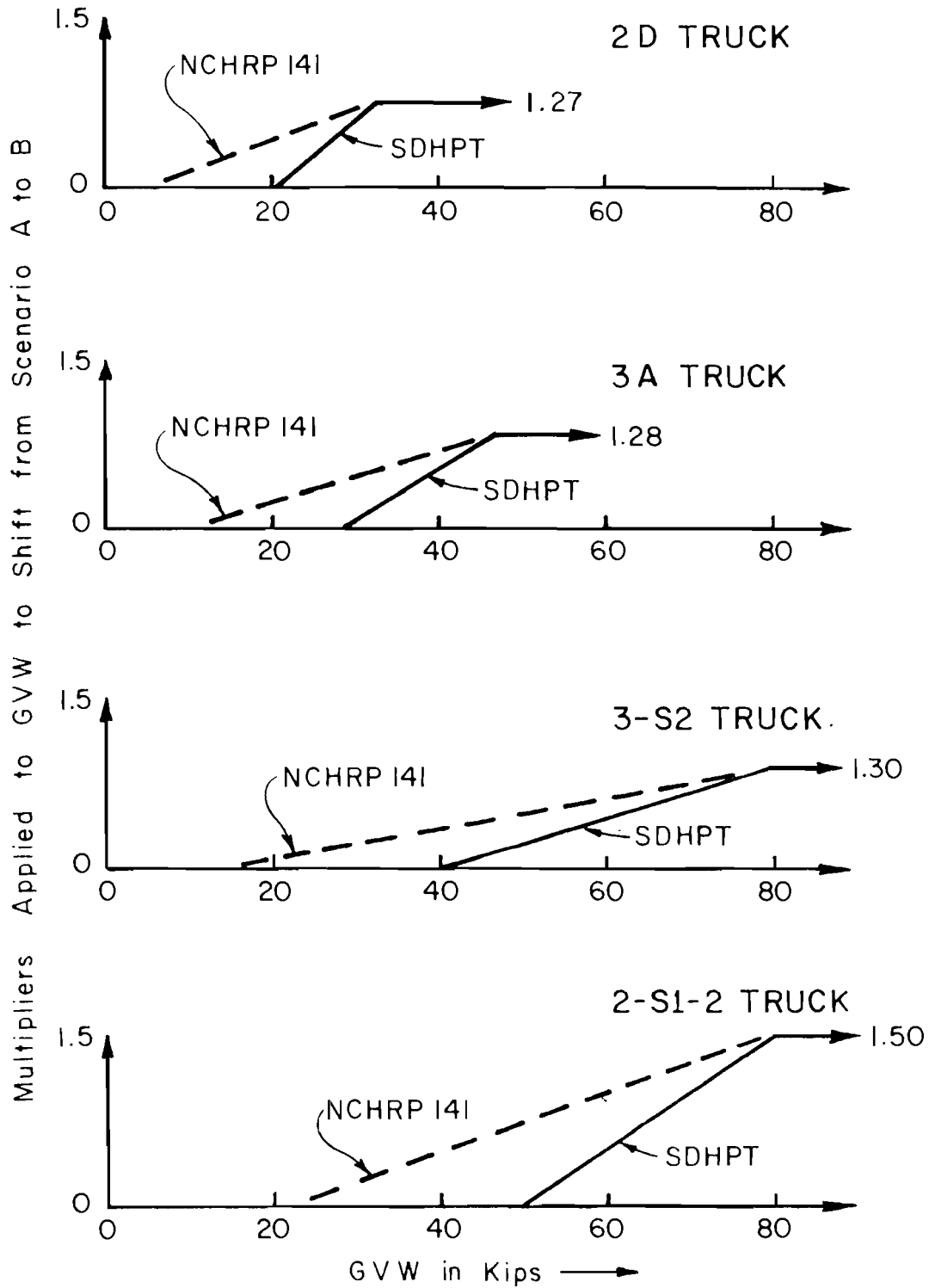


Fig 4. An example of multipliers adopted for shifting GVW distributions (Ref 17).

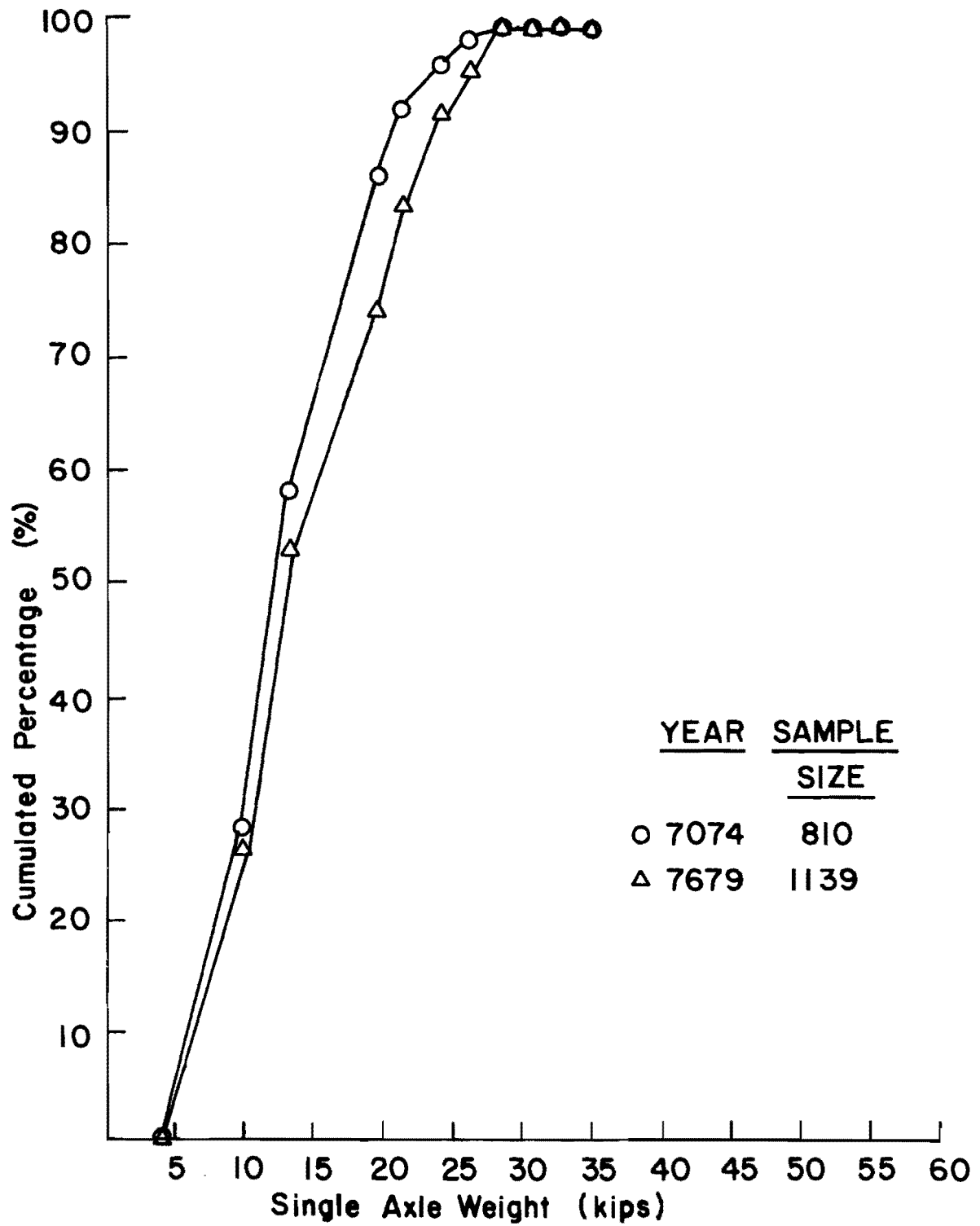


Fig 5. Shifting of GVW distribution of 2D after weight limits changed in 1975.

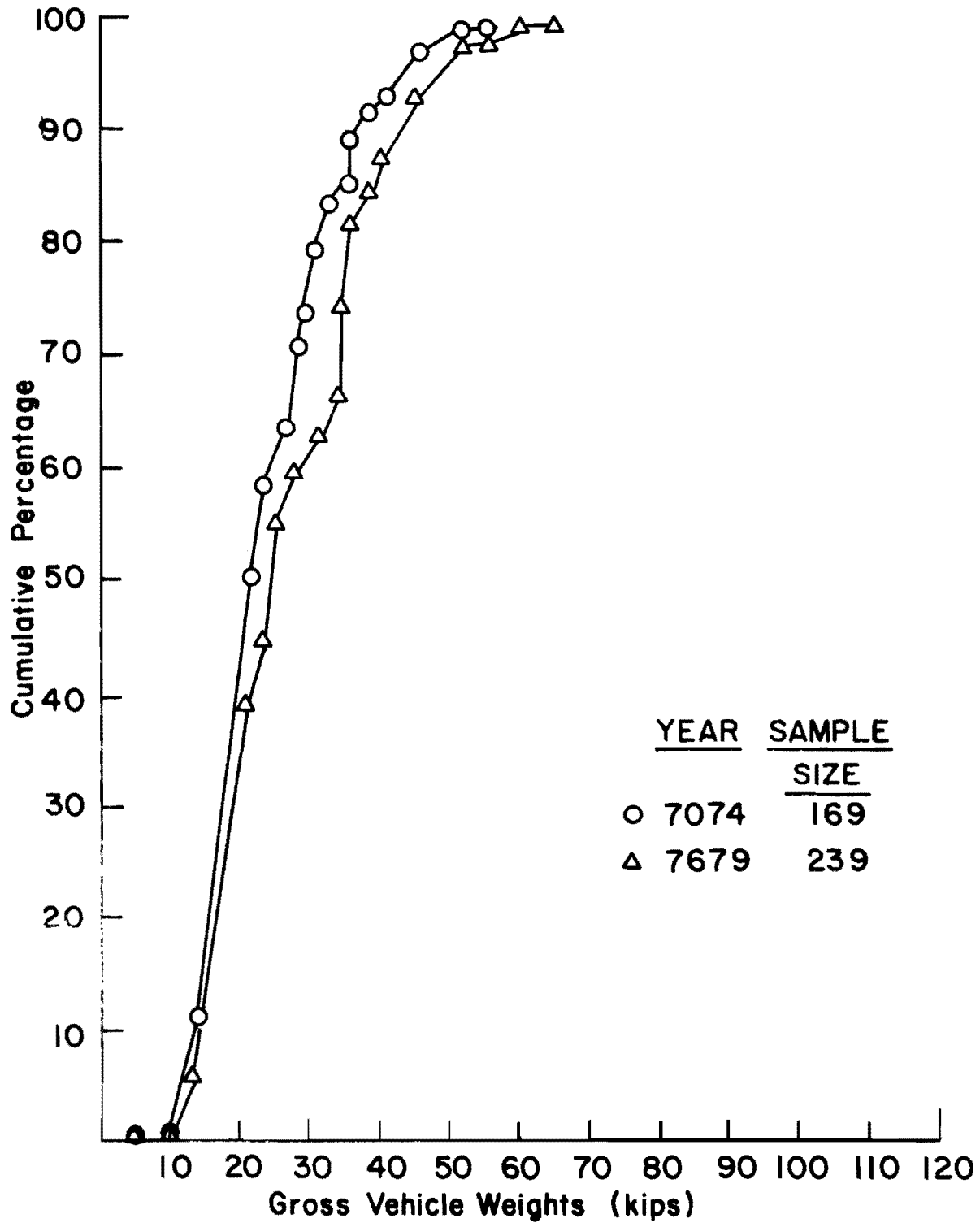


Fig 6. Shifting of GVW distribution of 3A after weight limits changed in 1975.

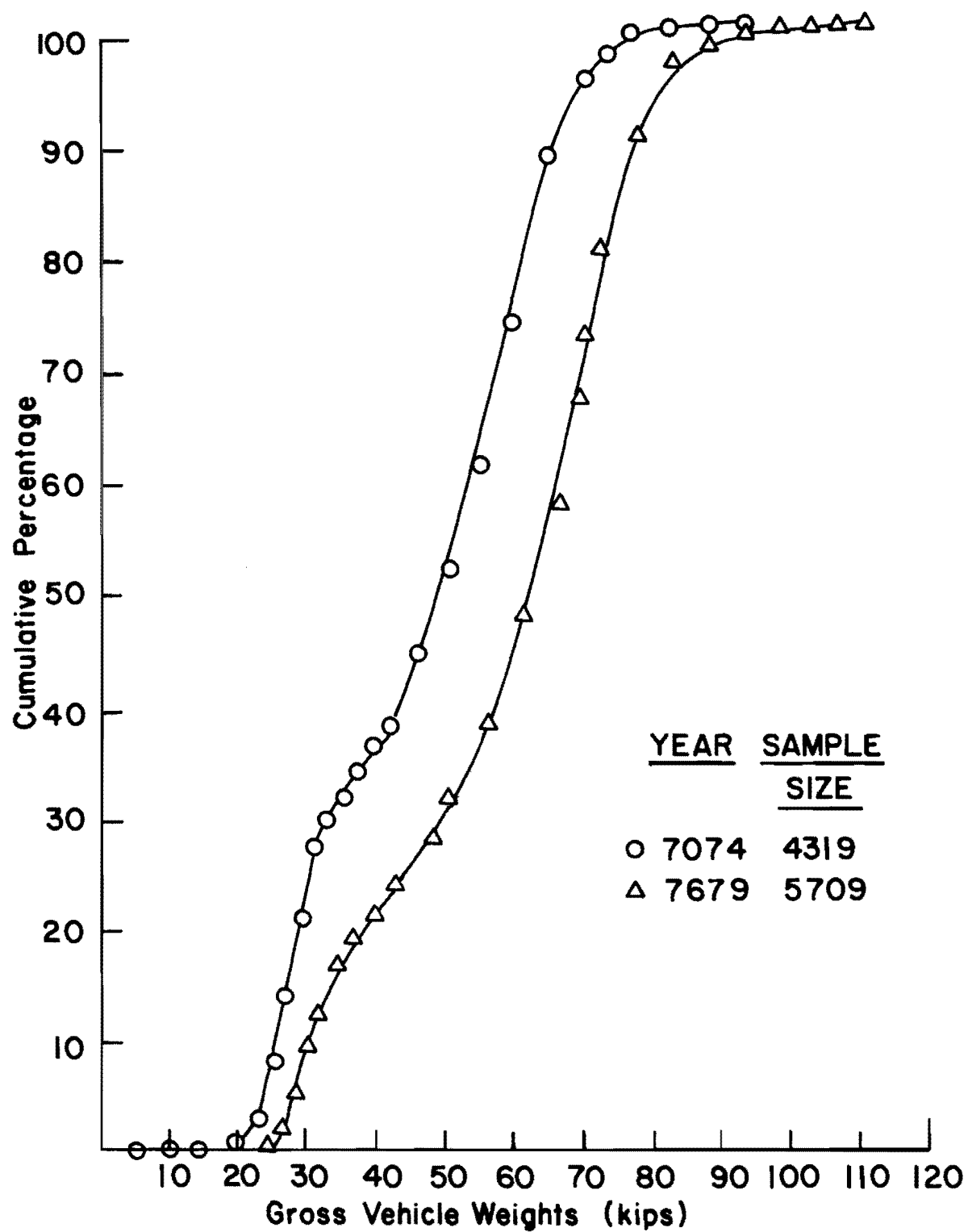


Fig 7. Shifting of GVW distribution of 3-S2 after weight limits changed in 1975.

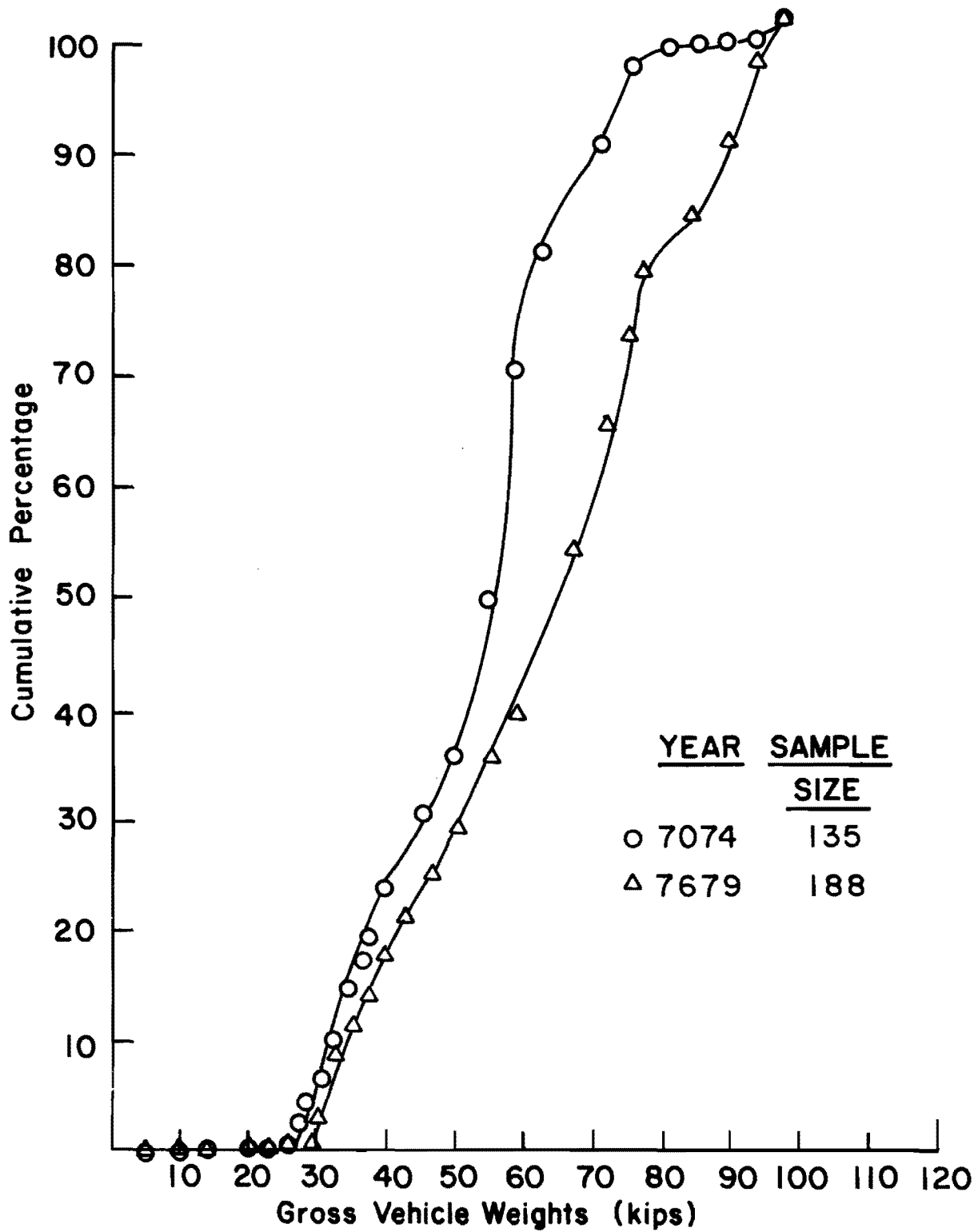


Fig 8. Shifting of GVW distribution of 2-S1-2 after weight limits changed in 1975.

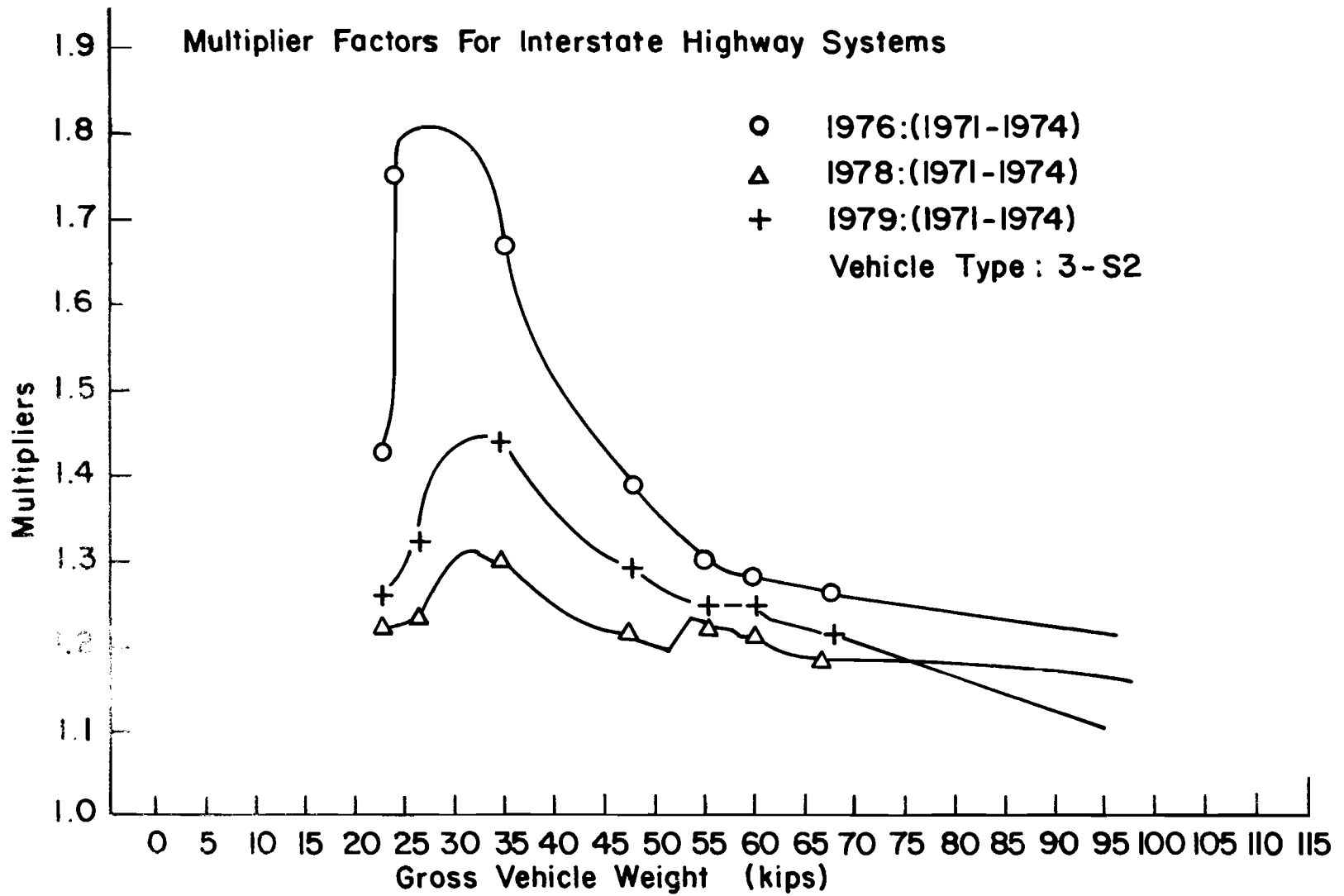


Fig 9. Comparison of NCHRP, SDHPT, and actual multiplying factors for Texas interstate rural highways.

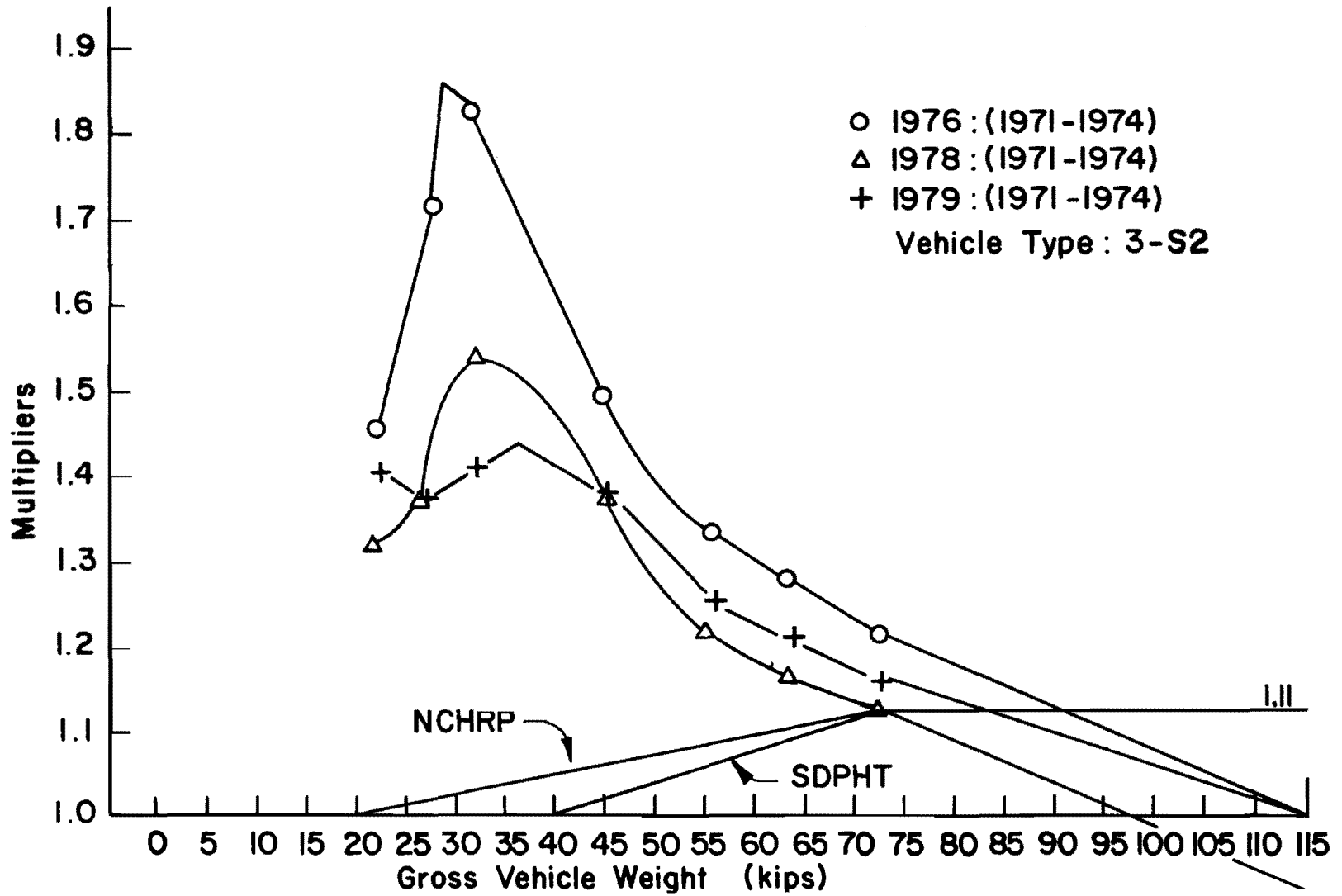


Fig 10. Comparison of NCHRP, SDPHT, and actual multiplying factors for other main rural highways.

commodities transported. It was stated that the 3-S2 and 2-S1-2 vehicles are less likely than 2D and 3A to be demand and volume constrained (Ref 7).

In Walton and Yu's study (Ref 13), they concluded that the redistribution of vehicle weight due to changes in size and weight laws varies from one vehicle class to another. They also pointed out that a specific highway class based methodology is preferred to a more general one.

Based on these findings and recommendations, further research to develop a more precise shifting methodology was pursued.

DESCRIPTION OF THE METHODOLOGY

The new shifting procedure extends the contribution of the previous shifting models. For instance,

1. Existing data within the state are analyzed and used as a base for projection as in the second FHWA method.
2. Multiplying factors are used as a means of performing the shifting as suggested in NCHRP procedure (Ref 15).
3. Initial shifting points in the accumulated percentage are used to take into account demand and volume constraint considerations (Ref 7).
4. A vehicle-class based methodology is used (Ref 13).

In addition to these contributions, some additional improvements have also been incorporated.

1. Most of the accumulated distribution curves of vehicle weight resemble a normal distribution pattern; therefore, both the mean and the variance of a curve are used to characterize the truck weight distribution pattern.
2. Past truck weight distribution data are used for trend analysis. Figure 11 illustrates that the trend of the mean truck weight did reflect the changes that occurred in truck size and weight laws in Texas.
3. More statistical analysis and testing are used in the shifting procedures.

Before discussing the procedure in more detail, it is worthwhile to review some of the estimators that are used.

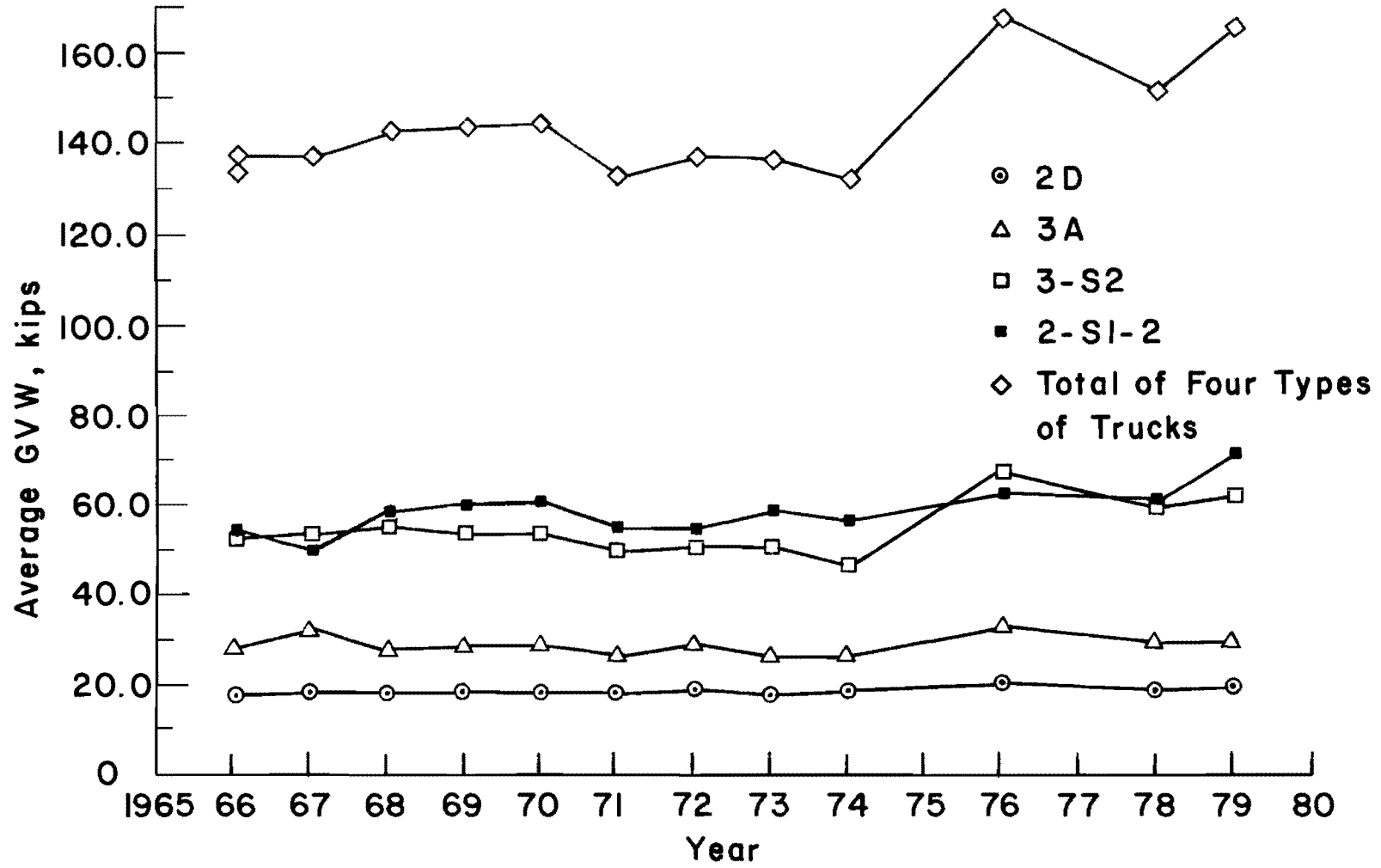


Fig 11. Average gross vehicle weight for four types of trucks on Texas interstate highways.

ESTIMATORS FOR THE SHIFTING PROCEDURE

When the sample size is sufficiently large (e.g., the number of observations is more than 100), the truck weight distribution data resemble a normal distribution pattern. Thus it is convenient and accurate to use both the mean and the variance of each distribution curve as the estimators to characterize a truck weight distribution curve. In Fig 11 the mean truck weights for four types of vehicles from 1966 to 1979 are shown. The trends suggest that the usage of mean truck weight as a detector of changes in the truck weight laws may be justified. For instance, the curves show significant jumps between 1974 and 1976. Within the same span of time, the truck size and weight laws in Texas were changed. Figure 12 shows a set of typical truck weight distribution curves for the 3-S2. It indicates that the curves shifted to the right following the changes in Texas weight laws in 1975. From Fig 11, substantial variations are observed in the truck weight distribution for 1978 and 1979. These variations can be confirmed by referring to Fig 12. The mean GVW increases as the curve shifts to the right and the mean GVW decreases as the curve moves to the left. Thus, the mean GVW can be used as an index to detect the direction of shifting of the truck weight distribution curve.

The other estimator used is the variance of the distribution curves. It is insufficient to use the mean as the only estimator. This insufficiency is shown in Figs 13 and 14. Figure 13 shows the plotting of both the mean and variance of a set of distribution curves for the 3-S2 on the Interstate Rural Highway System. Figure 14 shows a set of truck weight distribution curves from the same set of truck weight data. The mean weight curve in Fig 13 does not suggest any shifting in the truck weight distribution for the period 1966 through 1974. However, in actuality, some amount of shifting did occur, as is shown in Fig 14. By using just the mean GVW, such shifting trends may go undetected. However, with the second estimator, the variance of GVW distribution, such a shift is more readily apparent. Thus, the variance is required as a second estimator.

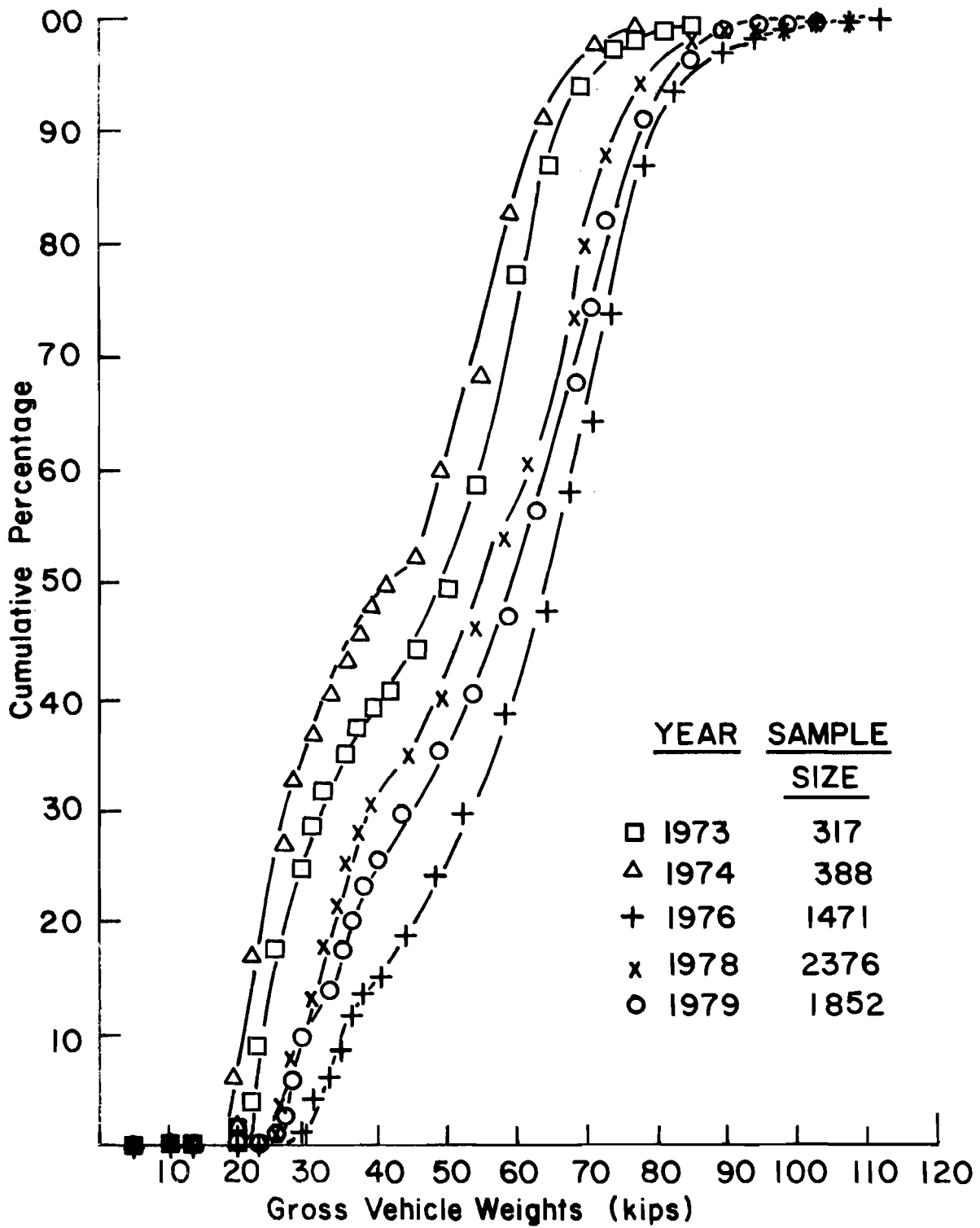


Fig 12. A set of typical weight distribution curves showing shifting occurred after weight limits changed in 1975.

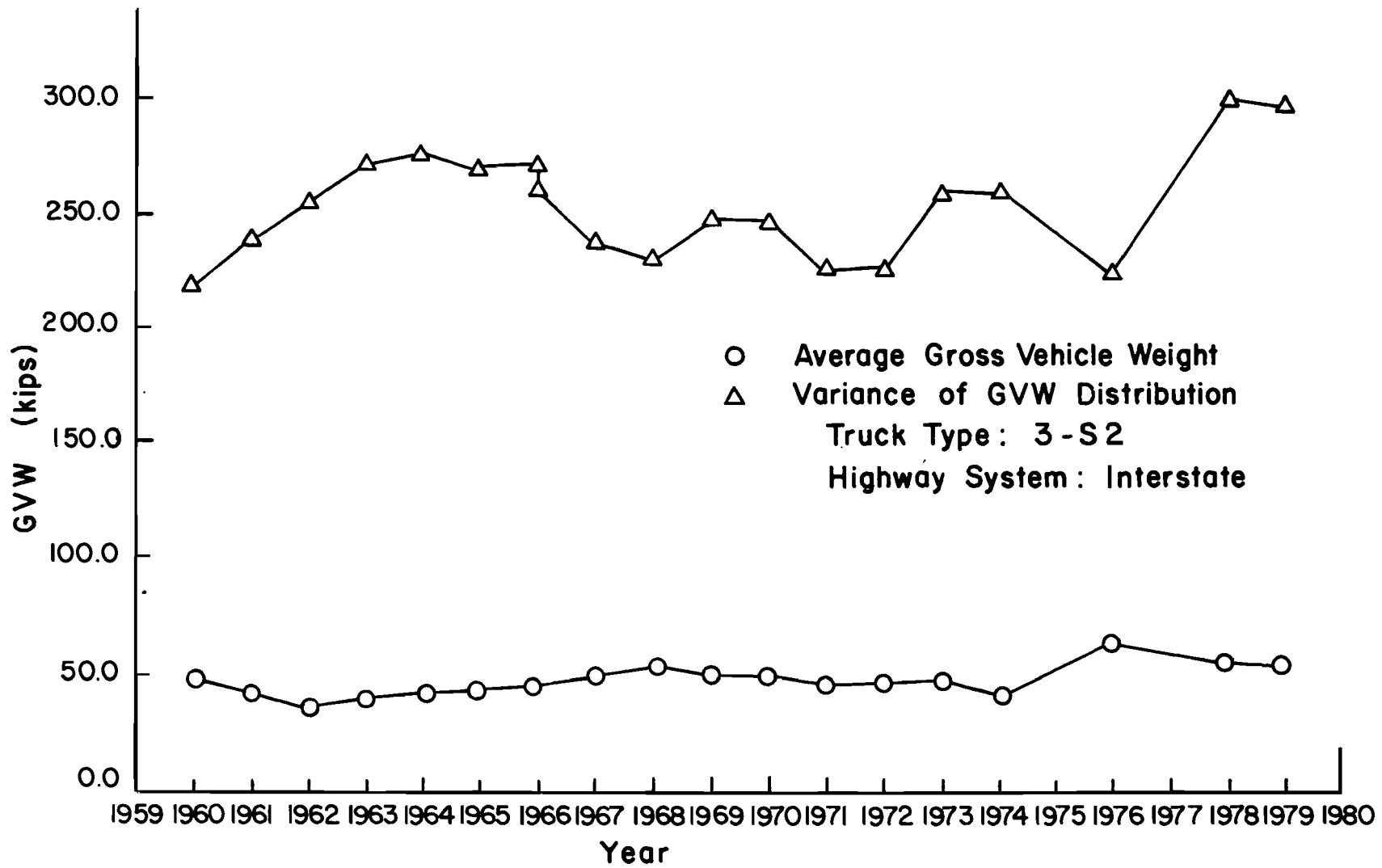


Fig 13. Trends of mean and variance of GVW distribution for 3-S2 on Texas interstate rural highways.

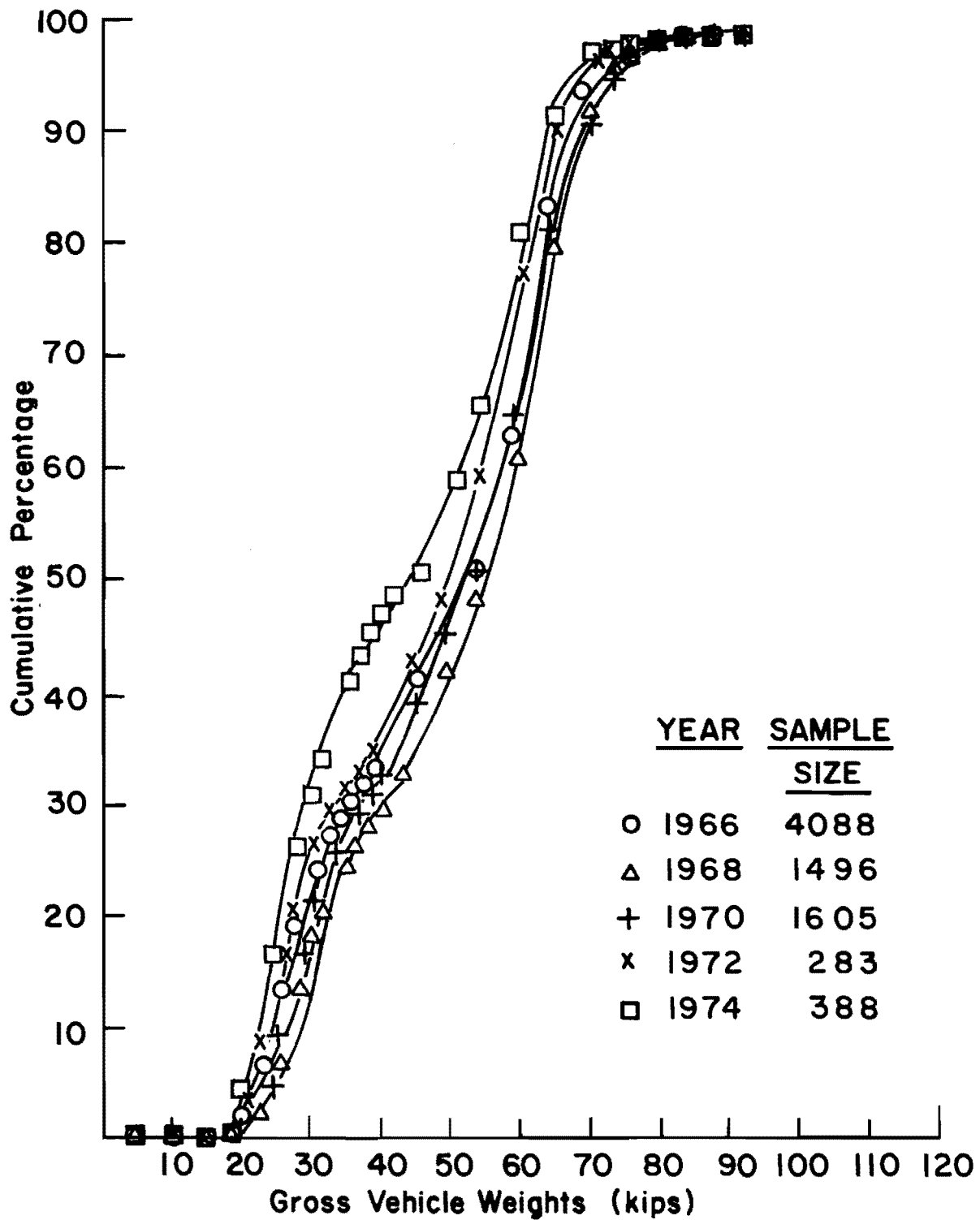


Fig 14. Truck GWW distribution curves obtained before the 1975 weight law changes.

DISCUSSION OF THE NEW SHIFTING METHODOLOGY

The shifting methodology is composed of two major parts. The first part is to analyze the past truck weight trend by studying the patterns of both the mean and variance of historical truck GVW distribution data. The second part is the prediction of the future truck GVW distribution. The prediction may be for a certain year given an existing or a proposed weight law. The first part of the procedure is discussed in the rest of this chapter. The second part of the procedure may be done either manually or with computer application. The manual application of the procedure is discussed in Chapter 4 and the computer application in Chapter 5. At any rate, the mean and variance of the predicted curve should be compatible with (i.e., within the desirable confidence levels) the respective values estimated from the first part of the procedure.

ANALYSIS OF HISTORICAL TRUCK WEIGHT DATA

At periodic intervals most states submit truck weight survey data to the FHWA. The survey data are processed and summarized by FHWA, with summaries of truck weight distribution data formatted into W-tables. These W-tables are the most complete information available on the vehicle weight carried by the highway system. In order to study truck weight trends, the means and the variances for each year for which data are available must be computed. The computation procedure (given below) is illustrated by the example shown in Table 1. In the example, the data for a 3-S2 truck type operating on Texas interstate rural highways are used. The data required are the GVW's which were obtained from the W-5 tables.

- Step 1. Enter the GVW distribution intervals in column A; compute the mid-GVW and enter the values in column B.
- Step 2. Enter the number of vehicles weighed in each GVW interval in column C.
- Step 3. Compute the products of values in columns B and C for each row; enter results in column D.
- Step 4. By rows, compute the square of the values in column B, then multiply the square with values in column C. Enter results in column E.
- Step 5. Find the summations of columns C, D, and E.

TABLE 1. EXAMPLE OF COMPUTATION OF MEAN AND VARIANCE FOR
TRUCK WEIGHT DISTRIBUTION DATA (3-S2, TEXAS
INTERSTATE RURAL HIGHWAYS, 1980)

(A) Truck Weight Distribution Intervals	(B) Mid- Point	(C) Accumulated Frequency (%)	(D) Percentage (%)	(E) B x D	(F) B ² x D
0.0-4.0	2.0	0.0	0.0	0.0	0.00
4.0-10.0	7.0	0.0	0.0	0.0	0.00
10.0-13.5	11.75	0.0	0.0	0.0	0.00
13.5-20.0	16.75	0.0	0.0	0.0	0.00
20.0-22.0	21.0	0.5	0.5	10.5	5.25
22.0-24.0	23.0	1.0	0.5	11.5	5.75
24.0-26.0	25.0	2.0	1.0	25.0	25.00
26.0-28.0	27.0	5.5	3.5	94.5	330.75
28.0-30.0	29.0	12.5	7.0	203.0	1,421.00
30.0-32.0	31.0	16.0	3.5	108.5	379.75
32.0-34.0	33.0	20.0	4.0	132.0	528.00
34.0-36.0	35.0	23.5	3.5	122.5	428.75
36.0-37.0	37.0	26.0	2.5	92.5	231.25
38.0-40.0	39.0	29.0	3.0	117.0	351.00
40.0-45.0	42.5	34.0	4.0	212.5	1,062.50
45.0-50.0	47.5	38.0	4.0	190.0	760.00
50.0-55.0	52.5	44.0	6.0	315.0	189.00
55.0-60.0	57.5	53.0	9.0	517.5	4,657.50
60.0-65.0	62.6	61.0	8.0	500.0	4,000.00
65.0-70.0	67.5	73.0	12.0	810.0	9,720.00
70.0-72.0	71.0	83.0	10.0	710.0	7,100.00
72.0-75.0	73.5	87.5	4.5	330.75	1,488.38
75.0-80.0	77.5	95.0	7.5	581.25	4,359.38
80.0-85.0	82.5	97.0	2.0	165.0	330.00
85.0-90.0	87.5	98.0	1.0	87.5	87.50
90.0-95.0	92.5	99.0	1.0	92.5	92.50
95.0-100	97.5	99.5	0.5	48.75	24.38
100.0-105	102.5	100.0	0.5	51.25	25.63
105.0-110	107.5	100.0	0.0	0.0	0.00
110.0-115	112.5	100.0	0.0	0.0	0.00
			$\Sigma = 100.00$	$\Sigma = 5,509.00$	$\Sigma = 338,814.25$

$$\text{MEAN} = \frac{5,509}{100} = \underline{55.09}; \quad \text{VARIANCE} = \frac{338,814.25 - (5,509)^2/100}{100} = \underline{353.23};$$

$$\text{STANDARD DEVIATION} = \underline{18.79}$$

Step 6. Compute the mean GVW using

$$\text{Mean GVW} = \frac{\Sigma(\text{Column D})}{\Sigma(\text{Column C})}$$

Step 7. Compute the variance:

$$\text{Variance} = \left[\frac{\Sigma(\text{Column E}) - \frac{\Sigma(\text{Column D}) \times \Sigma(\text{Column D})}{\Sigma(\text{Column C})}}{\Sigma(\text{Column C}) - 1} \right]$$

With all the available truck GVW weight data, compute the mean and variance for each year and plot both values versus time. Once the curves have been plotted, specific trends of GVW means and variances may be realized. An example for the 3-S2 truck type is shown in Fig 13. Although the figure suggests possible trends with respect to mean and variance, no specific regression model has been developed for the analysis.

The importance of a shifting methodology is not based on its ability to predict the new weight distribution within the span of the same weight laws. Rather, the major concern of a methodology is its capability to predict changes in distribution trends under proposed weight laws. In Fig 11, it is shown that for each type of truck there were two significant deviations in 1960 and 1975. These deviations or "jumps" could not have been predicted through extrapolation of previous trends.

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CHAPTER 3. A SHIFTING PROCEDURE TO INCORPORATE INFLUENCES
DUE TO CHANGES IN TRUCK WEIGHT LIMITS

From a detailed study of the average vehicle weight trends as replicated in the Texas data, some conclusions were drawn:

1. Within the span of same truck weight laws, the average GVW for each truck type did not change abruptly. Rather, the changes over a period of time were gradual.
2. Correlation among the four major trucks (i.e., 2D, 3A, 3-S2, and 2-S1-2) was studied; however, no significant correlation on the average GVW among the four truck types was observed.
3. The average GVW factor is defined as the ratio between the average GVW and the practical maximum GVW allowed by current weight laws for a specific truck type. The variation of this ratio throughout the years for a specific truck type is virtually insignificant.

DERIVATION OF AVERAGE GVW FACTORS

Among the three findings, the most significant one is the third item. Tables 2, 3, 4, and 5 show the average truck weights and ratios with respect to the practical maximum GVW for 2D, 3A, 3-S2, and 2-S1-2, respectively. The ratio can be expressed mathematically as

$$\text{Average GVW Factor} = \frac{\text{Average GVW}}{\text{Practical maximum GVW}}$$

For each type of truck, a linear regression analysis was applied to determine the relationship between the average GVW and the practical maximum GVW. In the regression, the independent variable was the practical maximum GVW and the dependent variable was the average GVW. The regression model had no constant term; it can be expressed as

$$Y = AX,$$

where
Y = Average GVW,
X = Practical maximum GVW, and
A = Coefficient.

TABLE 2. RELATIONSHIP OF AVERAGE GVW AND PRACTICAL MAXIMUM
GVW FOR 2D, TEXAS INTERSTATE RURAL HIGHWAYS

Year	Average Legal GVW (kips)	Practical Maximum GVW (kips)	Average GVW Factor
1960	12.39	24.60	0.50
1961	12.55	24.60	0.51
1962	12.17	24.60	0.49
1963	12.10	24.60	0.49
1964	12.11	24.60	0.49
1965	12.49	24.60	0.51
1966	12.31	24.60	0.50
1967	13.06	24.60	0.53
1968	12.82	24.60	0.52
1969	12.75	24.60	0.52
1970	12.79	24.60	0.52
1971	12.86	24.60	0.52
1972	13.00	24.60	0.53
1973	12.84	24.60	0.52
1974	13.34	24.60	0.54
1975	(Texas weight limits changed)		
1976	15.67	27.22	0.58
1978	13.87	27.22	0.51
1979	14.41	27.22	0.53

Note: 1976 data were not included in the following statistics.

Mean of GVW Factor = 0.51
Standard Deviation = 0.0147

One-Sample t-test = 1.24 (D.F. = 16)
Two-Sample t-test = -0.58 (D.F. = 16)

TABLE 3. RELATIONSHIP OF AVERAGE GVW AND PRACTICAL MAXIMUM
GVW FOR 3A, TEXAS INTERSTATE RURAL HIGHWAYS

Year	Average Legal GVW (kips)	Practical Maximum GVW (kips)	Average Legal Factor
1960	26.45	42.46	0.63
1961	19.90	42.26	0.47
1962	21.35	42.26	0.51
1963	22.62	42.26	0.54
1964	21.13	42.26	0.50
1965	21.19	42.26	0.50
1966	21.62	42.26	0.51
1967	24.50	42.26	0.58
1968	20.22	42.26	0.48
1969	21.59	42.26	0.51
1970	21.59	42.26	0.51
1971	20.15	42.26	0.48
1972	24.05	42.26	0.57
1973	21.25	42.26	0.50
1974	20.23	42.26	0.48
1975	(Texas Weight Limits Changed)		
1976	27.11	44.90	0.60
1978	22.45	44.90	0.50
1979	23.13	44.90	0.52

Note: 1960 and 1976 data were not included
in the following statistics.

Mean of GVW Factor = 0.51
Standard Deviation = 0.0302

One Sample t-test = -0.108 (D.F. = 15)
Two Sample t-test = 0.34 (D.F. = 17)

TABLE 4. RELATIONSHIP OF AVERAGE GVW AND PRACTICAL MAXIMUM
GVW FOR 3-S2, TEXAS INTERSTATE RURAL HIGHWAYS

Year	Average Legal GVW (kips)	Practical Maximum GVW (kips)	Average GVW Factor
1960	48.52	72.00	0.67
1961	46.68	72.00	0.65
1962	45.63	72.00	0.63
1963	46.51	72.00	0.65
1964	46.70	72.00	0.65
1965	47.22	72.00	0.66
1966	47.46	72.00	0.66
1967	47.91	72.00	0.67
1968	49.35	72.00	0.69
1969	47.51	72.00	0.66
1970	47.65	72.00	0.66
1971	44.92	72.00	0.62
1972	44.54	72.00	0.63
1973	45.21	72.00	0.63
1974	41.32	72.00	0.57
1975	(Texas Weight Limits Changed)		
1976	59.43	80.00	0.74
1978	53.20	80.00	0.67
1979	54.86	80.00	0.69

Note: 1974 and 1976 data were not included
in the following statistics.

Mean of GVW Factor = 0.66
Standard Deviation = 0.0183

One-Sample t-test = -1.15 (D.F. = 15)
Two-Sample t-test = -1.78 (D.F. = 14)

TABLE 5. RELATIONSHIP OF AVERAGE GVW AND PRACTICAL MAXIMUM
FVW FOR 2-S1-2, TEXAS INTERSTATE RURAL HIGHWAYS

Year	Average Legal GVW (kips)	Practical Maximum GVW (kips)	Average GVW Factor
1966	48.28	72.00	0.67
1967	45.40	72.00	0.63
1968	52.92	72.00	0.74
1969	53.16	72.00	0.74
1970	53.78	72.00	0.74
1971	50.17	72.00	0.70
1972	50.17	72.00	0.70
1973	53.88	72.00	0.75
1974	49.25	72.00	0.68
1975	(Texas Weight Limits Changed)		
1976	57.19	80.00	0.71
1978	53.65	80.00	0.67
1979	57.18	80.00	0.71

Note: 1974 and 1976 data were not included
in the following statistics.

Mean of GVW Factor = 0.072
Standard Deviation = 0.0359

One-Sample t-test = 0.271 (D.F. = 10)
Two-Sample t-test = 0.41 (D.F. = 9)

The statistical package MINITAB was used in the analysis. The coefficient for each type of truck obtained from the analysis can be used as the recommended average GVW factor. These coefficients are shown in Table 6.

TABLE 6. RECOMMENDED AVERAGE GVW FACTORS FOR FOUR TYPES OF TRUCKS OPERATING ON TEXAS INTERSTATE RURAL HIGHWAYS

Truck Type	Recommended Average GVW Factor
2D	0.51
3A	0.51
3-S2	0.66
2-S1-2	0.70

One-Sample t-test

In order to test the validity of these average GVW factors, two statistical tests, one-sample t-test and two-sample t-test, were used. The one-sample t-test is to test

$$H_0 : \mu = \mu_0 \quad \text{versus} \quad H_a : \mu \neq \mu_0$$

where

μ = the mean of average GVW factors observed from truck weight data,

μ_0 = the mean of average GVW provided by the regression model.

The null hypothesis would be rejected at an α level of significance if the t-value exceeds the limits provided by the student t-distribution statistical tables. The t-value of the sample is expressed in terms of the mean, standard deviations, and the number of observations. It can be expressed as

$$t = \frac{\bar{y} - \mu_o}{s/\sqrt{n}}$$

where

- \bar{y} = mean of the observed average GVW factors,
- μ_o = average GVW factor suggested by the regression analysis,
- s = standard deviation of the observed average GVW factors,
- n = number of observations.

The concept of rejection or acceptance of the null hypothesis is illustrated in Fig 15. Table 7 is the summary of the decision process. Since the computed t-values for four types of trucks are within the limits suggested by the student t-distribution, it can be concluded that the average GVW factors obtained from regression analysis may be used to represent the relationships between average GVW and practical maximum GVW for the four types of trucks.

TABLE 7. SUMMARY OF RESULTS FROM ONE SAMPLE T-TESTS

Truck Type	Degree of Freedom	Observed t-value	Student t-Distribution
2D	16	1.2423	+ 2.1199
3A	15	-0.1083	+ 2.1315
3-S2	15	-1.1505	+ 2.1314
2-S1-2	10	0.2706	+ 2.2281

*Confidence level = 0.95

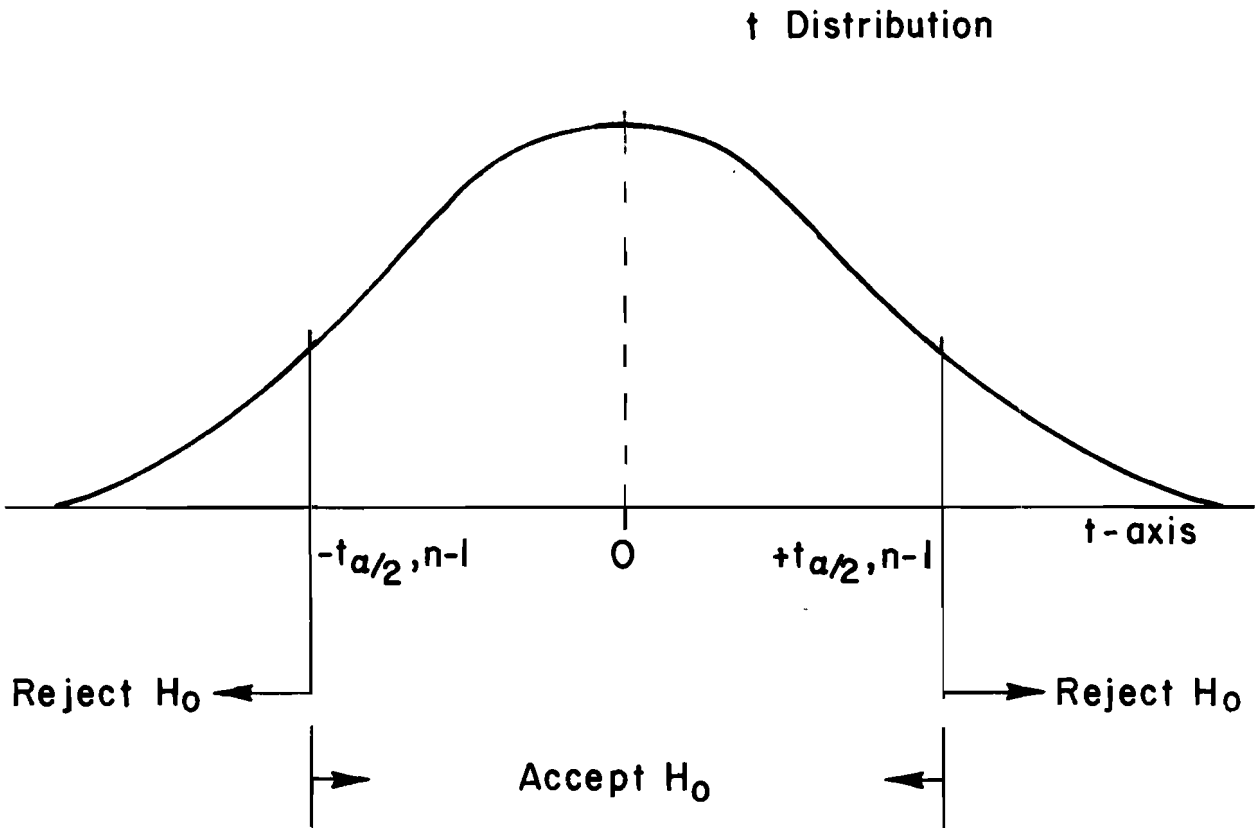


Fig 15. Distribution of t when H_0 is true. (Ref 8)

Two-Sample t-test

In the regression analysis of the average GVW and the practical maximum GVW, it was assumed that the relationship of these two parameters would not be affected by changes in truck weight limits. In order to validate such an assumption, a two-sample t-test was used to check the significance of variations of the average GVW factor before and after the weight law changes that occurred in 1975. The null and alternate hypotheses are expressed as

$$H_0 : \mu_0 = \mu_1$$

and

$$H_1 : \mu_0 \neq \mu_1$$

where

μ_0 = the mean of average GVW factors obtained from
1960 through 1974 and

μ_1 = the mean of average GVW factors obtained in 1975.

The concept of rejection or acceptance of the null hypothesis is similar to that illustrated earlier, in Fig 15. Computation of the two-sample t-value is different from that in the one-sample t-test. The t-value is dependent on the means, standard deviations, and numbers of observations in both samples. It was assumed that observations made before 1975 constituted one sample and those made after 1975 the other. By definition (Ref 8),

$$t = \frac{\bar{x} - \bar{y}}{S_p \sqrt{\frac{1}{n} + \frac{1}{m}}}$$

where \bar{x} = mean of average GVW factor before 1975,
 n = number of observations before 1975,
 \bar{y} = mean of average GVW factor after 1975,
 m = number of observations after 1975,
 S_p = pooled variance of the two samples,

and the pooled variance is defined as

$$S_p^2 = \frac{(n-1)S_x^2 + (m-1)S_y^2}{n + m - 2}$$

where

$$S_x^2 = \text{variance of average GVW factor before 1975,}$$

$$S_y^2 = \text{variance of average GVW factor after 1975.}$$

The decision process was summarized in Table 8. The computed t-values for the two sample tests are within the allowable range of the t-distribution. It shows that the variation between the means of two samples is not significant at the 95 percent level. Thus, it can be concluded that changes in weight laws in 1975 did not have a significant effect on the average GVW factors.

TABLE 8. SUMMARY OF RESULTS FROM TWO-SAMPLE T-TESTS

Truck Type	Degree of Freedom	Two-sample t-value	Student t-distribution Acceptable Range
2D	16	0.58	± 2.1199
3A	17	0.34	± 2.1098
3-S2	14	-1.78	± 2.1448
2-S1-2	9	0.41	± 2.2622

*Confidence level = 0.95

RELATIONSHIP BETWEEN PRACTICAL MAXIMUM GVW'S AND AVERAGE GVW FACTORS

Note that the practical maximum GVW is used in the equation above instead of maximum allowable GVW. By using practical maximum GVW, changes in both GVW and axle weight limits can be expressed in one single parameter. If maximum allowable GVW were used, the average GVW factor would yield incorrect predictions in cases where weight law changes occurred in either GVW or axle weight alone.

For illustrative purposes, consider the 2D. The total truck weight is bounded by axle weight limits as well as safety considerations. An increase in maximum GVW limit only will not affect the weight trend of the 2D. To attain the maximum GVW limits, 2D would have had to have a total axle weight as high as 36 kips before 1975. However, axle weight limits control the GVW of the 2D; therefore, an erroneous shift would be projected if care were not taken in developing the average GVW factors.

Due to the considerations of operational safety, the steering axle weight cannot reach the maximum allowable single axle weight limit. A review of the trends in steering axle weight distributions shows that there has not been a change in the past years. This can be seen in the steering axle weight distribution curves for 3A and 3-S2 in Figs 16, 17, and 18. For 2D and 2-S1-2, the single axle weight distribution curves represent steering axles and the loading axles as well. Thus, this analogy for 2D and 2-S1-2 may not be appropriately illustrated in the distribution curves.

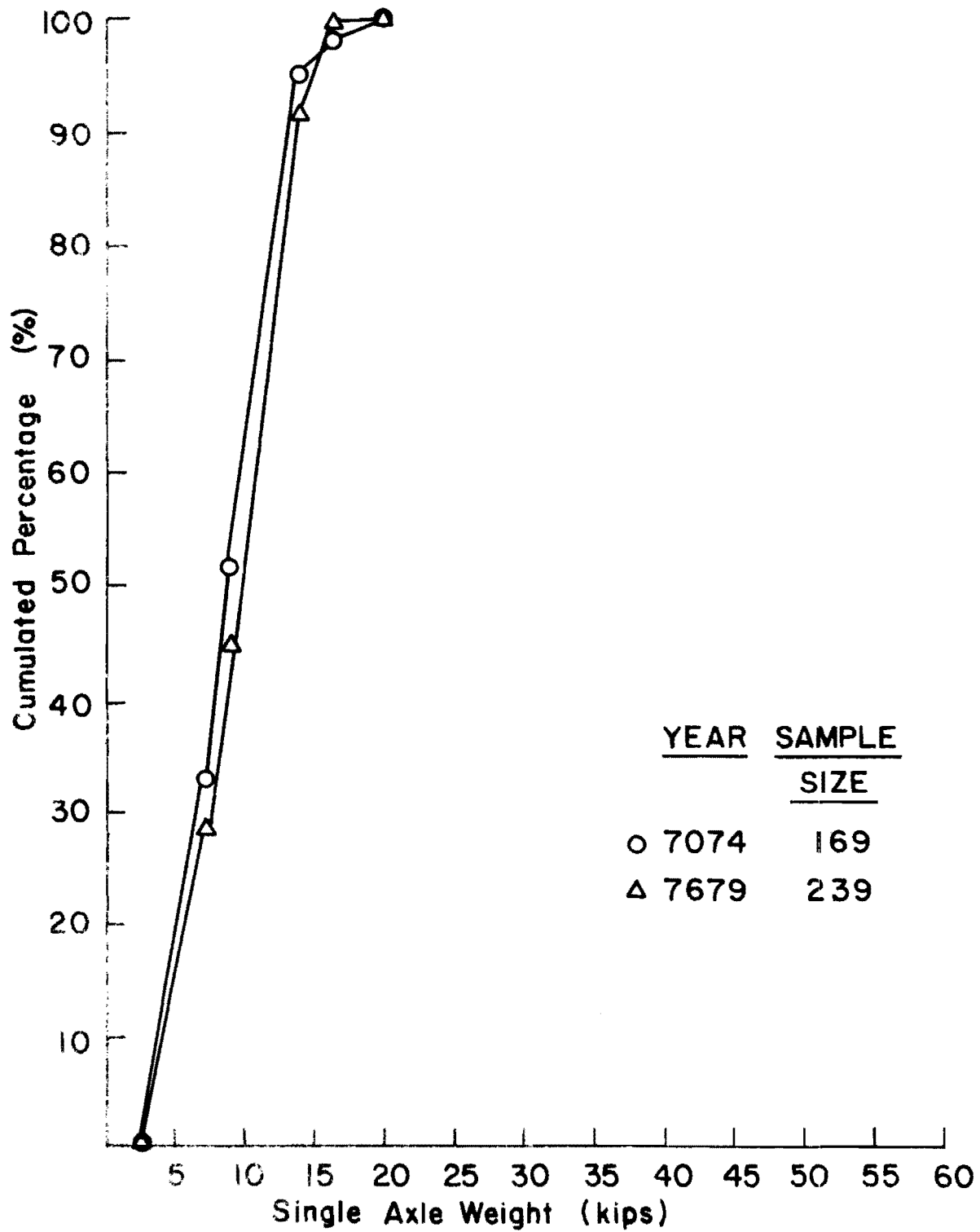
From the observation of historical data and review of the pertinent literature, practical maximum steering axle weights for four types of trucks are recommended. These limits are summarized in Table 9 (Ref 15).

The practical maximum steering axle limits for 2D and 3A provided in Table 9 were suggested by Whiteside (Ref 15). The steering axle limits for 3-S2 and 2-S1-2 were based on the values provided by the Texas Department of Public Safety.

The use of these steering axle limits is recommended to arrive at the practical maximum GVW limits. A summary of practical maximum GVW for Texas since 1951 is shown in Table 10.

With the practical maximum GVW as a function of the average GVW factor, engineers and planners may derive the practical maximum GVW for any proposed law and for selected truck types. With the available average GVW factor provided in Table 6, one can obtain the expected average truck weight under any proposed weight limits. From the expected average truck weight, a shifted curve can be obtained by using the methodology discussed in the next chapter.

The average GVW factors provided in Table 6 were derived from the Texas weight survey data. Whether such factors are transferrable to other states is unknown. It is believed that there may be some variation in the factors



Note: Data were combined to provide a smooth distribution curve.

Fig 16. Axle weight analysis (steering axle) for 3A, interstate rural, 1970-1979, State of Texas.

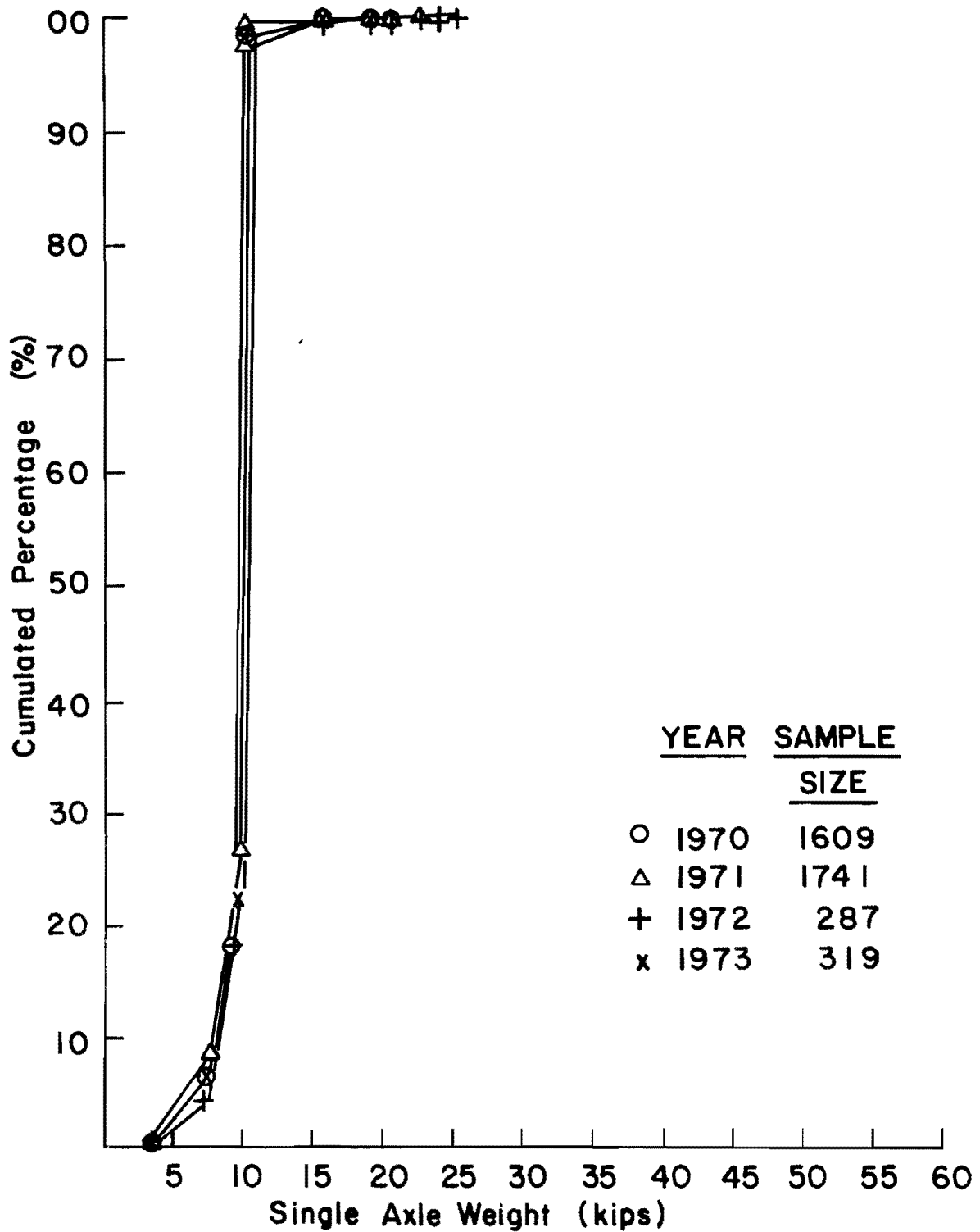


Fig 17. Axle weight analysis (steering axle) for 3-S2, interstate rural, 1970-1973, State of Texas (Ref 14).

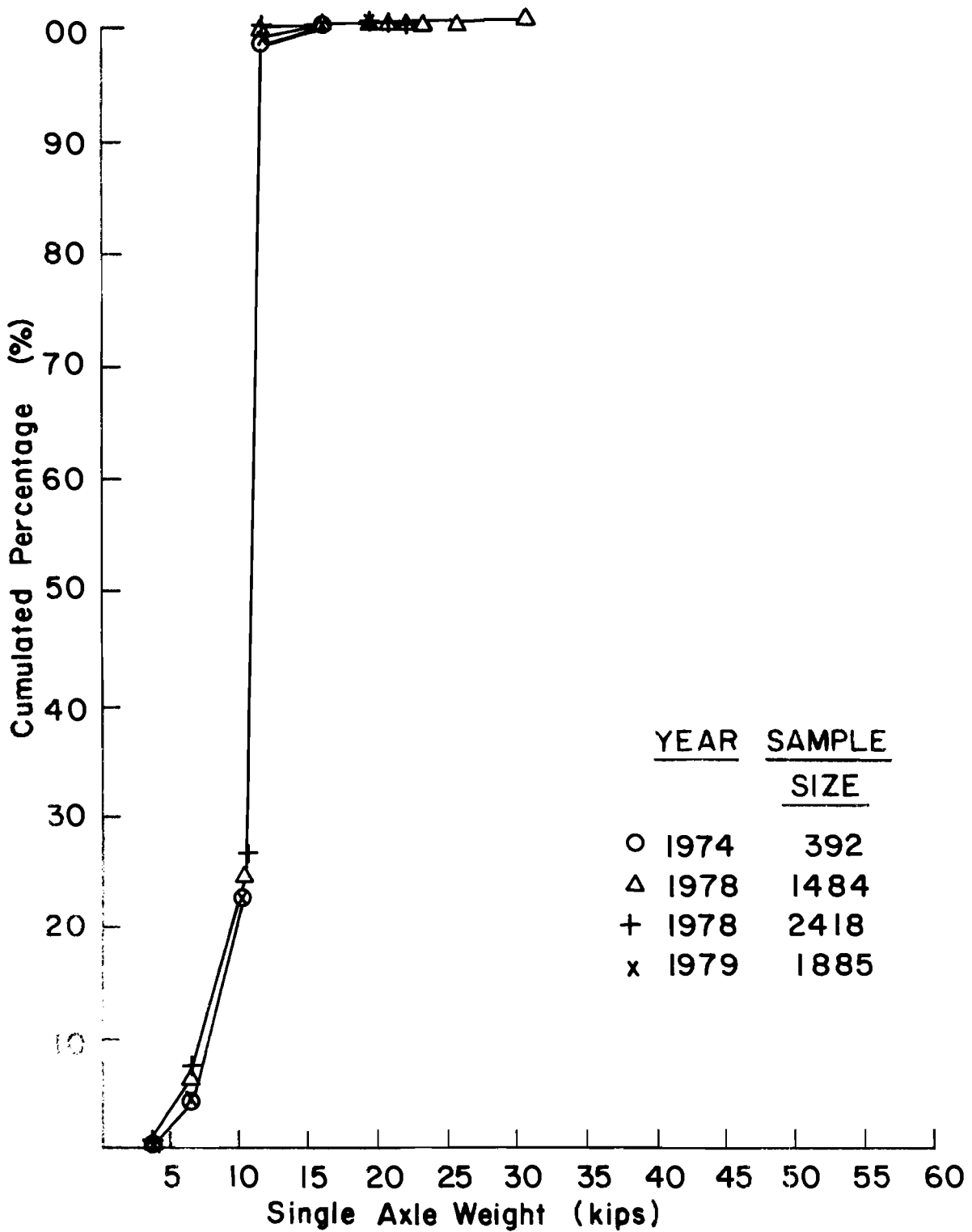


Fig 18. Axle weight analysis (steering axle) for 3-S2, interstate rural, 1974-1979, State of Texas (Ref 14).

TABLE 9. PRACTICAL MAXIMUM STEERING AXLE LIMITS FOR TRUCKS

Truck Type	Practical Maximum Steering Axle Limits
2D	7.22 Kips
3A	10.90 Kips
3-S2	12.0 Kips
2-S1-2	13.0 Kips

TABLE 10. PRACTICAL MAXIMUM GVW FOR TRUCKS IN TEXAS

Truck Type	1951-1959	1960-1974	1975-date
2D	24.6	24.6	27.22
3A	42.26	42.26	44.90
3-S2	58.4	72.0	80.0
2-S1-2	58.4	72.0	80.0

explained by different physical factors such as terrain, route systems in states and levels of enforcement in weight laws. Thus, if this methodology is used by states other than Texas, some adjustments in these factors are required. However, for practical purposes, these factors may be used to arrive at a reasonable prediction.

CHAPTER 4. MANUAL APPLICATION OF THE NEW SHIFTING PROCEDURE

In Chapter 2, the first part of the shifting procedure (i.e., analysis of historical truck weight data) is discussed thoroughly. In this chapter, the second part of the shifting procedure is discussed. This part of the procedure can be applied either manually or by resorting to computer programming. In this chapter, only the manual procedure is discussed. The computer application will be presented in the next chapter.

SHIFTING OF TRUCK WEIGHT DISTRIBUTION CURVE

Application of the manual shifting methodology is summarized in the flowchart on Fig 19. The methodology is composed of three major parts. The first part is to determine the expected mean and variance of the GVW distribution for a truck type under the proposed limits. This part involves the analysis of historical data and the application of the average GVW factors. It has been discussed explicitly in the previous chapters. The second part is to obtain a cumulative distribution curve from a set of representative truck weight data provided in the W-5 tables. The third part of the procedure is to shift the cumulative distribution curve so that the mean and variance of the shifted curve is within the acceptable tolerance of the parameters obtained in the first part of the procedure. In this part of the procedure, statistical testing is used to make the decision to accept or to reject a shifted curve.

In performing the procedure, it is necessary to shift a cumulative distribution curve and test the shifted curve with statistical tests. Once the tests are satisfied, the shifting procedure is completed and the latest shifted curve is the projected truck weight distribution curve.

In the following sections, the details of the shifting procedure and statistical testing are discussed.

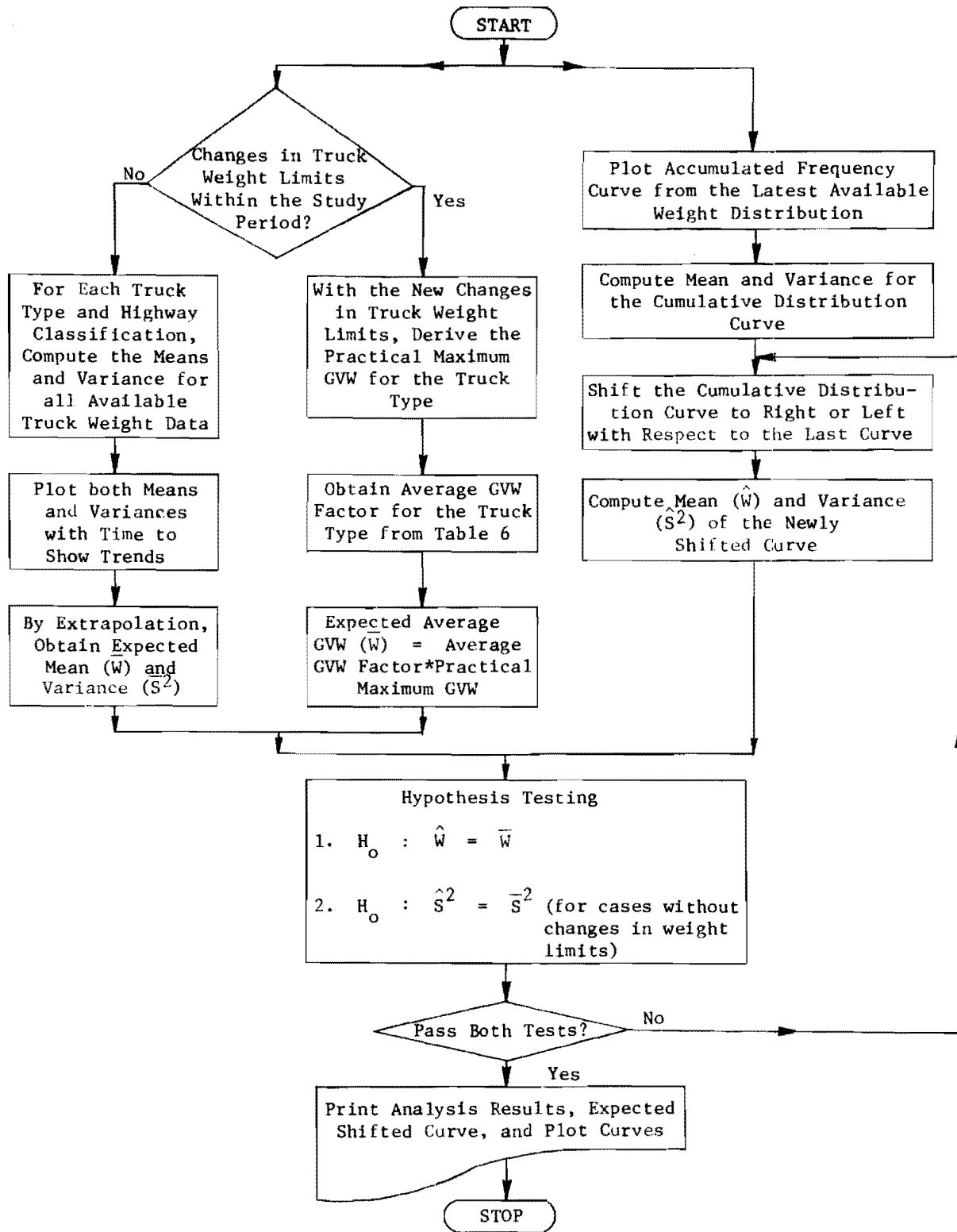


Fig 19. Flowchart showing the manual application of the shifting procedure.

Preparation of a Cumulative Frequency Curve

This part of the procedure is to provide a base curve from which the shifting may occur. It is preferable to use statistically significant data from the most recent years since the shift should incorporate the most up-to-date distribution trends.

- Step 1. Read data from W-4 or W-5 weight distribution tables. Sum the number of trucks weighed.
- Step 2. Calculate the percentage of trucks in each truck weight interval; obtain the cumulative percentage.
- Step 3. Plot the cumulative percentage for the truck weight distribution intervals.

Shifting of Curve to Obtain Expected Mean and Variance

It has been suggested by Larkin (Ref 7) that shifting for 2D and 3A starts at 50 percent and for 3-S2 and 2-S1-2 starts at 33.3 percent. However, these figures are based on Texas data. In the shifting procedure, users may start at any percentage that intuitively or explicitly represents this experience.

The shifting procedure is basically an iterative one. With adequate practice and experience, the number of iterations may be reduced. Obviously, the application of computer programming to handle the shifting procedure will reduce the time consumed in performing the iterations. The computer application is discussed in the next chapter. In this section, a manual step-by-step method is provided.

- Step 1. Choose an initial shifting point and start the procedure by shifting the accumulated distribution curve to the right or left from that of the unshifted curve according to the magnitude of the difference of the expected mean weight difference. The shifted curve should resemble the same pattern as the unshifted curve (Fig 20).
- Step 2. Compute the mean of the shifted curve. This can be done by taking the cumulative percentage of each weight interval and obtaining the percentage for the corresponding interval. The average weight for the shifted curve is the summation of the product of the mid-point intervals with the corresponding percentage.
- Step 3. Compute the variance of the shifted curve. Computation of variance is similar to that mentioned in the first part of the procedure. The computation of variance for the example is shown in Table 11.

- Step 4. To test the acceptability of the estimated curve, two statistical tests are applied. These two tests will be discussed later in this chapter. Briefly, the student t-test is used to test if the mean is within the 95 percent confidence intervals of the expected average truck weight. The chi-square test is used to test the variance (Ref 8). If either the mean or variance of the estimated curve are outside the confidence intervals of the corresponding values, go back to Step 1 and repeat the procedure again. If both mean and variance are within acceptability, go to the next step.
- Step 5. Once a distribution curve is accepted, a truck weight distribution table can be constructed.

The steps are shown in Figs 20 and 21 and the computation of mean and variance is shown in Table 11. The example demonstrates the prediction for the 3-S2 truck weight curve in 1978. The base year is 1970. This year was chosen because of its large sample population.

In 1975, the weight laws of Texas were changed as follows:

- (1) Gross Vehicle Weight from 72 kips to 80 kips,
- (2) Tandem Axle Weight from 32 kips to 34 kips,
- (3) Single Axle Weight from 18 kips to 20 kips.

From the 1975 weight laws, it can be derived that the practical maximum GVW is equal to 80.0 kips. From Table 6, the average GVW factor for 3-S2 is 0.66. Thus, the expected average GVW after the weight law changes is 52.80 kips.

When the average GVW factors were derived, only the legal vehicles were included in the computation of average GVW. Thus, for projecting future truck weight distributions, it is necessary to consider the percentage of the truck population that violate the weight laws. In applying the average GVW methodology, this consideration can be taken care of by a violation factor. If the population of violation is about 5 percent of the total population of a particular type of truck, the violation factor is then equal to 1.05. In the example, the adjusted GVW is 52.8×1.05 , or 55.44 kips.

As shown in Table 4, the average GVW for 1970 is 47.65 kips. From the 1970 weight distribution curve, a first shifting was attempted (Fig 20). From the shifted curve, an average GVW of 62.5 kips was obtained. By comparing it with the expected average GVW, 55.44 kips, it is obvious that the second curve should be somewhere between the unshifted and the first shifted curves. A new plotting is shown in Fig 21. From the new shifted curve, a mean of 55.09 and

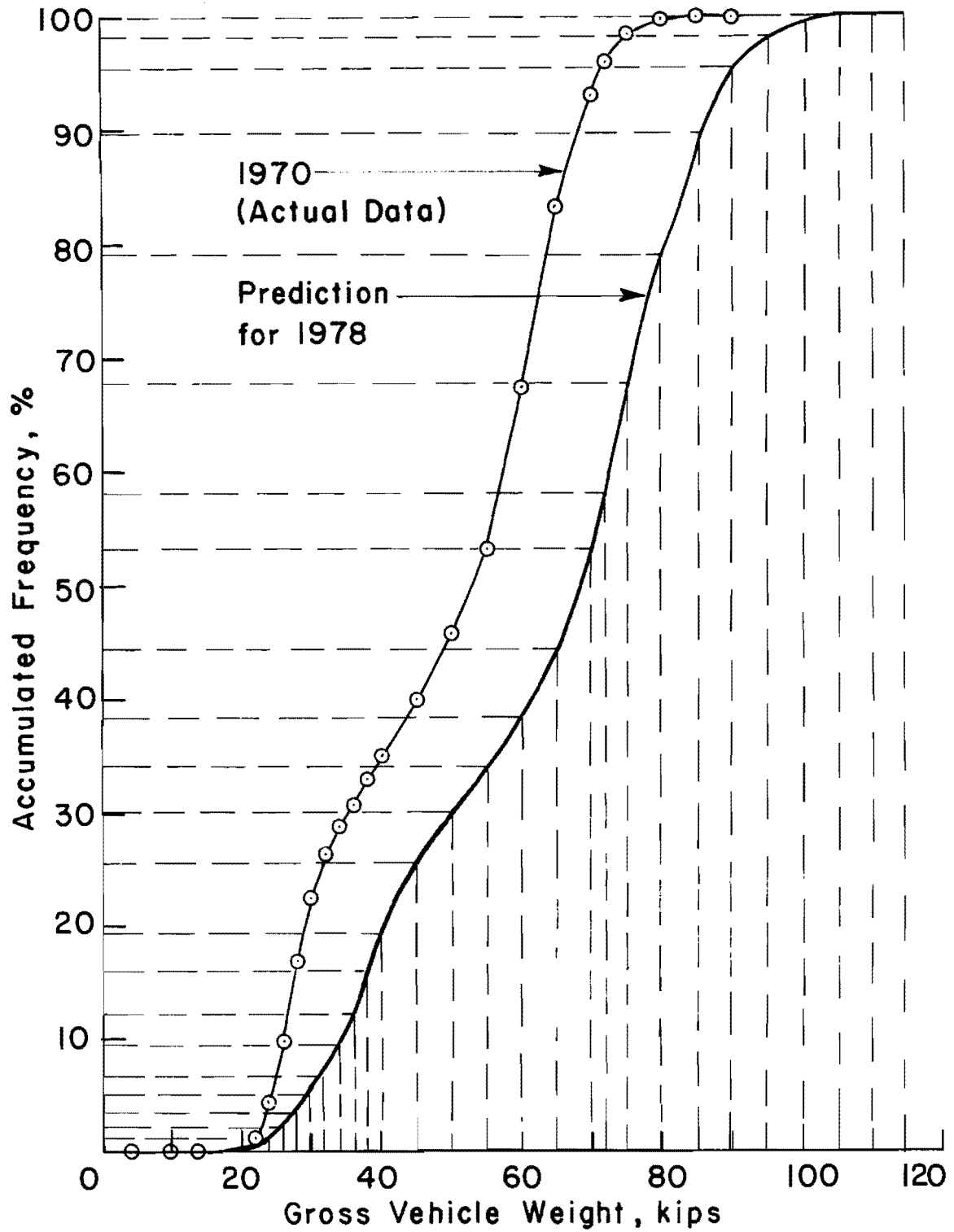


Fig 20. First trial shifting from 1970 data for the projection of 1978 GVW distribution, 3-S2, Texas interstate rural highways.

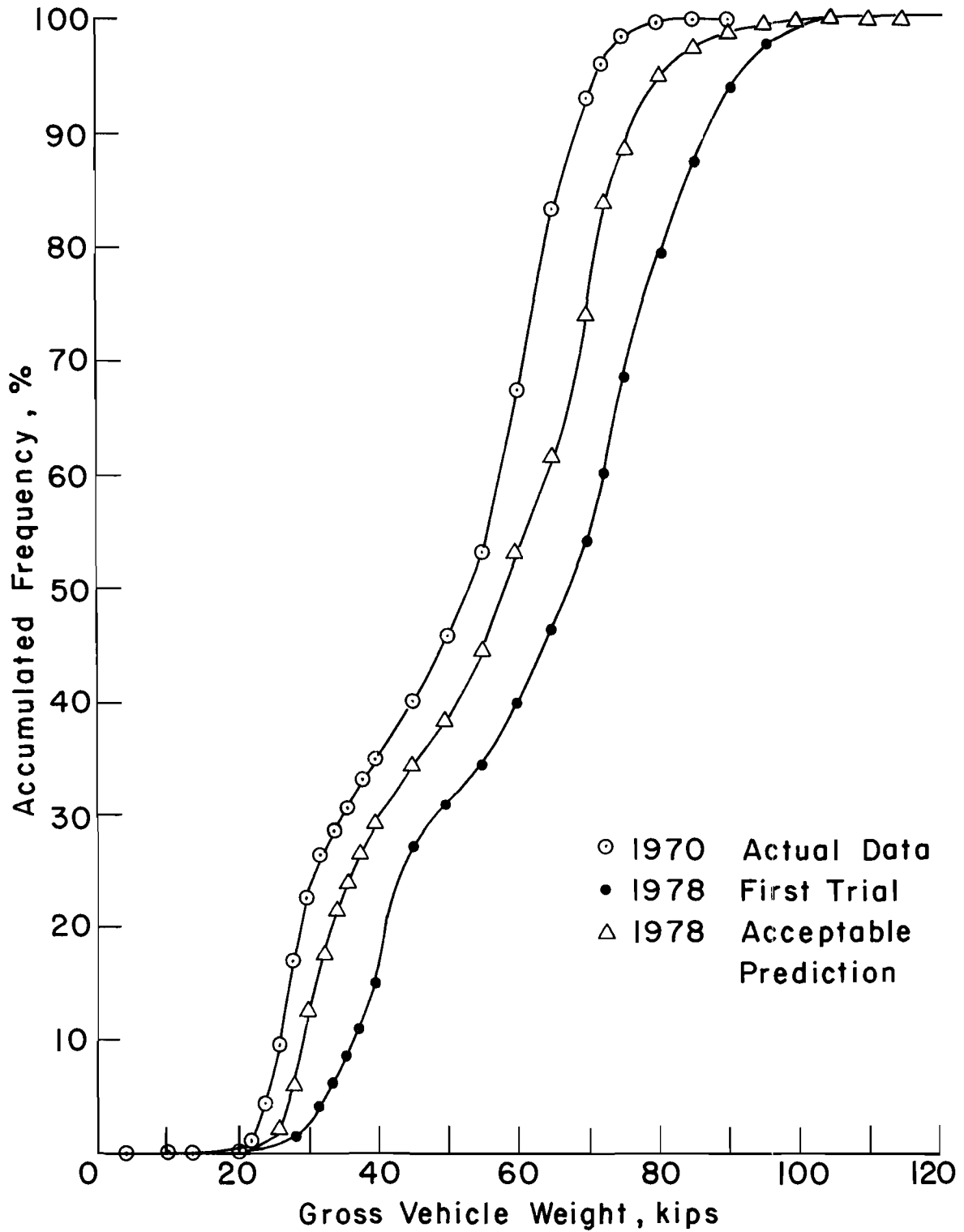


Fig 21. Acceptable shifting for the projection of 1978 GVW distribution, 3-S2, Texas interstate rural highways.

TABLE 11. COMPUTATION OF MEAN AND VARIANCE FROM AN ESTIMATED CUMULATED DISTRIBUTION CURVE

(A) GVW Distribution Intervals	(B) Mid-GVW Intervals	(C) Number of Trucks	(D) B x C	(E) B ² x C
0.0-4.0	2.0	0	0.0	0.00
4.0-10.0	7.0	0	0.0	0.00
10.0-13.5	11.75	0	0.0	0.00
13.5-20.0	16.75	2	33.5	561.13
20.0-22.0	21.0	15	315.0	6,615.00
22.0-24.0	23.0	51	1,173.0	26,979.00
24.0-26.0	25.0	85	2,125.0	53,125.00
26.0-28.0	27.0	117	3,159.0	85,293.00
28.0-30.0	29.0	92	2,668.0	77,372.00
30.0-32.0	31.0	61	1,891.0	58,621.00
32.0-34.0	33.0	37	1,221.0	40,293.00
34.0-36.0	35.0	31	1,085.0	37,975.00
36.0-38.0	37.0	39	1,443.0	53,391.00
38.0-40.0	39.0	32	1,248.0	48,672.00
40.0-45.0	42.5	79	3,357.5	142,693.75
45.0-50.0	47.5	95	4,512.5	214,343.75
50.0-55.0	52.5	117	6,142.5	322,481.25
55.0-60.0	57.5	229	13,167.5	757,131.25
60.0-65.0	62.5	254	15,875.0	992,187.50
65.0-70.0	67.5	157	10,597.5	715,331.25
70.0-72.0	71.0	48	3,408.0	241,968.00
72.0-75.0	73.5	39	2,866.5	210,687.75
75.0-80.0	77.5	20	1,550.0	120,125.00
80.0-85.0	82.5	4	330.0	27,225.00
85.0-90.0	87.5	1	87.5	7,656.25
90.0-95.0	92.5	0	0.0	0.00
		$\Sigma = 1,605$	$\Sigma = 78,256$	$\Sigma = 4,240,727.88$
$\text{MEAN} = \frac{78,256}{1,605} = \underline{48.76}; \quad \text{VARIANCE} = \frac{4,240,727.88 - \frac{(78,256)^2}{1,605}}{(1,605 - 1)} = \underline{265.06}$				
$\text{STANDARD DEVIATION} = \underline{16.28}$				

a variance of 352.23 are obtained. The standard deviation of the curve is 18.79. The computation of mean, standard deviation, and variance is shown in Table 11.

Before considering accepting or rejecting the second shifted curve, it is appropriate to consider some statistical tests applied to the procedure.

STATISTICAL TESTS APPLIED IN THE PROCEDURE

Since the shifting procedure is based on a logical iteration method, it is difficult and time-consuming to find a curve whose mean and variance are exactly the same as those predicted by regression analysis. Thus, for a given shifted curve, statistical tests are needed to examine whether the parameters are within tolerable limits, or confidence intervals, of the expected values. In the analyses of the mean and the variance, a student t-test and chi-squared test are applied, respectively.

Student t-test of the Mean

To examine the mean of a shifted curve to determine if it is within the confidence intervals of the mean estimated by regression analysis, the student t-test is employed (Ref 4, 8). For each shifted curve, a testing of null and an alternative hypothesis are established:

$$H_0 : \hat{W} = \bar{W}$$

$$H_1 : \hat{W} \neq \bar{W}$$

The null hypothesis states that the mean computed from a shifted curve is actually equal to the mean obtained from regression analysis of past trends. The alternate hypothesis states that they are not equal. H_0 should be rejected at the α level of significance if

$$t = \frac{\hat{W} - \bar{W}}{S/\sqrt{n}}$$

is either $\leq -t_{\alpha/2, n-1}$ or $\geq +t_{\alpha/2, n-1}$ (Ref 8).

Notations for the symbols are as follows:

\hat{W}	=	mean truck weight computed from a shifted curve,
\bar{W}	=	mean truck weight obtained from regression of past truck weight data,
S	=	standard deviation of truck weight computed from a shifted curve,
n	=	number of distribution intervals, degree of freedom,
α	=	level of significance; use $\alpha = 0.05$ in the shifting procedure, and
$t_{\alpha/2, n-1}$	=	student t-distribution with n degrees of freedom. Values for student t-distribution may be obtained from statistical tables.

Chi-Squared (χ^2) Test of the Variance

To determine the acceptance of a shifted curve based on its variance, a variance ratio test, or chi-squared (χ^2) test, is applied (Ref 4, 8). The chi-squared test is used to test:

$$H_0 : S^2 = S_o^2$$

versus

$$H_1 : S^2 \neq S_o^2$$

The null hypothesis states that the variance of a shifted curve is equal to that estimated from regression analysis. The alternate hypothesis states that the two variances are not equal. The null hypothesis should be rejected under two situations:

$$\frac{(n-1)S^2}{S_o^2} \leq \chi_{\alpha/2, n-1}^2$$

and

$$\frac{(n-1)S^2}{S_o^2} \geq \chi_{\alpha/2, n-1}^2$$

Notations for the symbols are as follows:

n	=	degree of freedom, or the number of weight intervals in the W-tables,
S^2	=	variance of the shifted curve,
S_o^2	=	variance obtained from regression analysis,
$\chi^2_{\alpha/2, n-1}$	=	chi-square distribution with level of significant $\alpha/2$, and $(n-1)$ degree of freedom.

From the example, the parameters of the shifted curve are

mean	=	55.09,
variance	=	353.23,
standard deviation	=	18.79.

From the average GVW, the expected mean is 55.44.

In order to accept the shifted curve, it is necessary to have a satisfactory t-test. The t-value for the shifted curve is

$$t = \frac{55.09 - 55.44}{18.79/\sqrt{30}} = -0.1020$$

The value 30 corresponds to the number of weight groups considered. From the t-distribution statistical table, for a level of significance of 0.05 and 29 degrees of freedom,

$$t_{0.025, 29} = \pm 2.0452$$

Since the t ratio is less than that from the t-distribution curve, it can be concluded that the second shifted curve is acceptable.

It should be pointed out that in response to weight law changes, only the average truck weight is used to predict a shifted curve. The variance is not used for the following reasons:

- (1) It is difficult to quantify the change in variance with respect to changes in weight laws. It is definite that the variance shows a jump at each increase of weight limits (Fig 13). However, the magnitude of a jump cannot be expressed in terms of the magnitude of changes in truck weight limits.

- (2) With careful selection of the base distribution curve and inducing that the shifting has the general shape of the unshifted curve, a shifted curve under proposed weight limits should yield a reasonably accurate estimate. The best base distribution curve is one that has sufficient sample size so that the curve is generally smooth. The next definition of a good base distribution curve is one that represents the latest distribution trend, which, in turn, conveys the latest technology and practice trends in the trucking industry.

Based on the available 1978 truck weight data, the results of a chi-squared test on the predicted truck weight distribution are shown in Table 12. At a confidence level of 0.05 and 29 degrees of freedom, the chi-square value obtained from a distribution table is 42.56 (Ref 8). Since the computed chi-square value, 1.47, is much lower than 42.56, it indicates that the projection is acceptable.

From experience gained in using the iterative procedure outlined herein, a few insights have occurred which may save time. Before starting to shift a curve, the mean of the curve should be computed. After the first shift, the mean weight of the shifted curve should also be computed. The next step is to decide to which side of the first shifted curve the next curve should be shifted. If the mean weight of the first shifted curve is above the expected weight provided by regression analysis obtained from the average GVW factor, the second shifted curve should be somewhere between the original curve and the first shifted curve. The position of the second shifted curve can be carefully chosen so as to minimize the number of shiftings.

GENERAL DISCUSSION OF THE METHODOLOGY

The methodology provided in this chapter, like other available methodologies, cannot render a precise prediction of what kind of shifting may occur under proposed weight law changes. However, it is the researchers' belief that this model can provide a more statistical and reasonable solution to the shifting problem. The accuracy of the prediction provided by this methodology should also be superior to other methodologies as it makes extensive use of past data.

TABLE 12. CHI-SQUARE TEST ON OBSERVED AND PREDICTED TRUCK WEIGHT DISTRIBUTIONS

GVW	Predicted Cumulated Frequency	Observed Cumulated Frequency
0.0-4.0	0.0	0.0
4.0-10.0	0.0	0.0
10.0-13.5	0.0	0.0
13.5-20.0	0.0	0.0
20.0-22.0	0.5	0.21
22.0-24.0	1.0	0.42
24.0-26.0	2.0	1.94
26.0-28.0	5.5	5.89
28.0-30.0	12.5	12.42
30.0-32.0	16.0	17.51
32.0-34.0	20.0	21.84
34.0-36.0	23.5	24.96
36.0-38.0	26.0	27.36
38.0-40.0	29.0	29.63
40.0-45.0	34.0	35.14
45.0-50.0	38.0	40.15
50.0-55.0	44.0	46.42
55.0-60.0	53.0	53.70
60.0-65.0	61.0	60.31
65.0-70.0	73.0	73.61
70.0-72.0	83.0	80.30
72.0-75.0	97.5	88.55
75.0-80.0	95.0	95.50
80.0-85.0	97.0	97.94
85.0-90.0	98.0	99.28
90.0-95.0	99.0	99.83
95.0-100	99.5	99.92
100.0-105	100.0	99.96
105.0-110	100.0	100.0
110.0-115	100.0	100.0

$$\chi^2 = 1.47$$

However, if this model is used manually with a non-programmable calculator, it requires much more work and time to arrive at the desirable result than other methodologies do and that may be considered an undesirable aspect of the methodology. Nevertheless, with available computer technology and sophisticated programmable hand calculators, much work and time can be saved. In the next chapter, a computer application of this methodology is presented.

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CHAPTER 5. COMPUTERIZED SHIFTING METHODOLOGY IN THE ESTIMATION OF TRUCK WEIGHT DISTRIBUTION

Due to the large amount of data and parameters required in the shifting methodology, the procedure is tedious and users may accidentally insert errors. To increase the accuracy of application and reduce computation time, computer programs in FORTRAN language have been developed for the shifting methodology and procedure discussed in the last chapter. However, due to some factors which will be discussed later, some modifications and adjustments in the procedure have been incorporated.

The computer program series consists of two major programs. One of the programs computes the means and variances of the available data. This program generally follows the guideline provided in the last chapter under the topic "Computation of mean and variance of truck weight distribution data." Equations used in the program are listed in the same section. Users need to input sample sizes for the corresponding truck weight intervals for each year. The computer will provide mean, standard deviation, and variation for each year. With a plotting option, users may obtain the graphical presentation. The program listing and user's manual are listed in Appendices 1 and 6, respectively. A flowchart of the program is shown in Fig 22.

The other program facilitates the shifting element of the methodology. This procedure required some modification, which will be discussed in the next section.

MODIFICATIONS OF SHIFTING METHODOLOGY TO BE COMPATIBLE WITH AUTOMATION

Generally, truck weight distributions resemble the normal distribution pattern. When the truck weight distribution is transformed into an accumulated distribution curve, it shows a traditional S-shaped curve. This S-shaped curve may be represented by the following expression (Ref 3):

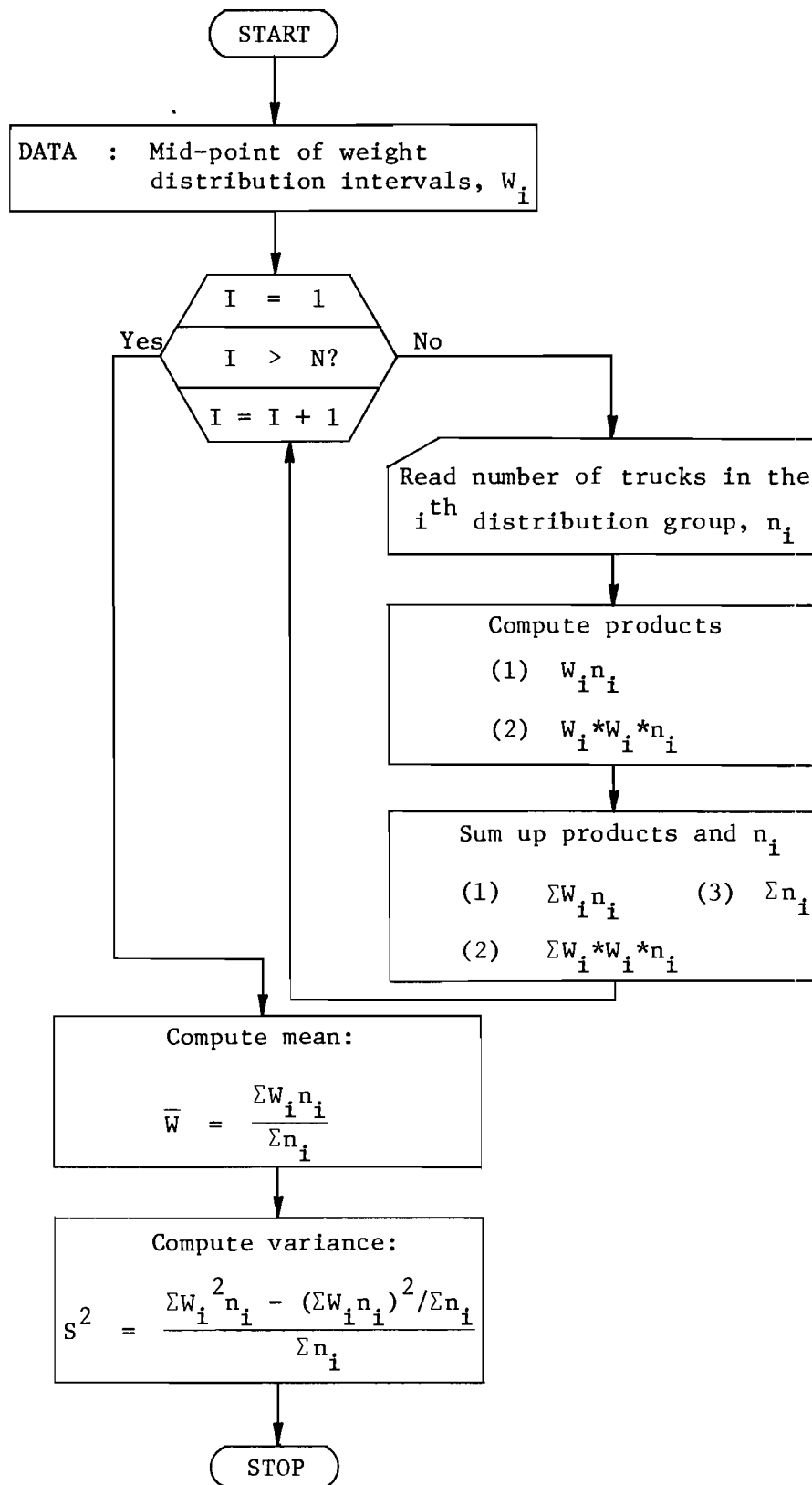


Fig 22. Flowchart showing procedure of computation of mean and variance.

$$Y = \frac{1}{A + B - CX}$$

where A, B, and C = coefficients of the S-curve
 Y = accumulated percentage, and
 X = mid-point of weight distribution
 intervals.

This expression is a non-linearizable equation in that the coefficients A, B, and C cannot be obtained from a linear regression analysis. Only through the trial and error method may these coefficients be found. Yet, certain errors may be induced in this process. These errors will be intensified when the actual shifting is performed. Thus, it is unprofitable and time-consuming to fit data and predict shifts utilizing this non-linearizable equation. To expedite the shifting procedure and to ensure acceptable results, a linear equation was derived.

In the computer model, a "detour" is made in order to make use of a compatible linear equation. The detour is based on the method of using multiplying factors, as suggested in the NCHRP report. Let GVW (1,i%) be the denotation of GVW for year one at the i% on the accumulated frequency curve and GVW (2,i%) be that of year two at the same percentage. Then,

$$\text{Multiplying factor (i\%)} = \frac{\text{GVW}(2,i\%)}{\text{GVW}(1,i\%)} \quad (\text{Eq 5-1})$$

For each of the two years, a cumulative frequency is computed from which the GVW's at one percent increment intervals may be obtained. The GVW (2,i%) should be based on the most recent data as it will affect the shape of the shifted curve through the multiplying factors. From 1 percent to 100 percent, the multiplying factors can be curves fitted into a modified linear equation. The normal form of the equation is given as (Ref 3)

$$Y_i = AX_i^B e^{CX_i}, \quad (\text{Eq 5-2})$$

and the linear form is

$$\log_e(Y_i) = \log_e A + B \log_e X_i + CX_i \quad (\text{Eq 5-3})$$

In this shifting procedure, the following deviations are used for Y_i and X_i :

Y_i = multiplying factors at one percent increment intervals,

X_i = GVW (1,i%).

Generally speaking, all the cumulative distribution curves for truck weight data can be fitted into this equation. However, to guard against any discrepancy, a chi-squared test is introduced in the computer shifting methodology to reject any bad fitting. The chi-squared test checks the goodness-of-fit between the observed Y's and the expected Y's computed by the fitted equation.

The fitting of data into Eq 5-3 is done by the least squares method. Regression coefficients A, B, and C are computed by solving the following system of equations:

$$\begin{bmatrix} n & \Sigma(\log_e X_i) & \Sigma X_i \\ \Sigma(\log_e X_i) & \Sigma(\log_e X_i)^2 & \Sigma(X_i * \log_e X_i) \\ \Sigma X_i & \Sigma(X_i \log_e X_i) & \Sigma(X_i)^2 \end{bmatrix} \begin{bmatrix} A \\ B \\ C \end{bmatrix} = \begin{bmatrix} \Sigma(\log_e Y_i) \\ \Sigma(\log_e X_i)(\log_e Y_i) \\ \Sigma(X_i \cdot \log_e Y_i) \end{bmatrix}$$

In the 3 x 3 matrix, n is equal to 100, which represents the number of multiplying ratios obtained from the cumulative frequency curve.

Once the regression coefficients A, B, and C are found, the values are used as inputs into Eq 5-2. With GVW (1,i%) as the X-values, the multipliers and their respective GVW (2,i%) may be found. In the computer model, the regression coefficients are used as initial values from which the shifting will be started. After checking against criteria which will be discussed later, a new set of regression coefficients may be obtained. With Eq 5-1, GVW (2,i%), which represents the GVW (i%) for a future year, can be computed by multiplying the new multiplying factor and GVW (1,i%). This operation can be expressed by the following equation:

$$\text{GVW (2,i\%)} = \text{GVW (1,i\%)} \times \text{multiplying factor (i\%)} \quad (\text{Eq 5-4})$$

The whole procedure is summarized in the flowchart shown in Fig 23.

As suggested in the previous chapter, the shifting of the distribution curve is performed by an iterative procedure. For each iteration, the shifted curve is to be accepted or rejected based on the compatibility of its mean and variance with those projected by regression analysis or average GVW factors. The iterative method that is programmed into FORTRAN language will be presented next.

ITERATION METHOD IN THE SHIFTING PROCEDURE

In the shifting procedure, Powell's method is used to perform the iterations (Refs 6 and 9). In Powell's method the three coefficients given in Eq 5-2 for the proposed year are located through a series of iterations so as to minimize the objective function. Each iteration involves a search for a minimum along a set of three linearly independent directions. These directions are the coordinate directions initially, but at each iteration a new direction is defined to replace one of the initial directions. The new directions formed after a series of iterations will be mutually conjugate (Ref 6). The objective function used in the Powell method is the difference between the mean computed from a curve characterized by the coefficients A, B, and C and the mean obtained from regression analysis or average GVW factors. It can be expressed as follows:

$$f(u) = \bar{W} - \left[\text{GVW}(1,i\%) * u(1) * \text{GVW}(1,i\%)^{u(2)} * \right. \\ \left. \text{Exp}(u(3) * \text{GVW}(1,i\%)) \right]$$

where

- $f(u)$ = objective function to be minimized by Powell's method
- \bar{W} = mean weight obtained from regression analysis or average GVW factors,
- $\text{GVW}(1,i\%)$ = GVW for base year at 1% increment intervals,
- $u(1)$, $u(2)$, and $u(3)$ are the regression coefficients; they correspond to the coefficients A, B, and C, respectively, as given in Eq 5-2.

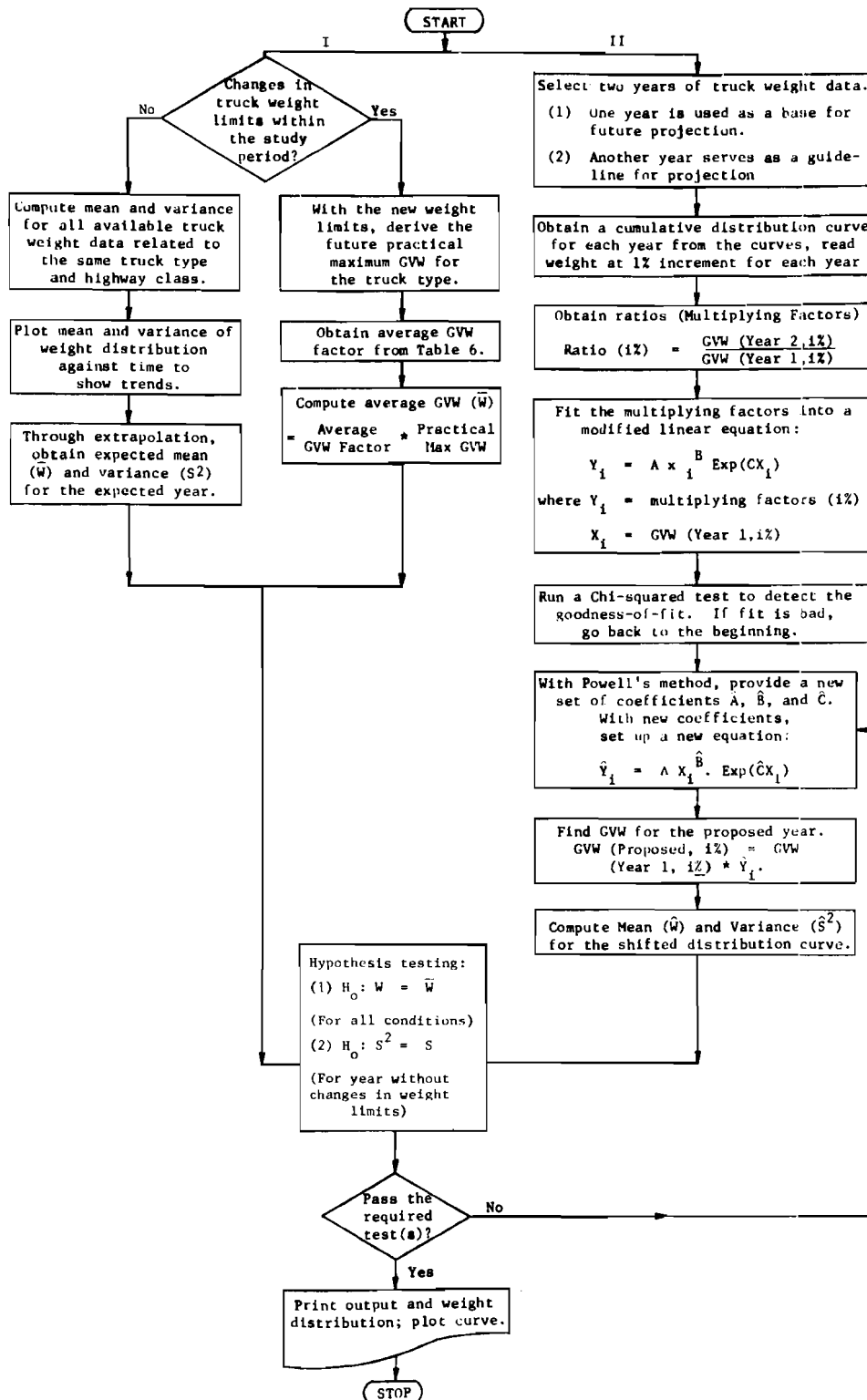


Fig 23. Flowchart showing the computerized shifting methodology.

For the Powell method, better initial values for $u(1)$, $u(2)$, and $u(3)$ ensure a faster convergence. Thus, in choosing truck weight distribution data, this principle should serve as a guide. One should choose data with sufficient sample size so that smooth cumulative curves and, thus, better initial values for the coefficients may be provided.

LIMITATIONS OF THE COMPUTER PROGRAM

In general, the Powell method works well with most of the initial coefficients that were derived from available truck weight data. However, the possibility that the iteration does not converge cannot be eliminated. In any case, if it fails to converge and provide new coefficients, one should consider using another set of truck weight data so as to provide a new set of initial coefficients.

Since the objective function to be minimized by Powell's method is expressed in terms of the means of truck weight only, the new truck weight distribution will be shaped according to the mean rather than the variance. Hence, distribution with unacceptable variance may be derived from the computer shifting methodology. Recommendations for a remedy are given as follows:

- (1) Change either set of the truck weight distribution data; i.e., change either GVW (1,i%) or GVW (2,i%) or even obtain and use new sets of data.
- (2) Due to certain problems in data collection procedures, some data may not reflect expected trends. Obviously, erroneous data should not be used unless altered by combining data that was collected in different years. The process of combination dilutes any extremities in a set of data and, thus, smooths the distribution curve.
- (3) To predict the shifting that occurred after changes in weight laws, one should not be overly concerned about the variance of a new shifted curve provided the most recent truck weight data were used. Generally, the Powell method generates new coefficients that provide a distribution curve with a pattern compatible with that characterized by the initial coefficients.

APPLICATIONS OF THE COMPUTERIZED SHIFTING METHODOLOGY

To illustrate the usage of the computerized shifting methodology, results of two runs are presented in this section. Input data are given in Appendix 6. In order to check the precision and accuracy of the prediction, shifting was performed for past years. This would provide the comparison of the results derived from the computer model and actual field data. The first run is to demonstrate a shifting within the span of the same weight limits. The second run is to demonstrate a shifting to cope with the weight law change.

Based on the study of weight trends as discussed in the last chapter, it is shown that, within the span of the same weight laws, the mean and variance change gradually. The changes of the mean and variance over a period of time, provided that there were no weight law changes, can be fitted into a certain regression model. In this example, a prediction for 2D on Interstate highways in 1974 was estimated by the computer model. In Fig 24, both trends, for the mean and variance, of 2D from 1959 to 1973 are shown. It was assumed that the data for 1974 and afterwards were not available. The trend observed from 1960-1973 shows that the mean can be fitted into a linear regression model. For 1974, the expected mean weight is thus 13.38 kips. The variance basically does not show any major variation. Thus, the average of the variance, 32.0 (or Standard Deviation of 5.6), is used. The data collected in 1973 were used as the base to project for 1974 distribution. Figure 25 shows the predicted and actual distribution curves for 1974. Table 13 shows the comparison of the accumulative frequencies of the two curves. The chi-squared value of 2.02 shows that the goodness of fit is exceptional. Both the student t-test and the chi-squared test on mean and variance are acceptable.

The second example illustrates prediction of a truck distribution curve for 1978. In the prediction, the data after 1975 were assumed to be unavailable. In this example, the basic difference from the previous example is that there was a change in weight law in 1975. Thus, this example deals with the shifting including the effects of weight law changes. For later comparison of various methodologies, the example will be the same as in the example discussed in the last chapter. In this example, the 3-S2 on Interstate highways was used. The prediction was for 1978. From Table 6, in the previous chapter,

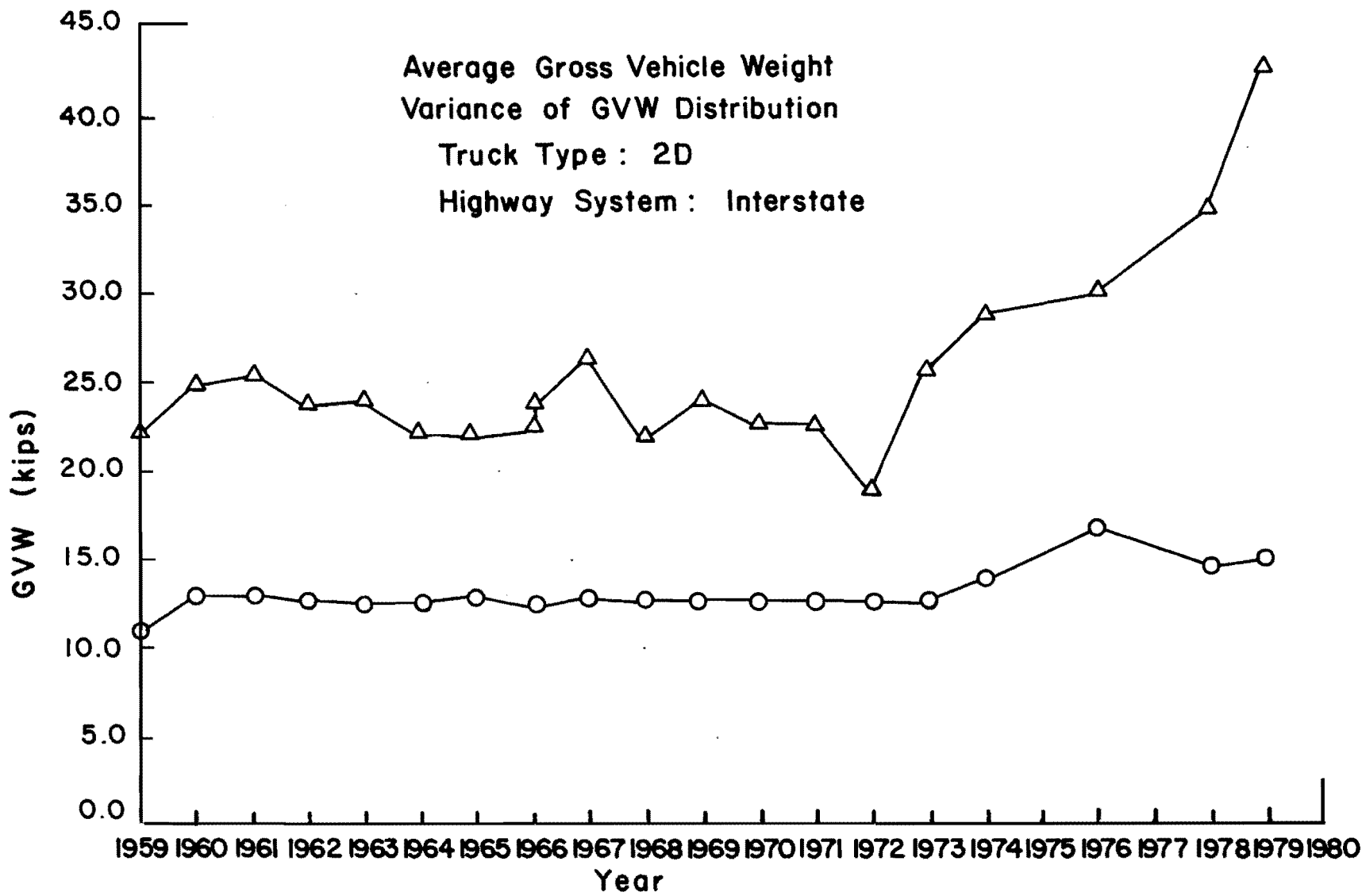


Fig 24. Trends of mean and variance for truck GVW distribution (2D, Texas interstate rural highways).

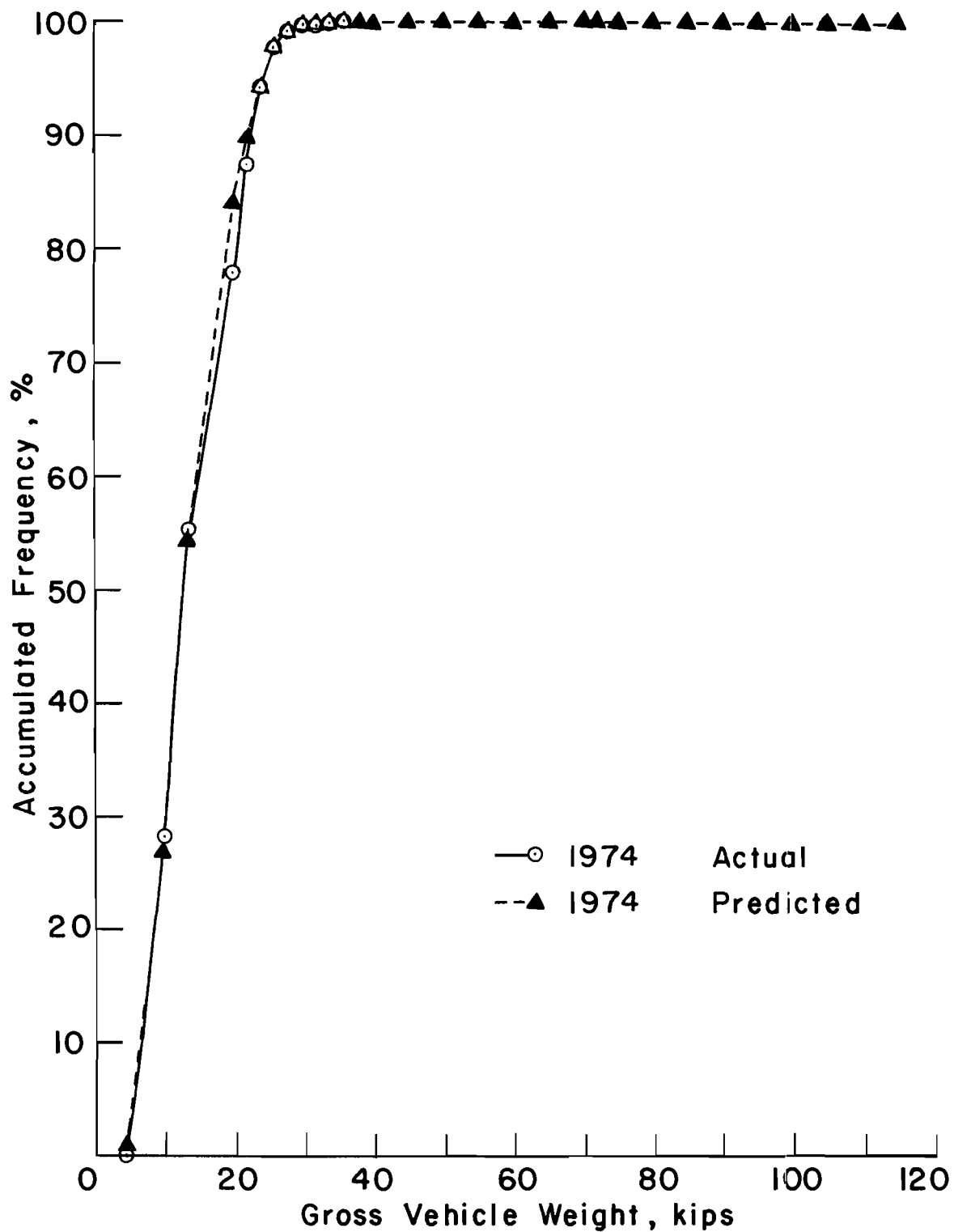


Fig 25. Comparison of accumulated frequency distribution curves derived from actual field data and computerized shifting methodology (2D, 1974, Texas Interstate rural).

TABLE 13. COMPARISON OF OBSERVED ACCUMULATED FREQUENCIES OF ACTUAL FIELD DATA AND EXPECTED ACCUMULATED FREQUENCIES FROM SHIFTING MODEL FOR 2D ON TEXAS INTERSTATE RURAL HIGHWAYS (PREDICTION IS FOR 1974)

GVW Distribution Intervals	Observed Accumulated Frequency	Expected Accumulated Frequency
0.0-4.0	0.0	.9986
4.0-10.0	30.61	26.7740
10.0-13.5	51.02	54.4751
13.5-20.0	81.63	83.9244
20.0-22.0	89.80	89.7085
22.0-24.0	95.92	94.1995
24.0-26.0	95.92	97.9639
26.0-28.0	97.96	99.1991
28.0-30.0	97.96	99.5054
30.0-32.0	97.96	99.8118
32.0-34.0	97.96	100.0000
34.0-36.0	100.00	100.0000

$$\chi^2 = 2.02$$

the average GVW factor for 3-S2 was 0.66. The practical maximum GVW after the 1975 weight law changes was 80 kips. Thus, the expected mean weight was 80.0×0.66 , or 52.8 kips. Assuming five percent of trucks were overloaded the expected mean weight thus becomes 52.8×1.05 , or 55.44 kips. For the reasons discussed earlier in the last chapter, the variance is not important in this shifting application. The inputs for this example are shown in Appendix 6. Data sets collected in 1974 and 1970 are used as the prediction basis. Figure 26 shows the predicted and actual distribution curves for 1978. Table 14 presents the comparison of the accumulated frequencies of the two curves. The chi-squared value shows that the prediction fits well with actual field data.

COMPARISON OF VARIOUS SHIFTING METHODOLOGIES

The NCHRP and SDHPT shifting methodologies are similar in their procedures. Both methodologies apply the concept of multiplying factors. Before the initial weight is reached, the multiplying factor remains as unity. Then, the multiplying factor increases gradually from the initial weight to the present practical maximum GVW, where it levels off. The difference between the two methodologies centers on the initial weights for each type of truck. For prediction of truck weight distribution after the 1975 changes, the initial weights given in Table 15 were used in each methodology.

TABLE 15. INITIAL WEIGHTS OF FOUR TRUCK TYPES USED IN NCHRP AND SDHPT METHODOLOGIES (BASED ON 1974 TRUCK WEIGHT DATA)

	<u>NCHRP (kips)</u>	<u>SDHPT(kips)</u>
2-D	4.0	20.0
3-A	13.5	30.0
3-S2	13.5	40.0
2-S1-2	24.0	50.0

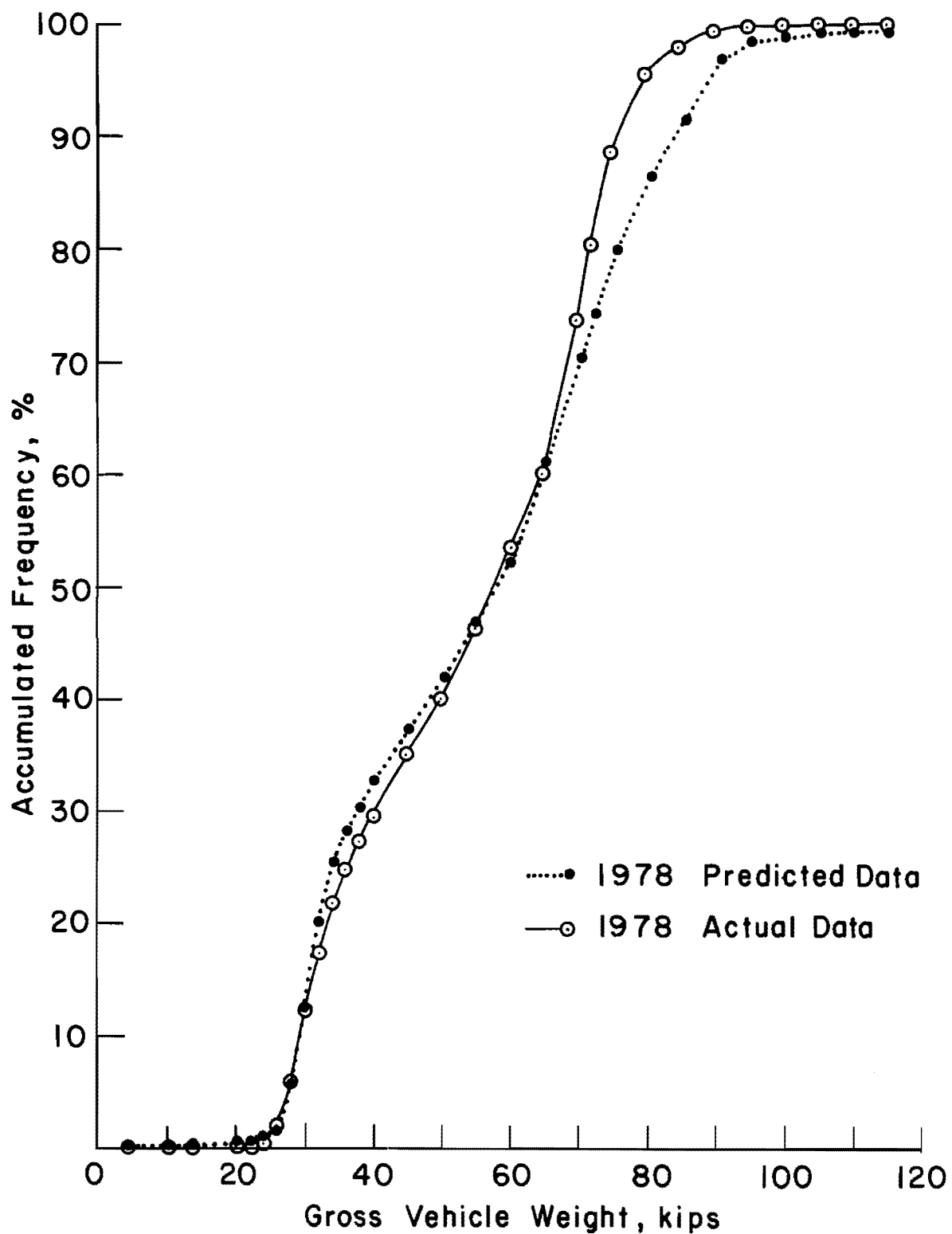


Fig 26. Comparison of actual and predicted GVW distribution curves for 3-S2 on Texas Interstate rural highways.

TABLE 14. COMPARISON OF CUMULATIVE FREQUENCIES OF WEIGHT DISTRIBUTION FOR THE 3-S2 TRUCKS BASED ON ACTUAL FIELD DATA AND PREDICTION BY COMPUTERIZED SHIFTING MODAL (PREDICTION IS FOR 1978)

Distribution Intervals (Kips)	Observed Cumulated Frequency (%)	Expected Cumulated Frequency (%)	
0.0- 4.0	0.0	0.1534	
4.0-10.0	0.0	0.3835	
10.0-13.5	0.0	0.5177	
13.5-20.0	0.08	0.7669	
20.0-22.0	0.21	0.8436	
22.0-24.0	0.42	0.9203	
24.0-26.0	1.94	0.9970	
26.0-28.0	5.89	5.1920	
28.0-30.0	12.42	12.6243	
30.0-32.0	17.51	20.3565	
32.0-34.0	21.84	25.4414	
34.0-36.0	24.96	28.3071	
36.0-38.0	27.36	30.4514	
38.0-40.0	29.63	32.8258	
40.0-45.0	35.14	37.5394	
45.0-50.0	40.15	42.1638	
50.0-55.0	46.42	47.0900	
55.0-60.0	53.70	52.4316	
60.0-65.0	60.31	61.4669	
65.0-70.0	73.61	70.8808	
70.0-72.0	80.30	74.7253	
72.0-75.0	88.55	80.3150	
74.0-80.0	95.50	86.9204	
80.0-85.0	97.94	92.1033	
85.0-90.0	99.28	97.4287	
90.0-95.0	99.83	99.0476	
95.0-100	99.92	99.3687	
100.0-105	99.96	99.6897	
105.0-110	100.00	100.00	
110.0-115	100.00	100.00	$\chi^2 = 8.26$

The computerized shifting methodology also applies the concept of multiplying factors. However, the multiplying factors do not show a linear trend along the truck GVW.

For the comparison of the shifted curves provided by the three methodologies, two illustrations were used. These illustrations are based on the following assumptions:

- (1) The truck weight data for the 1975 periods were not available for reference.
- (2) The only available information is the magnitude of changes in weight limits.

Due to an insufficient sample size for 3A and 2-S1-2, illustrations of shifting for these two types of trucks are not provided. The two illustrations used in this section are based on the 1974 truck weight data for 2D and 3-S2, assuming five percent of trucks were running overweight in both cases.

The following is the input information for each methodology.

1. 2D, Interstate highway system

a. NCHRP—initial weight = 4.0 kips

PMGVWP = 24.50 kips

PMGVWF = 27.22 kips

b. SDHPT—initial weight = 20.0 kips

PMGVWP = 24.50 kips

PMGVWF = 27.22 kips

c. Computerized Average GVW Factor Methodology

- expected mean weight = $0.41 \times 27.22 \times 1.05 = 14.58$ kips

- expected variance = 100.0 kips

- 1974 truck weight data as latest year

- 1970 truck weight data as base year

- initial shifting point = 0 kips

The shifted curves are plotted in Fig 27. For comparison with the actual weight distribution, a curve for the actual data is provided in the same figure.

Table 16 shows the distributions provided from each methodology and the chi-squared test results for the goodness of fit.

2. 3-S2, Interstate Highway System

- a. NCHRP—Initial weight = 13.5 kips
 PMGVWP = 72.0 kips
 PMGVWF = 80.0 kips
- b. SDHPT—Initial weight = 40.0 kips
 PMGVWP = 72.0 kips
 PMGVWF = 80.0 kips
- c. Computerized Average GVW Factor methodology
- expected mean weight = $0.66 \times 80.0 \times 1.05 = 55.44$ kips
 - expected variance = 400.0 kips
 - 1974 truck weight data as latest year
 - 1970 truck weight data as base data
 - initial shifting point = 0 kips

The comparison between the shifted curves and the actual data is shown in Fig 28. Table 17 shows the distributions and chi-squared test goodness of fit results. The lower the chi-squared value, the better the fit of the predicted curve with the actual curve.

GENERAL DISCUSSION OF THE METHODOLOGY

Based on previous analyses, it can be concluded that the computerized shifting methodology performs satisfactorily in predicting future truck weight distribution trends. With a practical maximum GVW and the average GVW factor for a certain type of truck, engineers and planners may assume a reasonable violation rate and project the corresponding truck weight distribution. Presently, there is no specific regression model to predict the future violation rate. It is an element dependent on the interaction of several factors. The factors include, but are not limited to, the following:

- (1) degree of weight law enforcement,
- (2) availability of other truck types which can be used to reach the maximum GVW,
- (3) price of gasoline, and
- (4) other highway legislation, such as speed limits.

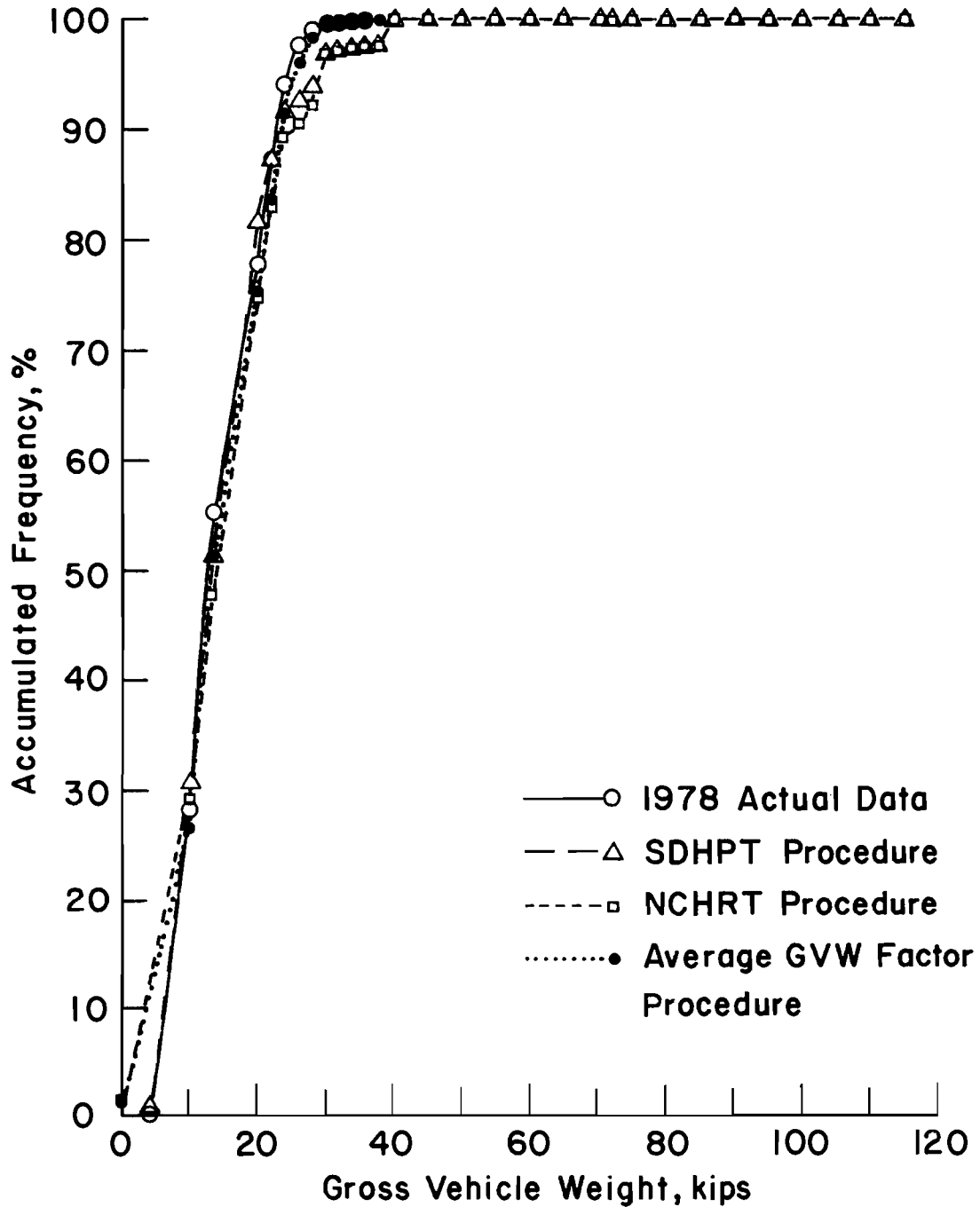


Fig 27. Comparison of GVW distribution curves derived from actual field data, NCHRP, SDHPT, and the computerized AGVWF shifting procedure (2D, Texas Interstate rural, 1978).

TABLE 16. COMPARISON OF PROJECTED CUMULATED FREQUENCY CURVES GENERATED FROM AVAILABLE SHIFTING METHODOLOGIES AND THE ACTUAL FIELD DATA FOR 2D ON TEXAS INTERSTATE RURAL HIGHWAYS (PROJECTION IS FOR 1978)

GVW Distribution Intervals	Actual Field Data	NCHRP Shifting Methodology	SDHPT Shifting Methodology	Computerized Average GVW Factor Methodology
0-4.0	0-0	0.95	0.95	0.93
4.0-10	28.33	29.08	30.64	26.64
10-13.5	55.35	47.41	51.02	51.29
13.5-20	77.86	74.76	81.50	75.21
20-22	87.43	82.54	86.98	83.54
22-24	94.18	89.29	91.54	91.32
24-26	97.75	90.57	92.54	96.12
26-28	99.06	92.45	93.78	98.41
28-30	99.62	96.94	96.94	99.13
30-32	99.62	97.19	97.19	99.37
32-34	99.81	97.40	97.40	99.60
34-36	100.00	97.60	97.60	99.84
36-38	100.00	97.80	97.80	100.00
38-40	100.00	100.00	100.00	100.00
40-45	100.00	100.00	100.00	100.00
Chi-squared Value:		4.33	2.63	1.76

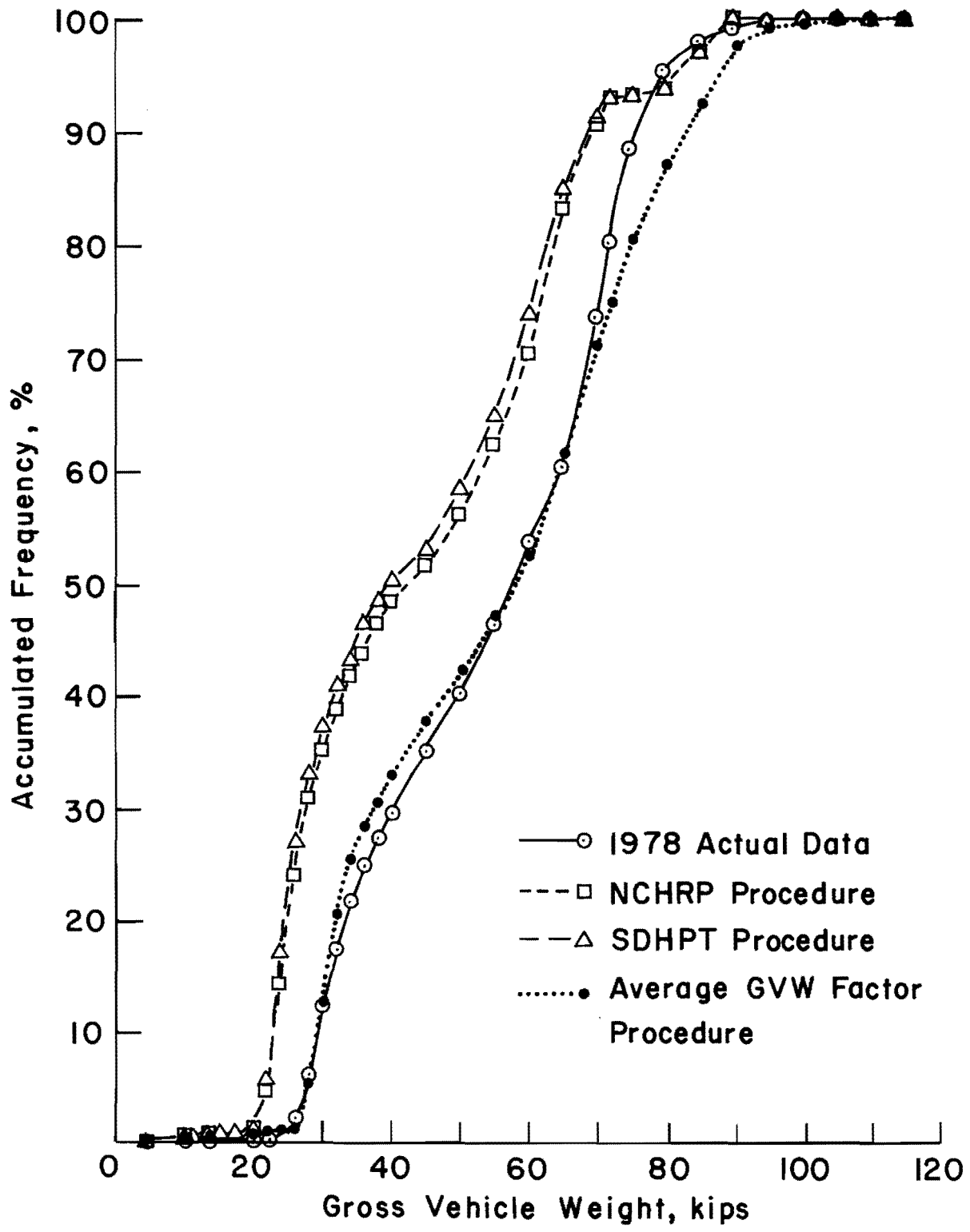


Fig 28. Comparison of GVW distribution curves derived from actual field data, NCHRP, SDHPT, and the computerized AGVWF shifting procedure (3-S2, Texas Interstate rural, 1978).

TABLE 17. COMPARISON OF PROJECTED CUMULATIVE FREQUENCY CURVES GENERATED FROM AVAILABLE SHIFTING METHODOLOGIES AND THE ACTUAL DATA FOR 3-S2 ON TEXAS INTERSTATE RURAL HIGHWAYS (PROJECTION FOR 1978)

GWV Distribution Intervals	Actual Field Data	NCHRP Shifting Methodology	SDHPT Shifting Methodology	Computerized Average GWV Factor Methodology
0.0-4.0	0.00	.1996	.2020	0.1534
4.0-10.0	0.00	.4989	.5049	0.3835
10.0-13.5	0.00	.6736	.6816	0.5177
13.5-20.0	0.00	.9979	1.2932	0.7669
20.0-22.0	0.21	4.4618	5.3447	0.8436
22.0-24.0	0.42	14.3453	17.0088	0.9203
24.0-26.0	1.94	24.1679	27.0374	0.9970
26.0-28.0	5.89	30.8721	32.9852	5.1920
28.0-30.0	12.42	35.3293	37.1071	12.6243
30.0-32.0	17.51	38.9926	40.9620	20.3565
32.0-34.0	21.84	41.8016	43.0606	25.4414
34.0-36.0	24.96	43.9880	46.1142	28.3071
36.0-38.0	27.36	46.6611	48.5965	30.4514
38.0-40.0	29.63	48.8201	50.1955	30.8258
40.0-45.0	35.14	51.8905	52.9377	37.5394
45.0-50.0	40.15	56.1453	58.3009	42.1638
50.0-55.0	46.42	62.5450	64.8687	47.0900
55.0-60.0	53.70	70.4924	73.9259	52.4316
60.0-65.0	60.31	83.0160	84.9037	61.4669
65.0-70.0	73.61	90.5734	91.6666	70.8808
70.0-72.0	80.30	93.0143	93.0878	74.7253
72.0-75.0	88.55	93.3409	93.4138	80.3150
75.0-80.0	95.50	93.8852	93.9572	86.9204
80.0-85.0	97.94	97.2693	97.5279	92.1033
85.0-90.0	99.28	100.0000	100.0000	97.4287
90.0-95.0	99.83	100.0000	100.0000	99.0476
95.0-100.0	99.92	100.0000	100.0000	99.3687
100.0-105.0	99.96	100.0000	100.0000	99.6897
105.0-110.0	100.00	100.0000	100.0000	100.0000
110.0-115.0	100.00	100.0000	100.0000	100.0000
		$\chi^2=149.96$	$\chi^2=172.06$	$\chi^2= 8.26$

In predicting future truck weight distribution, engineers and planners may exercise their judgment in defining the percentage violations. One suggestion, however, is to run the program with different violation factors. This method is to test the sensitivity of prediction to violation.

Besides predicting future GVW distribution, the computerized shifting procedure may be used to predict future axle weight distribution. This topic will be presented in the next chapter. The procedure can also be used to predict additional damages due to different degrees of violation.

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CHAPTER 6. PREDICTIONS OF AXLE WEIGHT DISTRIBUTION AND EQUIVALENT 18-KIP SINGLE AXLE LOAD

In the procedure for estimation of maintenance cost and highway rehabilitation cost for changing motor vehicle size and weight limits, one important element is the prediction of total equivalent 18-kip single axle load (18-KESAL). The relationship of the element with other components in the procedure is shown in Fig 29. The GVW distribution is directly affected by the axle weight distribution, which, in turn, directly affects the computation of total 18-KESAL. In previous chapters, much attention was given to the prediction of GVW distribution under proposed weight limits. In this chapter, the prediction of axle weight distribution is presented.

ESTIMATION OF TANDEM AXLE WEIGHT DISTRIBUTION FROM AVAILABLE DATA

In this section, the discussion is focused on two types of trucks, the 3A and 3-S2. Predictions for 2D and 2-S1-2 are not included in the discussion because it is not possible to separate the loading axle weight distribution from the single axle weight distribution given in W-4 tables. For 3A and 3-S2, the axle weight distributions given in the W-4 tables provide information for the steering axle and loading axle weight distributions. Due to the availability of axle weight data, it is thus possible to use past trends in predicting the future axle weight distribution.

For the single unit truck symbolized by 3A, the single axle is the steering axle while the tandem axle is the loading axle. Thus, for one particular truck it is obvious that the gross vehicle weight is the summation of the single axle weight and the tandem axle weight. This relationship is expressed in an equation as follows:

$$GVW = SAW + TAW \qquad \qquad \qquad (Eq 6-1)$$

For the 3-S2, which has one single axle (steering axle) and two tandem axles, the gross vehicle weight is obviously equal to the summation of the steering

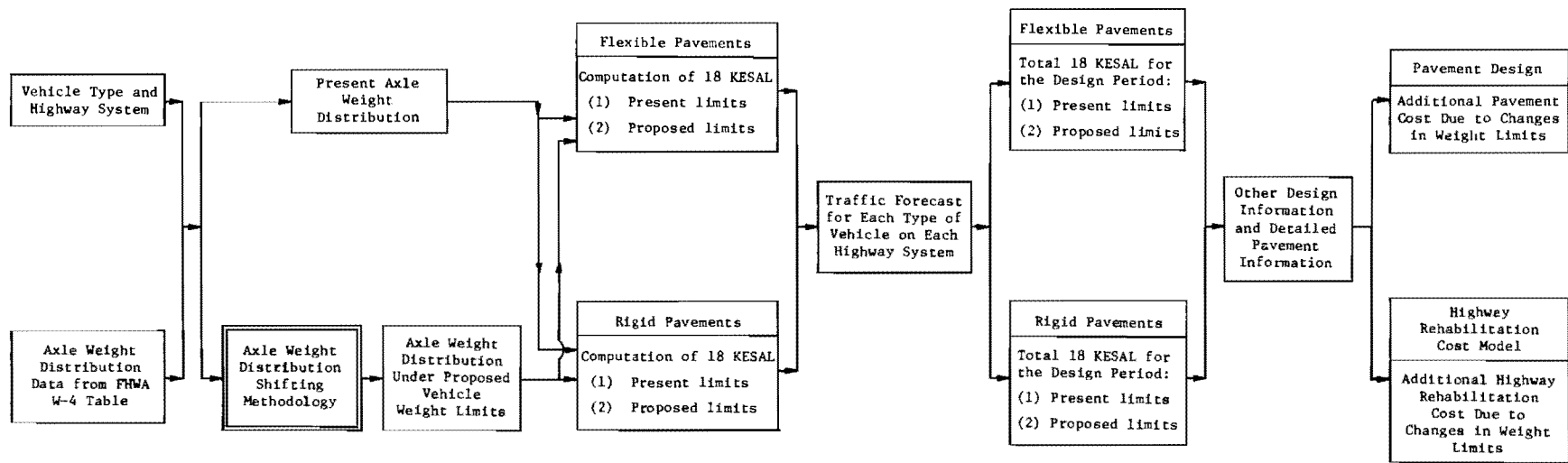


Fig 29. Schematic representation illustrating the procedure to estimate the additional pavement cost and highway rehabilitation cost for changing motor vehicle weight limits.

axle weight and the weight of tandem axles. It can be expressed as follows:

$$GVW = SAW + 2 TAW \quad (\text{Eq 6-2})$$

In consideration of these two equations, attempts were made to relate the three kinds of weight distribution data. The concept was to explore the relationship of GVW, SAW, and TAW for 3A and 3-S2 so that a prediction for TAW distribution could be made possible from the prediction of GVW distribution.

Let $GVW(i\%)$, $SAW(i\%)$ and $TAW(i\%)$ be the GVW, SAW, and TAW at $i\%$ along the truck weight cumulated percentage curves for either 3A or 3-S2. For the single unit trucks, 3A, prediction of $TAW(i\%)$ was based on the following equation:

$$TAW_{3A}(i\%) = \left[GVW(i\%) - SAW(i\%) \right] \quad (\text{Eq 6-3})$$

and for 3-S2, the following equation was used:

$$TAW_{3-S2}(i\%) = 0.5 \left[GVW(i\%) - SAW(i\%) \right] \quad (\text{Eq 6-4})$$

In the analysis, predicted TAW values were based solely on the GVW and SAW distribution data while the field data for TAW's were used as actual data for comparison. Once the $TAW(i\%)$ values were obtained, a predicted cumulative percentage curve was constructed. The distributions of the predicted TAW's and the actual TAW's were plotted in a graph for comparison. In the study, data collected in different years were used to prove the relationships stated in Eqs 6-3 and 6-4. These years represent a spectrum of different conditions. For instance, 1970 was chosen to show the trend of the 70's. Year 1974 was used to reflect the weight distribution before the changes in Texas weight limits. Year 1976 was known as an unusual year in that the weight data reflected that weights for different trucks increased significantly after the 1975 change. Year 1979 was used to reflect the latest trends. The distribution curves for 3A are shown in Fig 30 (a-d) and 3-S2 in Fig 31 (a-d). Along with the distribution curves, the predicted actual TAW distribution data were analyzed for the goodness-of-fit with chi-squared values shown in Table 18. Both the graphical presentation and the

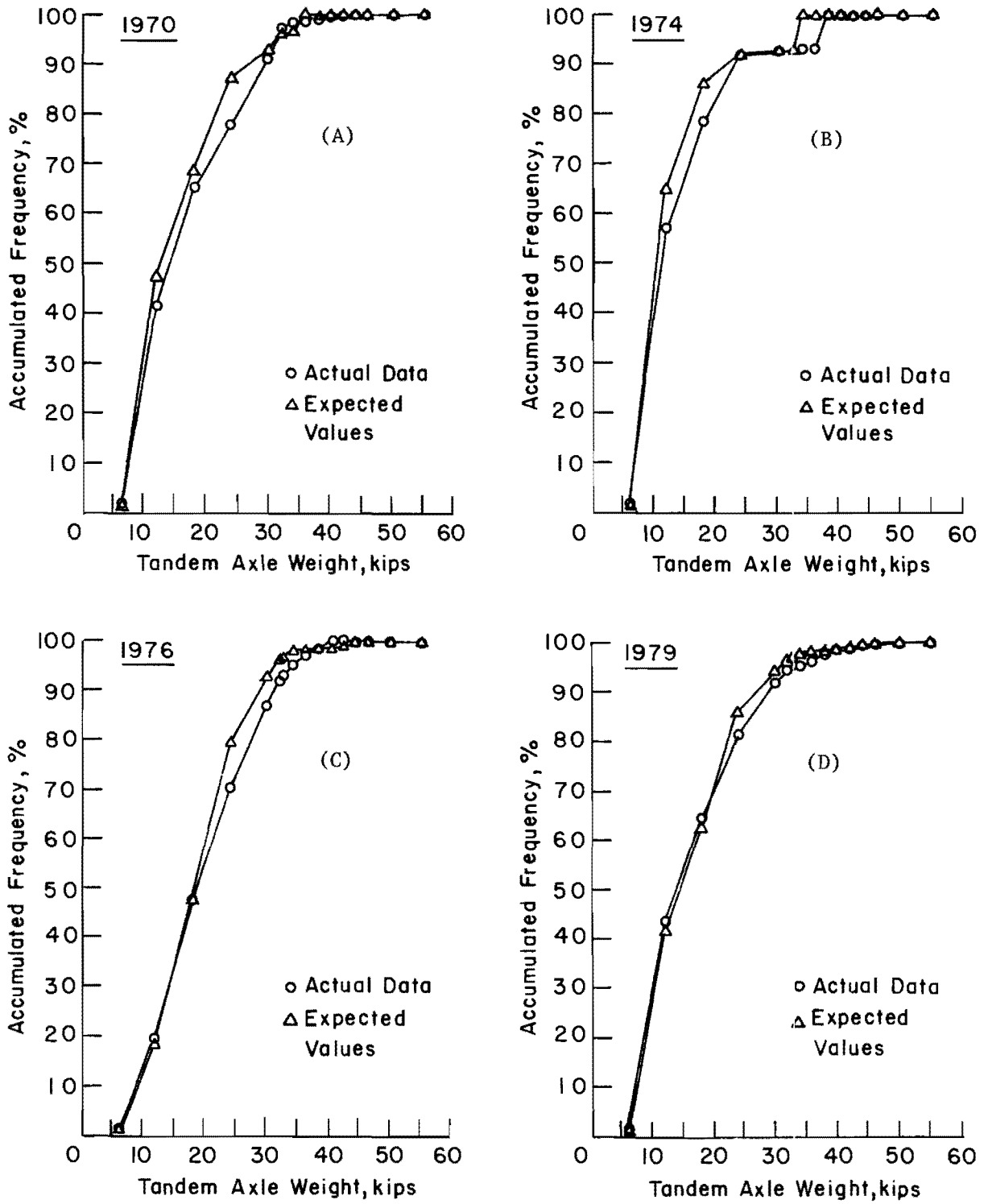


Fig 30. Comparison of actual and expected tandem axle distribution for 3A on Texas Interstate rural highways.

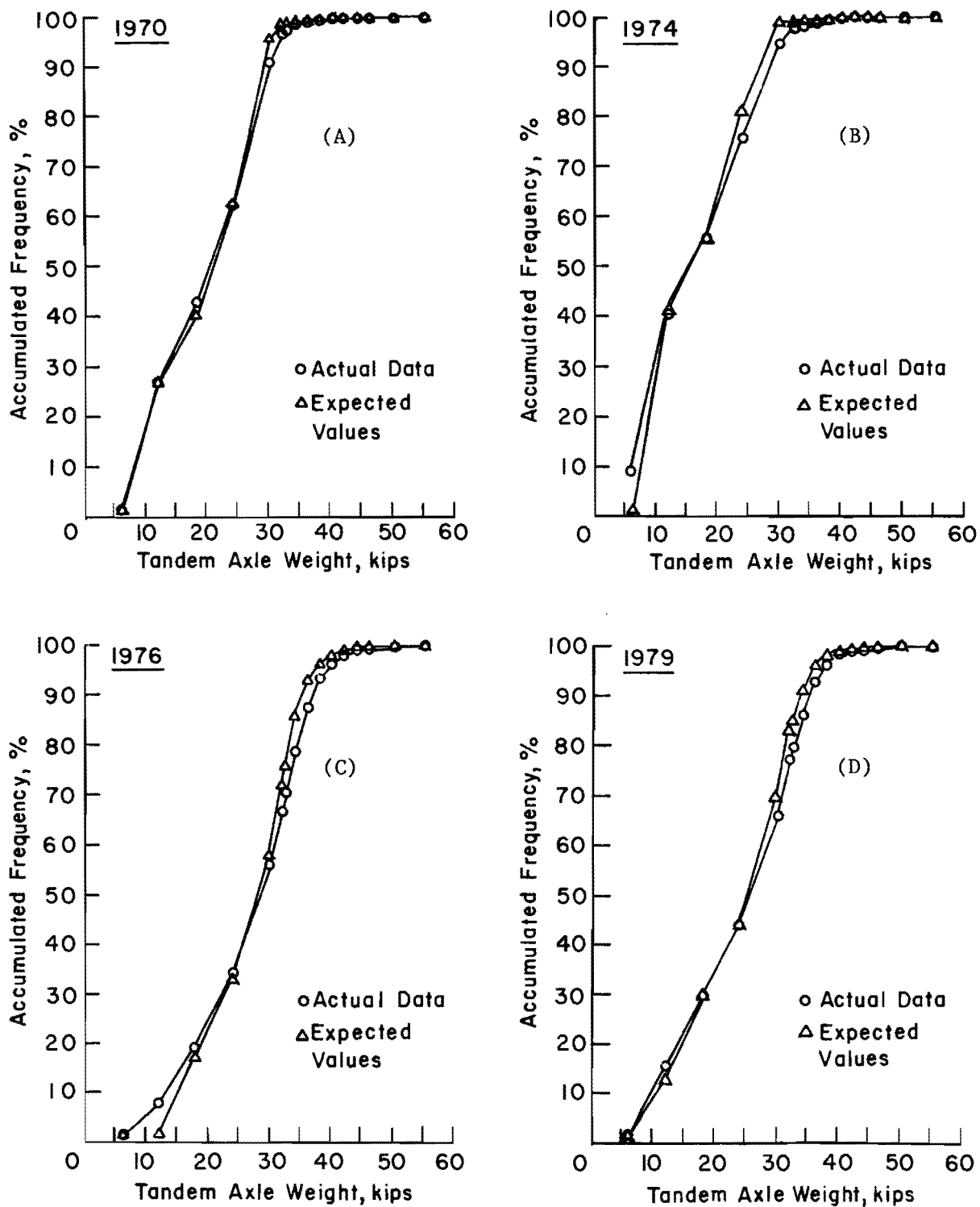


Fig 31. Comparison of actual and expected tandem axle distribution for 3-S2 on Texas Interstate rural highways.

TABLE 18. CHI-SQUARED VALUES TO SHOW THE GOODNESS-OF-FIT BETWEEN ACTUAL AND PREDICTED TANDEM AXLE WEIGHT DISTRIBUTION CURVES

<u>3A</u>	<u>Chi-squared value</u>
1970	20.68
1974	75.06
1976	19.58
1979	18.24

<u>3-S2</u>	<u>Chi-squared value</u>
1970	9.08
1974	33.85
1976	12.87
1979	10.35

chi-square values indicate that the predicted TAW distribution agrees with the actual TAW field data.

From the study of the relationship between gross vehicle weight and axle weight distributions, it can be concluded that the simple relationship of GVW, SAW, and TAW of a single truck can be applied to the weight distribution data. The relationships stated in Eqs 6-1 and 6-2 are valid for 3A and 3-S2 weight distributions, respectively. Thus, for a given year, if GVW and SAW distributions are available, it is possible to obtain the TAW distribution. This finding is essential to predictions of TAW distributions and 18-KESAL under the proposed truck weight limits.

PREDICTION FOR TANDEM AXLE WEIGHT DISTRIBUTION UNDER PROPOSED TRUCK WEIGHT LIMITS

From the extensive study of truck weight distribution patterns, it was observed that the change in axle weight as well as gross vehicle weight limits in 1975 did not change the distribution of steering axle weight. This statement is based on the analysis of steering axle weight distribution curves in Figs 16, 17, and 18. Because of practical and operational safety considerations, the steering axle weight distribution did not change even though the weight laws changed. Thus, for prediction purposes, it is acceptable to use the present steering axle weight distribution as the future steering axle weight distribution under the proposed weight laws. Along with this concept, it is possible to predict a tandem axle weight distribution for both 3A and 3-S2 with the application of the average GVW factor concept mentioned in the previous chapters. The procedures are shown in the flowchart in Fig 32 and discussed as follows:

- (1) With the previous stated methodology, use the average GVW factor methodology to obtain a GVW distribution curve for a proposed truck weight limit.
- (2) Obtain the SAW distribution for the truck type from the latest truck weight data in W-4 tables.
- (3) Read the GVW (i%) and SAW (i%) from the GVW and SAW distribution curves.
- (4) Use the appropriate equation for the truck type. For 3-S2,

$$TAW(i\%) = 0.5 \left[GVW(i\%) - SAW(i\%) \right]$$

and for 3A,

$$\text{TAW}(i\%) = \text{GVW}(i\%) - \text{SAW}(i\%)$$

5. From the TAW(i%) values, plot the distribution curve.

To illustrate the application of the procedure, an example using the 3-S2 Texas Interstate Highway is provided in the next section.

EXAMPLE OF PREDICTION OF TANDEM AXLE WEIGHT DISTRIBUTION

In order to determine the accuracy of the prediction, an illustration using prior data is developed. In this example, the prediction is made for the tandem axle weight distribution for the 3-S2 on Texas Interstate Highway in 1978. The purpose of this example is to illustrate the prediction of tandem axle weight under the proposed truck weight laws. The data available for prediction were composed of GVW and SAW distribution from 1959 through 1975. The 1978 GVW prediction, provided in the prior chapters, serves as the basis for predicting the TAW distribution. Tables 19 and 20 show the predicted GVW distribution for 1978 and the single axle weight distribution for 1974 respectively. These weight distribution data serve as inputs for the TAW prediction. The procedure is coded into a computer program. The listing of the program TAWEXP is included in Appendix 3. Figure 33 shows the cumulative percentage curves of the actual and predicted TAW distribution. A chi-squared test on the actual and predicted curves is shown in Table 21. Both the plotting and the chi-squared value indicate that the prediction is within acceptable tolerance.

CALCULATION OF EQUIVALENT 18-KIP SINGLE AXLE LOAD

To assess the impacts on pavement structures due to changes in legal weight limits, one has to compute the equivalent 18-kip single axle load applications for the present and proposed weight limits. The difference between the two load applications is the additional impact affected by changes in weight limits. The direct source of truck weight data used in the computation of the total number of 18-kip ESAL is the W-4 tables. Equivalent factors for both flexible and rigid pavements are provided in the W-4 tables. These factors, when multiplied by the number of axle loads within a given

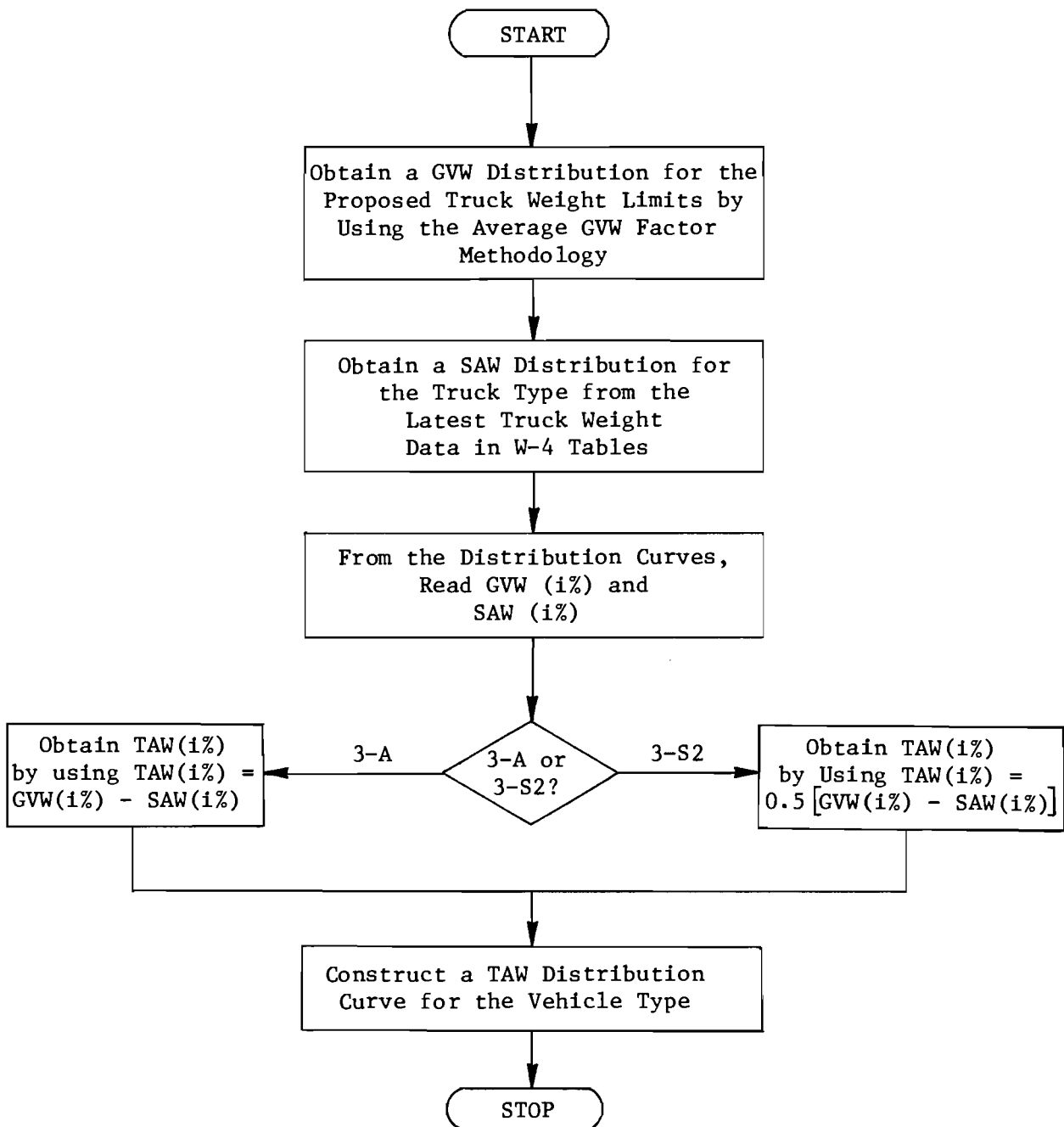


Fig 32. Flowchart for predicting tandem axle weight distribution for 3A and 3-S2.

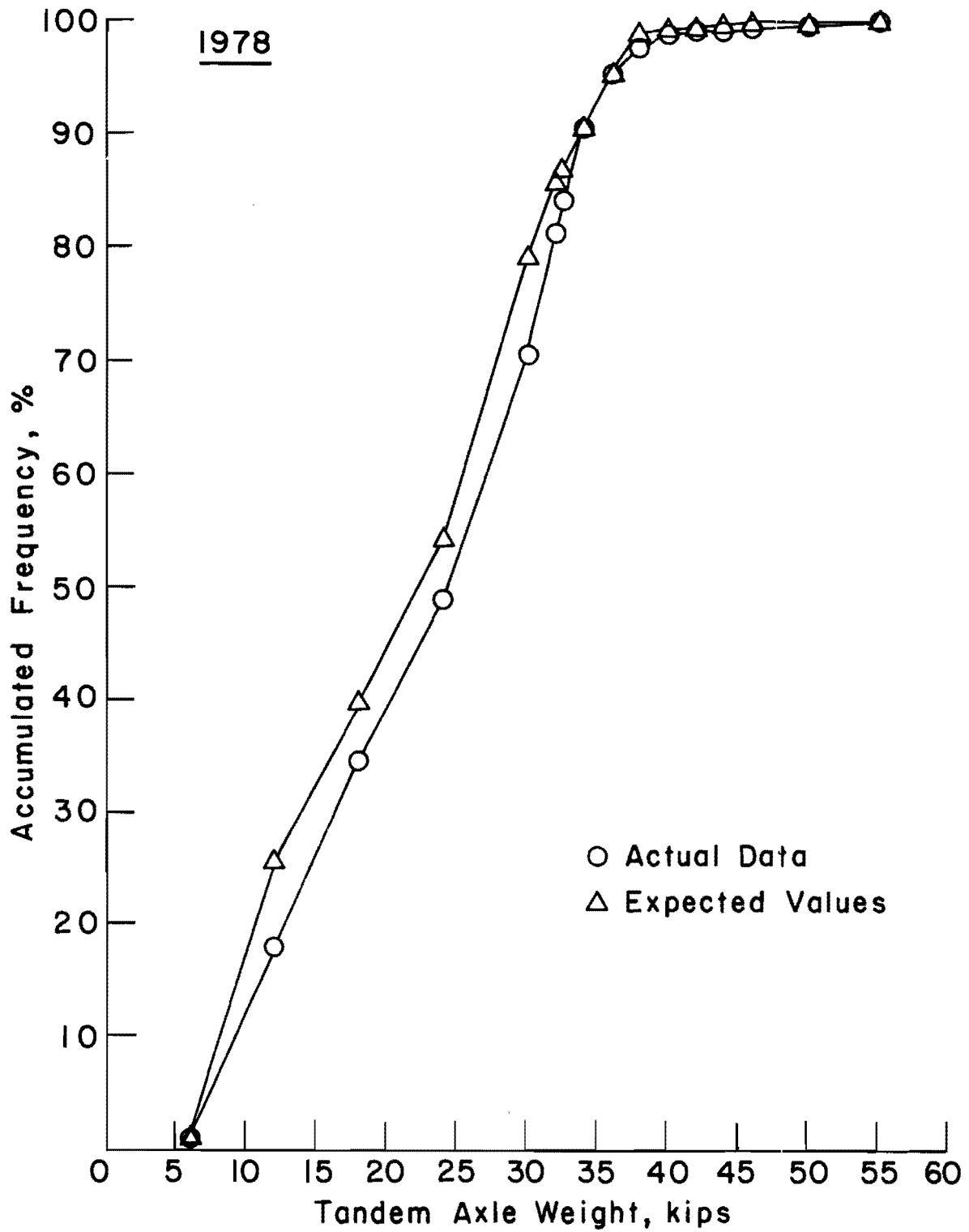


Fig 33. Comparison of actual and predicted tandem axle weight distributions for 3-S2 on Texas Interstate rural highways.

TABLE 19. PROJECTED GVW DISTRIBUTION FOR 1978, 3-S2,
TEXAS INTERSTATE RURAL HIGHWAYS (INPUT
DATA FOR PROJECTION OF TAW, 1978)

Weight Distribution (kips)	Percentage	Accumulated Percentage
4.0000	.1534	.1534
10.0000	.2301	.3835
13.5000	.1342	.5177
20.0000	.2492	.7669
22.0000	.0767	.8436
24.0000	.0767	.9203
26.0000	.0767	.9970
28.0000	4.1951	5.1920
30.0000	7.4323	12.6243
32.0000	7.7321	20.3565
34.0000	5.0849	25.4414
36.0000	2.8658	28.3071
38.0000	2.1443	30.4514
40.0000	2.3744	32.8258
45.0000	4.7136	37.5394
50.0000	4.6244	42.1638
55.0000	4.9262	47.0900
60.0000	5.3416	52.4316
65.0000	9.0353	61.4669
70.0000	9.4139	70.8808
72.0000	3.8445	74.7253
75.0000	5.5897	80.3150
80.0000	6.6054	86.9204
85.0000	5.1828	92.1033
90.0000	5.3254	97.4287
95.0000	1.6190	99.0476
100.0000	.3210	99.3687
105.0000	.3210	99.6897
110.0000	.3103	100.0000
115.0000	0	100.0000

TABLE 20. SINGLE AXLE WEIGHT DISTRIBUTION OF 3-S2 ON
TEXAS INTERSTATE RURAL HIGHWAYS (INPUT
DATA FOR PROJECTION OF TAW, 1978)

Data Obtained in 1974

A	B	C	D
End of SAW Interval (kips)	Sample Size	Percentage	Accumulated Percentage
3.	1.	.26	.26
7.	13.	3.32	3.57
8.	69.	17.60	21.17
12.	301.	76.79	97.96
16.	8.	2.04	100.00

Total Number of Trucks Weighed = 392

TABLE 21. PREDICTION OF 1978 TANDEM AXLE WEIGHT DISTRIBUTION BASED ON PROJECTED 1978 GVW AND ACTUAL 1974 SAW DISTRIBUTION DATA

Tandem Axle Weight (kips)	Actual Cumulated Percentage	Expected Cumulated Percentage
6.00	.95	.54
12.00	17.89	20.09
18.00	34.51	37.39
24.00	48.86	49.75
30.00	70.37	72.14
32.00	81.23	80.37
32.50	84.05	81.85
34.00	90.49	86.10
36.00	95.20	90.67
38.00	97.60	95.14
40.00	98.82	99.08
42.00	99.11	99.34
44.00	99.25	99.60
46.00	99.39	99.87
50.00	99.66	100.00
55.00	100.00	100.00

Chi-squared value = 14.5815

weight interval, give the number of 18-KESAL applications. The summation of the load applications throughout the whole span of weight intervals gives the total loading effect on the pavement by the sample trucks. Equivalent factors for other pavement conditions may be obtained by the equations or nomographs provided in the "AASHTO Interim Guide for Design of Pavement Structures," published by the American Association of State Highway and Transportation Officials (AASHTO) (Ref 1). An example illustrating the procedure for computing 18-KESAL is given in Table 22.

The equivalent 18-KESAL applications for the proposed weight limits can be computed by resorting to the shifted axle weight distribution curve. In the previous chapters, both the procedures and the example of shifting GVW and axle weight distribution curves have been presented. In this section, an example is used to illustrate the application of the shifting methodology in arriving at the 18-KESAL applications. The flowchart in Fig 34 summarizes the procedure.

For illustrative purposes, the predicted tandem axle weight distribution obtained earlier is used to compute the equivalent 18-kip single axle load. Both flexible and rigid pavement 18-KESAL for actual and predicted axle weight distributions are provided in Tables 23 and 24, respectively. In both rigid and flexible pavement, the differences between the actual and predicted 18-KESAL are within 6 percent.

COMMENT ON THE AXLE WEIGHT SHIFTING METHODOLOGY

The shifting procedure for GVW distribution depends on the GVW distribution data. Its accuracy is directly affected by the size and quality of the samples. The shifting for TAW distribution depends on both GVW and SAW distributions. Therefore, the accuracy of the prediction of future axle weight distributions is dependent upon the quality of the present axle weight distribution data and the sample size. An illustration of the importance of data to the procedure is reflected in Fig 30(b), where the number of both single axles and tandem axles available in the W-4 table was 14.

To remedy the deficiency in sample size, users may be able to combine data representing the same category. This may be significant for the steering

TABLE 22. EXAMPLE OF DETERMINATION OF EQUIVALENT
18-KIP (80-kN) SINGLE AXLE LOADS FROM
LOADOMETER STATION DATA (Ref 16)

Axle Load Groups, lb	Representative Axle Load, lb	Equiv Factor ¹	No. of Axles ²	Equiv 18-kip Single Axles
Single Axles				
Under 3,000	2,000	0.0003	512	0.2
3,000- 6,999	5,000	0.012	536	6.4
7,000- 7,999	7,500	0.0425	239	10.2
8,000-11,999	10,000	0.12	1,453	174.4
12,000-15,999	14,000	0.40	279	111.6
16,000-18,000	17,000	0.825	106	87.5
18,001-20,000	19,000	1.245	43	53.5
20,001-21,999	21,000	1.83	4	7.3
22,000-23,999	23,000	2.63	3	7.9
24,000 and over	-	-	0	-
			Subtotal	459.0
Tandem Axles				
Under 6,000	4,000	0.01	9	-
6,000-11,999	9,000	0.008	337	2.7
12,000-17,999	15,000	0.055	396	21.8
18,000-23,999	21,000	0.195	457	89.1
24,000-29,999	27,000	0.485	815	395.3
30,000-32,000	31,000	0.795	342	271.9
32,001-33,999	33,000	1.00	243	243.0
34,000-35,999	35,000	1.245	173	215.4
36,000-37,999	37,000	1.535	71	109.0
38,000-39,999	39,000	1.875	9	16.9
40,000-41,999	41,000	2.275	0	-
42,000-43,999	43,000	2.74	1	2.7
44,000 and over	-	-	0	-
			Subtotal	1,367.8
			Total	1,826.8
Total, all trucks = 3,146				

¹For $p_t = 2.5$ and $NS = 3.0$

²Loadometer station data for 3,146 trucks

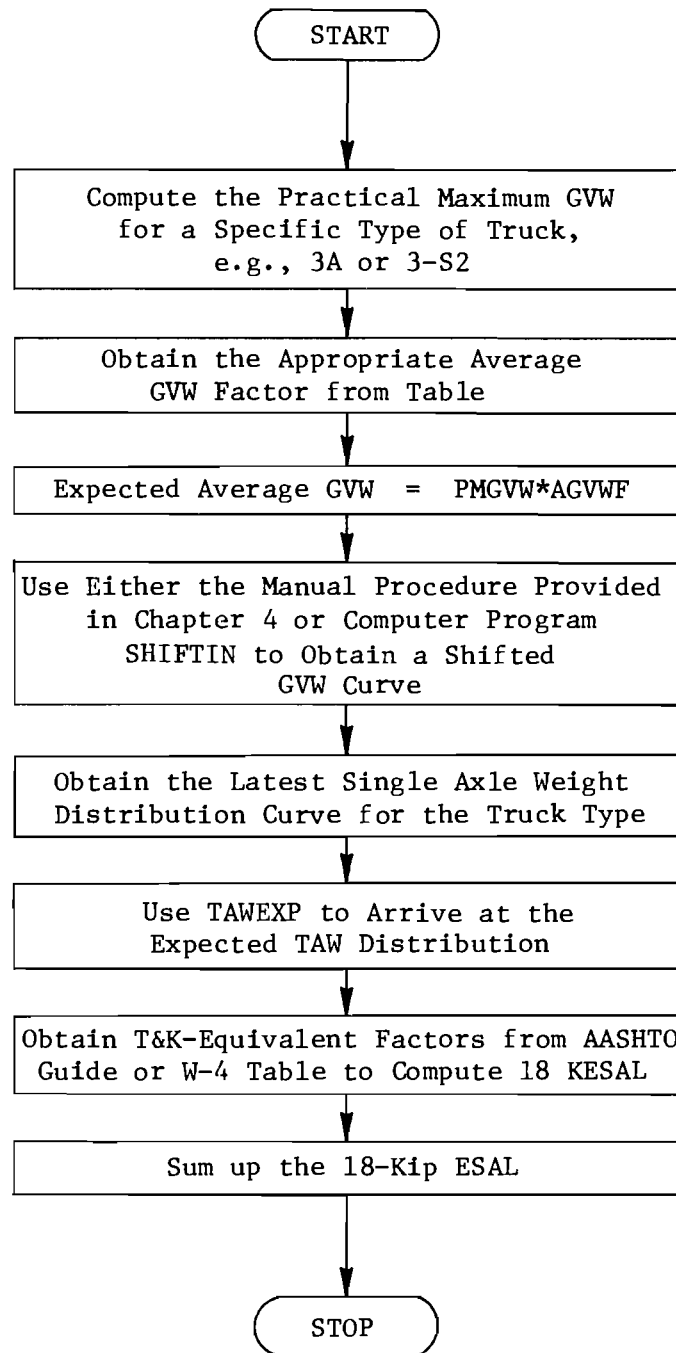


Fig 34. Shifting procedure and computation of 18-kip equivalent single axle load.

TABLE 23. COMPUTATION OF ACTUAL AND PREDICTED 18 KESAL FOR FLEXIBLE PAVEMENT (3-S2, TEXAS INTERSTATE HIGHWAYS)

Tandem Axle Weight Groups	Observed Sample	Predicted Sample	Flexible Pavement 18-K Equivalence Factor	Observed 18-KESAL Applications	Predicted 18-KESAL Applications
0- 5,999	1	25.6	0.010	0.01	0.3
6,000-11,999	848	927.5	0.010	8.48	9.3
12,000-17,999	790	820.7	0.044	34.76	36.1
18,000-23,999	676	586.4	0.1480	100.05	86.8
24,000-29,999	1019	1062.2	0.4260	434.09	452.5
30,000-32,000	519	390.4	0.7530	390.81	294.0
32,001-32,500	135	70.2	0.8850	119.48	62.1
32,501-33,999	312	201.6	1.0020	312.62	202.0
34,000-35,999	222	216.8	1.2300	273.06	266.7
36,000-37,999	116	212.1	1.5330	117.83	325.1
38,000-39,999	53	186.9	1.8850	99.91	352.3
40,000-41,999	32	12.3	2.2890	73.25	28.2
42,000-43,999	13	12.3	2.7490	35.74	33.8
44,000-45,999	4	12.8	3.2690	13.08	41.8
46,000-49,999	2	6.2	4.1700	8.34	25.9
50,000-55,000	2	0.0	5.100	10.20	0.0
	$\Sigma = 4744$	$\Sigma = 4744.0$		$\Sigma = 2092.00$	$\Sigma = 2217.0$

$$\Delta = \frac{2217 - 2092}{2092} = 5.98\%$$

TABLE 24. COMPUTATION OF ACTUAL AND PREDICTED 18 KESAL FOR RIGID PAVEMENT (3-S2, TEXAS INTERSTATE HIGHWAYS)

Tandem Axle Weight Groups	Observed Sample	Predicted Sample	Rigid Pavement 18-K Equivalence Factor	Observed 18-KESAL Applications	Predicted 18-KESAL Applications
0- 5,999	1	25.6	0.01	0.01	0.3
6,000-11,999	848	927.5	0.01	8.48	9.3
12,000-17,999	790	820.7	0.062	48.98	50.9
18,000-23,999	676	586.4	0.253	171.0	148.4
24,000-29,999	1019	1062.2	0.729	742.9	774.3
30,000-32,000	519	390.4	1.305	677.3	509.5
32,001-32,500	135	70.2	1.542	208.2	108.2
32,501-33,999	312	201.6	1.752	548.2	353.2
34,000-35,999	222	216.8	2.165	480.6	447.7
36,000-37,999	116	212.1	2.721	315.6	577.1
38,000-39,999	53	186.9	3.373	178.8	630.4
40,000-41-999	32	12.33	4.129	132.1	50.9
42,000-43,999	13	12.33	4.997	65.0	61.6
44,000-45,999	4	12.8	5.987	23.9	76.6
46,000-49,999	2	6.2	7.725	15.5	47.9
50,000-55,000	2	0.0	10.16	20.3	0.0
	$\Sigma = 4744$	$\Sigma = 4744.0$		$\Sigma = 3637.0$	$\Sigma = 3846.3$

$$\Delta = \frac{3846.3 - 3637.0}{3637} \times 100\% = 5.75\%$$

axle distribution of 3A and 3-S2. Since the SAW distribution curves did not shift throughout the years, the combination of data will surely improve the accuracy of prediction.

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CHAPTER 7. SUMMARY AND RECOMMENDATIONS

The objective of this research project was to develop a shifting methodology which could be used to predict precisely future GVW distributions and axle weight distributions and 18-kip equivalent single axle load application in response to legal weight limit changes. The methodology presented in previous chapters has proved that the objective of the study has been attained. No single methodology is perfect in all aspects, and the methodology discussed in this report is no exception. However, as far as precision is concerned, the shifting methodology is highly desirable. It was developed by building upon the contributions of earlier shifting methodologies, especially that provided by Walton and Yu's research conducted at the Center for Transportation Research

SUMMARY OF NEW CONCEPTS USED IN THE SHIFTING METHODOLOGY

While developing the shifting methodology, several new concepts were introduced to facilitate more precise predictions.

- (1) Extensive use of historical truck weight data in projecting future weight distribution—All available truck weight data were used in the analysis. Several computer programs were written to facilitate the analysis and statistical modeling. In the prediction of future truck weight distribution, two sets of the latest available weight distribution data were used. This practice can accurately capture the latest weight distribution trends in forecast future trends.
- (2) Extensive use of statistical methods in analyzing historical data—Statistical methods such as using mean and variance to predict a normal distribution curve are the theme of the shifting methodology. Statistical test methods such as the chi-squared method and student t-tests are used extensively in the procedure. Computer statistical packages such as SPSS and MINITAB were used in sorting and analysis of data.
- (3) Computer application in conducting the shifting procedure—Due to the large amount of historical data and a large number of required input parameters, computer application became a necessity in performing the shifting procedure. Computer programming has facilitated the

procedure by integrating analysis, statistical testing, regression modeling, and forecasting into one single package. It thus reduces the time required in step-by-step manual shifting procedures. Thus, unnecessary human error introduced in the procedure can be reduced to a minimum.

- (4) Concept of using a mean and a variance to predict future distribution—Both the means and variances for the weight distribution curves usually suggest specific trends over a period of time. These trends can be represented by regression models. By using these models, one may predict the two parameters for future truck weight distributions. With the suggested shifting procedure, one may obtain a future weight distribution curve with desirable precision.
- (5) Concept of using an average GVW factor for projection of average GVW under proposed limit—The average GVW factor is used to relate a known parameter to an unknown parameter such as the future maximum GVW to the future average GVW. From the proposed truck weight limits, one may derive the future maximum practical GVW for a certain type of truck. By multiplying the future maximum practical GVW with a given average GVW factor, one may obtain an average GVW for the truck type under the proposed weight limits. Once the future average GVW is obtained, one may project a future truck weight distribution by using the suggested shifting methodology.

ASSUMPTIONS MADE IN THE DEVELOPMENT OF THE SHIFTING METHODOLOGY

In deriving the shifting procedure, two assumptions were made:

- (1) The prediction of weight distribution does not take weight violation into account. In arriving at the average GVW for each type of vehicle, a maximum allowable GVW was input into the program so that any sample with weight greater than this value would not be included in the computation of average weight. Thus, the average GVW factors provided in previous chapters can be used only to predict future legal average GVW. However, if the percentage of truck weight violations is to be taken into consideration, one may adjust the average GVW factors accordingly.
- (2) Size effects were neglected in the analysis process. Vehicle operational characteristics are affected by both volume and demand constraints. Thus, changes in size limits will have definite effects on truck weight distributions. However, due to the complexity of the issue, size effects were neglected in the development of the procedure. Hence, it is difficult to quantify the impacts due to changes in size limits. It is the authors' belief that trucks subjected to volume constraint are a relatively low percentage of the total truck population. It is even less plausible that these types of trucks would affect truck weight distribution data significantly. Thus, to cope with changes in both size and weight limits, one may concentrate one's effort on analyzing the effects of weight limit changes.

RECOMMENDATIONS

Although the main data structure concentrated on the Texas interstate highway system, the shifting procedure can be used for other types of highway systems, and is applicable to other states. If facilities such as computer hardware and FORTRAN language compilers are available, the AGVWF shifting methodology is strongly recommended.

As mentioned earlier, in previous chapters, the size of a data base is vital to the prediction of future weight distribution trends. An insufficient data base will generally handicap the precision of any estimation.

A large data base is a prerequisite to a precise prediction. In recent years, many truck weighing stations in Texas had been closed due to insufficient operation funds. Obviously, a shut-down of a weighing station sacrifices a certain degree of precision in prediction. Consequently, this adverse effect will be reflected in the inefficient design of highway systems. Thus, for a long-term investment on the existing federal and state highway systems, it is strongly recommended that truck weighing activities should be intensified and improvements made in operating efficiency.

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APPENDIX 1

SOURCE PROGRAM OF "MEANWGT"—TO COMPUTE MEAN AND
VARIANCE OF TRUCK WEIGHT DISTRIBUTION DATA

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FILE: MEANWGT FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

      PROGRAM MEANWGT (INPUT,OUTPUT,TAPE5=INPUT,TAPE6=OUTPUT)
C
C PROGRAM SAVGWT CALCULATES THE AVGWT OF SUMB AND SUMAB WHERE
C SUMB IS THE SUMMATION OF B AND SUMAB IS THE SUMMATION OF A
C TIMES B
C
C
C VARIABLE DESCRIPTION:=
C AVGW1 -- AVGWT OF SUMB AND SUMAB
C SUMA  -- SUMMATION OF A
C SUMAB -- SUMMATION OF A TIMES B
C AB    -- A TIMES B
C A, B, GVW, AXLE ARE CONSISTENT WITH PREVIOUS PROGRAMS.
C
C
      DIMENSION A(30), B(30), GVW(2,30), AXLE(2,30), IYEAR,A2(30)
      DIMENSION IROAD(2),IVEH(3),ISTATE(2)
      REAL AVGW1, AB(30), SUMB, SUMAB, A1(30)
      INTEGER IN
      DATA (GVW(1,I), I=1,30)/4.0, 10.0, 13.5, 20.0, 22.0, 24.0,
+26.0, 28.0, 30.0, 32.0, 34.0,36.0, 38.0, 40.0, 45.0,50.0,55.0,
+60.0, 65.0, 70.0, 72.0, 75.0, 80.0, 85.0, 90.0, 95.0, 100.0,
+105.0, 110.0, 115.0 /
      DATA (GVW(2,I), I=1,26) /10.0, 12.0, 14.0, 16.0, 18.0, 20.0,
+22.0, 24.0, 26.0, 28.0, 30.0, 35.0, 40.0, 45.0, 50.0, 55.0,
+60.0, 65.0, 70.0, 75.0, 80.0, 85.0, 90.0, 95.0, 100.0, 105.0/
      DATA (AXLE(1,I), I=1,13) / 3.0, 7.0, 8.0, 12.0, 16.0, 18.0,
+18.0, 20.0, 22.0, 24.0, 26.0, 30.0, 35.0 /
      DATA (AXLE(2,I), I=1,16) / 6.0, 12.0, 18.0, 24.0, 30.0, 32.0,
+32.0, 34.0, 36.0, 38.0, 40.0, 42.0, 44.0, 46.0, 50.0, 55.0 /
C
C INPUT VALUES FROM DATA SET P1AVGWT UNTIL END OF FILE
C
C
      PRINT HEADING FOR OUTPUT
C
      CALL TITLE
C
      READ IN WEIGHT LIMITS BEFORE AND AFTER WEIGHT CHANGES
C
      READ (5,100) WGT1,WGT2
C
      1 CALL INIT( A,B,AB,SUMB,SUMAB,AVGW1,STDEV,SUMA2B,IN)
      READ(5,10,END=999) IYEAR,IROAD,IVEH,ISTATE,IN
10  FORMAT(14,X,2A10,3A10,2A10,I5)
      READ(5,11) IFLAG
11  FORMAT(I4)
      READ(5,100) ( B(I), I = 1, IN)
100  FORMAT(12F0.1)
C

```

FILE: MBRNMG1 FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

C  DETERMINE IFLAG. IF IFLAG IS 6000, USED DATA IN AXLE(1,I)
C  IF IFLAG IS 9000 AND IYEAR IS LESS THAN 1959 USE DATA IN GVW(2,I)
C  IF IFLAG IS 9000 AND IYEAR IS GREATER THAN 1959 USE GVW(1,I)
C  IF IFLAG IS 3000 USE DATA IN AXLE(2,I)
C
      UNHALF = IN / 2
      IF ( IFLAG .EQ. 3000 ) GO TO 3000
      IF ( IFLAG .EQ. 6000 ) GO TO 6000
      IF ((IFLAG.EQ.9000) .AND. (IYEAR.LE.1958)) GO TO 9002
      IF ((IFLAG.EQ.9000) .AND. (IYEAR.GT.1958)) GO TO 9001
C
C  TRANSFER VALUES FROM DATA DECLARED IN DATA ISTATEMENTS AND SUMS
C  ALL VALUES.
C
      3000 DO 12 I = 1, IN
            A(I) = AXLE(1,I)
            IF (IYEAR.LE.1975) WGTLM = WGT1
            IF (IYEAR.GT.1975) WGTLM = WGT2
      12  CONTINUE
C
C  CONTINUE PROCESSING UNTIL END OF FILE
C
      GO TO 1000
C
      6000 DO 13 I = 1, IN
            A(I) = AXLE(2,I)
            IF (IYEAR.LE.1975) WGTLM = WGT1
            IF (IYEAR.GT.1975) WGTLM = WGT2
      13  CONTINUE
C
C  CONTINUE PROCESSING UNTIL END OF INPUT DATA
C
      GO TO 1000
C
      9001 DO 14 I = 1, IN
            A(I) = GVW(1,I)
            IF (IYEAR.LE.1975) WGTLM = WGT1
            IF (IYEAR.GT.1975) WGTLM = WGT2
      14  CONTINUE
C
C  CONTINUE PROCESSING
C
      GO TO 1000
C
      9002 DO 15 I = 1, IN
            A(I) = GVW(2,I)
            IF (IYEAR.LE.1975) WGTLM = WGT1
            IF (IYEAR.GT.1975) WGTLM = WGT2
      15  CONTINUE
C
C
C  CONTINUE PROCESSING
C
      GO TO 1000
C

```

FILE: HEANWGF FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

C THIS PART OF THE PROGRAM CALCULATES MEANS AND STD. DEV.
C
C
1000 AMIDPT = A (NHALF)
SUMAB1 = 0.0
DO 10 I=1, IN
IF (I.EQ.1) A1(1) = A(1) / 2.0
IF (I.NE.1) A1(I) = (A(I-1) + A(I)) / 2.0
A2(I) = A1(I) - AMIDPT
IF (A1(I).GT.WGLIM) GO TO 16
SUMB = SUMB + B(I)
SUMAB1 = SUMAB1 + A1(I) * B(I)
SUMAB = SUMAB + A2(I) * B(I)
SUMA2B = SUMA2B + A2(I) * A2(I) * B(I)
10 CONTINUE
AVGWT = SUMAB1 / SUMB
STDEV = SQRT (( SUMA2B - SUMAB * SUMAB / SUMB)/(SUMB-1.))
C PRINT THE OUTPUTS AND THEN PROCEED TILL END-OF-FILE
C
CALL OUT ( A, B, AB, AVGWT, STDEV, IN, IYEAR, IROAD,
+ IVEH, ISTATE, SUMB, SUMAB)
GO TO 1
999 STOP
END

SUBROUTINE INIT ( A, B, AB, SUMB, SUMAB, AVGWT, STDEV, SUMA2B, IN)
REAL A(1), B(1), AB(1), SUMB, SUMAB, AVGWT
DIMENSION IROAD(2), IVEH(3), ISTATE(2)
C THIS SUBROUTINE INITILIZE ALL VARIABLES USED IN PROGRAM.
C ALL VARIABLES ARE SET TO 0.
C
DO 1 I = 1, 30
A(I) = 0.0
B(I) = 0.0
AB(I) = 0.0
1 CONTINUE
C
IN = 0
SUMB = 0
SUMAB = 0
SUMA2B = 0.0
STDEV = 0.0
AVGWT = 0
RETURN
END
C
C
C
C
C THIS SUBROUTINE PRINTS THE HEADING FOR OUTPUT
C
SUBROUTINE TITLE
*TITLE (0,10)
10 FORMAT(*1*,//5X,*VEHICLE TYPE*,25X,*HWY SYSTEM*,12X,*STATE*,

```

FILE: DEANWGI FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```
*21X,*YEAR*,9X,*AVERAGE*,6X,*STD DEV.*//)
RETURN
END
```

```
SUBROUTINE OUT ( A, B, AB, AVGWT,STDEV, IN, IYEAR, IROAD,
+ IVEH, ISTATE, SUMB, SUMAB)
REAL A(1),B(1),AB(1),SUMB,SUMAB,AVGWT
INTEGER IN, IYEAR, IVEH(3), ISTATE(2), IROAD(2)
```

```
C
C
C
```

```
DO NOT PRINT ALL PARAMETERS IN THE SUBROUTINE.
```

```
11 WRITE(6,11) IVEH,IROAD,ISTATE,IYEAR,AVGWT,STDEV
   FORMAT(5X,3A10,5X,2A10,5X,2A10,5X,I4,5X,F10.4,F10.4)
RETURN
END
```

APPENDIX 2

SOURCE PROGRAM OF "SHIFTIN"—
A COMPUTERIZED SHIFTING PROCEDURE

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PROGRAM SHIFLIN (INPUT,OUTPUT,TTY,PLOT,TAPES=INPUT,
 + TAPES6=OUTPUT,TAPES7=TTY) SH100010

THIS PROGRAM DOES THE TRUCK WEIGHT DISTRIBUTION SHIFTING
 PROCEDURE. THE SHIFTING METHODOLOGY WAS DEVELOPED BY
 PAUL NG OF CENTER FOR TRANSPORTATION RESEARCH, UNDER THE
 SUPERVISION OF DR.C.MICHAEL WALTON OF CIVIL ENGINEERING
 DEPARTMENT, UNIVERSITY OF TEXAS AT AUSTIN. SH100020
 SH100030
 SH100040
 SH100050
 SH100060
 SH100070
 SH100080
 SH100090
 SH100100
 SH100110

FUNCTION OF THE PROGRAM: SH100120

THE PROGRAM READS IN TRUCK WEIGHT DISTRIBUTIONS FROM THE
 W-4 AND W-5 TABLE SH100130
 SH100140
 SH100150
 TABLES ARE THE SUMMARIES OF TRUCK WEIGHT DATA, WHICH WAS
 COLLECTED BY WEIGHING STATIONS OR PORTABLE WEIGHT-IN-MOTION
 WEIGHING MACHINES. BY REGRESSION ANALYSIS, FUTURE AVERAGE
 TRUCK WEIGHT WILL BE COMPUTED AND USED AS A PART OF THE
 INPUT DATA. BY VARIOUS REGRESSION METHODS, OPTIMIZATION,
 COMPUTATION, AND DATA PROCESSING, PROGRAM /SHIFLIN/ WILL
 PROJECT THE TRUCK WEIGHT DISTRIBUTION DATA FOR THE PRE-
 DICTED YEAR(S). THE OUTPUT WILL BE PRESENTED BOTH IN
 TABLES AND CHARTS, AND GRAPHS. SH100160
 SH100170
 SH100180
 SH100190
 SH100200
 SH100210
 SH100220
 SH100230
 SH100240
 SH100250

THE FOLLOWING OUTPUTS WILL BE FURNISHED BY /SHIFLIN/
 SH100260
 SH100270

- *1. RATIO CURVE OF THE INPUT TRUCK WEIGHT DATA SH100280
- *2. REGRESSION OF LINEARIZED EQUATION FOR FITTING RATIO CURVE SH100290
- *3. CHI-SQUARE TEST OF ITEM 2 SH100300
4. OPTIMIZATION OF DIFFERENCE IN MEANS IN THE GUESSING PROCEDURES SH100310
5. NEW DISTRIBUTION CURVE (IN TERMS OF PERCENTAGE) SH100320
6. NEW DISTRIBUTION CURVE (IN TERMS OF WEIGHT DISTRIBUTION GROUPS) SH100330
 SH100340

IN ADDITION TO THE HARDCOPY PRINT-OUT, /SHIFLIN/ ALSO PROVIDES
 GRAPHICAL OUTPUTS: SH100350
 SH100360

- *1. ACCUMULATED DISTRIBUTION CURVE SH100370
- *2. HISTOGRAM SHOWING THE WEIGHT DISTRIBUTION SH100380
 SH100390
 SH100400

ITEMS MARKED WITH AN ASTERISK ARE THE OPTIONAL OUTPUTS. SH100410
 SH100420

PROGRAM /SHIFLIN/ HAS BEEN TESTED WITH TEXAS TRUCK WEIGHT
 DATA. THE RESULTS WERE THEN COMPARED WITH 1980 TRUCK
 WEIGHT DATA, WHICH WAS NOT AVAILABLE AT THE TIME WHEN THE
 PROGRAM WAS DEVELOPED. THE GOODNESS OF FIT OF THE PROJECT-
 ION WAS HIGHLY DESIRABLE. SH100430
 SH100440
 SH100450
 SH100460
 SH100470
 SH100480
 SH100490
 SH100500
 SH100510
 SH100520
 SH100530
 SH100540
 SH100550

 DESCRIPTION OF THE SHIFTING METHODOLOGY

FILE: SHIFTLN FORTRAN 3

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

C      /SHIFTLN/ ALLOWS TWO CATEGORIES OF INPUT DATA, NAMELY,          SH100560
C      1. RAW DATA --- (RAWDATA), AND                                  SH100570
C      2. COEFFICIENTS --- (COEFFIC)                                    SH100580
C      SH100590
C      SH100600
C      THE FIRST OPTION, RAWDATA, PERFORMS ACCORDING TO THESE STEPS:  SH100610
C      SH100620
C      STEP I.                                                           SH100630
C      SUBROUTINE /INDATA/ READS IN TWO YEARS OF TYPICAL TRUCK        SH100640
C      WEIGHT DISTRIBUTION DATA. THE DATA MAY HAVE ONE OF THE        SH100650
C      FOLLOWING NATURES:                                               SH100660
C      SH100670
C      1. GROSS VEHICLE WEIGHT,                                         SH100680
C      2. SINGLE AXLE WEIGHT, AND                                       SH100690
C      3. TANDEM AXLE WEIGHT.                                           SH100700
C      OTHER INFORMATION REQUIRED FOR THE EXECUTION OF /SHIFTLN/        SH100710
C      IS AS FOLLOWING:                                                 SH100720
C      1. OPTIONS RELATED TO THE PRINTING                               SH100730
C      2. AVERAGE TRUCK WEIGHT FOR THE PROJECTED YEAR                  SH100740
C      3. OPTIONS RELATED TO PLOTTING                                   SH100750
C      SH100760
C      STEP II.                                                         SH100770
C      SH100780
C      SUBROUTINE /RATIO/ ANALYZES DATA. TRUCK WEIGHTS AT 5%         SH100790
C      FREQUENCY INTERVALS WILL BE OBTAINED. RATIOS OF THE            SH100800
C      WEIGHTS OF TWO GIVEN YEARS WILL THEN BE COMPUTED.              SH100810
C      THE OUTPUT ITEMS ARE :                                          SH100820
C      1. ACCUMULATED FREQUENCIES AT 5 % INTERVALS                     SH100830
C      2. LATEST AVAILABLE TRUCK WEIGHT DATA                           SH100840
C      3. BASE YEAR TRUCK WEIGHT DATA                                  SH100850
C      4. RATIO BETWEEN THE LAST TWO ITEMS                             SH100860
C      SH100870
C      STEP III                                                         SH100880
C      SH100890
C      SUBROUTINE /REGRESS/ USES THE LAST TWO OUTPUT ITEMS            SH100900
C      ISSUED FROM STEP II AS THE PARAMETERS FOR REGRESSION           SH100910
C      ANALYSIS. THE LINEAR EQUATION USED IS:                          SH100920
C      SH100930
C      LN(RATIO) = LN (A) + B * LN (WGT2) + C * WGT2                  SH100940
C      SH100950
C      NORMAL FORM OF THE ABOVE EQUATION IS GIVEN AS:                  SH100960
C      SH100970
C      SH100980
C      RATIO = A * (WGT2 ** B) * EXP ( C * WGT2)                       SH100990
C      SH101000
C      THE METHOD OF LEAST SQUARES IS USED IN THE LINEAR               SH101010
C      REGRESSION ANALYSIS.                                           SH101020
C      THE OUTPUT OF THIS STEP IS THE COEFFICIENTS A, B, AND C        SH101030
C      FOR THE TWO GIVEN YEARS, I.E. THE LATEST AVAILABLE YEAR        SH101040
C      AND THE BASE YEAR. THESE COEFFICIENTS WILL BE TRANSFERRED      SH101050
C      TO /OPTIMSL/, THE OPTIMIZATION PROGRAM, TO PROVIDE SOME         SH101060
C      GENERAL GUIDELINES FOR SEARCHING NEW COEFFICIENTS.             SH101070
C      SH101080
C      SH101090
C      THE FOLLOWING STEPS ARE COMMON TO *RAWDATA* AND *COEFFIC*       SH101100

```

FILE: SHIFTLN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

C
C
C      STEP IV
C
C      TRANSFER BASE YEAR TRUCK WEIGHT DATA, WGT2'S, TO THE
C      OPTIMIZATION ROUTINE /OPTMSE/ FOR SEARCHING THE NEW
C      COEFFICIENTS A, B, AND C.
C
C      STEP V
C
C      SET UPPER AND LOWER LIMITS FOR THE NEW A, B, AND C; THEN
C      TRANSFER THE LIMITS TO /OPTMSE/.
C      STEP VI
C
C      /OPTMSE/, WITH THE LIMITS AND GENERAL GUIDELINES, STARTS TO
C      SEARCH FOR THE NEW COEFFICIENTS.
C      *****
C
COMMON /FLAGS/ IFLAG(10),NFLAGS
COMMON /CONSIS/ A,B,C,ANEW,BNEW,CNEW
COMMON /LIMITS/ AHIGH,ALOW,BHIGH,BLOW,CHIGH,CLOW
COMMON /VARIES/ WGTBASE(20),RAT(101)
COMMON /INDEX/ LVEN(3),IROAD(2),ISTATE(2),IFLG
COMMON /WGTLIA/ SUMWGT,EXPWGT,SUMVAR,EXPVAR,CPT
INTEGER COMMAND(2)
PROGRAM STARTS AT THIS POINT
READ INPUT DATA

CALL INPROG (COMMAND)
NSTOP = 0

DETERMINE WHICH STEP TO TAKE.....

IF (COMMAND.EQ.'RAWDATA')
+CALL RAWDATA
1 IF (COMMAND.EQ.'COEFFIC')
+CALL COEFFS

CHECK THE GOODNESS-OF-FIT OF EQUATION BY USING /CHISQ/ ROUTINE

CALL XSQUARE(NSTOP)

THE NULL HYPOTHESIS:
H0: A, B, AND C ARE THE TRUE COEFFICIENTS FOR EQUATION
Y = A * (X ** B) * EXP (C * X)

IF CALCULATED CHI-SQ VALUE IS TOO HIGH, REJECT THE NULL
HYPOTHESIS AND STOP PROCESSING

IF (NSTOP.NE.0) WRITE (7,601)
IF (NSTOP.NE.0) WRITE (6,601)
601 FORMAT (*1*,,//10X,*A,B,AND C ARE NOT THE TRUE COEFFICIENTS*/
+10X,*OF THE EQUATION:*
+10X,35H Y = A * ( X ** B ) * EXP ( C * X )

```

FILE: SHIFLIN FORTRAN 8

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

      +,)
      IF (NSIOP.NE.0) STOP
      IF NULL HYPOTHESIS CANNOT BE REJECTED, PROCEED ON AND
      OBTAIN THE COEFFICIENTS FOR THE FUTURE YEAR
      CALL OPTMSE
      USE SUBROUTINE /CALWGT/ TO FIND THE EXPECTED VALUES
      CALL CALWGT
      FIND THE EXPECTED S-CURVE AND THEN PLOT THE CURVE
      CALL SCURVE
      I PLOT = 10
      DO 10 I=1,NFLAGS
      IF (IFLAG(I).EQ. 'NOPLO' ) I PLOT = 0
      CONTINUE
      IF ( I PLOT.NE. 0) CALL SPLOT
      CALL CONCLUD
      THIS = 10
      DO 11 I=1,NFLAGS
      IF (IFLAG(I).EQ. 'NOHIS') IHIS=0
      CONTINUE
      IF (IHIS.NE.0) CALL HISTOGRM
      WRITE(7,002)
002  FORMAT (///10X, *TRUCK WEIGHT SHIFTING METHODOLOGY*
      //10X, *PROGRAM /SHIFLIN/ VERSION 1*
      //10X, *AUGUST 1, 1981*
      //10X, *CIVIL ENGINEERING DEPARTMENT*
      //10X, *THE UNIVERSITY OF TEXAS AT AUSTIN*//)
999  STOP
      END
      *****
      A SUBROUTINE STARTS AT THIS POINT
      *****
      SUBROUTINE RAWDATA
      COMMON /CONSIS/ A, B, C, ANEW, BNEW, CNEW
      COMMON /LIMITS/ AHIGH, ALOW, BHIGH, BLOW, CHIGH, CLOW
      COMMON /TKWGT/ TKWGT(2, 101), WGT PROJ(101)

```

```

SH101600
SH101670
SH101680
SH101690
SH101700
SH101710
SH101720
SH101730
SH101740
SH101750
SH101760
SH101770
SH101780
SH101790
SH101800
SH101810
SH101820
SH101830
SH101840
SH101850
SH101860
SH101870
SH101880
SH101890
SH101900
SH101910
SH101920
SH101930
SH101940
SH101950
SH101960
SH101970
SH101980
SH101990
SH102000
SH102010
SH102020
SH102030
SH102040
SH102050
SH102060
SH102070
SH102080
SH102090
SH102100
SH102110
SH102120
SH102130
SH102140
SH102150
SH102160
SH102170
SH102180
SH102190
SH102200

```


FILE: SHIFPIN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

C
COMMON /CONSIS/ A,B,C,ANEW,BNEW,CNEW
COMMON /VARIES/ WGTBASE(20),RAT(101)
COMMON /FLAGS/ IFLAG(10),NFLAGS
COMMON /INDEX/ IVEH(3),IROAD(2),ISTATE(2),IFLG
COMMON /LIMITS/ ANIGH,ALOW,BHIGH,BLOW,CHIGH,CLOW
DIMENSION IFMT(5)
C
C
C READ IN COEFFICIENTS A,B,C
READ (5,100) A,B,C
100 FORMAT(5F10.4)
C READ IN LABELS OF DATA-- E.G. TRUCK TYPE, HWY SYS, AND STATE.
C
C READ 101,IYEAR,IROAD,IVEH,ISTATE,NA
101 FORMAT(I4,X,2A10,3A10,2A10,I5)
C READ IN THE READING FORMAT OF INPUT DATA
READ (5,102) IFMT
102 FORMAT(8A10)
C READ IN TRUCK DISTRIBUTION GROUPS
READ(5,IFMT) (WGTBASE(I),I = 1,NA)
C SET UP LIMITS FOR A,B,C, AND THEN RETURN TO /SHIFPIN/
C
C
C ANIGH = 2.0 * A
C ALOW = 0.5 * A
C BHIGH = 2.0 * B
C BLOW = 0.5 * B
C CHIGH = 2.0 * C
C CLOW = 0.5 * C
C
C
C RETURN
C
C
C
C *****
C A SUBROUTINE STARTS AT THIS POINT
C *****
C
C
C
C *****
C SUBROUTINE RATIO
C
C THIS PROGRAM IS MODIFIED TO HANDLE THE RATIOS IN A
C FINER MANNER. THAT IS, INSTEAD OF USING 5 PERCENT
C INTERVALS, IT USES 1 PERCENT INTERVAL. THE REASON
C OF USING 1 PERCENT INTERVAL IS TO OBTAIN A MORE
C PRECISE REGRESSION ANALYSIS AND A BETTER SHAPE OF
C ACCUMULATED FREQUENCY CURVES. DATA:JUNE 18 1981.
C
C
C

```

```

SH102760
SH102770
SH102780
SH102790
SH102800
SH102810
SH102820
SH102830
SH102840
SH102850
SH102860
SH102870
SH102880
SH102890
SH102900
SH102910
SH102920
SH102930
SH102940
SH102950
SH102960
SH102970
SH102980
SH102990
SH103000
SH103010
SH103020
SH103030
SH103040
SH103050
SH103060
SH103070
SH103080
SH103090
SH103100
SH103110
SH103120
SH103130
SH103140
SH103150
SH103160
SH103170
SH103180
SH103190
SH103200
SH103210
SH103220
SH103230
SH103240
SH103250
SH103260
SH103270
SH103280
SH103290
SH103300

```


FILE: SHIFIN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

020 DO 014 I=1,20 SHI03860
014 WDIST(I) = GVW(2,I) SHI03870
      NGROUP = 26 SHI03880
      DO 02, (SAMSIZ(J),J=1,N) SHI03890
        ILIGHT(K)=0 SHI03900
        DO 00 I=1,N SHI03910
          IF(ILIGHT(K).NE.0) GO TO 65 SHI03920
          IF(SAMSIZ(I).GT.0.0) ILIGHT(K)=I SHI03930
05 IF(SAMSIZ(I).GT.0.0) IHEAVY(K)=I SHI03940
06 CONTINUE SHI03950
        SUMB=0.0 SHI03960
        DO 10 I=1,N SHI03970
09 SUMB=SUMB + SAMSIZ(I) SHI03980
        DO 11 I=1,N SHI03990
          I(1) = SAMSIZ(I)/SUMB * 100.0 SHI04000
11 CONTINUE SHI04010
          YSUM(1) = Y(1) SHI04020
          DO 12 I=2,N SHI04030
12 YSUM(I) = YSUM(I-1) + Y(I) SHI04040
          A=1 SHI04050
          CHECK = 0.00000001 SHI04060
          I=1 SHI04070
20 IF(.E.Q.2) CHECK=CHECK - 0.00000001 SHI04080
          IF(CHECK.GT.100.1) GO TO 999 SHI04090
          IF(.E.Q.N) GO TO 41 SHI04100
          IF(YSUM(I) - CHECK) 40,41,42 SHI04110
40 I=I+1 SHI04120
          GO TO 20 SHI04130
41 VALUE(K,M) = WDIST(I) SHI04140
          A=M+1 SHI04150
          CHECK = CHECK + 1.0 SHI04160
          GO TO 20 SHI04170
42 YSUM1=0.0 SHI04180
          IF(.E.Q.1) YSUM1=YSUM(I-1) SHI04190
          IF(YSUM(I)-YSUM1) 20,20,43 SHI04200
43 AIM1=0.0 SHI04210
          IF(.E.Q.1) AIM1= WDIST(I-1) SHI04220
          VALUE(K,M) =AIM1 + (CHECK - YSUM1)/ SHI04230
          (YSUM(I) - YSUM1) * (WDIST(I) - AIM1) SHI04240
          A=M+1 SHI04250
          CHECK =CHECK + 1.0 SHI04260
          GO TO 20 SHI04270
999 CONTINUE SHI04280
        DO 30 I=2,101 SHI04290
30 RAT(I) = VALUE(1,I)/VALUE(2,I) SHI04300
          SHI04310
          IRETURN = 0 SHI04320
          DO 32 I=1,NFLAGS SHI04330
            IF(.E.Q.(I).EQ.'NORAT') IRETURN = 999 SHI04340
32 CONTINUE SHI04350
            IF(IRRETURN.NE.0) GO TO 9999 SHI04360
            PRINT 499 SHI04370
499 FORMAT(*1*,//10X,*MULTIPLIERS OF THE INPUT TRUCK * SHI04380
          +*WEIGHT DATA*//,10.,42(==)/) SHI04390
            PRINT 500,IVER SHI04400

```


FILE: SHFTLN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

PRINT 500,IROAD SH104410
PRINT 500,ISTATE SH104420
500 FORMAT(15X,8A10) SH104430
DO 35 K=1,2 SH104440
IF (IYEAR(K).GT.4000) PRINT 510,K,I1(K),I2(K) SH104450
IF (IYEAR(K).LT.2000) PRINT 515,K,IYEAR(K) SH104460
35 CONTINUE SH104470
IF (IFLG.EQ.9000) PRINT 520 SH104480
520 FORMAT(/,17X,*ACCUMULATED*,6X,*YEAR(1)*,7X,*YEAR(2)*, SH104490
+X,*YEAR(1)/YEAR(2)*/,17X,*FREQUENCY*,8X,*GVW (KIPS)*, SH104500
+X,*GVW (KIPS)*// SH104510
IF (IFLG.EQ.3000) PRINT 518 SH104520
518 FORMAT(/,17X,*ACCUMULATED*,6X,*YEAR(1)*,7X,*YEAR(2)*, SH104530
+X,*YEAR(1)/YEAR(2)*/,17X,*FREQUENCY*,8X,*SAW (KIPS)*, SH104540
+X,*SAW (KIPS)*// SH104550
IF (IFLG.EQ.0000) PRINT 519 SH104560
519 FORMAT(/,17X,*ACCUMULATED*,6X,*YEAR(1)*,7X,*YEAR(2)*, SH104570
+X,*YEAR(1)/YEAR(2)*/,17X,*FREQUENCY*,8X,*TAW (KIPS)*, SH104580
+X,*TAW (KIPS)*// SH104590
ACFREQ(1) = 0.0 SH104600
PRINT 400,ACFREQ(1),VALUE(1,1),VALUE(2,1) SH104610
DO 50 I=1,20 SH104620
ACFREQ(I) = ACFREQ(I-1) + 5.0 SH104630
IS = 1 + I * 5 SH104640
PRINT 400,ACFREQ(I),VALUE(1,IS),VALUE(2,IS),RAT(IS) SH104650
50 CONTINUE SH104660
IL1=ILIGHT(1) SH104670
IL2=ILIGHT(2) SH104680
IH1=IHEAVY(1) SH104690
IH2=IHEAVY(2) SH104700
A11=0.0 SH104710
IF (IL1.NE.1) A11=WDIST(IL1-1) SH104720
A21=0.0 SH104730
IF (IL2.NE.1) A21=WDIST(IL2-1) SH104740
PRINT 522,A11,WDIST(IL1),A21,WDIST(IL2) SH104750
PRINT 521,WDIST(IH1-1),WDIST(IH1),WDIST(IH2-1),WDIST(IH2) SH104760
400 FORMAT(9X,3F15.4,6X,F8.2) SH104770
51 FORMAT(I4,X,2A10,3A10,2A10,I5) SH104780
52 FORMAT(12F6.1) SH104790
510 FORMAT(/15X,*YEAR(*,I2,*) = *,I4,* - *,I4) SH104800
515 FORMAT(/15X,*YEAR(*,I2,*) = *,I4) SH104810
522 FORMAT(/9X,*THE LIGHTEST TRUCK*/9X SH104820
+,*RECORDED IS IN THE */9X, SH104830
+*DISTRIBUTION GROUP: *,F5.1,*--*,F5.1, SH104840
+3X,F5.1,*--*,F5.1/) SH104850
521 FORMAT(/9X,*THE HEAVIEST TRUCK*/9X, SH104860
+,*RECORDED IS IN THE */9X, SH104870
+*DISTRIBUTION GROUP: *,F5.1,*--*,F5.1, SH104880
+3X,F5.1,*--*,F5.1) SH104890
9999 RETURN SH104900
END SH104910
C SH104920
C***** SH104930
C SH104940
C a SUBROUTINE STARTS AT THIS POINT SH104950

```

FILE: SHIFIN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

C
C*****
C
C      SUBROUTINE REGRESS
C
C      THIS SUBROUTINE DOES THE REGRESSION ANALYSIS FOR THE
C      MATHEMATICAL MODEL. THE METHOD OF LEAST SQUARES IS
C      COMMON /VARIES/ X(20),Y(101)
C      COMMON /CONSTS/ A,B,C,ANEW,BNEW,CNEW
C      EMPLOYED IN REGRESSING LINEAR EQUATIONS
C
C      DIMENSION XREG(20),YREG(20),TRANX(20)
C      +,TRANX(20),ANAF(3,3),BVEC(3),CVEC(3),AINV(3,3)
C      COMMON/FLAGS/ IFLAG(10),NFLAGS
C
C      SELECT DATA FOR ANALYSIS, FIND THE MEAN AND STANDARD DEVIATION
C      OF VARIABLES --- SET UP CRITERIA TO ACCEPT OR REJECT
C
C      SUMY = 0.0
C      SELECT THE MIDDLE PART OF DISTRIBUTION AND USE DATA IN THE
C      REGION TO COMPUTE MEAN AND S.D.
C
C      DO 10 I=5,19
C      IS = 1 + I * 5
C      XREG(I) = X(IS)
C      YREG(I) = Y(IS)
C      SUMY = SUMY + YREG(I)
10  CONTINUE
C      COMPUTE MEAN
C      YBAR = SUMY / 15.0
C      COMPUTE STANDARD DEVIATION OF Y'S
C
C      YSQSUM = 0.0
C      DO 11 I= 5,19
C      YSQSUM = YSQSUM + (YREG(I) - YBAR) ** 2.0
11  CONTINUE
C      SD = SQRT (YSQSUM / 14.00)
C      SET UP LIMITS FOR ACCEPTANCE, 90% CONFIDENCE INTERVALS
C      TOP = YBAR + 1.96 * SD
C      BOTTOM = YBAR - 1.96 * SD
C
C      REJECT YREG VALUES THAT ARE NOT IN THE 90% CONFIDENCE INTERVALS
C      IAKI = 0
C      THIS DO LOOP REJECT/ACCEPT VARIABLES THAT ARE TO BE REGRESSED
C
C      DO 12 I=1,20
C      IS = 1 + I * 5
C      IF (Y(IS) .GT. TOP .OR. Y(IS) .LT. BOTTOM) GO TO 12
C      IAKI = IAKI + 1
C      XREG(IAKI) = X(IS)
C      YREG(IAKI) = Y(IS)
12  CONTINUE
C
C      USE LEAST SQUARE METHOD TO FIND A,B, AND C
C      FORM OF EQUATION:

```

```

SH104900
SH104970
SH104980
SH104990
SH105000
SH105010
SH105020
SH105030
SH105040
SH105050
SH105060
SH105070
SH105080
SH105090
SH105100
SH105110
SH105120
SH105130
SH105140
SH105150
SH105160
SH105170
SH105180
SH105190
SH105200
SH105210
SH105220
SH105230
SH105240
SH105250
SH105260
SH105270
SH105280
SH105290
SH105300
SH105310
SH105320
SH105330
SH105340
SH105350
SH105360
SH105370
SH105380
SH105390
SH105400
SH105410
SH105420
SH105430
SH105440
SH105450
SH105460
SH105470
SH105480
SH105490
SH105500

```


FILE: SHIFILN FORTRAN 9

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

CVEC (1) = CUMT                                SH106060
CVEC (2) = CUMXF                                SH106070
CVEC (3) = CUMYT                                SH106080
C
C CALL /MATINV/ TO FIND THE INVERSE OF MATRIX AMAT SH106090
C
C CALL MATINV (AMAT,3,AINV)                      SH106100
C
C CALL MATINV (AMAT,3,AINV)                      SH106110
C
C FIND THE UNKNOWNNS BY MATRIX MULTIPLICATION SH106120
C
C (BVEC) = [AINV] * (CVEC)                       SH106130
C
C CALL MATMUL (AINV,CVEC,BVEC)                   SH106140
C
C EQUATE VALUES FOR A,B,C,AND THEN COMPUTE THE R-SQ VALUE SH106150
C
C A = EXP (bVEC(1))                               SH106160
C B = bVEC(2)                                     SH106170
C C = bVEC(3)                                     SH106180
C
C *****                                         SH106190
C
C INSERT THE R-SQ EQUATION HERE                   SH106200
C
C *****                                         SH106210
C
C CHECK PRINTING OPTIONS AND ACT ACCORDINGLY SH106220
C
C IREG = 0                                         SH106230
C DO 15 I = 1,NFLAGS                               SH106240
C
C IF (IPLAS(I).EQ.'NOREG') IREG = 10             SH106250
C CONTINUE                                         SH106260
C
C IF (IREG.EQ.0) CALL REGOUT (AMAT,BVEC,CVEC,AINV,RSQ) SH106270
C RETURN                                           SH106280
C END                                              SH106290
C
C *****                                         SH106300
C
C A SUBROUTINE STARTS AT THIS POINT              SH106310
C
C *****                                         SH106320
C
C SUBROUTINE REGOUT (AMAT,BVEC,CVEC,AINV,RSQ) SH106330
C
C THIS SUBROUTINE PRINTS COUTPUT FROM /REGRESS/ SH106340
C
C DIMENSION AMAT (3,3) ,BVEC (3) ,CVEC (3) ,AINV (3,3) SH106350
C
C PRINT 100                                        SH106360
C 100 FORMAT(*1* ,//10X ,*REGRESSION ANALYSIS TO FIT MULTIPLIERS * , SH106370
C *IN10 AN EQUATION*// ,10X,55 (***) ,//) SH106380
C PRINT 101                                        SH106390
C
C *****                                         SH106400
C
C *****                                         SH106410
C
C *****                                         SH106420
C
C *****                                         SH106430
C
C *****                                         SH106440
C
C *****                                         SH106450
C
C *****                                         SH106460
C
C *****                                         SH106470
C
C *****                                         SH106480
C
C *****                                         SH106490
C
C *****                                         SH106500
C
C *****                                         SH106510
C
C *****                                         SH106520
C
C *****                                         SH106530
C
C *****                                         SH106540
C
C *****                                         SH106550
C
C *****                                         SH106560
C
C *****                                         SH106570
C
C *****                                         SH106580
C
C *****                                         SH106590
C
C *****                                         SH106600

```


FILE: SHIFILM FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

      DREW = X(2)                                SH107160
      CNEW = X(3)                                SH107170
      CALL SECND(A2)                             SH107180
      PRINT 600,A1                               SH107190
      PRINT 601,KCOUNTS,LIN,FX,(X(I),I=1,N)     SH107200
      PRINT 602,A2                               SH107210
      PRINT 30, X(1),X(2),X(3),SUMWGT,SUMVAR     SH107220
30  FORMAT(///10X,*COEFFICIENTS OF THE EQUATION*//20X,
      ** A = *,F10.4,/20X,* B = *,F10.4,/20X,* C = *,F10.4,
      +//20X,*AVERAGE WEIGHT = *,F10.4,/20X,*VARIANCE = *,F10.4//) SH107230
      JOO FORMAT(14H TIME IS NOW =,F20.3)        SH107240
      601 FORMAT(/// I10,48H FUNCTION EVALUATIONS WITHIN POWELL ROUTINE ANDS SH107250
      1 // I10,47H FUNCTION EVALUATIONS DURING THE LINE SEARCHES. SH107260
      2 // 18H FUNCTION VALUE = ,E20.8          SH107270
      3 // 18H VARIABLE VALUES:- / (X,5E20.8 )) SH107280
      602 FORMAT(/// 14H TIME IS NOW = , F20.8) SH107290
      RETURN                                     SH107300
      END                                         SH107310
C                                               SH107320
C                                               SH107330
C                                               SH107340
C *****                                     SH107350
C                                               SH107360
C          A SUBROUTINE STARTS AT THIS POINT    SH107370
C                                               SH107380
C *****                                     SH107390
C                                               SH107400
      SUBROUTINE POWELL (STEP,ICONVG)            SH107410
      COMMON /ONE/ X,Y,S,FX,FY,N,KCUNT,LIN,NDRV,DIRECT,BEFORE,FIRST SH107420
      DIMENSION X(10),Y(10),S(10),DIRECT(10,10),BEFORE(10),FIRST(10) SH107430
      1      ,W(10),SECND(10)                    SH107440
      EQUIVALENCE (W,SECND)                     SH107450
C *** N = THE NUMBER OF VARIABLES.             SH107460
C      KCUNT = THE NUMBER OF FUNCTIONS EVALUATIONS NOT IN LINEAR SEARCH. SH107470
C      ICONVG = THE FINAL CONVERGENCE TEST DESIRED. SH107480
C              = 1, TERMINATE AS SOON AS TESTING IS SATISFIED. SH107490
C              = 2, AS SOON AS THE TESTING CRITERIA ARE SATISFIED INCREASES SH107500
C                  ALL THE VARIABLES BY 10*ACC AND SOLVE PROBLEM AGAIN. SH107510
C      THEN PERFORM A LINE SEARCH BETWEEN THE SOLUTIONS IF DIFFERENT SH107520
C      SOLUTIONS ARE DEEMED TO BE FOUND.       SH107530
C      STEP = THE INITIAL STEP SIZE.           SH107540
C      ACC = THE REQUIRED ACCURACY IN THE FUNCTION AND VECTOR VALUES. SH107550
C      LPRINT IPRINT= 1 FOR COMPLETE PRINT OUT OR IPRINT = 2 FINAL SH107560
C      ANSWER ONLY                              SH107570
      ACC = .0001                                SH107580
      LPRINT=1                                   SH107590
      NDRV=1                                    SH107600
      N1=N-1                                    SH107610
      STEP=STEP                                  SH107620
C *** SET UP THE INITIAL DIRECTION MATRIX (USING UNIT VECTORS). SH107630
      DO 2 I=1,N                                 SH107640
      DO 1 J=1,N                                 SH107650
      1 DIRECT(J,I)=0.                            SH107660
      2 DIRECT(I,I)=1.                            SH107670
C *** EVALUATED THE FUNCTION AT THE INITIAL VARIABLE VALUES. SH107680
      DO CALL FUN(X,FX)                          SH107690
      KCUNT=KCUNT+1                              SH107700

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FILE: SHIFTEM FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

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C *** SAVE THE FINAL FUNCTION VALUE (F1) AND THE FINAL VARIABLE VALUES SH107710
C (BEFORE) FROM THE PREVIOUS CYCLE. SH107720
      PRINT 36 SH107730
      30 FORMAT(8X,*FX*,12X,*X(1)*,14X,*X(2)*,12X,*X(3)*/) SH107740
      3 F1=FX SH107750
      DO 4 I=1,N SH107760
      + BEFORE(I)=X(I) SH107770
      GO TO (801,802), IPRINT SH107780
      801 PRINT 901,FX,(X(I),I=1,N) SH107790
      901 FORMAT (/5E16.8) SH107800
C *** START SEARCHING HERE. SH107810
      802 SUM=0. SH107820
C AT THE END OF THE CYCLE, SUM WILL CONTAIN THE MAXIMUM CHANGE IN SH107830
C THE FUNCTION VALUE FOR ANY SEARCH DIRECTION, AND ISAVE INDICATES SH107840
C THE DIRECTION VECTOR TO WHICH IT CORRESPONDS. SH107850
      DO 9 I=1,N SH107860
C S CONTAINS THE INITIAL STEP SIZES IN THE I-TH DIRECTION. SH107870
      DO 5 J=1,N SH107880
      5 S(J)=DIRECT(J,I)*STEP SH107890
C FIND THE MINIMUM IN THE I-TH DIRECTION, AND THE CHANGE IN FUNCTIONS SH107900
C VALUE. SH107910
      CALL COGGIN SH107920
      A=FX-FY SH107930
      IF (A-SUM) 7,7,6 SH107940
      6 ISAVE=I SH107950
      SUM=A SH107960
C TRANSFER THE NEW FUNCTION AND VARIABLE VALUES TO FX AND X. SH107970
      DO 8 J=1,N SH107980
      8 X(J)=Y(J) SH107990
      9 FX=FY SH108000
C *** NOW INVESTIGATE WHETHER A NEW SEARCH DIRECTION SHOULD BE INCORPOR- SH108010
C ATED INSTEAD OF THE ISAVE DIRECTION. SH108020
      F2=FX SH108030
      DO 10 I=1,N SH108040
      10 * (I)=2.0*X(I)-BEFORE(I) SH108050
      CALL FUN(*,F3) SH108060
      KOUNT=KOUNT+1 SH108070
      A=F3-F1 SH108080
      IF (A) 11,19,19 SH108090
      11 A=2.0*(F1-2.0*F2+F3)*((F1-F2-SUM)/A)**2 SH108100
      IF (A-SUM) 12,19,19 SH108110
C *** A NEW SEARCH DIRECTION IS REQUIRED. FIRST REMOVE ROW ISAVE. SH108120
      12 IF (ISAVE=N) 13,15,15 SH108130
      13 DO 14 I=ISAVE,N1 SH108140
      I1=I+1 SH108150
      DO 14 J=1,N SH108160
      14 DIRECT(J,I1)=DIRECT(J,I) SH108170
C SET THE N-TH DIRECTION VECTOR EQUAL TO THE NORMALISED DIFFERENCE SH108180
C BETWEEN THE INITIAL AND FINAL VARIABLE VALUES FOR LAST CYCLE. SH108190
      15 A=0. SH108200
      DO 16 J=1,N SH108210
      DIRECT(J,N)=X(J)-BEFORE(J) SH108220
      16 A=DIRECT(J,N)**2+A SH108230
      A=1.0/SQRT(A) SH108240
      DO 17 J=1,N SH108250

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FILE: SH1P1N FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

DIRECT(J,N)=DIRECT(J,N)*A
17 S(J)=DIRECT(J,N)*STEP
CALL COGGIN
FA=FY
DO 18 I=1,N
18 X(I)=Y(I)
C *** TEST FOR CONVERGENCE.
19 CALL TEST(F1,FX,BEFORE,X,FLAG,N,ACC)
IF(FLAG) 22,22,20
C *** CONVERGENCE NOT YET ACHIEVED. COMPUTE A NEW STEP SIZE AND
C GO BACK TO 3.
20 IF(F1-FX) 121,120,120
121 STEP=-0.4*SQRT(ABS(F1-FX))
GO TO 123
120 STEP=0.4*SQRT(F1-FX)
123 IF(STEPA-STEP) 21,3,3
21 STEP=STEPA
GO TO 3
C *** CONVERGENCE ACHIEVED. IF ICONVG=2, INCREASE ALL VARIABLES BY
C 10*ACC AND GO BACK TO 3.
22 GO TO (23,24),ICONVG
23 RETURN
24 GO TO (25,27),NTRY
25 NTRY=2
DO 26 I=1,N
FIRST(I)=A(I)
26 A(I)=X(I)+ACC*10.
FIRST=FX
GO TO 100
C *** CONVERGENCE ATTAINED USING TWO DIFFERENT STARTING POINTS. CONSTRUCT
C UNIT VECTOR BETWEEN SOLUTIONS AND SEARCH DIRECTION FOR A MINIMUM.
27 FSECND=FX
A=0.
DO 28 I=1,N
SECND(I)=X(I)
S(I)=FIRST(I)-SECND(I)
28 A=A+S(I)**2
IF(A) 29,23,29
29 A=STEP/SQRT(A)
DO 30 I=1,N
30 S(I)=S(I)*A
CALL COGGIN
C *** TEST IF NEW POINT IS SUFFICIENTLY CLOSE TO EITHER OF THE TWO
C SOLUTIONS. IF SO RETURN.
CALL TEST(FFIRST,FY,FIRST,Y,FLAG,N,ACC)
IF(FLAG) 32,32,31
31 CALL TEST(FSECND,FY,SECND,Y,FLAG,N,ACC)
IF(FLAG) 32,32,34
32 DO 33 I=1,N
33 X(I)=Y(I)
FA=FY
RETURN
C *** FINAL SOLUTION NOT ACCURATE ENOUGH. REPLACE THE FIRST DIRECTION
C VECTOR BY INTER-SOLUTION VECTOR (NORMALISED) AND RECYCLE
34 A=A/STEP

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FILE: SHIFILN FORTRAN 8

VM/SP CONVERSATIONAL MONITOR SYSTEM

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DO 35 I=1,N                               SH108810
DIRECT(I,1)=(FIRST(I)-SECND(I))*A          SH108820
35 FIRST(I)=SECND(I)                       SH108830
GO TO 3                                     SH108840
END                                          SH108850
SUBROUTINE TEST (PI,FF,RI,RF,FLAG,N,ACC)    SH108860
COMMON /CONSIS/ A,B,C,ANEW,BNEW,CNEW       SH108870
DIMENSION RI(10),RF(10)                   SH108880
FLAG=+2.                                    SH108890
IF (ABS(PI)-ACC) 2,2,1                      SH108900
1 IF (ABS((PI-FF)/PI)-ACC) 3,3,7           SH108910
2 IF (ABS(PI-FF)-ACC) 3,3,7                SH108920
3 DO 6 I=1,N                                SH108930
IF (ABS(RI(I))-ACC) 5,5,4                   SH108940
4 IF (ABS((RI(I)-RF(I))/RI(I))-ACC) 6,6,7  SH108950
5 IF (ABS(RI(I)-RF(I))-ACC) 6,6,7          SH108960
6 CONTINUE                                  SH108970
FLAG=-2.                                    SH108980
7 RETURN                                    SH108990
END                                          SH109000
C                                           SH109010
C*****                                     SH109020
C                                           SH109030
C      A SUBROUTINE STARTS AT THIS POINT    SH109040
C                                           SH109050
C*****                                     SH109060
C                                           SH109070
SUBROUTINE COGGIN                           SH109080
COMMON /CCONSIS/ A,B,C,ANEW,BNEW,CNEW       SH109090
COMMON /ONE/ X,Y,S,FX,FY,N,KOUNTS,LIN,NDRV,H,SIG,DELG SH109100
DIMENSION X(10),Y(10),S(10),SIG(10),DELG(10),H(10,10) SH109110
C *** THE INITIAL VARIABLE VALUES ARE IN X, AND THE CORRESPONDING SH109120
C *** FUNCTION VALUE IS FX.                 SH109130
C *** THE SEARCH DIRECTION VECTOR IS S, AND THE INITIAL STEP SIZE STEP. SH109140
C *** LIN IS USED TO COUNT THE NUMBER OF FUNCTION EVALUATIONS AND N IS SH109150
C *** THE NUMBER OF VARIABLES.             SH109160
FA=FB=FC=FX                                  SH109170
DA=DB=DC=0.                                  SH109180
STEP=1.0                                       SH109190
D=STEP                                         SH109200
K=-2                                           SH109210
M=0                                             SH109220
C *** START THE SEARCH THE BOUND THE MINIMUM SH109230
1 DO 2 I=1,N                                   SH109240
2 Y(I)=X(I)+D*S(I)                             SH109250
CALL FUN(Y,F)                                   SH109260
A=K+1                                           SH109270
LIN=LIN+1                                       SH109280
IF (F-FA) 5,3,6                                  SH109290
C *** NO CHANGE IN FUNCTION VALUE. RETURN WITH VECTOR CORRESPONDING TO SH109300
C FUNCTION VALUE OF FA, BECAUSE IF THE FUNCTION VALUE IS INDEPENDENT SH109310
C OF THIS SEARCH DIRECTION, THEN CHANGES IN THE VARIABLE VALUES MAY SH109320
C UPSET THE MAIN PROGRAM CONVERGENCE TESTING. SH109330
3 DO 4 I=1,N                                   SH109340
4 Y(I)=X(I)+DA*S(I)                             SH109350

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FILE: SHFITIN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

      FY=FA
      RETURN
C *** THE FUNCTION IS STILL DECREASING. INCREASE THE STEP SIZE BY
C DOUBLE THE PREVIOUS INCREASE IN STEP SIZE.
      5 FC=FB $ FB=FA $ FA=F
      DC=DB $ DB=DA $ DA=D
      D=2.0*D+STEP
      GO TO 1
C *** MINIMUM IS BOUNDED IN AT LEAST ONE DIRECTION.
      6 IF (K) 7,8,9
C MINIMUM IS BOUNDED IN ONE DIRECTION ONLY. REVERSE THE SEARCH
C DIRECTION AND RECYCLE.
      7 FB=F
      DB=D $ D=-D $ STEP=-STEP
      GO TO 1
C MINIMUM IS BOUNDED IN BOTH DIRECTIONS AFTER ONLY TWO FUNCTION
C EVALUATIONS [ONE EITHER SIDE OF THE ORIGIN]. PROCEED TO THE
C PARABOLIC INTERPOLATION.
      8 FC=FB $ FB=FA $ FA=F
      DC=DB $ DB=DA $ DA=D
      GO TO 21
C THE MINIMUM IS BOUNDED AFTER AT LEAST TWO FUNCTION EVALUATIONS IN
C THE SAME DIRECTION. EVALUATE THE FUNCTION AT STEP SIZE=(DA+DB)/2.
C THIS WILL YIELD 4 EQUALLY SPACED POINTS BOUNDING THE MINIMUM.
      9 DC=DB $ DB=DA $ DA=D
      FC=FB $ FB=FA $ FA=F
      10 D=0.5*(DA+DB)
      DO 11 I=1,N
      11 Y(I)=X(I)+D*S(I)
      CALL FUN(Y,F)
      LIN=LIN+1
C *** NOW HAVE THAT FA>FB<FC AND THAT FA>F<FC ASSUMING THAT THE
C FUNCTION IS UNIMODAL. REMOVE EITHER POINT A OR POINT B IN SUCH A
C WAY THAT THE FUNCTION IS BOUNDED AND FA>FB<FC [THE CORRESPONDING
C STEP SIZES ARE DA>DB>DC OR DA<DB<DC ].
      12 IF ((DC-D)*(D-DB)) 15,13,18
C *** LOCATION OF MINIMUM IS LIMITED BY ROUNDING ERRORS. RETURN WITH B.
      13 DO 14 I=1,N
      14 Y(I)=X(I)+DB*S(I)
      FY=FB
      RETURN
C *** THE POINT D IS IN THE RANGE DA TO DB.
      15 IF (F-FC) 16,13,17
      16 FC=FB $ FB=F
      DC=DB $ DB=D
      GO TO 21
      17 FA=F
      DA=D
      GO TO 21
C *** THE POINT D IS IN THE RANGE DB TO DC
      18 IF (F-FC) 19,13,20
      19 FA=FB $ FB=F
      DA=DB $ DB=D
      GO TO 21
      20 FC=F

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FILE: SHAFTIN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

      DC=D                                SH109910
C *** NOW PERFORM THE PARABOLIC INTERPOLATION.  SH109920
      21 A=FA*(DB-DC)+FB*(DC-DA)+FC*(DA-DB)  SH109930
      IF (A) 22,30,22                      SH109940
      22 B=0.5*((DB*DB-DC*DC)*FA+(DC*DC-DA*DA)*FB+(DA*DA-DB*DB)*FC)/A  SH109950
C CHECK THAT THE POINT IS GOOD. IF SO, EVALUATE THE FUNCTION.  SH109960
      IF ((DA-D)*(D-DC)) 13,13,23          SH109970
      23 DO 24 I=1,N                        SH109980
      24 Y(I)=X(I)+D*S(I)                   SH109990
      CALL FUN(Y,F)                          SH100000
      LEB=LIN+1                              SH100100
C *** CHECK FOR CONVERGENCE. IF NOT ACHEIVED, RECYCLE.  SH100200
      IF (ABS(FB)-0.00001) 25,25,26         SH100300
      25 A=1.0 5 GO TO 27                   SH100400
      26 A=1.0/FB                           SH100500
      27 IF (ABS((FB-F)*A)-.0001) 28,28,12  SH100600
C *** CONVERGENCE ACHIEVED. RETURN WITH THE SMALLER OF F AND FB.  SH100700
      28 IF (F-FB) 29,13,13                SH100800
      29 FY=F                                SH100900
      RETURN                                  SH101000
C *** THE PARABOLIC INTERPOLATION WAS PREVENTED BY THE DIVISOR BEING  SH101100
C ZERO. IF THIS IS THE FIRST TIME THAT IT HAS HAPPENED, TRY AN  SH101200
C IMMEDIATE STEP SIZE AND RECYCLE; OTHERWISE GIVE UP AS IT LOOKS  SH101300
C LIKE A LOST CAUSE.                        SH101400
      30 IF (3) 31,31,13                    SH101500
      31 M=M+1                              SH101600
      GO TO 10                               SH101700
      END                                    SH101800
      BLOCK DATA                            SH101900
      COMMON /ONE/  X,Y,S,FX,FY,N,OUNTS,LIN,NDRV,H,SIG,DELG  SH102000
      DIMENSION X(10),Y(10),S(10),SIG(10),DELG(10),H(10,10)  SH102100
      COMMON /CONSIS/ A,B,C,ANEW,BNEW,CNEW  SH102200
      DATA N /3/                            SH102300
      END                                    SH102400
C                                           SH102500
C *****                                SH102600
C                                           SH102700
C A SUBROUTINE STARTS AT THIS POINT        SH102800
C                                           SH102900
C *****                                SH103000
C                                           SH103100
      SUBROUTINE FUN(X,FX)                   SH103200
      DIMENSION X(10)                       SH103300
C THIS SUBROUTINE PROVIDE THE EQUATION AND RESTRAINTS  SH103400
C FOR THE MAIN PROGRAM. THE EQUATION IS  SH103500
C                                           SH103600
C                                           SH103700
C Y = A * X ** B * EXP ( C * X )          SH103800
C                                           SH103900
C                                           SH104000
C X(1) = A                                  SH104100
C X(2) = B                                  SH104200
C X(3) = C                                  SH104300
C                                           SH104400
C                                           SH104500

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FILE: SHIFILM FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

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COMMON /WGTLIN/ SUMWGT,EXPWGT,SUMVAR,EXPVAR,CPT          SHI10460
COMMON /VARIES/ #GIBASE(20),RAT(101)                  SHI10470
COMMON /PLOTD/ ACCPR(35),B(35)                         SHI10480
COMMON /BASIC/ A(35),SAMSIZ(35),NA,NGROUP             SHI10490
COMMON /PASS/ NPASS                                    SHI10500
DIMENSION A1(35),A2(35),FACT(101)                    SHI10510
COMMON /TKWGT/ TKWGT(2,101),WGTPROJ(101)             SHI10520
IFLG = 0                                               SHI10530
IF(IFLG.E2.0) GO TO 100                                SHI10540
FK = ABS ( 10000000000.00 / X(1) * X(2) / X(3) )     SHI10550
GO TO 999                                             SHI10560
100 DO 10 I = 2, 95                                    SHI10570
    PI = FLOAI(I+1)                                    SHI10580
    IF(PI.LE.CPT) GO TO 5                               SHI10590
    FACT(I) = X(1)*(TKWGT(2,I) ** X(2)) * EXP(TKWGT(2,I) * X(3))
    WGTPROJ(I) = IKWGT(2,I) * FACT(I)                  SHI10600
    GO TO 10                                           SHI10620
5    FACT(I) = RAT(I)                                  SHI10630
    WGTPROJ(I) = IKWGT(2,I) * FACT(I)                  SHI10640
10    CONTINUE                                         SHI10650
    DO 11 I = 90,101                                   SHI10660
    FACT(I) = 0.5 * (FACT(90) + FACT(95))              SHI10670
    WGTPROJ(I) = IKWGT(2,I) * FACT(I)                  SHI10680
11    CONTINUE                                         SHI10690
    SUMWGT = 0.0                                       SHI10700
    SUMVAR = 0.0                                       SHI10710
    NPASS = 0                                           SHI10720
    CALL SCURVE                                         SHI10730
C                                                       SHI10740
C    CALCULATE THE VARIANCE OF S-CURVE                 SHI10750
    SUMB = 0.0                                          SHI10760
    SUMAB1 = 0.0                                        SHI10770
    SUMAB = 0.0                                        SHI10780
    SUMA2B = 0.0                                       SHI10790
    MC = NGROUP / 2                                     SHI10800
    AMIDPT = A (MC)                                    SHI10810
    DO 16 I=1, NGROUP                                  SHI10820
    IF(I.EQ.1) A1(I) = A(1) / 2.0                       SHI10830
    IF(I.NE.1) A1(I) = (A(I-1) + A(I)) / 2.0           SHI10840
    A2(I) = A1(I) - AMIDPT                              SHI10850
    SUMB = SUMB + B(I)                                  SHI10860
    SUMAB1 = SUMAB1 + A1(I) * B(I)                      SHI10870
    SUMAB = SUMAB + A2(I) * B(I)                       SHI10880
    SUMA2B = SUMA2B + A2(I) * A2(I) * B(I)             SHI10890
16    CONTINUE                                         SHI10900
    SUMWGT = SUMAB1 / SUMB                              SHI10910
    SUMVAR = (SUMA2B - SUMAB * SUMAB / SUMB) / SUMB     SHI10920
    FK = ABS (SUMWGT - EXPWGT)                          SHI10930
999    RETURN                                          SHI10940
    END                                               SHI10950
C                                                       SHI10960
C                                                       SHI10970
C*****                                                SHI10980
C                                                       SHI10990
C    A SUBROUTINE STARTS AT THIS POINT                 SHI11000

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FILE: SHIFIN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

C
C*****
C
C
C      THIS SUBROUTINE IS TO FIND THE MATRIX-INVERSE
SUBROUTINE MATINV(AA,N,AINV)
DIMENSION AA(3,3),AINV(3,3),A(10,20),ID(10)
NN=N+1
NZ=N*N
DO 200 I=1,N
DO 200 J=1,N
200  A(I,J) = AA(I,J)
      K=1
      DO 1 I=1,N
      DO 1 J=NN,NZ
      A(I,J) = 0.
1      CONTINUE
      DO 21 I=1,N
      A(I,N+I) = 1.
21     ID(I) = I
2      CONTINUE
      KA=K+1
      IS=K
      II=K
      B=ABS(A(K,K))
      DO 3 I=K,N
      DO 3 J=K,N
      IF (ABS(A(I,J)) - B) 3,3,31
31     IS=I
      IF=J
      C=ABS(A(I,J))
3      CONTINUE
      IF (IS-K) 4,4,41
41     DO 42 J=K,N2
      C=A(IS,J)
      A(IS,J)=A(K,J)
42     A(K,J)=C
4      CONTINUE
      IF (II-K) 5,5,51
51     IC=ID(K)
      ID(K)=ID(II)
      ID(II)=IC
      DO 52 I=1,N
      C=A(I,II)
      A(I,II)=A(I,K)
52     A(I,K)=C
5      CONTINUE
      IF (A(K,K)) 6,120,6
6      CONTINUE
      DO 7 J=KK,N2
      A(K,J)=A(K,J)/A(K,K)
      DO 7 I=KK,N
      W=A(I,K)*A(K,J)
      A(I,J)=A(I,J)-W
      IF (ABS(A(I,J)) - .0001*ABS(W)) 71,7,7

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SHI11010
SHI11020
SHI11030
SHI11040
SHI11050
SHI11060
SHI11070
SHI11080
SHI11090
SHI11100
SHI11110
SHI11120
SHI11130
SHI11140
SHI11150
SHI11160
SHI11170
SHI11180
SHI11190
SHI11200
SHI11210
SHI11220
SHI11230
SHI11240
SHI11250
SHI11260
SHI11270
SHI11280
SHI11290
SHI11300
SHI11310
SHI11320
SHI11330
SHI11340
SHI11350
SHI11360
SHI11370
SHI11380
SHI11390
SHI11400
SHI11410
SHI11420
SHI11430
SHI11440
SHI11450
SHI11460
SHI11470
SHI11480
SHI11490
SHI11500
SHI11510
SHI11520
SHI11530
SHI11540
SHI11550

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FILE: SHIPILN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

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71  A(I,J)=0.                                SH111500
7   CONTINUE                                SH111570
    K=KK                                     SH111580
    IF(K-N) 2,01,120                         SH111590
01  IF(A(N,N))0,120,8                       SH111600
0   CONTINUE                                SH111610
    DO 9 J=NN,N2                             SH111620
    A(N,J)=A(N,J)/A(N,N)                    SH111630
9   CONTINUE                                SH111640
    N1=N-1                                    SH111650
    DO 10 M=1,N1                              SH111660
    I=N-M                                     SH111670
    II=I+1                                    SH111680
    DO 10 K=II,N                             SH111690
    DO 10 J=NN,N2                             SH111700
    A(I,J)=A(I,J)-A(I,K)*A(K,J)             SH111710
10  CONTINUE                                SH111720
    DO 11 I=1,N                               SH111730
    DO 11 J=1,N                               SH111740
    IF(ID(J)-1)11,111,11                    SH111750
111 DO 112 K=NN,N2                          SH111760
112 AINV(I,K-N)=A(J,K)                     SH111770
11  CONTINUE                                SH111780
    RETURN                                    SH111790
120 WRITE(2,1000)                            SH111800
    WRITE(3,1000)                            SH111810
    RETURN                                    SH111820
1000 FORMAT(10X,19n MATRIX IS SINGULAR)    SH111830
    END                                       SH111840
C                                           SH111850
C                                           SH111860
C*****                                     SH111870
C                                           SH111880
C      A SUBROUTINE STARTS AT THIS POINT    SH111890
C                                           SH111900
C*****                                     SH111910
C                                           SH111920
C                                           SH111930
C      SUBROUTINE MATMUL (AINV,CVEC,BVEC)   SH111940
C                                           SH111950
C      THIS SUBROUTINE DOES MATRIX-MULTIPLICATION. SH111960
C      ONLY 3X3,(3) = (3) OPERATIONS CAN BE DONE IN THIS SUBROUTINE SH111970
C                                           SH111980
C      DIMENSION AINV(3,3),CVEC(3),BVEC(3) SH111990
C      THIS DO LOOP OPERATES MATRIX MULTIPLICATION SH12000
C                                           SH12010
C      DO 10 I=1,3                            SH12020
C          BVEC(I) = 0.0                       SH12030
C          DO 11 J=1,3                         SH12040
C              BVEC(I) = BVEC(I) + AINV(I,J) * CVEC(J) SH12050
11  CONTINUE                                SH12060
10  CONTINUE                                SH12070
    RETURN                                    SH12080
    END                                       SH12090
C                                           SH12100

```

FILE: SHAFIN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

C
C*****
C          A SUBROUTINE STARTS AT THIS POINT
C*****
C
C          SUBROUTINE XSQUARE (NSTCP)
C          COMMON /FLAGS/ IFLAG(10),NFLAGS
C          COMMON /BASIC/ WDIST(35),SAMSIZ(35),NA,NGROUP
C          DIMENSION IFFT(5),CHISQ(35),YEXP(20),COL3(20),COL4(20),COL5(20),
C          + X(20),Y(20)
C          COMMON /CONSTS/ A,B,C,ANEW,BNEW,CNEW
C          COMMON /VARIES/ WGTBASE(20),RAT(101)
C          DATA (CHISQ(I),I=1,30)/ 3.841,5.991,7.815,9.488,11.070,12.592,
C          .14.007,15.507, 16.910, 18.307, 19.75, 21.026, 22.362, 23.685,
C          .24.990,26.296,27.587, 28.369, 30.144, 31.410, 32.671, 33.924,
C          .25.172, 36.415, 37.652, 38.885, 40.113, 41.337, 42.557, 43.773/
C          NP = 20
C          DO 10 I = 1, 20
C            IS = 1 + I*5
C            X(I) = WGTBASE(I)
C            Y(I) = RAT(IS)
10          CONTINUE
C          DO 20 I=1,20
C            YEXP(I) = A * X(I) ** B * EXP (X(I) *C)
20          CONTINUE
C          SUMCOL3=0.0
C          DO 25 J=1,NP
C            COL3(J) = YEXP(J) - Y(J)
C            COL4(J) = COL3(J) * COL3(J)
C            COL5(J) = COL4(J) / YEXP(J)
C            SUMCOL5 = SUMCOL5 + COL5(J)
25          CONTINUE
C          NXSQ = 10
C          DO 26 I=1,NFLAGS
C            IF (IFLAG(I).EQ.'NOCHI') NXSQ = 0
26          CONTINUE
C          IF (NXSQ.EQ.0) GO TO 990
C          PRINT 500
500          FORMAT(*1*,/10X,*CHI-SQUARE TESTING ON GOODNESS-OF-FIT OF *,
C          **THE MULTIPLIER EQUATION*/,10X,64(==),//)
C          PRINT 505
505          FORMAT(10X,*EQUATION FITTED:*,10X,
C          + 35NYEXP = A * (X ** B) * EXP (X * C) ,/)
C          PRINT 510,A,B,C
510          FORMAT(/10X,**HERE      A = *,F10.4/
C          + 20X,*B = *,F10.4,/20X,*C = *,F10.4/)
C          PRINT 515
515          FORMAT(18X,* (1) *,12X,* (2) *,12X,* (3) *,12X,* (4) *,12X,* (5) *,
C          ./10X,*ACTUAL*,9X,*EXPECTED*,7X,* (1) - (2) *,6X,* (3) X (3) *,8X,
C          .* (4) / (2) *,/13X,*VALUES*,9X,*VALUES*//)
C          DO 30 J=1,NP
C            PRINT 520,J,Y(J),YEXP(J),COL3(J),COL4(J),COL5(J)

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FILE: SUFFIM FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

30  CONTINUE                               SH112660
320  FORMAT(7X,13,5(F13.6,3X)/)           SH112670
      PRINT 525,SUMCOL5                     SH112680
325  FORMAT(51X,*CHI-SQUARE VALUE = *,F15.6/) SH112690
      NDEG = 18                             SH112700
      IF (SUMCOL5.GT.CHI SQ(NDEG))         SH112710
      .PRINT 535                             SH112720
      IF (SUMCOL5.GT.CHI SQ(NDEG))         SH112730
      +NSTOP = 10                           SH112740
      IF (SUMCOL5.LE.CHI SQ(NDEG)) PRINT 540 SH112750
535  FORMAT(10X,*THE CHI-SQUARE VALUE EXCEEDS THE 5% SIGNIFICANT VALUE*SH112760
      .,/10X,*THE COEFFICIENTS GIVEN SHOULD NOT BE RELIED UPON.*/) SH112770
540  FORMAT(10X,*THE CHI-SQUARE VALUE IS WITHIN THE 5 PERCENT SIGNI*, SH112780
      .*EFFICIENT *,                          SH112790
      .*VALUE.*,/10X,*THUS,THE COEFFICIENTS MAY BE USED FOR THE EQUATION*SH112800
      ./)                                     SH112810
990  CONTINUE                               SH112820
      RETURN                                  SH112830
      END                                    SH112840
                                           SH112850
                                           SH112860
                                           SH112870
                                           SH112880
C                                           SH112890
C*****                                     SH112900
C                                           SH112910
C      A SUBROUTINE STARTS AT THIS POINT    SH112920
C                                           SH112930
C*****                                     SH112940
C                                           SH112950
C                                           SH112960
C      SUBROUTINE CALWGT                    SH112970
C      THIS SUBROUTINE PRODUCES W-4 OF W-5 TABLES FOR THE SH112980
C      WEIGHT DISTRIBUTION                  SH112990
C                                           SH113000
C      COMMON /VARIES/ WGTBASE(20),RAT(101) SH113010
C      COMMON /WGTLLM/ SUMWGT,EXPWGT,SUMVAR,EXPVAR,CPT SH113020
C      COMMON /CONSIS/ A,B,C,ANEW,BNEW,CNEW SH113030
C      COMMON /TKWGT/ TKWGT(2,101),WGTPROJ(101) SH113040
C      DIMENSION FACT(101)                 SH113050
C                                           SH113060
C      THIS DO LOOP CALCULATES THE PROJECTED WEIGHT AT 1 % SH113070
C      INTERVALS AND COMPUTES THE AVERAGE WEIGHT SH113080
C                                           SH113090
C      PROJWGT = 0.0                       SH113100
C      DO 10 I = 2, 95                      SH113110
C      FACT(I) = ANEW * (TKWGT(2,I) ** BNEW) * EXP(TKWGT(2,I) * CNEW) SH113120
C      ZF = FLOAT(I+1)                      SH113130
C      IF (ZF.LE.CPT) GO TO 5                SH113140
C      WGTPROJ(I) = TKWGT(2,I) * FACT(I)    SH113150
C      GO TO 10                              SH113160
C      FACT(I) = RAT(I)                     SH113170
C      WGTPROJ(I) = TKWGT(2,I) * FACT(I)    SH113180
10  CONTINUE                               SH113190
C                                           SH113200

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FILE: SHIFLIN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

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CALL SYMBOL (1.5,-.6,.2,13HAUGUST 1,1981,0.,13)          SHI13760
CALL PLOT(0.,0.,999)                                     SHI13770
C GO TO NEXT PAGE AND PLOT CURVE                          SHI13780
ENLARG = 1.0                                           SHI13790
REDUCE = 0.9                                           SHI13800
IF (IFLG.EQ.3000.OR.IFLG.EQ.6000) ENLARG=2.0          SHI13810
SIZE1=.14                                             SHI13820
SIZE2= .11                                           SHI13830
FACTOR = 1./ENLARG                                    SHI13840
C TO CALL ZSTAPLOT SUBROUTINES -- INITIATE              SHI13850
CALL PLOTS (0,0,4LPLOT)                                SHI13860
C TO MOVE PEN TO THE ALLCATED ORIGIN                    SHI13870
CALL PLOT (1.8,1.5,-3)                                  SHI13880
CALL FACTOR (REDUCE)                                    SHI13890
C TO PLOT A 7 INCH AXIS WITH TICK MARKS AND ANNOTATIONS SHI13900
X=0.0                                                  SHI13910
Z=0.0                                                  SHI13920
DO 100 I=1,12                                          SHI13930
CALL PLOT (X,0.0,3)                                     SHI13940
X = X + 0.5                                           SHI13950
Z = Z + 10.0*FACTOR                                    SHI13960
CALL PLOT (X, 0.0, 2)                                   SHI13970
CALL PLOT (X, 0.1,2)                                   SHI13980
CALL NUMBER (X-0.09,-.25, 0.105,Z,0.0,-1)            SHI13990
100 CONTINUE                                           SHI14000
X = 0.0                                               SHI14010
DO 310 I=1,120                                        SHI14020
CALL PLOT (X,0.0,3)                                     SHI14030
CALL PLOT (X,0.05,2)                                   SHI14040
X = X +0.05                                           SHI14050
310 CONTINUE                                           SHI14060
C TO WRITE THE TITLE FOR THE PLOT                       SHI14070
CALL SYMBOL(0.0,-1.0,SIZE1,                            SHI14080
+44HTRUCK WEIGHT DISTRIBUTION SHIFTING PROCEDURE ,0.,44) SHI14090
C TO WRITE SYMBOLS FOR X-AXIS                           SHI14100
IF (IFLG.EQ.3000) CALL SYMBOL (0.75,-.5,SIZE1,        SHI14110
+28H SINGLE AXLE WEIGHT (KIPS) ,0.,28)                SHI14120
IF (IFLG.EQ.6000) CALL SYMBOL (0.75,-.5,SIZE1,        SHI14130
+28H TANDEM AXLE WEIGHT (KIPS) , 0.,28)               SHI14140
IF (IFLG.EQ.9000) CALL SYMBOL (0.75,-.5,SIZE1,        SHI14150
+28H GROSS TRUCK WEIGHT (KIPS) ,0.,28)               SHI14160
C TO PLOT AN 8-INCH Y-AXIS WITH TICK MARKS AND NO.     SHI14170
Y = 0.0                                               SHI14180
W = 0.0                                               SHI14190
DO 101 I=1,10                                         SHI14200
CALL PLOT (0.0,Y,3)                                    SHI14210
C SCALE THE Y-AXIS                                     SHI14220
Y = Y + 0.3                                           SHI14230
W = W + 1.0                                           SHI14240
CALL PLOT (0.0,Y,2)                                    SHI14250
CALL PLOT (0.1,Y,2)                                    SHI14260
C PUT NUMBERS UNDER TICK MARKS ( NUMBER = XXX.XX)     SHI14270
IDIGIT = 0                                             SHI14280
CALL NUMBER (-0.15,Y-.1, 0.105,10.*W,90.0,IDIGIT)    SHI14290
101 CONTINUE                                           SHI14300

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FILE: SHIFTIM FORTRAN 3

VM/SP CONVERSATIONAL MONITOR SYSTEM

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      Y = 0.0
      DO 311 I = 1,100
      Y = Y + 0.08
      CALL PLOT (0.0,Y,3)
      CALL PLOT ( 0.05,Y,2)
311  CONTINUE
C     TO PLOT SYMBOLS FOR Y-AXIS
      CALL SYMBOL (-.4,1.5,SIZE1,
+30H ACCUMULATED FREQUENCY (X)      ,90.,30)
C     TO PRINT THE HIGHWAY TYPE
      X=1.2
      Y=9.0
      CALL SYMBOL (0.,Y,SIZE1,IROAD,0.,20)
      Y=Y-.25
C     TO PRINT THE VEHICLE TPE
      CALL SYMBOL(0.,Y,SIZE1,IVEH,0.,30)
      Y=Y-.25
C     PRINT THE STATE
      CALL SYMBOL(0.,Y,SIZE1,ISTATE,0.,20)
C     DEFINE THE SCALE FOR THE GRAPH
      XSCALE = 20. * FATOR
      N = NGROUP
      WDIST(N+1) = 0.
      WDIST(N+2) = XSCALE
      ACCFR(N+1) = 0.
      ACCFR(N+2) = 12.5
      CALL LINE (WDIST(1),ACCFR(1),N,1,1,1)
      CALL PLOT(0.,0.,999)
      RETURN
      END
C
C
C*****
C     A SUBROUTINE STARTS AT THIS POINT
C*****
C
C     SUBROUTINE SCURVE
C     THIS SUBROUTINE CONVERTS CUTPUT INTO W-4 OR W-5 TABLES
C
      COMMON /PLOTD/ ACCFR(35),PERCENT(35)
      COMMON /TKWGT/ TKWGT(2,101),WGTPROJ(101)
      COMMON /BASIC/ WDIST(35),SAMSIZ(35),NA,NGROUP
      COMMON /WGILIM/ SUMWGT,EXPWGT,SUMVAR,EXPVAR,CPT
      COMMON /PASS/ MPASS
C
      WGTPROJ(1) = 0.0
      M = 1
      L = 2
5     IF (M.EQ. NGROUP) GO TO 13
      IF (L.GE.101) GO TO 13
      IF (WGTPROJ(L) - WDIST(M)) 10,11,12
10    L = L + 1
      SH114310
      SH114320
      SH114330
      SH114340
      SH114350
      SH114360
      SH114370
      SH114380
      SH114390
      SH114400
      SH114410
      SH114420
      SH114430
      SH114440
      SH114450
      SH114460
      SH114470
      SH114480
      SH114490
      SH114500
      SH114510
      SH114520
      SH114530
      SH114540
      SH114550
      SH114560
      SH114570
      SH114580
      SH114590
      SH114600
      SH114610
      SH114620
      SH114630
      SH114640
      SH114650
      SH114660
      SH114670
      SH114680
      SH114690
      SH114700
      SH114710
      SH114720
      SH114730
      SH114740
      SH114750
      SH114760
      SH114770
      SH114780
      SH114790
      SH114800
      SH114810
      SH114820
      SH114830
      SH114840
      SH114850

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FILE: SHIFIN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

      GO TO 5
11  ACCFR(M) = (I-1) * 1.0
      I = I + 1
      M = M + 1
      GO TO 5
12  ACCFR(M) = ((WDIST(M) - WGTPROJ(I-1))/(WGTPROJ(I) - WGTPROJ(I-1))
      + (1 - 2) ) * 1.0
      M = M + 1
      GO TO 5
13  DO 14 I = M, NGROUP
      ACCFR(I) = 100.0
14  CONTINUE
      PERCENT(1) = ACCFR(1)
C    THIS DO LOOP CALCULATES THE PERCENTAGE OF THE DISTRIBUTION
C
      DO 15 I = 2, NGROUP
      PERCENT(I) = ACCFR(I) - ACCFR(I-1)
15  CONTINUE
      IF(MPASS.EQ.0) GO TO 99
C
C    PRINT OUTPUTS
C
      *WRITE(6,600)
600  FORMAT(*1*,/
      +/10A,*PROJECTED WEIGHT DISTRIBUTION*,/10X,29(==*)//)
      *WRITE(6,601)
601  FORMAT(10X,*WEIGHT*,15X,*PERCENTAGE*,10X,
      +*ACCUMULATED*/7X,*DISTRIBUTION*,33X,*FREQUENCY*//)
C
C    THIS DO LOOP PRINTS THE OUTPUT
C
      DO 16 I = 1,NGROUP
      WRITE(6,602) WDIST(I), PERCENT(I), ACCFR(I)
16  CONTINUE
602  FORMAT(10X,F10.4,10X,F10.4,10X,F10.4)
      WRITE(6,603) SUMWGT, SUMVAR
603  FORMAT(/10X,*AVERAGE WEIGHT = *,F10.4/,
      + 10X,* VARIANCE = *,F10.4/)
99  MPASS = 10
      RETURN
      END
C    SUBROUTINE
C
C*****
C
C    A SUBROUTINE STARTS AT THIS POINT
C*****
C
      SUBROUTINE IMPROG (COMMAND)
      COMMON /FLAGS/ IFLAG(10),NFLAGS
      COMMON /WGTLLM/ SUMWGT,EXPWGT,SUMVAR,EXPVAR,CPT
      INTEGER COMMAND(1)
      READ(5,100) COMMAND

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FILE: SHIFIN FORTRAN B

VN/SP CONVERSATIONAL MONITOR SYSTEM

```

100 FORMAT(A7)                                SHI15410
      I = 0                                    SHI15420
1    I = I + 1                                SHI15430
      NFLAGS=I                                SHI15440
      IF(NFLAGS.GI.10) GO TO 2                SHI15450
      READ(5,110) IFLAG(I)                   SHI15460
      IF(IFLAG(I).EQ.' ') GO TO 2            SHI15470
      GO TO 1                                  SHI15480
2    READ(5,120) EXPWGT,EXPVAR,CPT           SHI15490
110  FORMAT(A5)                                SHI15500
120  FORMAT(5F10.5)                            SHI15510
      RETURN                                    SHI15520
      END                                        SHI15530
                                           SHI15540
                                           SHI15550
                                           SHI15560
C                                           SHI15570
C*****                                     SHI15580
C                                           SHI15590
C      A SUBROUTINE STARTS AT THIS POINT     SHI15600
C                                           SHI15610
C*****                                     SHI15620
C                                           SHI15630
C      SUBROUTINE CONCLUD                     SHI15640
C                                           SHI15650
C      THIS SUBROUTINE MAKES THE CONCLUSION  SHI15660
C                                           SHI15670
C      COMMON /NGILIM/ SUMWGT,EXPWGT,SUMVAR, SHI15680
C      COMMON /BASIC/ A(35),SAMSIZ(35),NA,NG SHI15690
C                                           SHI15700
C      WHILE(0,600)                            SHI15710
600  FORMAT(*1*,//10X,*CONCLUSION OF ANALYSIS SHI15720
      WHILE(0,010) EXPWGT,EXPVAR             SHI15730
610  FORMAT(/10X,*INPUT ESTIMATORS:*/       SHI15740
      +12X,*EXPECTED MEAN = *F10.2,/        SHI15750
      +12X,*EXPECTED VARIANCE = *F10.2/)    SHI15760
      WHILE(0,020) SUMWGT,SUMVAR            SHI15770
620  FORMAT(/10X,*COMPUTED ESTIMATORS:*/    SHI15780
      +12X,* MEAN = *F10.2,/               SHI15790
      +12X,* VARIANCE = *F10.2/)          SHI15800
C                                           SHI15810
C                                           SHI15820
C      COMPUTE THE T- AND CHISQ-VALUES       SHI15830
C                                           SHI15840
C      REFERENCE:                             SHI15850
C      R.J.LARSEN, M.L.MARK                   SHI15860
C      AN INTRODUCTION TO MATHEMATICAL STATI SHI15870
C      APPLICATION                            SHI15880
C                                           SHI15890
C      PRENTICE HALL 1980                     SHI15890
C      T-TEST: PG 324; CHISQ-TEST: PG 288   SHI15900
C                                           SHI15910
C       $T = \text{ABS}(\bar{X} - \bar{Y}) / (\text{SDEV} / \text{SQRT}(N))$  SHI15920
C                                           SHI15930
C                                           SHI15940
C      RT = NGROUP * 1.0                     SHI15950

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FILE: SHIFLIN FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

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      T = ABS (EXPWGT - SUMWGT) / SQRT (SUMVAR / RT)                SH115960
C                                                                    SH115970
C      COMPUTE THE CHISQ VALUE                                     SH115980
C                                                                    SH115990
C      CHISQ = (N-1) * COMPUTED VARIANCE / EXPECTED VARIANCE     SH116000
C                                                                    SH116010
C      CHISQ = (NA - 1) * SUMVAR / EXPVAR                         SH116020
C      DF = FLOAT (NA)                                           SH116030
C                                                                    SH116040
C      PRINT COMMENTS AND RESULTS                                SH116050
C      WRITE (6,630) T,CHISQ,DF                                  SH116060
630  FORMAT(/10X,*STATISTICAL TESTING*/,10X,19(*-*)//,          SH116070
+10X,*T-TEST (TO TEST THE ACCEPTABILITY OF THE COMPUTED MEAN)*/ SH116080
+10X,*CHISQ-TEST (TO TEST THE ACCEPTABILITY OF THE VARIANCE)*/ SH116090
+//10X,*T-TEST*//                                              SH116100
+12X,*NULL HYPOTHESIS      : COMPUTED MEAN = EXPECTED MEAN*/   SH116110
+//12X,*ALTERNATE HYPOTHESIS: THEY ARE NOT EQUAL*/            SH116120
+//15X,*T-VALUE = *,F10.4//                                    SH116130
+10X,*CHISQ-TEST*//                                           SH116140
+12X,*NULL HYPOTHESIS      : COMPUTED VARIANCE = EXPECTED VARIANCE* SH116150
+//12X,*ALTERNATE HYPOTHESIS: THEY ARE NOT EQUAL*//          SH116160
+15X,*CHISQ-VALUE = *,F10.4//                                  SH116170
+15X,*DEGREE OF FREEDOM = *,F10.0//                            SH116180
      WRITE (6,640)                                             SH116190
640  FORMAT(/,                                                SH116200
+10X,*ENGINEERS ARE RESPONSIBLE TO CHECK BOTH THE*/           SH116210
+10X,*T-AND CHISQ-VALUES WITH THE T-AND CHISQ-DISTRIBUTION*/ SH116220
+10X,*TABLES RESPECTIVELY.*//                                  SH116230
+10X,*IF BOTH HYPOTHESES ARE ACCEPTABLE, THE COMPUTED*/       SH116240
+10X,*WEIGHT DISTRIBUTION CURVE SHOULD BE ACCEPTABLE.*//     SH116250
+10X,*IF ONE OF THE HYPOTHESES IS REJECTED, THEN IT*/        SH116260
+10X,*IS UP TO THE ENGINEERS TO USE THEIR OWN JUDGEMENT*/    SH116270
+10X,*TO ACCEPT OR REJECT THE DISTRIBUTION CURVE.*//)        SH116280
C                                                                    SH116290
C                                                                    SH116300
      RETURN                                                    SH116310
      END                                                        SH116320
C                                                                    SH116330
C*****                                                         SH116340
C                                                                    SH116350
C      A SUBROUTINE STARTS AT THIS POINT                         SH116360
C                                                                    SH116370
C*****                                                         SH116380
C                                                                    SH116390
C      SUBROUTINE HISTOGRM                                       SH116400
C                                                                    SH116410
C      THIS SUBROUTINE PLOTS A HISTOGRAM TO SHOW THE DISRIBUTION OF
C      TRUCK WEIGHT                                             SH116420
C                                                                    SH116430
C                                                                    SH116440
C      COMMON /INDEX/ IVEH (3) ,IROAD (2) ,ISTATE (2) ,IFLG     SH116450
C      COMMON /BASIC/  D (35) ,SAMSIZ (35) ,NA,NGROUP           SH116460
C      COMMON /PLOTS/  ACCPR (35) ,PERCENT (35)                 SH116470
C                                                                    SH116480
C      CALL PLOTS (0,0,4EPLCT)                                   SH116490
C      CALL PLOT (1.5,2.5,-3)                                    SH116500

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FILE: SHL11N FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

      SIZE1=0.14                                SHL16510
      SIZE2=0.10                                SHL16520
C      DEFINE LENGTH OF X-AXIS                  SHL16530
      ALONG = 0.25 * (NGROUP +4)                SHL16540
C      LOCATE THE STARTING POINT OF X-AXIS HEADING SHL16550
      ALOC = 0.5 * (ALONG - (SIZE1 * 25.0))     SHL16560
C      DEFINE SIZE OF LETTERS IN TITLE        SHL16570
      TSIZE = XLONG / 46.0                     SHL16580
C      DRAW A LINE FOR X-AXIS                  SHL16590
      CALL PLOT(XLONG,0.0,2)                   SHL16600
      CALL PLOT(0.0,0.0,3)                     SHL16610
      XGO = 0.25                                SHL16620
      CALL PLOT(XGO,0.0,3)                     SHL16630
      CALL PLOT(XGO,0.05,2)                   SHL16640
      XVALUE = 0.0                             SHL16650
      CALL NUMBER (0.2,-0.15,SIZE2,XVALUE,0.0,-1) SHL16660
      CALL PLOT(XGO,0.0,3)                   SHL16670
      DO 10 I= 1,NGROUP                       SHL16680
      XGO = XGO + 0.25                         SHL16690
      CALL PLOT(XGO,0.0,3)                   SHL16700
      CALL PLOT(XGO,0.05,2)                   SHL16710
      CALL NUMBER (XGO-0.1,-0.15,SIZE2,D(I),0.0,-1) SHL16720
10     CONTINUE                               SHL16730
      IF (ALOG.EQ.3000) CALL SYMBOL           SHL16740
      +(ALOC,-0.5,SIZE1,25HSINGLE AXLE WEIGHT (KIPS) ,0.0,25) SHL16750
      IF (ALOG.EQ.6000) CALL SYMBOL           SHL16760
      +(ALOC,-0.5,SIZE1,25HTANDEM AXLE WEIGHT (KIPS) ,0.0,25) SHL16770
      IF (ALOG.EQ.9000) CALL SYMBOL           SHL16780
      +(ALOC,-0.5,SIZE1,25HGROSS TRUCK WEIGHT (KIPS) ,0.0,25) SHL16790
      CALL SYMBOL(0.0,-1.5,TSIZE,            SHL16800
      +7A1TRUCK WEIGHT DISTRIBUTION SHIFTING METHODOLOGY ,0.0,47) SHL16810
      CALL PLOT(0.0,0.0,3)                   SHL16820
      CALL PLOT(0.0,0.05,2)                  SHL16830
      DO 15 I=1,10                            SHL16840
      YVALUE = I * 5.0                       SHL16850
      YGO = I * 0.5                          SHL16860
      CALL PLOT(0.0,YGO,3)                   SHL16870
      CALL PLOT(0.05,YGO,2)                  SHL16880
      CALL NUMBER (-0.6,YGO,SIZE2,YVALUE,0.0,1) SHL16890
15     CONTINUE                               SHL16900
      CALL SYMBOL (-1.0,1.8,SIZE1,10HPERCENTAGE ,90.0,10) SHL16910
      CALL SYMBOL (0.0,6.5,SIZE1,IROAD,0.0,20) SHL16920
      CALL SYMBOL (0.0,6.2,SIZE1,IVEH,0.0,30) SHL16930
      CALL SYMBOL (0.0,5.0,SIZE1,ISTATE,0.0,20) SHL16940
      CALL PLOT(0.0,0.0,3)                   SHL16950
      DO 20 I=1,NGROUP                       SHL16960
      XGO = 0.25 * I                          SHL16970
      CALL PLOT(XGO,0.0,3)                   SHL16980
      YGO = PERCENT(I)/10.0                  SHL16990
      CALL PLOT(XGO,YGO,2)                   SHL17000
      CALL NUMBER (XGO+0.1,YGO+0.1,SIZE2,PERCENT(I),90.0,1) SHL17010
      CALL PLOT(XGO,YGO,3)                   SHL17020
      CALL PLOT(XGO+0.25,YGO,2)              SHL17030
      CALL PLOT(XGO+0.25,0.0,2)             SHL17040
20     CONTINUE                               SHL17050

```

FILE: SHIPIN FORTRAN B

VM/SP CONVERSATIONAL MONIIR SYSTEM

CALL PLOT(0.0,0.0,999)
RETURN
END

SHI17060
SHI17070
SHI17080

APPENDIX 3

SOURCE PROGRAM OF "TAWEXP"—SHIFTING PROGRAM
FOR TANDEM AXLE WEIGHT DISTRIBUTION

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FILE: TAWEXP FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

PROGRAM TAWEXP (INPUT, OUTPUT, PLOT)
C THIS PROGRAM IS USED TO ANALYZE THE PREDICTION FOR
C TANDEM AXLE DISTRIBUTION. IT CAN BE APPLIED TO 2
C TYPES OF TRUCKS ONLY -- 3-A AND 3-S2. THE SEQUENCE
C OF INPUT DATA SHOULD BE ACCORDING TO THE FOLLOWING
C ORDER :
C 1. GROSS VEHICLE WEIGHT DISTRIBUTION DATA
C 2. SINGLE AXLE WEIGHT DIST. DATA OF THE SAME YEAR,
C 3. TANDEM AXLE WEIGHT DIST. DATA OF THE SAME YEAR.
C
C THE RESULT OF THE ANALYSIS IS PRESENTED IN ACTUAL
C AND PREDICTED TANDEM AXLE WEIGHT DISTRIBUTION AND
C THE CHI-SQUARE VALUE TO SHOW THEIR GOODNESS OF FIT.
C
C DATE OF FIRST VERSION: OCTOBER 12, 1961.
C WRITTEN BY PAUL NG FOR THE CENTER FOR TRANSP. RESEARCH.
C
DIMENSION VALUE (3, 101), TAWE (101)
COMMON /BASIC/ WDIST (35), SAMSIZ (35), N, NGROUP
COMMON /PLOT/ ACCFR (2, 35)
COMMON /INDEX/ IVEH (3), IROAD (2), ISTATE (2), IFLG, IYEAR
DIMENSION YSUM (35), Y (35)
DIMENSION GVW (2, 35), AXLE (2, 16)
DATA (GVW (1, I), I=1, 30) / 4., 10., 13.5, 20., 22., 24., 26., 28., 30., 32.,
+ 34., 36., 38., 40., 45., 50., 55., 60., 65., 70., 72., 75., 80.,
+ 85., 90., 95., 100., 105., 110., 115. /
DATA (GVW (2, I), I=1, 26) / 10., 12., 14., 16., 18., 20., 22., 24., 26., 28., 30.,
+ 35., 40., 45., 50., 55., 60., 65., 70., 75., 80., 85., 90., 95., 100., 105. /
DATA (AXLE (1, I), I=1, 13) / 3., 7., 8., 12., 16., 18., 18.5, 20., 22., 24., 26.,
+ 30., 35. /
DATA (AXLE (2, I), I=1, 16) / 6., 12., 18., 24., 30., 32., 32.5, 34., 36., 38.,
+ 40., 42., 44., 46., 50., 55. /
DO 333 K=1, 3
READ 51, IYEAR, IROAD, IVEH, ISTATE, N
DENOM = 1.0
IF (IVEH (1) .EQ. '3-S2 (33200)') DENOM = 2.0
IF (IVEH (1) .EQ. '3-A (230000)') DENOM = 1.0
READ 51, IFLG
IF (IFLG .EQ. 3000) GO TO 601
IF (IFLG .EQ. 6000) GO TO 602
IF (IFLG .EQ. 9000) GO TO 603
READ 52, (WDIST (I), I=1, N)
GO TO 5
601 DO 610 I=1, 13
610 WDISI (I) = AXLE (1, I)
NGROUP = 13
GO TO 5
602 DO 612 I=1, 16
612 WDISI (I) = AXLE (2, I)
NGROUP = 16
GO TO 5
603 IF (IYEAR .LE. 1958) GO TO 620
DO 613 I=1, 30
WDISI (I) = GVW (1, I)

```

FILE: TAMEXP FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

613 CONTINUE
    NGROUP = 30
    GO TO 5
620 DO 614 I=1,26
614 *WDIST(I) = GVM(2,I)
    NGROUP = 26
    READ 52, (SAMSIZ(J),J=1,N)
    SUMB=0.0
    DO 10 I=1,N
10    SUMB=SUMB + SAMSIZ(I)
    DO 11 I=1,N
11    Y(I) = SAMSIZ(I)/SUMB * 100.0
    CONTINUE
    YSUM(1) = Y(1)
    DO 12 I=2,N
12    YSUM(I) = YSUM(I-1) + Y(I)
    M=1
    CHECK = 0.00000001
    L=1
20    IF (M.EQ.2) CHECK=CHECK - 0.00000001
    IF (CHECK.GE.100.1) GO TO 999
    IF (L.EQ.N) GO TO 41
    IF (YSUM(L) - CHECK) 40,41,42
40    L=L+1
    GO TO 20
41    VALUE(K,M) = WDIST (I)
    M=M+1
    CHECK = CHECK + 1.0
    GO TO 20
42    YSUMI1=0.0
    IF (L.GT.1) YSUMI1=YSUM(I-1)
    IF (YSUM(L)-YSUMI1) 20,20,43
43    AIM1=0.0
    IF (L.GT.1) AIM1= WDIST (I-1)
    VALUE(K,M) =AIM1 + (CHECK - YSUMI1)/
    * (YSUM(L) - YSUMI1) * (WDIST(I) - AIM1)
    M=M+1
    CHECK =CHECK + 1.0
    GO TO 20
999 CONTINUE
    ASQ = 0.0
    DO 30 I=2,101
    TAME(I) = (VALUE(1,I) - VALUE(2,I))/DENOM
    DIFF = TAME(I) - VALUE(3,I)
    DIFFSQ = DIFF * DIFF
    CONST = DIFFSQ / TAME(I)
    ASQ = ASQ + CONST
30 CONTINUE
    PUT ACTUAL DATA INTO VALUE(1,I),AND EXPECTED VALUES INTO VALUE(2,I)
    DO 31 I = 2,101
    VALUE(2,I) = TAME(I)
    VALUE(1,I) = VALUE(3,I)
31 CONTINUE
    K = 1
101 M = 1

```

FILE: FAWEXP FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

      VALUE(K,1) = 0.0
      I = 2
105  IF (3.EQ.26) GO TO 113
      IF (1.GT.101) GO TO 113
      K1 = K + 1
      IF (VALUE(K,I) - AXLE(2,M)) 110,111,112
110  I = I + 1
      GO TO 105
111  ACCFR(K,M) = (I - 1) * 1.0
      I = I + 1
      M = M + 1
      GO TO 105
112  ACCFR(K,M) = ((AXLE(2,M) - VALUE(K,I-1)) /
      . (VALUE(K,I) - VALUE(K,I-1)) + (I-2)) * 1.0
      M = M + 1
      GO TO 105
113  DO 114 I = M,16
      ACCFR(K,I) = 100.0
114  CONTINUE
      K = K + 1
      IF (K.EQ.2) GO TO 101
      PRINT 500,IYEAR,IROAD,ISTATE,IVEH
500  FORMAT(*1*,//10X,*AXLE WEIGHT DISTRIBUTION ANALYSIS*//
+10X,14,//10X,2A10,//10X,2A10,//10X,3A10//)
      PRINT 503
503  FORMAT(10X,*TANDEM*,4X,*ACTUAL*,8X,*EXPECTED*/
+10X,* AXLE *,3X,*CUMMULATED*,5X,*CUMMULATED*/
+10X,*WEIGHT*,3X,*PERCENTAGE*,5X,*PERCENTAGE*/
      DO 499 I=1,16
499  PRINT 501,AXLE(2,I),ACCFR(1,I),ACCFR(2,I)
      PRINT 502,XSQ
501  FORMAT(6X,F10.2,3X,F10.2,3X,F10.2)
502  FORMAT(//10X,*CHI-SQUARE VALUE = *,F10.4/)
51  FORMAT(14,X,2A10,3A10,2A10,I5)
52  FORMAT(1ZF0.1)
      CALL SPLOT
      STOP
      END
      SUBROUTINE SPLOT
      DIMENSION XAXIS(4),YAXIS(4),TITLE(5),ACC(35)
      INTEGER XAXIS,YAXIS,TITLE
      COMMON /BASIC/ ADIST(35),SAMSIZ(35),N,NGROUP
      COMMON /INDEX/ IVEH(3),IRCAD(2),ISTATE(2),IFLG,IYEAR
      COMMON /PLOT/ ACCFR(2,35)
C     THIS SUBROUTINE PLOTS A TITLE PAGE
C     INITIATE ZETAPLOT
C
      ENLARG = 1.0
      REDUCE = 0.9
      IF (IFLG.EQ.3000.OR.IFLG.EQ.6000) ENLARG=2.0
      SIZE1=.14
      SIZE2=.11
      FATOR = 1./ENLARG
C     TO CALL ZETAPLOT SUBROUTINES -- INITIATE
      CALL PLOTS(0,0,4LPLOT)

```

FILE: PAWEAP FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

C      TO MOVE PEN TO THE ALLOCATED ORIGIN
      CALL PLOT (1.8,1.5,-3)
      CALL FACTOR (REDUCE)
C      TO PLOT A 7 INCH AXIS WITH TICK MARKS AND ANNOTATIONS
      X=0.0
      Z=0.0
      DO 100 I=1,12
      CALL PLOT (X,0.0,3)
      X = X + 0.5
      Z = Z + 10.0*FACTOR
      CALL PLOT (X, 0.0, 2)
      CALL PLOT (X, 0.1,2)
      CALL NUMBER (X-0.09,-.25, 0.105,Z,0.0,-1)
100    CONTINUE
      X = 0.0
      DO 310 I=1,120
      CALL PLOT (X,0.0,3)
      CALL PLOT (X,0.05,2)
      X = X +0.05
310    CONTINUE
C      TO WRITE THE TITLE FOR THE PLOT
      CALL SYMBOL(0.0,-1.0,SIZE1,
+44H AXLE WEIGHT DISTRIBUTION SHIFTING PROCEDURE ,0.,44)
C      TO WRITE SYMBOLS FOR X-AXIS
      CALL SYMBOL (0.75,-.5,SIZE1,
+26H FIFTEN AXLE WEIGHT (KIPS) , 0.,28)
C      TO PLOT AN 8-INCH Y-AXIS WITH TICK MARKS AND NO.
      Y = 0.0
      W = 0.0
      DO 101 I=1,10
      CALL PLOT (0.0,Y,3)
C      SCALE THE Y-AXIS
      Y = Y + 0.8
      W = W + 1.0
      CALL PLOT (0.0,Y,2)
      CALL PLOT (0.1,Y,2)
C      PUT NUMBERS UNDER TICK MARKS ( NUMBER = XXX.XX)
      IDIGIT = 0
      CALL NUMBER (-0.15,Y-.1, 0.105,10.*W,90.0,1DIGIT)
101    CONTINUE
      Y = 0.0
      DO 311 I =1,100
      Y = Y + 0.06
      CALL PLOT (0.0,Y,3)
      CALL PLOT ( 0.05,Y,2)
311    CONTINUE
C      TO PLOT SYMBOLS FOR Y-AXIS
      CALL SYMBOL (-.4,1.5,SIZE1,
+30H ACCUMULATED FREQUENCY (%) ,90.,30)
C      TO PRINT THE HIGHWAY TYPE
      X=1.2
      Y=9.0
      CALL SYMBOL(0.,Y,SIZE1,IROAD,0.,20)
      Y=Y-.2
C      TO PRINT THE VEHICLE TYE

```

FILE: FANEXP FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

      CALL SYMBOL(0.,Y,SIZE1,IVEH,0.,30)
      Y=Y-.25
C     PRINT THE STATE
      CALL SYMBOL(0.,Y,SIZE1,ISTATE,0.,20)
C     PRINT THE YEAR
      Y = Y -.25
      YEAR = FLOAT(IVEAR)
C     CALL NUMBER(0.,Y,SIZE1,YEAR,0.,-1)
C     DEFINE THE SCALE FOR THE GRAPH
      XSCALE = 20. * FATOR
      N = NGROUP
      DO 312 I=1,2
      DO 313 K = 1,N
        ACC(K) = ACCPR(I,K)
313  CONTINUE
      WDISI(N+1) = 0.
      WDISI(N+2) = XSCALE
      ACC(N+1) = 0.
      ACC(N+2) = 12.5
      CALL LINE (WDIST(1),ACC(1),N,1,1,I)
      IF (1.EQ.1) CALL SYMBOL (4.0,2.0,.11,1,0.,-1)
      IF (1.EQ.1) CALL SYMBOL (4.4,2.0,.11,11ACTUAL DATA ,0.0,11)
      IF (1.EQ.2) CALL SYMBOL (4.0,1.8,.11,1,0.,-1)
      IF (1.EQ.2) CALL SYMBOL (4.4,1.8,.11,15EXPECTED VALUES ,0.0,15)
312  CONTINUE
      CALL PLOT(0.,0.,999)
      RETURN
      END

```

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APPENDIX 4

SOURCE PROGRAM FOR SHIFTING OF TRUCK WEIGHT DISTRIBUTION
BASED ON NCHRP/SDHPT PROCEDURE

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FILE: NCHRP FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

PROGRAM NCHRP (INPUT,OUTPUT)
C THIS PROGRAM PROJECTS THE SHIFTING OF TRUCK WEIGHT DISTRIBUTION
C CURVES. THE METHODOLOGY USED IS BASED UPON NCHRP 141 PROCEDURE
C OR THE SDHFI PROCEDURE DEVELOPED BY TEXAS HIGHWAY DEPARTMENT.
DIMENSION IROAD(2),IVEH(3),ISTATE(2),SAMSIZ(30),GVW(30),
+YSUM(30),Y(30),VALUE(105),ACCFR(30),PERCT(30)
DATA (GVW(I),I=1,30)/4.,10.,13.5,20.,22.,24.,26.,28.,30.,32.,
+34.,36.,38.,40.,45.,50.,55.,60.,65.,70.,72.,75.,80.,
+85.,90.,95.,100.,105.,110.,115./
READ 50,SGVW,PMGVWP,PMGVWF
50 FORMAT(3F10.5)
READ 51,IYEAR,IROAD,IVEH,ISTATE,N
READ 51,IFLAG
51 FORMAT(I4,X,2A10,3A10,2A10,I5)
READ 52,(SAMSIZ(I),I=1,N)
52 FORMAT(12F6.1)
SUMB = 0.0
DO 10 I=1,N
10 SUMB = SUMB + SAMSIZ(I)
DO 11 I=1,N
11 I(I) =SAMSIZ(I) / SUMB * 100.0
CONTINUE
YSUM(1) = Y(1)
DO 12 I= 2,N
12 YSUM(I) = YSUM(I-1) + Y(I)
M = 1
CHECK = 0.000001
I = 1
20 IF (I.EQ.2) CHECK = CHECK - 0.000001
IF (CHECK.GT.100.1) GO TO 999
IF (I.EQ.N) GO TO 41
IF (YSUM(I) - CHECK) 40,41,42
40 I = I + 1
GO TO 20
41 VALUE(M) = GVW(I)
M = M + 1
CHECK = CHECK + 1.0
GO TO 20
42 YSUMI1 = 0.0
IF (I.GT.1) YSUMI1 = YSUM(I - 1)
IF (YSUM(I) - YSUMI1) 20,20,43
43 AIM1 = 0.0
IF (I.GI.1) AIM1 = GVW(I-1)
VALUE(M) = AIM1 + (CHECK - YSUMI1) /
- (YSUM(I) - YSUMI1) * (GVW(I) - AIM1)
M = M + 1
CHECK = CHECK + 1.0
GO TO 20
999 DO 150 I=1,100
IF (VALUE(I).LT.SGVW) GO TO 150
IF (VALUE(I).GT.SGVW.AND.VALUE(I).LT.PMGVWP)
+VALUE(I) = VALUE(I) * (1.0 +(PMGVWF/PMGVWP - 1.0)/
+(PMGVWP - SGVW) * (VALUE(I) - SGVW))
IF (VALUE(I) .GE. PMGVWP)

```

FILE: NCHRP FORTRAN B

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

+VALUE(I) = VALUE(I) * PMGVWF / PMGVWP
100 CONTINUE
C
C THIS PART OF PROGRAM CONVERTS VALUES BACK TO PERCENTAGE
C
VALUE(1) = 0.0
A = 1
I = 2
105 IF (A.EQ.30) GO TO 113
IF (I.GT.101) GO TO 113
IF (VALUE(A) - GVW(A)) 110,111,112
110 I = I + 1
GO TO 105
111 ACCFR(A) = (I - 1) * 1.0
I = I + 1
A = A + 1
GO TO 105
112 ACCFR(A) = ((GVW(A) - VALUE(I-1)) / (VALUE(I) - VALUE(I-1))
+ (I-2)) * 1.0
A = A + 1
GO TO 105
113 DO 114 I=1,30
ACCFR(I) = 100.0
114 CONTINUE
PERCT(1) = ACCFR(1)
SAMSIZ(1) = PERCT(1) * SUMB / 100.0
DO 115 I=2,30
PERCT(I) = ACCFR(I) - ACCFR(I-1)
SAMSIZ(I) = PERCT(I) * SUMB / 100.0
115 CONTINUE
C THIS PART PRINTS THE OUTPUT
C
PRINT 600, IYEAR, IROAD, IVEH, ISTATE
600 FORMAT(*1*,10X,*PROJECTION BY NCHRP/SDHPT METHODOLOGY*//,
+10X,14,/10X,2A10,/10X,3A10,/10X,2A10/)
PRINT 601
601 FORMAT(10X,*GVW INTERVALS*,10X,*ACCUMULATED FREQUENCY*/)
DO 116 I=1,30
PRINT 602, GVW(I), ACCFR(I)
116 CONTINUE
NA = 30
PRINT 117, IYEAR, IROAD, IVEH, ISTATE, NA
117 FORMAT(*1*,/I4,X,2A10,3A10,2A10,I5)
PRINT 119, IFLAG
119 FORMAT(I4)
PRINT 118, (SAMSIZ(I), I=1,NA)
118 FORMAT(12F6.1)
602 FORMAT(10X,F10.4,15X,F10.4)
STOP
END

```

APPENDIX 5

INPUT FORMAT AND ILLUSTRATION FOR "MEANWGT"

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INPUT FORMAT FOR "MEANWGT"

1. First card—Description of Data I

Format: (I4, X, 2A10, 3A10, 2A10, I5)

- a. YEAR Year in which data was collected
- b. IROAD Highway system in which the truck weighing stations
 were located
- c. IVEH Vehicle type
- d. ISTATE State in which data was collected
- e. N Number of distribution intervals contained in the data

2. Second card—Description of Data II

Format: (I4)

- a. IFLAG To indicate the type of truck weight
 i.e., 3000 for single axle weight
 6000 for tandem axle weight
 9000 for gross vehicle weight

3. Third card to Fifth card—Sample sizes for corresponding truck weight distribution groups (one to three cards)

Format: (12F6.1)

- a. $\overline{[SAMSIZ(i), I = 1, N]}$ Number of trucks recorded in the corresponding truck weight distribution groups

Note: Repeat the above sequence to compute for more years.
Leave a blank card to terminate.

FILE: APENDC DATA 3

VM/SP CONVERSATIONAL MONITOR SYSTEM

```

.....*.....1.....*.....2.....*.....3.....*.....4.....*.....5.....*.....6.....*.....7.....*.....
1970.INTERSTATE RURAL 3-S2(332000) STATE OF TEXAS 2
9000.GROSS VEHICLE WEIGHT
 0. 0. 0. 2. 15. 51. 85. 117. 92. 61. 37. 31.
39. 32. 79. 95. 117. 229. 254. 157. 48. 39. 23. 4.
 1.
1971.INTERSTATE RURAL 3-S2(332000) STATE OF TEXAS 2
9000.GROSS VEHICLE WEIGHT
 0. 0. 0. 14. 41. 95. 125. 114. 77. 48. 54. 33.
35. 27. 127. 128. 201. 278. 217. 94. 12. 1. 3. 1.
 1.
1972.INTERSTATE RURAL 3-S2(332000) STATE OF TEXAS 2
9000.GROSS VEHICLE WEIGHT
-0. -0. -0. 2. 5. 17. 23. 13. 16. 9. 2. 6.
 5. 6. 17. 19. 32. 51. 38. 15. 2. 2. 2. -0.
 0. 1.
1973.INTERSTATE RURAL 3-S2(332000) STATE OF TEXAS 2
9000.GROSS VEHICLE WEIGHT
-0. -0. -0. 2. 7. 17. 30. 21. 15. 11. 7. 10.
 4. 4. 14. 18. 27. 59. 33. 24. 8. 3. 2. 1.
1974.INTERSTATE RURAL 3-S2(332000) STATE OF TEXAS 2
9000.GROSS VEHICLE WEIGHT
-0. -0. -0. 4. 16. 46. 39. 23. 16. 15. 8. 12.
10. 6. 12. 27. 33. 58. 34. 22. 0. 1. 5. 1.
1976.INTERSTATE RURAL 3-S2(332000) STATE OF TEXAS 3
9000.GROSS VEHICLE WEIGHT
 0. 0. 0. 0. 0. 0. 2. 9. 29. 25. 53. 32.
30. 18. 67. 62. 91. 130. 138. 146. 90. 138. 206. 101.
42. 20. 11. 1. 3. 1.
1978.INTERSTATE RURAL 3-S2(332000) STATE OF TEXAS 3
9000.GROSS VEHICLE WEIGHT
 0. 0. 0. 2. 3. 5. 36. 94. 155. 121. 103. 74.
57. 54. 131. 119. 149. 173. 157. 316. 159. 196. 165. 58.
32. 13. 2. 1. 1. 0.
1979.INTERSTATE RURAL 3-S2(332000) STATE OF TEXAS 2
9000.GROSS VEHICLE WEIGHT
 0. 0. 0. 0. 0. 5. 24. 54. 92. 82. 74. 54.
41. 45. 91. 96. 112. 126. 154. 221. 112. 159. 171. 99.
31. 9. 7. 3.

```


APPENDIX 6

INPUT FORMAT AND ILLUSTRATION FOR "SHIFTIN"

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INPUT FORMAT AND ILLUSTRATION FOR "SHIFTIN"

- (1) Nature of data (one card)
 Format: (A7)
- (a) [COEFF] COEFFICIENTS for a linearized curve. The coefficients are the $\ln(A)$, B , and C of the following equation.
- $$\ln Y = \ln A + B \ln x + Cx$$
- The input of COEFFICIENTS will provide a base for the program to start shifting.
- (b) [RAWDATA] To identify that the data provided in the file is drawn from raw field data. Regression analysis is required before actual shifting can be started.
- (2) Optional commands (up to five cards)
 Format: (A5)
- These optional commands can be used to suppress certain outputs. These options are:
- (a) [NORATIO] To suppress the output from the subroutine /RATIO/, which is to compute the ratios of truck weight for two years at five percent interval.
- (b) [NOREGRE] To suppress the output from the subroutine /REGRESS/, which is to perform a regression analysis and to fit the ratios obtained from subroutine /RATIO/ to a straight line.
- (c) [NOCHISQ] To suppress the output from the subroutine /CHISQ/, which is to perform a CHI-SQUARE analysis on the curve fitted by /REGRESS/ and the actual data.
- (d) [NOPLOT] To suppress the plotting routine from generating an accumulated frequency truck weight distribution curve (shifted curve).
- (e) [NOHISTO] To suppress the plotting routine from generating a histogram for the shifted truck weight curve.

NOTE: Leave a blank card to terminate options.

INPUT FORMAT AND ILLUSTRATION FOR "SHIFTIN" (cont.)

- (3) Estimator—Input card (one card)

Format: (5F10.5)

This card contains three parameters. The sequence of the parameters are:

- (a) [EXPWGT] Expected average weight for the predicted year. The expected average weight is obtained either from historical trend analysis or from an average GVW factor.
- (b) [EXPVAR] Expected variance for the predicted year. The variance for truck weight is obtained from historical trend analysis. To project a weight distribution curve for a new set of proposed truck weight limits, EXPVAR should be \approx 10-15 percent higher than the latest available distribution.
- (c) [CPT] Critical point from which shifting starts to occur. It is expressed in terms of percentage, i.e. for 10 percent input CPT = 10.0.

- (4) Latest truck weight distribution data (three to five cards)

- (a) First card—description of data I

Format: (I4, x, 2A10, 3A10, 2A10, I5)

- (i) IYEAR Year in which data was collected.
- (ii) IROAD Highway system in which the truck weighing stations were located.
- (iii) IVEH Vehicle type.
- (iv) ISTATE State in which data was collected.
- (v) N Number of distribution intervals contained in the data.

- (b) Second card—description of data II

Format: (I4)

- (i) IFLG To indicate the type of truck weight, i.e. 3,000 single axle weight
6,000 tandem axle weight
9,000 gross vehicle weight

INPUT FORMAT AND ILLUSTRATION FOR "SHIFTIN" (cont.)

- (c) Third card—Fifth card—sample sizes for corresponding truck weight distribution groups (one to three cards)
- Format: (12F6.1)
- (i) [SAMSIZ The number of trucks recorded in the corresponding truck weight distribution groups.
 (I), The distribution weights are input inside
 I=1,N] the program in DATA statements. Check W-4 or W-5 tables for the weights of the distribution groups. Each card should not contain more than 12 numbers
- (5) Base year truck weight distribution data (three to five cards)

Format:

Description of this item is exactly the same as than in item 4. The importance of this item is that the general shape of curve of the base year affects the shape of the predicted years. Thus, it is important to choose data with a large sample size for the base year data. Any unsmoothness in a curve due to scarcity of sample will sacrifice the accuracy of prediction.

A sample input is shown on the next page. This sample is the actual input data used for the shifting of truck weight distribution curve for 1978 for the truck 3-S2 on Texas highways. The output of the shifting is shown in the latter part of the appendix.

FILE: APENDA2 DATA B

VM/SP CONVERSATIONAL MONITOR SYSTEM

.....*.....1.....*.....2.....*.....3.....*.....4.....*.....5.....*.....6.....*.....7.....*.....

RAWDATA

14.58	100.0	0.0											
1974.INTERSTATE RURAL		2D(220000)										STATE OF TEXAS	1.
9000.GROSS VEHICLE WEIGHT													
-0.	15.	10.	15.	4.	3.	0.	1.	0.	0.	0.	1.		
1978.INTERSTATE RURAL		2D(220000)										STATE OF TEXAS	1.
9000.GROSS VEHICLE WEIGHT													
0.	151.	144.	120.	51.	36.	19.	7.	3.	0.	1.	1.		

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APPENDIX 7

INPUT FORMAT AND ILLUSTRATION FOR "TAWEXP"

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INPUT FORMAT FOR "TAWEXP"

Three sets of data are to be arranged in the following order:

1. predicted GVW distribution for the interested year,
2. actual SAW distribution from the latest available year,
3. optional: actual TAW distribution to be compared with the output.

For each set of data, input format is similar to that listed for "MEANWGT."
The third set of data is optional. If no comparison is expected, leave a blank card to terminate the data set.

FILE: APENDB DATA B

VM/SP CONVERSATIONAL MONITOR SYSTEM

.....*.....1.....*.....2.....*.....3.....*.....4.....*.....5.....*.....6.....*.....7.....*.....

1978.INTERSTATE RURAL 3-S2(332000) STATE OF TEXAS 3

9000.GROSS VEHICLE WEIGHT

.165 .2445 .1426 .2049 .0815 .0815 3.016 7.181 8.350 5.554 3.120 2.234

2.465 2.031 4.639 5.002 5.454 7.410 9.669 9.965 3.823 4.332 5.457 5.961

2.296 .3000 .3354 .3354 .0078 0.000

1974.INTERSTATE RURAL 3-S2(332000)

STATE OF TEXAS

3000.SINGLE AXLE

1. 13. 69. 301. 8.

1978.INTERSTATE RURAL 3-S2(332000)

STATE OF TEXAS

6000.TANDEM AXLE

1. 840. 790. 676. 1019. 519. 135. 312. 222. 116. 53. 32.

13. 4. 2. 2.

APPENDIX 8

SAMPLE OUTPUT FROM "SHIFTIN"

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MULTIPLIERS OF THE INPUT TRUCK *WEIGHT DATA

3-82(332000)
 INTERSTATE RURAL
 STATE OF TEXAS

YEAR (1) = 1974

YEAR (2) = 1970

| ACCUMULATED
FREQUENCY | YEAR(1)
GVW (KIPS) | YEAR(2)
GVW (KIPS) | YEAR(1)/YEAR(2) |
|--------------------------|-----------------------|-----------------------|-----------------|
| 0 | 13.5000 | 13.5000 | |
| 5.0000 | 21.9250 | 24.2882 | .90 |
| 10.0000 | 22.8174 | 26.1282 | .87 |
| 15.0000 | 23.6609 | 27.5000 | .86 |
| 20.0000 | 24.5949 | 29.1087 | .84 |
| 25.0000 | 25.5897 | 31.2869 | .82 |
| 30.0000 | 26.9913 | 34.3871 | .76 |
| 35.0000 | 28.9750 | 39.9840 | .72 |
| 40.0000 | 31.4933 | 45.0526 | .70 |
| 45.0000 | 35.2667 | 49.2763 | .72 |
| 50.0000 | 39.6667 | 52.8419 | .75 |
| 55.0000 | 46.1852 | 55.6496 | .83 |
| 60.0000 | 49.7778 | 57.4017 | .87 |
| 65.0000 | 52.7576 | 59.1539 | .89 |
| 70.0000 | 55.3966 | 60.8169 | .91 |
| 75.0000 | 57.9600 | 62.3967 | .91 |
| 80.0000 | 58.7414 | 63.9764 | .92 |
| 85.0000 | 60.7059 | 65.8997 | .92 |
| 90.0000 | 63.5588 | 68.4554 | .93 |
| 95.0000 | 67.1818 | 71.3229 | .94 |
| 100.0000 | 85.0000 | 90.0000 | .94 |

THE LIGHTEST TRUCK
 RECORDED IS IN THE
 DISTRIBUTION GROUP: 13.5- 20.0 13.5- 20.0

THE HEAVIEST TRUCK
 RECORDED IS IN THE
 DISTRIBUTION GROUP: 80.0- 85.0 85.0- 90.0

REGRESSION ANALYSIS TO FIT MULTIPLIERS INTO AN EQUATION

A = MATRIX (3 X 3)

| | | |
|---------|---------|----------|
| 20.00 | 77.24 | 1015.93 |
| 77.24 | 301.10 | 4052.58 |
| 1015.93 | 4052.58 | 57748.10 |

INVERSE OF MATRIX A (3X3)

| | | |
|--------|--------|------|
| 91.23 | -32.48 | .67 |
| -32.48 | 11.62 | -.24 |
| .67 | -.24 | .01 |

C= VECTOR

| |
|---------|
| -3.31 |
| -12.56 |
| -153.52 |

FORM OF MATRIX MULTIPLICATION

[A(INV)](C(VEC)) = (B(VEC))

B=VECTOR (COEFFICIENTS A,B,AND C)

| | |
|---------|--------|
| LN(A) = | 1.9122 |
| B = | -.7876 |
| C = | .0190 |

CHI-SQUARE TESTING ON GOODNESS-OF-FIT OF THE MULTIPLIER EQUATION

EQUATION FITTED: $Y_{EXP} = A * (X ** B) * EXP (X * C)$

WHERE A = 6.7684
 B = -.7876
 C = .0190

| | (1)
ACTUAL
VALUES | (2)
EXPECTED
VALUES | (3)
(1) - (2) | (4)
(3)X(3) | (5)
(4)/(2) |
|----|-------------------------|---------------------------|--------------------|----------------|----------------|
| 1 | .002700 | .869907 | -.832794 | .001075 | .001236 |
| 2 | .873286 | .850462 | -.022824 | .000521 | .000613 |
| 3 | .860395 | .838407 | -.021989 | .000484 | .000577 |
| 4 | .844932 | .826539 | -.018393 | .000338 | .000409 |
| 5 | .817906 | .813817 | -.004090 | .000017 | .000021 |
| 6 | .762744 | .798336 | .035592 | .001267 | .001587 |
| 7 | .724658 | .791197 | .066539 | .004427 | .005506 |
| 8 | .699034 | .792905 | .093870 | .008812 | .011113 |
| 9 | .715692 | .900512 | .084820 | .007194 | .008987 |
| 10 | .750667 | .810678 | .060011 | .003601 | .004442 |
| 11 | .829929 | .820867 | -.009062 | .000082 | .000100 |
| 12 | .867182 | .828143 | -.039039 | .001524 | .001840 |
| 13 | .891860 | .836997 | -.055772 | .003111 | .003720 |
| 14 | .910874 | .844257 | -.066617 | .004438 | .005257 |
| 15 | .914616 | .852547 | -.062069 | .003853 | .004519 |
| 16 | .918173 | .861354 | -.056819 | .003228 | .003748 |
| 17 | .921186 | .872762 | -.048424 | .002345 | .002687 |
| 18 | .928470 | .889074 | -.039397 | .001552 | .001746 |
| 19 | .941939 | .908922 | -.033017 | .001090 | .001109 |
| 20 | .944444 | 1.078565 | .134120 | .017968 | .021678 |
| | | | CHI-SQUARE VALUE = | .076075 | |

THE CHI-SQUARE VALUE IS WITHIN THE 5 PERCENT SIGNIFICANT VALUE.
 THUS, THE COEFFICIENTS MAY BE USED FOR THE EQUATION

RESULTS OBTAINED FROM THE POWELL METHOD
 =====

| FX | X(1) | X(2) | X(3) |
|---------------|---------------|----------------|---------------|
| .14322279E+02 | .67679738E+01 | -.79756438E+00 | .18970216E-01 |
| .82483325E-06 | .91262080E+01 | -.79756438E+00 | .18970214E-01 |
| FX | X(1) | X(2) | X(3) |
| .33066588E+01 | .91272080E+01 | -.79656438E+00 | .19970214E-01 |
| .26806692E-05 | .91272080E+01 | -.80129808E+00 | .19970214E-01 |
| TIME IS NOW = | 6.218 | | |

o FUNCTION EVALUATIONS WITHIN POWELL ROUTINE AND
 266 FUNCTION EVALUATIONS DURING THE LINE SEARCHES.

FUNCTION VALUE = .26806692E-05

VARIABLE VALUES:-

.91272080E+01 -.80129808E+00 .19970214E-01

TIME IS NOW = 8.17900000

COEFFICIENTS OF THE EQUATION

A = .91272
 B = -.8013
 C = .0200

AVERAGE WEIGHT = 55.4400
 VARIANCE = 427.4632

PROJECTED TRUCK WEIGHT DISTRIBUTION (51 INTERVALS)

=====

| ACC.FREQ.
(PERCENT) | FACTORS | WEIGHT
(KIPS) |
|------------------------|---------|------------------|
| 5.0000 | 1.1505 | 27.9428 |
| 10.0000 | 1.1257 | 29.4123 |
| 15.0000 | 1.1105 | 30.5381 |
| 20.0000 | 1.0957 | 31.8974 |
| 25.0000 | 1.0801 | 33.7924 |
| 30.0000 | 1.0621 | 37.5844 |
| 35.0000 | 1.0557 | 42.2104 |
| 40.0000 | 1.0616 | 47.8272 |
| 45.0000 | 1.0750 | 52.9712 |
| 50.0000 | 1.0915 | 57.6756 |
| 55.0000 | 1.1075 | 61.6326 |
| 60.0000 | 1.1188 | 64.2218 |
| 65.0000 | 1.1311 | 66.9073 |
| 70.0000 | 1.1436 | 69.5488 |
| 75.0000 | 1.1562 | 72.1445 |
| 80.0000 | 1.1696 | 74.8276 |
| 85.0000 | 1.1869 | 78.2165 |
| 90.0000 | 1.2115 | 82.9369 |
| 95.0000 | 1.2204 | 87.0309 |
| 100.0000 | 1.2204 | 109.8328 |

PROJECTED WEIGHT DISTRIBUTION
 =====

| WEIGHT DISTRIBUTION | PERCENTAGE | ACCUMULATED FREQUENCY |
|---------------------|------------|-----------------------|
| 4.0000 | .1534 | .1534 |
| 10.0000 | .2301 | .3835 |
| 13.5000 | .1342 | .5177 |
| 20.0000 | .2492 | .7669 |
| 22.0000 | .0767 | .8436 |
| 24.0000 | .0767 | .9203 |
| 26.0000 | .0767 | .9970 |
| 28.0000 | 4.1951 | 5.1920 |
| 30.0000 | 7.4323 | 12.6243 |
| 32.0000 | 7.7321 | 20.3565 |
| 34.0000 | 5.4849 | 25.8414 |
| 36.0000 | 2.8658 | 28.7071 |
| 38.0000 | 2.1443 | 30.8514 |
| 40.0000 | 2.3744 | 32.8258 |
| 45.0000 | 4.7136 | 37.5394 |
| 50.0000 | 4.6244 | 42.1638 |
| 55.0000 | 4.9262 | 47.0900 |
| 60.0000 | 5.3416 | 52.4316 |
| 65.0000 | 9.0353 | 61.4669 |
| 70.0000 | 9.4139 | 70.8808 |
| 72.0000 | 3.8445 | 74.7253 |
| 75.0000 | 5.5897 | 80.3150 |
| 80.0000 | 6.6054 | 86.9204 |
| 85.0000 | 5.1824 | 92.1033 |
| 90.0000 | 5.3254 | 97.4287 |
| 95.0000 | 1.6190 | 99.0476 |
| 100.0000 | .3210 | 99.3687 |
| 105.0000 | .3210 | 99.6897 |
| 110.0000 | .3103 | 100.0000 |
| 115.0000 | 0 | 100.0000 |

AVERAGE WEIGHT = 55.4400
 VARIANCE = 427.4632

CONCLUSION OF ANALYSIS

INPUT ESTIMATORS:

EXPECTED MEAN = 55.44
 EXPECTED VARIANCE = 400.00

COMPUTED ESTIMATORS:

MEAN = 55.44
 VARIANCE = 427.46

STATISTICAL TESTING

T-TEST (TO TEST THE ACCEPTABILITY OF THE COMPUTED MEAN)
 CHISQ-TEST (TO TEST THE ACCEPTABILITY OF THE VARIANCE)

T-TEST

NULL HYPOTHESIS : COMPUTED MEAN = EXPECTED MEAN

ALTERNATE HYPOTHESIS: THEY ARE NOT EQUAL

T-VALUE = .6000

CHISQ-TEST

NULL HYPOTHESIS : COMPUTED VARIANCE = EXPECTED VARIANCE

ALTERNATE HYPOTHESIS: THEY ARE NOT EQUAL

CHISQ-VALUE = 30.0911

DEGREE OF FREEDOM = 30.

ENGINEERS ARE RESPONSIBLE TO CHECK BOTH THE
 T-AND CHISQ-VALUES WITH THE T-AND CHISQ-DISTRIBUTION
 TABLES RESPECTIVELY.

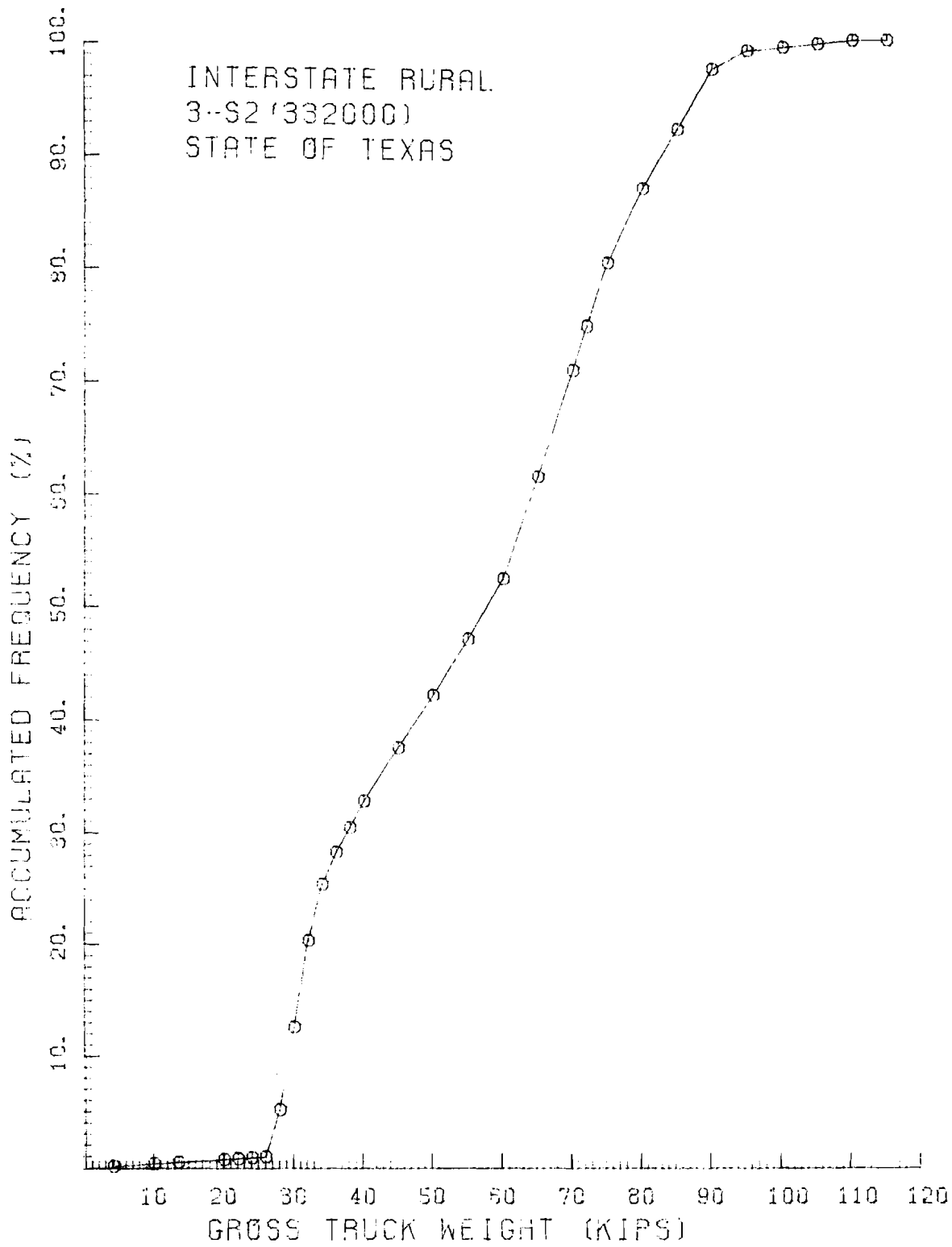
IF BOTH HYPOTHESES ARE ACCEPTABLE, THE COMPUTED
 WEIGHT DISTRIBUTION CURVE SHOULD BE ACCEPTABLE.

IF ONE OF THE HYPOTHESES IS REJECTED, THEN IT
 IS UP TO THE ENGINEERS TO USE THEIR OWN JUDGEMENT
 TO ACCEPT OR REJECT THE DISTRIBUTION CURVE.

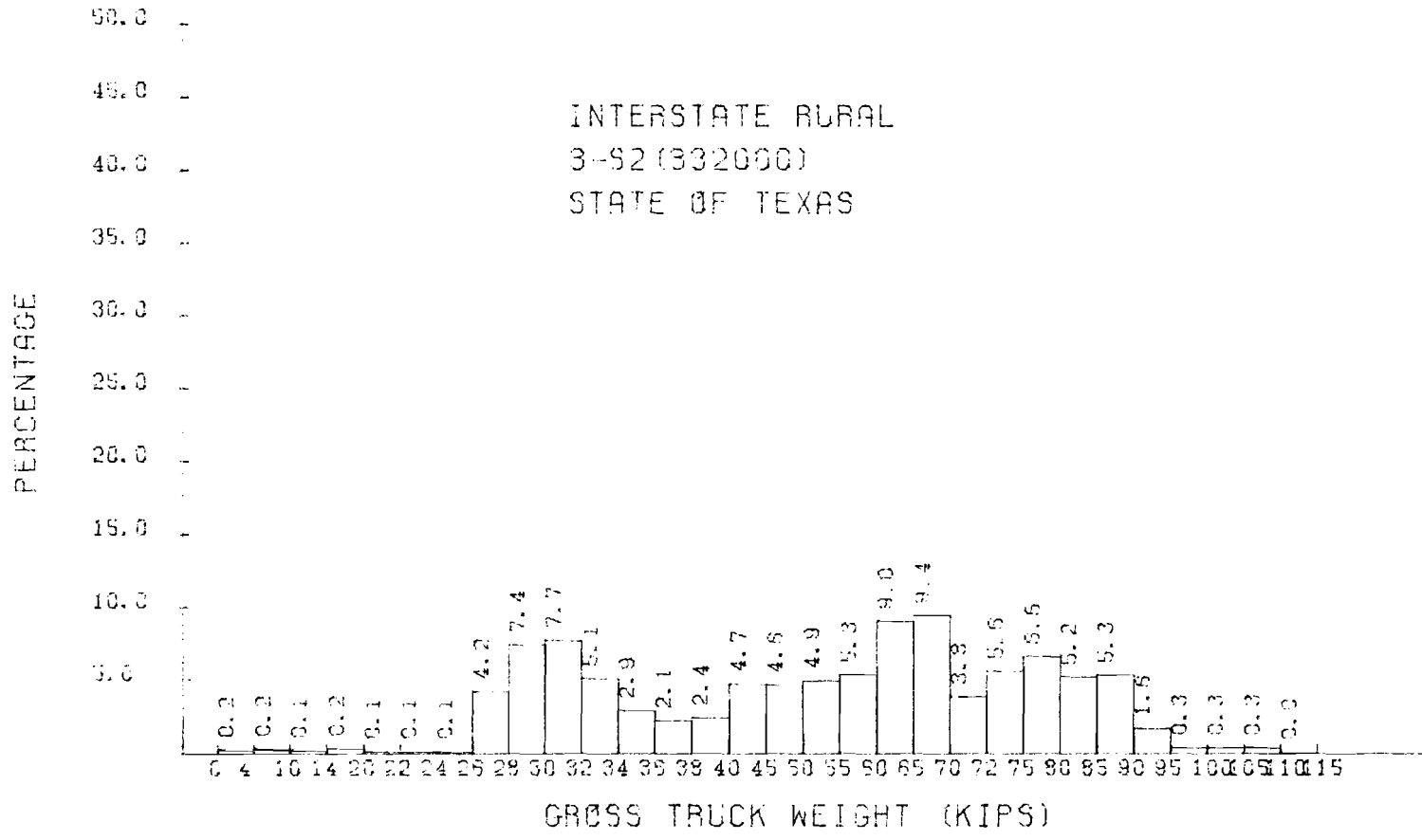
TRUCK WEIGHT
DISTRIBUTION
SHIFTING
METHODOLOGY

22 APR 82

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AUGUST 1, 1981



TRUCK WEIGHT DISTRIBUTION SHIFTING PROCEDURE



TRUCK WEIGHT DISTRIBUTION SHIFTING METHODOLOGY